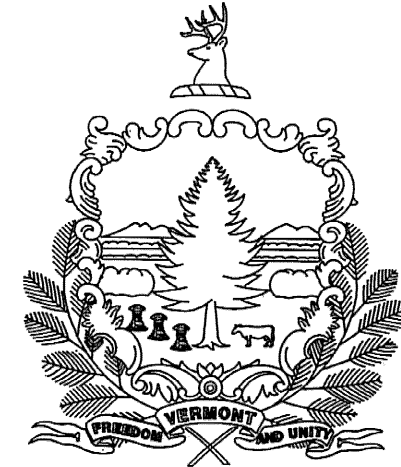


STATE OF VERMONT  
 AGENCY OF TRANSPORTATION



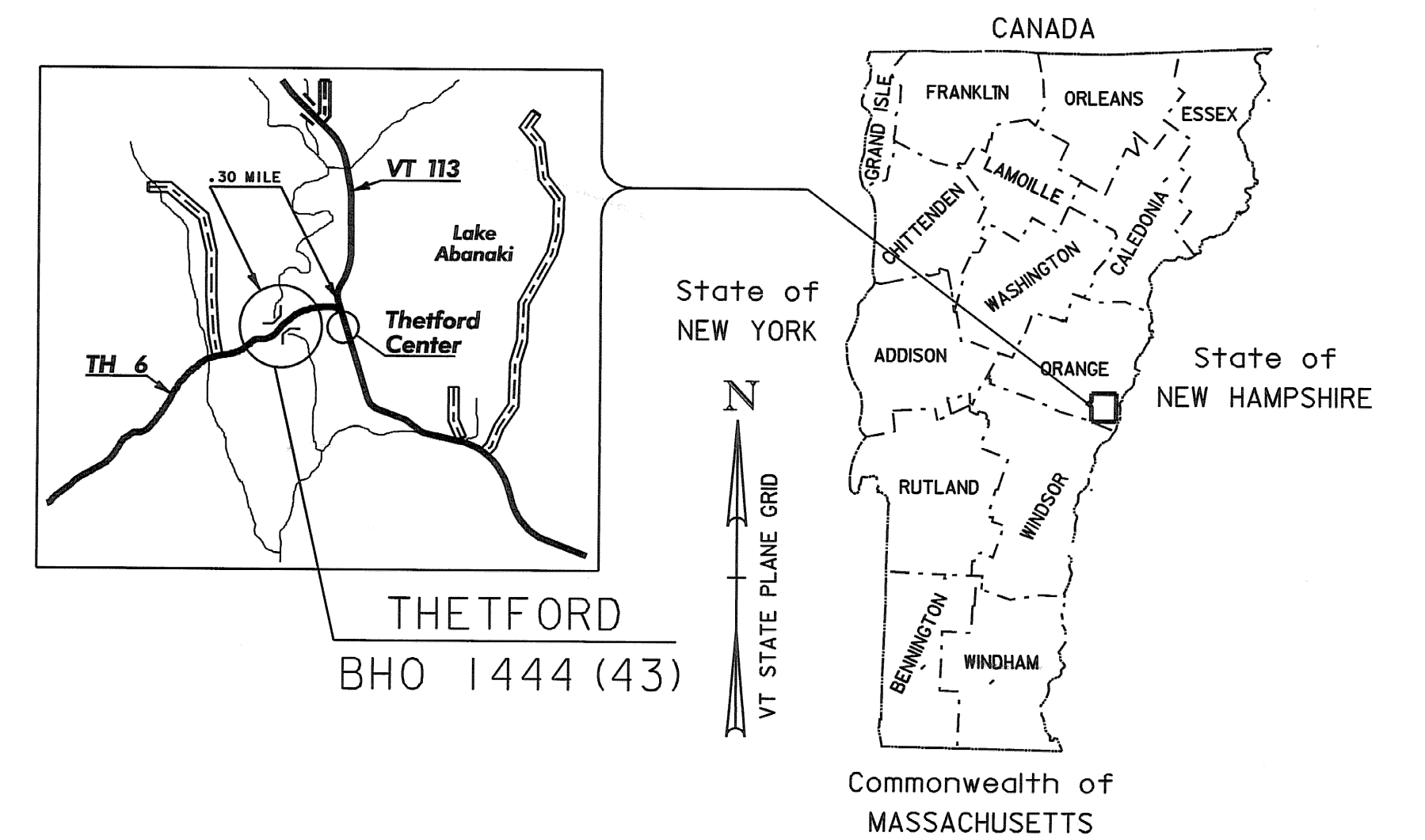
PROPOSED IMPROVEMENT  
 BRIDGE REHABILITATION PROJECT  
 TOWN OF THETFORD CENTER  
 COUNTY OF ORANGE

