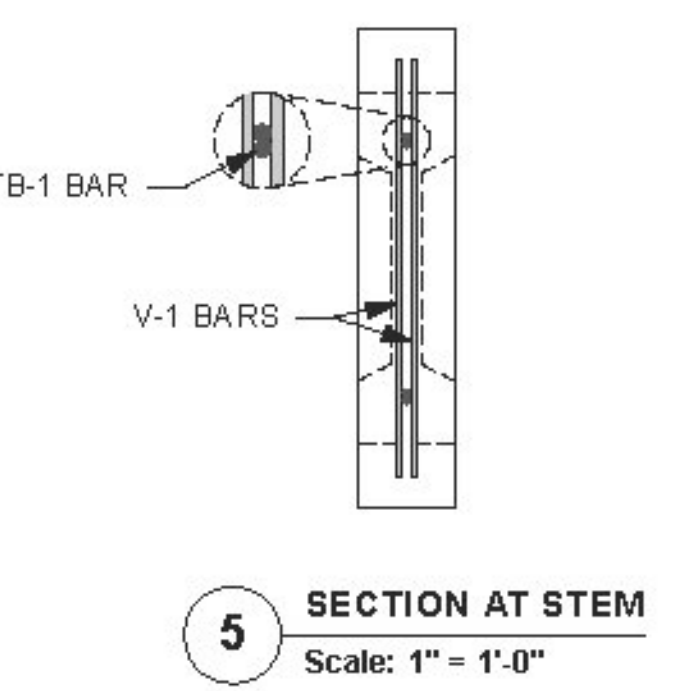
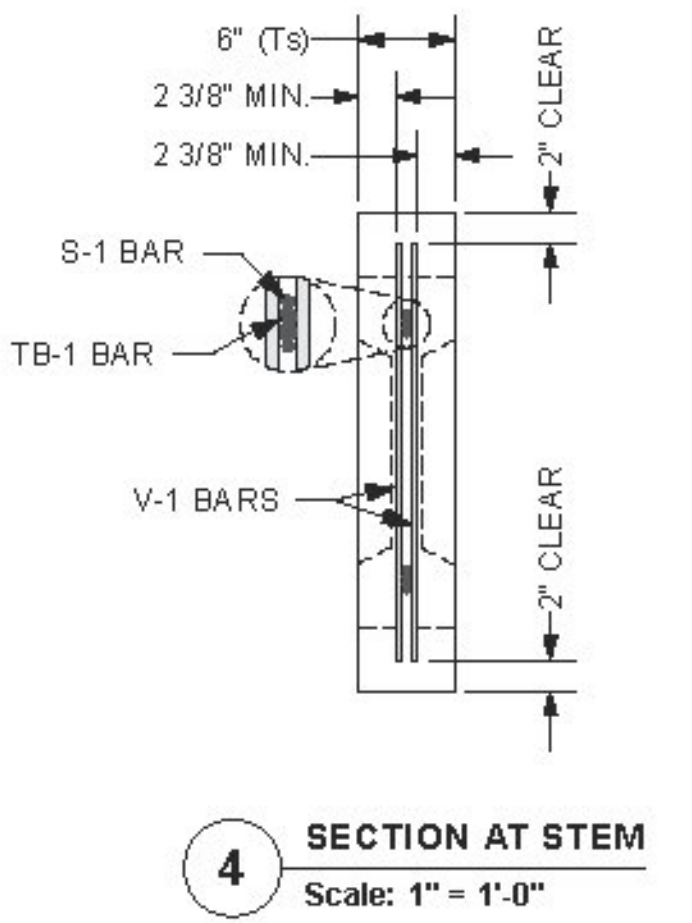