THETFORD BHO 1444 (43)  
 BRIDGE NO. 27 TOWN HIGHWAY 6 (CLASS 2)

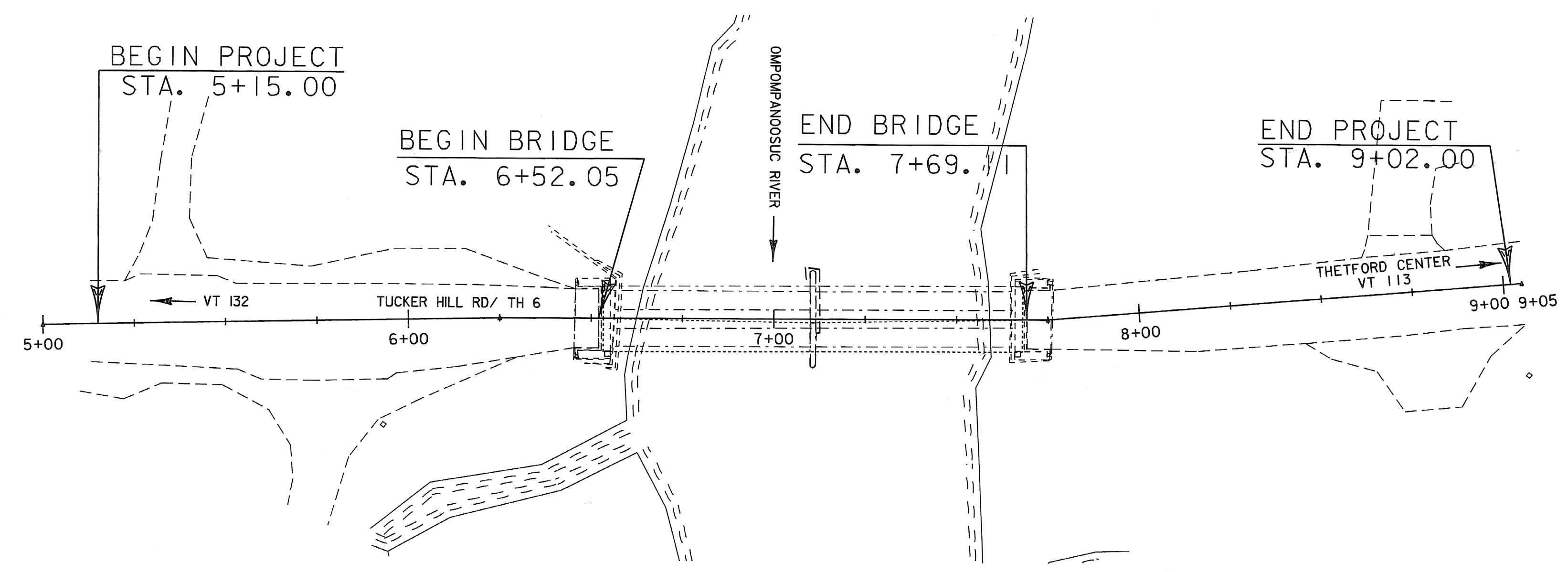
PROJECT LOCATION: BEGINNING APPROXIMATELY 0.30 MILES SOUTH WEST ON TOWN HIGHWAY NO. 6 MEASURED FROM THE INTERSECTION OF TH. 6 AND VT 113 AND EXTENDING NORTH EAST 0.07 MILES.

PROJECT DISCRIPTION: REHABILITATION OF SAYERS COVERED BRIDGE

LENGTH OF ROADWAY = 269.94 FT.  
 LENGTH OF BRIDGE = 117.06 FT.  
 LENGTH OF PROJECT = 387.0 FT.

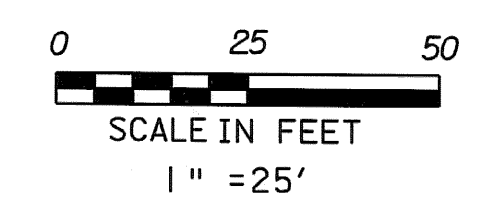


RECORD PLANS	
CONTRACTOR:	ALPINE CONSTRUCTION, LLC - SCHUYLerville, NY
RESIDENT ENGINEER:	DAN BREER
CONSTRUCTION BEGAN:	APRIL 2, 2007
CONSTRUCTION COMPLETE:	SEPTEMBER 14, 2008
RECORD PLANS BY:	DAN BREER & NICK GARBACIK
I HEREBY CERTIFY THAT ALL THE CONSTRUCTION REQUIRED BY THIS SET OF DRAWINGS HAS BEEN ACCOMPLISHED AS INDICATED HEREIN.	
BY:	<i>DW Breer</i> RESIDENT ENGINEER
DATE:	10/30/09
NOTE: Any further information concerning final quantities, amounts or other details relative to this project may be found at Central Files in the electronic archives.	



CONVENTIONAL SYMBOLS	
COUNTY LINE	
TOWN LINE	
LIMITS OF ACCESS	
POINT OF ACCESS	
FENCE LINE	
STONE WALL	
TRAVELED WAY	
GUARD RAIL	
RAILROAD	
SURVEY LINE	
CULVERT	
POWER POLE	
TELEPHONE POLE	
TREES	
CONTROL OF ACCESS	
PROPERTY LINE	
R.O.W. TAKING LINE	
SLOPE RIGHTS	
TOP OF CUT	
TOE OF SLOPE	

SURVEYED BY : GILMAN  
 SURVEYED DATE : 2003  
 DATUM  
 VERTICAL NAVD88  
 HORIZONTAL NAD 83 (1996)



THESE PLANS ARE SUBJECT TO SUCH ENGINEERING CHANGES AS MAY BE REQUIRED BY THE FEDERAL HIGHWAY ADMINISTRATION OR THE DIRECTOR OF PROGRAM DEVELOPMENT.  
 CONSTRUCTION IS TO BE CARRIED ON IN ACCORDANCE WITH THESE PLANS AND THE STANDARD SPECIFICATIONS FOR CONSTRUCTION DATED 1995, AS APPROVED BY THE FEDERAL HIGHWAY ADMINISTRATION ON MARCH 15, 1990 FOR USE ON THIS PROJECT, INCLUDING ALL SUBSEQUENT REVISIONS AND SUCH REVISED SPECIFICATIONS AND SPECIAL PROVISIONS AS ARE INCORPORATED IN THESE PLANS.

DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATOR
APPROVED <i>Mark P. Ricketts</i> DATE 10-10-06
DIRECTOR OF PROGRAM DEVELOPMENT
APPROVED <i>Paul J. Hunt</i> DATE 9-27-06
PROJECT MANAGER : M. EVANS-MONGEON
PROJECT NAME : THETFORD
PROJECT NUMBER : BHO 1444 (43)
SHEET 1 OF 60 SHEETS

# PRELIMINARY INFORMATION SHEET

## INDEX OF SHEETS

1. TITLE SHEET
2. PRELIMINARY INFORMATION SHEET
3. ROAD TYPICAL
4. BRIDGE TYPICAL
- 5.-6. QUANTITY SHEETS.
7. FINISHED PORTAL OPENING
8. TIE SHEET
9. LAYOUT SHEET
10. PROFILE
11. MATERIAL TRANSITION / BANKING DIAGRAM
- 12.-14. TEMPORARY DETOUR SIGNAGE AND ROUTING
15. SIGN LAYOUT SHEET (PERM.)
16. TRAFFIC SIGN SUMMARY SHEET.
17. PLAN AND ELEVATION.
- 18.-19. GENERAL NOTES.
20. EROSION CONTROL NARRATIVE
- 21.-23. EROSION AND SEDIMENT CONTROL PLAN
- 24.-27. EROSION AND SEDIMENT CONTROL DETAILS
28. NORTH TRUSS REPLACEMENT EXISTING / PROPOSED
29. TIMBER TRUSS ASSEMBLY AND CONFIGURATION SHEET
30. GLUE LAMINATED DECK SECTION DETAILS
31. GLUE LAMINATED DECK LAYOUT SHEET
32. PLANK WEARING COURSE LAYOUT
33. TIMBER CURB ATTACHMENT TO DECK LAYOUT SHEET
34. LOWER CHORD TO EXTERIOR GIRDER ATTACHMENT DETAILS
35. ABUTMENT #1 TYPICAL
36. ABUTMENT CAP #1 DETAIL SHEET
37. ABUTMENT CAP #1 REBAR LAYOUT SHEET
38. REINFORCING STEEL SCHEDULE
39. STEEL BACKED TIMBER RAIL POST PLACEMENT SHEET
40. STEEL BACKED TIMBER TRANSITION RAIL TO TIMBER BRIDGE CURB.
41. STEEL BACKED TIMBER TRANSITION RAIL TO TIMBER BRIDGE CURB DETAILS.
42. STEEL BACKED TIMBER RAIL STRAIGHT SPLICE PLATE DETAILS
43. STEEL BACKED TIMBER RAIL ANGLED SPLICE PLATE DETAILS
44. STEEL BACKED TIMBER RAIL ANCHOR DETAILS
- 45.-48. 11" & MAINLINE CROSS SECTIONS
- 49.-53. OMPOMPANOOSUC CHANNEL SECTIONS
- 54.-60. SCANNED RECORD PLANS