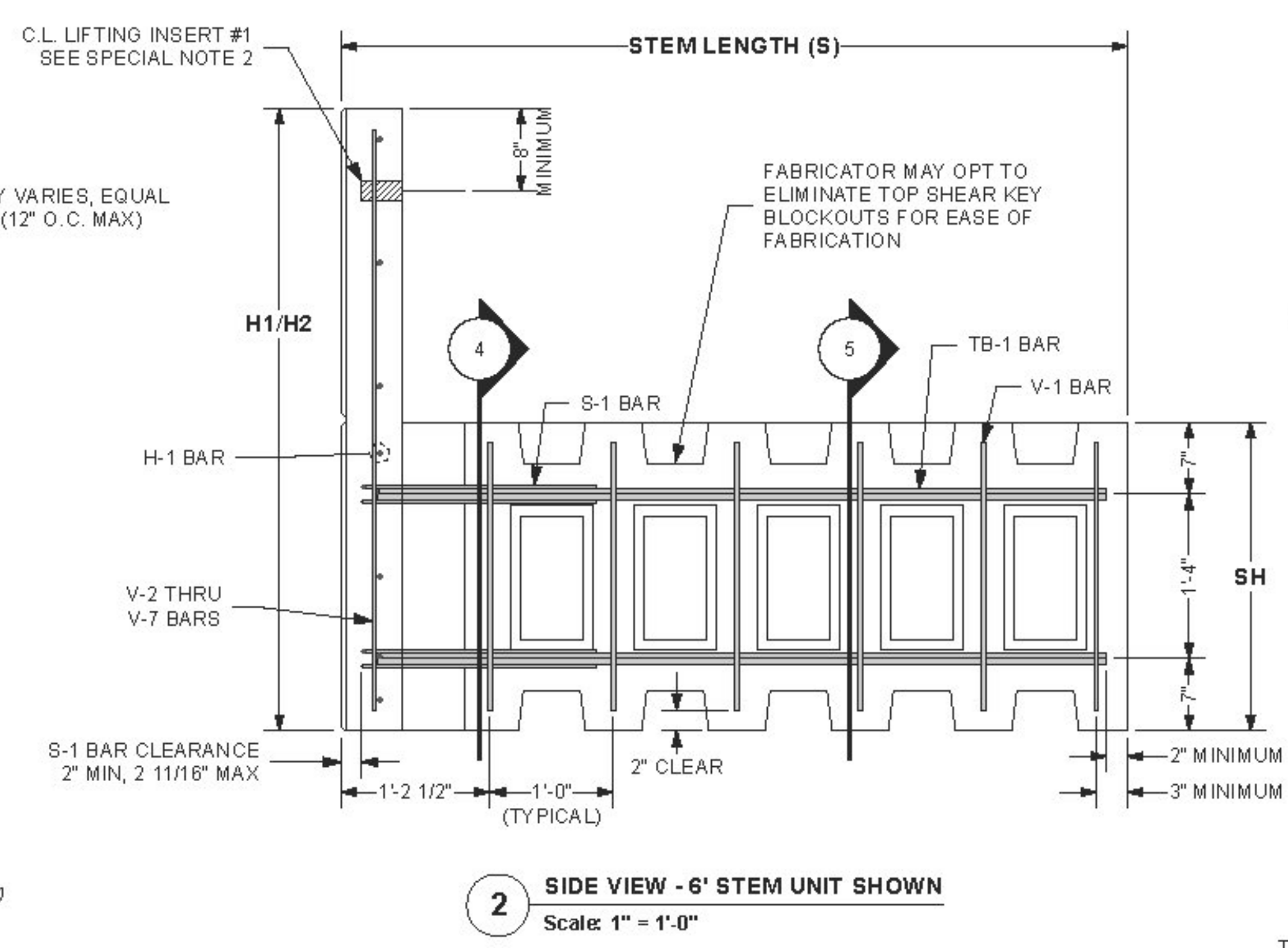
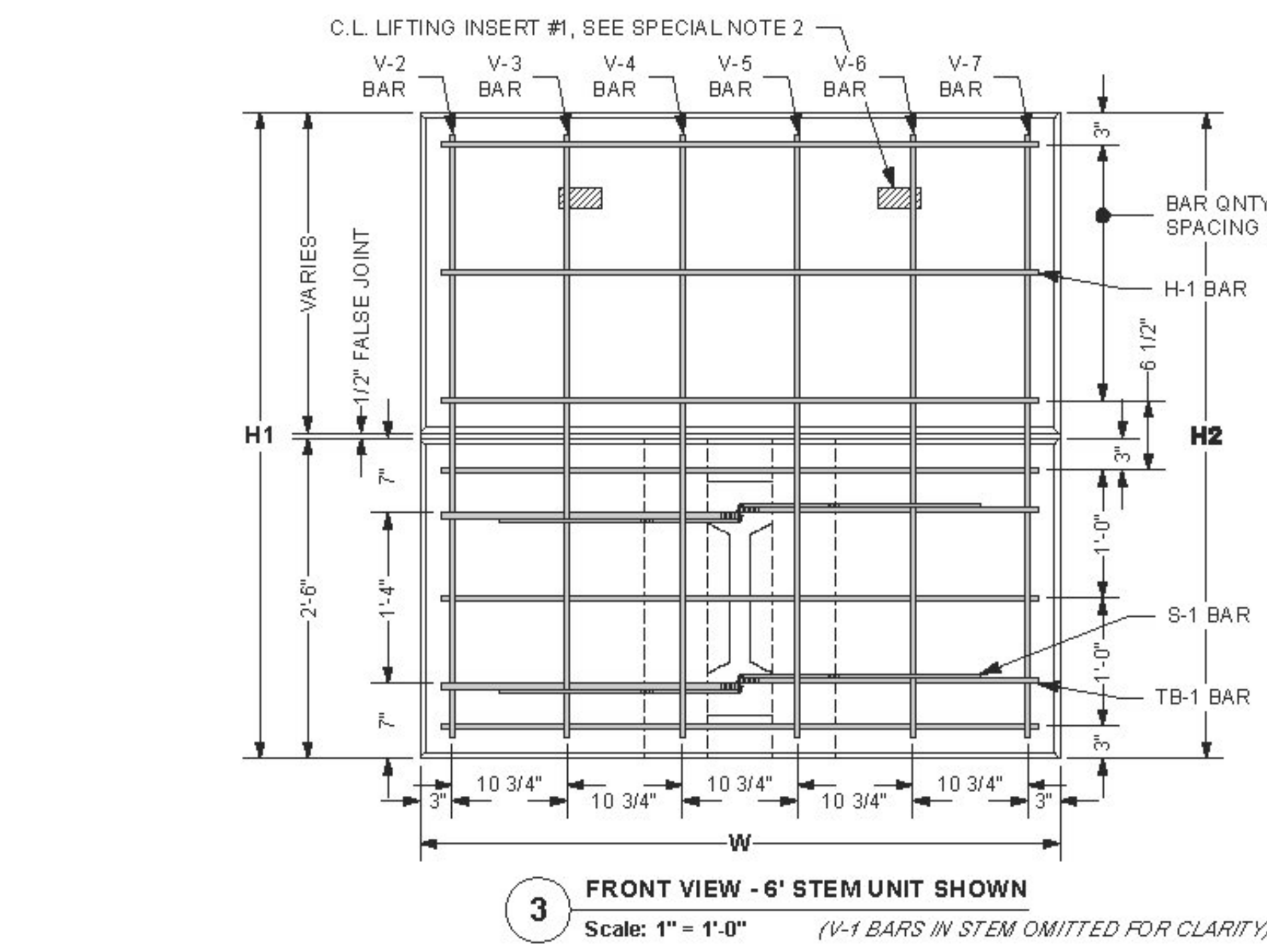


- GENERAL NOTES:**
- PRIMARY REFERENCE:
    - AASHTO, LRFD BRIDGE DESIGN SPECIFICATION, 5TH EDITION 2010 (WITH INTERIMS)
  - T-WALL@ CONCRETE:
    - F'c = 5000 psi (MINIMUM) @ 28 DAYS
    - MINIMUM STRIPPING STRENGTH: 2500 psi
  - T-WALL@ REINFORCING STEEL:
    - BLACK
    - Fy = 60 ksi (GRADE 60)
    - WELDING IS NOT PERMITTED**
  - MARKING OF PRECAST UNITS:
    - CLEARLY MARK EACH PRECAST UNIT ON THE BUTT END OF THE STEM WITH THE UNIT TYPE (i.e. 2.5x5.0x06 STD), THE DATE OF MANUFACTURE, THE LOT NUMBER (IF APPLICABLE), AND THE TRADEMARK 'T-WALL@'.
  - REINFORCING FABRICATION AND PLACEMENT TOLERANCES:
    - THE STRUCTURAL DESIGN OF PRECAST UNITS ASSUMES 2 INCHES OF CONCRETE COVER OVER ALL REINFORCING BARS.
    - UNLESS OTHERWISE NOTED IN CONTRACT DOCUMENTS OR REFERENCED SPECIFICATIONS, TOLERANCES ON CONCRETE COVER SHALL BE ± 3/8 INCHES.
    - UNLESS OTHERWISE NOTED IN CONTRACT DOCUMENTS OR REFERENCED SPECIFICATIONS, TOLERANCES ON BAR PLACEMENT SHALL BE:
      - VERTICAL LOCATION OF TB-1 BARS: ± 3/8 INCHES
      - LOCATION / SPACING OF H-1, V-1 & V-2 BARS: ± 1 INCH
    - REGARDLESS OF THE SPECIFIED PLACEMENT TOLERANCES, CONCRETE COVER SHALL BE MAINTAINED WITHIN ± 3/8 INCHES AS PREVIOUSLY NOTED.
    - ALL REINFORCING BARS SHALL BE CUT AND BENT FOLLOWING REQUIREMENTS OF THE CRSI MANUAL OF STANDARD PRACTICE.
    - UNLESS NOTED OTHERWISE, TOLERANCES FOR BAR FABRICATION SHALL MEET REQUIREMENTS OF STANDARD A CI 318 AND THE CRSI MANUAL OF STANDARD PRACTICE.