## LIST OF STANDARDS

E-100 CONSTRUCTION APPROACH	JAN. 2, 2004
E-100A SIDE ROAD CONSTRUCTION APPROACH SIGNS	JAN. 2, 2004
E-101 CONSTRUCTION SIGN DETAILS	MAY 30, 2003
E-102 CONSTRUCTION SIGN DETAILS	JUNE 30, 2003
E-102A CONSTRUCTION SIGN DETAILS	MAY 1, 2004
E-107 DELINEATION, BARRACADES AND DETOURS FOR CONSTRUCTION AREAS	JUNE 30, 2003
E-121 STANDARD SIGN PLACEMENT	AUG. 8, 1995
E-136A U.S. ROUTE MARKER SIGN DETAILS	AUG. 8, 1995
E-155 WARNING SIGN DETAILS	MAY 1, 2004
E-160 FLANGED CHANNEL SIGN POST	MAY 20, 1999

## HYDROLOGIC DATA

DRAINAGE AREA: 51.3 SQUARE MILES  
 CHARACTER OF TERRAIN: HILLY, MOSTLY WOODED, RURAL  
 STREAM CHARACTERISTICS: PROBABLY INCISED, LITTLE TO NO FLOODPLAIN, SINUOUS  
 NATURE OF STREAMBED: SILT, LEDGE DOWNSTREAM OF BRIDGE

PEAK FLOW DATA  
 Q 2.33 = 1500 cfs    Q 50 = 5000 cfs  
 Q 10 = 2800 cfs    Q 100 = 8500 cfs  
 Q 25 = 4000 cfs    Q 500 = 8900 cfs

DATE OF FLOOD OF RECORD: 1973  
 ESTIMATED DISCHARGE: UNKNOWN  
 WATER SURFACE ELEV.: UNKNOWN  
 NATURAL STREAM VELOCITY @ Q25 = 5.3 FPS  
 ICE CONDITIONS: MODERATE  
 DEBRIS: MODERATE  
 DOES THE STREAM REACH MAXIMUM HIGHWATER ELEV. RAPIDLY? NO  
 IS ORDINARY RISE RAPID? NO  
 IS STAGE AFFECTED BY UPSTREAM OR DOWNSTREAM CONDITIONS? YES  
 IF YES, DESCRIBE: IF DOWNSTREAM FLOOD DAM IS FULL, BACKWATER COULD REACH AN ELEVATION OF 564.0.

WATERSHED STORAGE: 2.9X  
 HEADWATERS: UNIFORM & IMMEDIATELY ABOVE SITE

## EXISTING STRUCTURE

STRUCTURE TYPE: COVERED BRIDGE ON ROLLED BEAMS    YEAR BUILT: 1839  
 CLEAR SPAN (NORMAL TO STREAM): 104'    MODIFIED IN: 1963  
 VERTICAL CLEARANCE ABOVE STREAMBED: 17'  
 WATERWAY OF FULL OPENING: 1600 SQ. FT.  
 DISPOSITION OF STRUCTURE: REHABILITATION  
 TYPE OF MATERIAL UNDER SUBSTRUCTURE: N/A

WATER SURFACE ELEV. @ Q2.33 = 553.3'    VELOCITY = 3.7 FPS  
 Q10 = 555.5'    "    4.8 FPS  
 Q25 = 556.7'    "    5.4 FPS  
 Q50 = 557.8'    "    6.0 FPS  
 Q100 = 558.8'    "    6.8 FPS

## PROPOSED STRUCTURE

STRUCTURE TYPE: EXISTING BRIDGE TO BE REHABILITATED, NO CHANGES  
 CLEAR SPAN (NORMAL TO STREAM): 104'  
 VERTICAL CLEARANCE ABOVE STREAMBED: 17'  
 WATERWAY OF FULL OPENING: 1600 SQ. FT.

WATER SURFACE ELEV. @ Q2.33 = 553.3'    VELOCITY = 3.7 FPS  
 Q10 = 555.5'    "    4.8 FPS  
 Q25 = 556.7'    "    5.4 FPS  
 Q50 = 557.8'    "    6.0 FPS  
 Q100 = 558.8'    "    6.8 FPS

LONG TERM STREAM BED CHANGES: DOWNSTREAM LEDGE AND FALLS WILL LIMIT ANY STREAM GRADE CHANGES.  
 IS THE ROADWAY OVERTOPPED BELOW THE Q100? NO    FREQUENCY ABOVE Q100 RELIEF ELEVATION: 568.2'    DISCHARGE OVER ROAD @ Q100: NONE

UPSTREAM STRUCTURE: TOWN: THETFORD    DISTANCE: 11,000'  
 HIGHWAY NO.: TH 20    STRUCTURE NO.: BR 26  
 STRUCTURE TYPE: SIMPLE SPAN PLATE GIRDER  
 CLEAR SPAN: 91.5'    CLEAR HEIGHT: 17'  
 YEAR BUILT: 1985    FULL WATERWAY: 1555 SQ. FT.

DOWNSTREAM STRUCTURE: TOWN: THETFORD    DISTANCE: 18,800'  
 HIGHWAY NO.:    STRUCTURE NO.:  
 STRUCTURE TYPE: FLOOD DAM WITH SPILLWAY ELEVATION = 564.0'  
 CLEAR SPAN:    CLEAR HEIGHT:     
 YEAR BUILT:    FULL WATERWAY:

IS THE ROADWAY OVERTOPPED BELOW THE Q100? NO    FREQUENCY ABOVE Q100 RELIEF ELEVATION: 568.2'    DISCHARGE OVER ROAD @ Q100: NONE  
 AVERAGE LOW ELEVATION OF SUPERSTRUCTURE: 564.8'  
 VERTICAL CLEARANCE @ Q25 = 8.1'

SCOUR: MAXIMUM CONTRACTION SCOUR UP TO Q50 = 1'. MAXIMUM PIER SCOUR UP TO Q50 = 5'. TOTAL MAXIMUM SCOUR AT PIER = 6', UNLESS LEDGE IS HIGHER.  
 REQUIRED CHANNEL PROTECTION: STONE FILL, TYPE 1

\* NO WORK IN THE CHANNEL IS ANTICIPATED, AND THEREFORE NO STONE FOR CHANNEL PROTECTION IS REQUIRED.

## DESIGN CRITERIA

1. DESIGN LIVE LOAD AASHTO: H 20
  2. DESIGN SPAN: 2 SPANS AT 57'-0" EACH
  3. ALLOWABLE LOAD FOR SPREAD FOOTINGS ON SOIL: 4 KSF ON LEDGE    10 KSF
  4. ALLOWABLE LOAD FOR PILING: N/A    TYPE: M/A    ESTIMATED LENGTH: N/A
  5. STRUCTURAL STEEL AASHTO GRADE: 50
  6. REINFORCING STEEL GRADE: 60
  7. CONCRETE HIGH PERFORMANCE CLASS A: 4" x 4000 PSI  
 CONCRETE HIGH PERFORMANCE CLASS B: 4" x 3500 PSI
- TRAFFIC MAINTENANCE:  
 1. IS TRAFFIC TO BE MAINTAINED? YES    IF YES, ON EXISTING STRUCTURE: NO    OR ON TEMPORARY BRIDGE: NO    OR TEMPORARY DETOUR ROUTE: YES  
 2. TEMPORARY BRIDGE REQUIREMENTS: ONE OR TWO WAY: NO    TRAFFIC CONTROL SIGNALS REQUIRED: NO
- ARE SIDEWALKS REQUIRED? NO    IF SO, ON WHAT SIDE?