- SPECIAL NOTES:**
- FRONT FACE OF T-WALL@ UNITS FINISH TREATMENT:
    - PLAIN STEEL FORM FINISH
  - LIFTING INSERTS CAPACITY:
    - TWO QUICKLIFT 'QL050G' LIFTING INSERTS OR EQUAL, SPACED AT LEAST 30" APART.
    - 2000 LBS (1 TON) MINIMUM RATED WORKING LOAD CAPACITY.
    - MINIMUM CONCRETE STRENGTH SHALL BE 3,500 psi PRIOR TO STRIPPING AND LIFTING OPERATIONS.
  - 1/2" FALSE JOINT LOCATION:
    - IF H-1 IS GREATER THEN 2'-6", THEN FIRST FALSE JOINT WILL BE 2'-6" FROM THE BOTTOM OF THE UNIT.
    - IF H-1 IS GREATER THEN 5'-0", THEN SECOND FALSE JOINT WILL BE 2'-6" ABOVE THE FIRST FALSE JOINT.
    - THE FALSE JOINT WILL 1/2" HIGH AND 1/2" DEEP.

**REBAR SCHEDULES**

Unit Dims	Bar Mark	Qty	Size	Length	Dim "A"	Bar Weight	Bend Dia	Remarks
H=VARIES	H-1	VARIES	#4	4'-8"				SEE SLOPED TOP UNIT SCHEDULE
W=5'-0"	V-1	12 ea	#4	2'-2"		17.37 lbs		
S=6'-4 1/2"	V-2 THRU V-7	1 ea	#5	VARIES				SEE SLOPED TOP UNIT SCHEDULE
SH=2'-6"	S-1	4 ea	#4	3'-4 1/2"		9.02 lbs	D=3"	
	TB-1	4 ea	#4	8'-3"	5'-11"	22.04 lbs	D=3"	

**HIGHWAY REBAR**

**SLOPED TOP UNIT SCHEDULE:**

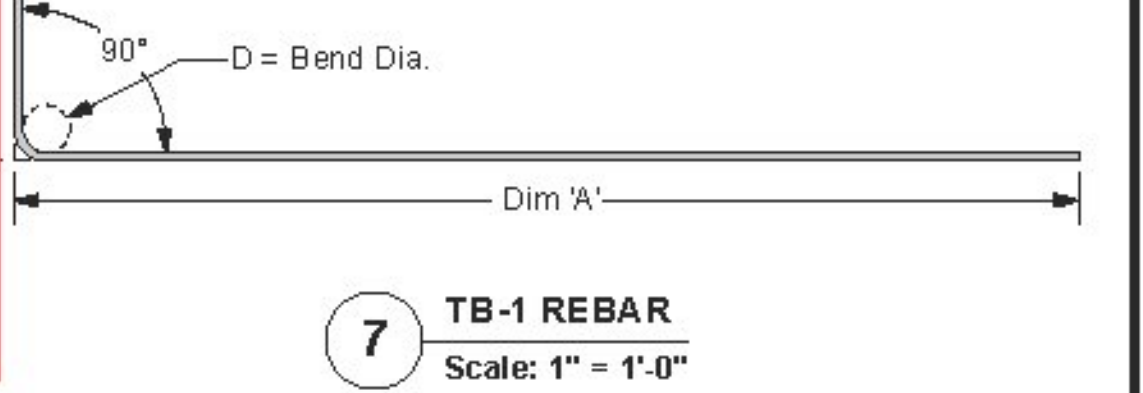
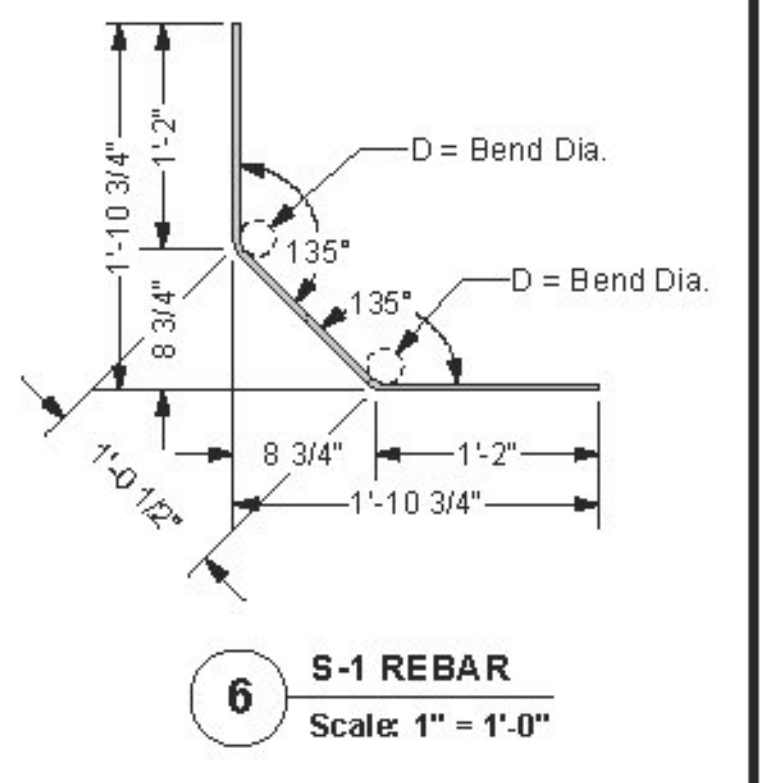
MARK No.	QNTY	STEM	WIDTH	H1	H2	H-1 BAR	V-2 BAR	V-3 BAR	V-4 BAR	V-5 BAR	V-6 BAR	V-7 BAR	VOL	WEIGHT	AREA
SP2	1 ea	6'-4 1/2"	5'-0"	3'-10 1/8"	3'-11"	5 ea	3'-5 5/8"	3'-3 3/4"	3'-2"	3'-1 1/8"	2'-9 1/2"	0.56 cy	2,266 lbs	17.30 sf	
SP3	1 ea	6'-4 1/2"	5'-0"	5'-7 1/2"	4'-10 3/8"	7 ea	5'-3"	5'-1 1/8"	4'-11 3/8"	4'-10 3/8"	4'-8 5/8"	0.72 cy	2,934 lbs	26.20 sf	
SP4	1 ea	6'-4 1/2"	5'-0"	4'-10 3/8"	4'-11 1/4"	6 ea	4'-5 7/8"	4'-4"	4'-2 1/4"	4'-1 1/4"	3'-11 1/2"	0.65 cy	2,649 lbs	22.40 sf	
SP5	1 ea	6'-4 1/2"	5'-0"	4'-11 1/4"	3'-4 1/8"	5 ea	3'-8 3/4"	3'-6 7/8"	3'-5 1/8"	3'-4 1/8"	3'-2 3/8"	0.58 cy	2,364 lbs	18.60 sf	
SP6	1 ea	6'-4 1/2"	5'-0"	5'-10 5/8"	5'-11 1/2"	7 ea	5'-6 1/8"	5'-4 1/4"	5'-2 1/2"	5'-1 1/2"	4'-11 3/4"	0.75 cy	3,032 lbs	27.50 sf	
SP7	1 ea	6'-4 1/2"	5'-0"	4'-8 1/2"	6'-5 1/2"	7 ea	4'-5 5/8"	4'-9 7/8"	5'-2"	5'-4 1/8"	5'-8 1/4"	0.76 cy	3,063 lbs	27.93 sf	
SP8	1 ea	6'-4 1/2"	5'-0"	3'-11"	5'-6"	7 ea	3'-8 1/8"	4'-0 3/8"	4'-4 1/2"	4'-6 5/8"	4'-10 3/4"	0.68 cy	2,767 lbs	23.93 sf	
SP9	1 ea	6'-4 1/2"	5'-0"	3'-11 1/2"	4'-10 1/2"	6 ea	2'-10 3/4"	3'-2 7/8"	3'-7"	3'-9 1/8"	4'-1 1/4"	0.61 cy	2,471 lbs	20.02 sf	
SP11	1 ea	6'-4 1/2"	5'-0"	3'-11 7/8"	4'-11 7/8"	6 ea	2'-11 1/8"	3'-3 1/2"	3'-7 3/4"	3'-10"	4'-2 3/8"	0.62 cy	2,497 lbs	20.38 sf	
SP12	1 ea	6'-4 1/2"	5'-0"	2'-5 3/8"	4'-3 3/8"	5 ea	2'-2 5/8"	2'-7"	2'-11 1/4"	3'-1 1/2"	3'-5 7/8"	0.55 cy	2,231 lbs	16.82 sf	
SP15	1 ea	6'-4 1/2"	5'-0"	5'-3 1/2"	4'-6 3/4"	6 ea	4'-11"	4'-9 1/4"	4'-7 1/2"	4'-6 5/8"	4'-4 7/8"	0.70 cy	2,816 lbs	24.62 sf	
SP16	1 ea	6'-4 1/2"	5'-0"	4'-6 3/4"	3'-10"	6 ea	4'-2 1/4"	4'-0 1/2"	3'-10 3/4"	3'-9 7/8"	3'-6 1/2"	0.63 cy	2,542 lbs	20.97 sf	
SP17	1 ea	6'-4 1/2"	5'-0"	3'-10"	3'-11 1/4"	5 ea	3'-5 1/2"	3'-3 3/4"	3'-2"	3'-1 1/8"	2'-11 3/8"	0.56 cy	2,268 lbs	17.32 sf	
SP18	1 ea	6'-4 1/2"	5'-0"	5'-7 3/4"	4'-11"	7 ea	5'-3 3/8"	5'-1 5/8"	4'-11 7/8"	4'-11"	4'-9 1/4"	0.73 cy	2,951 lbs	26.43 sf	

**Approved As Noted**

This review is only for general conformance with the design concept and the information given in the Construction Documents. Corrections or comments made on the shop drawings during the review do not relieve the Contractor from compliance with the requirements of the Plans and Specifications. Review of a specific item shall not include review of an assembly of which an item is a component. The Contractor is responsible for dimensions to be confirmed and corrected at the job site; information that pertains solely to the fabrication process or to the means, methods, techniques, sequences and procedures of construction; coordination of the Work with that of other trades and performing all Work in a safe and satisfactory manner.

Date 8/8/2014  
By T. Traver

**McFarland Johnson**



The design contained on these drawings is based upon information provided by the owner. On the basis of this information, The Neel Company has designed, and is responsible for, the internal stability of the structure only. External stability, including foundation and slope stability, is the responsibility of the owner.

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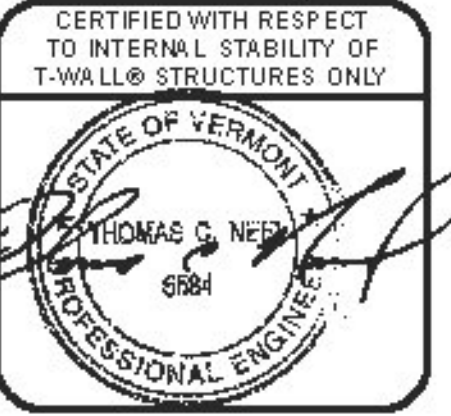
**PRECASTER:** CONCRETE SYSTEMS, INC. CSI  
**PROJECT #:** T21882

**CONTRACTOR:** AL. ST. ONGE CONTRACTORS  
**PROJECT #:**

**DESIGNER**

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WEB: WWW.NEELCO.COM

**PROJECT #:** TW4301



**REVISIONS**

NO.	REVIEWER COMMENTS	ABC	8-6-14
1			

**RIGIDIFIED TUBE FRP ARCH (RTFA) PROJECT**

FAIRFIELD, VT

SHOP DRAWINGS  
SLOPED TOP UNITS  
REBAR AND DIMENSIONS

T-WALL@ RETAINING WALL SYSTEM

SCALE: AS NOTED

DATE: 4/21/14

DESIGNED BY: KD

DRAWN BY: ABC

CHECKED BY: CCG

SHEET: 8