## LOAD FACTOR LOAD RATING (TONS)

LOADING LEVELS (LOAD FACTOR)	TRUCK						
	H20	H20	SS2	6 AXLE	3A. STR.	4A. STR.	5A. SCMI
INVENTORY A=2.175 B=1.00	26	47					
POSTED A=1.559 B=1.00	31	56	105		60	86	105
OPERATING A=1.309 B=1.67		62	118	149	67	96	

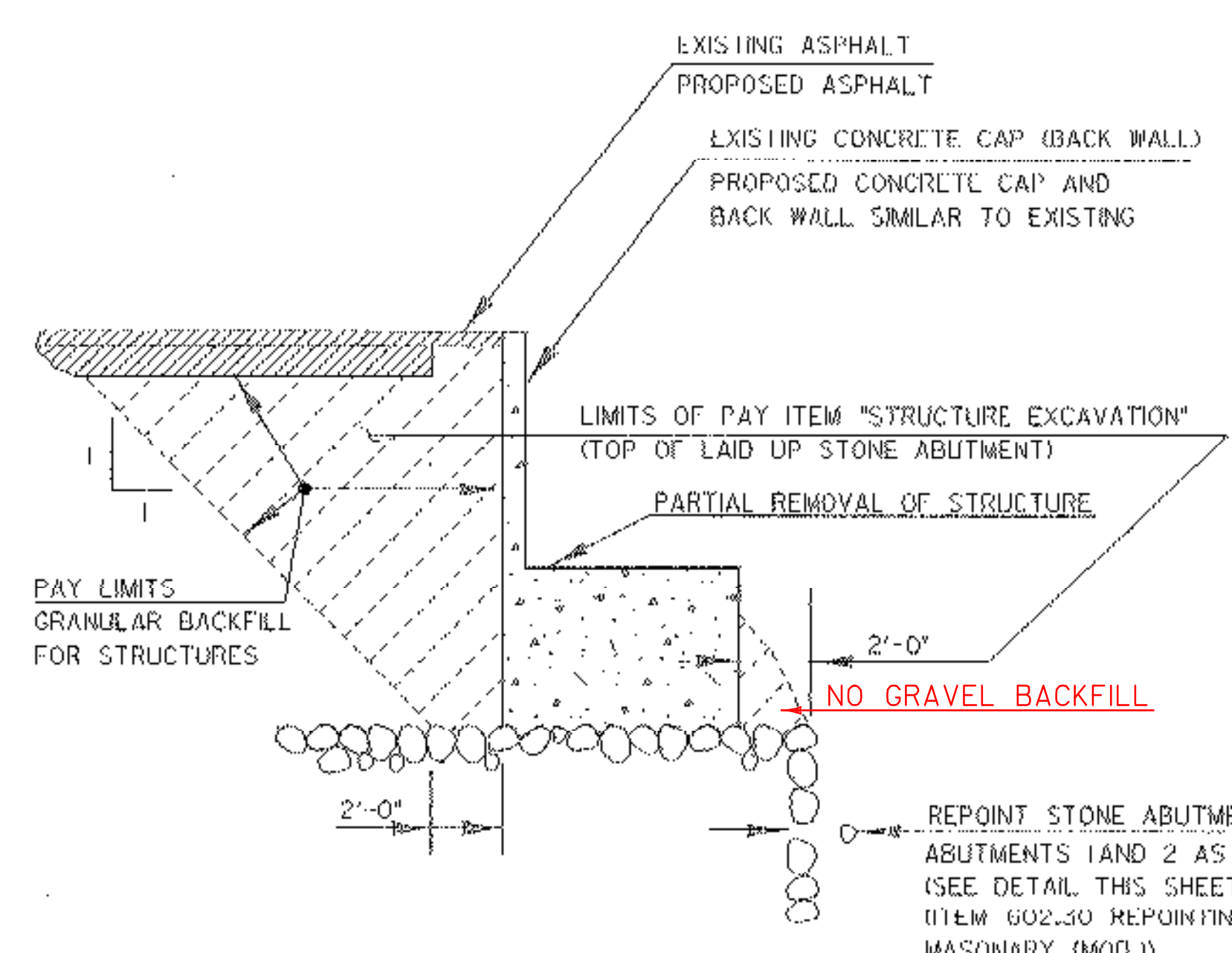
## TRAFFIC DATA

YEAR	ADT	DIV	%	%	ADTT
2006	1000	170	71	4	30
2026	1300	190	71	5	60

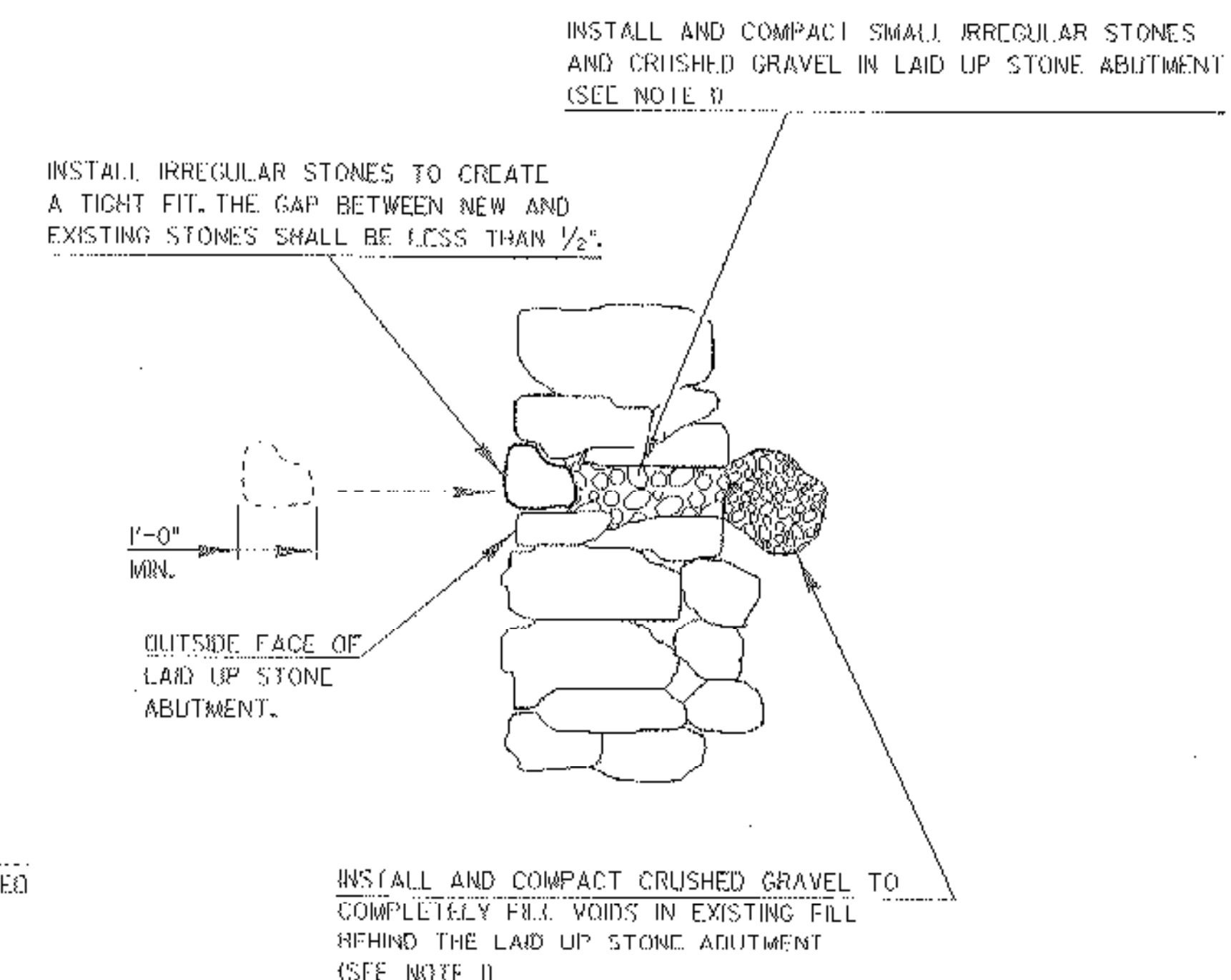
18 kip ESAL for flexible pavement from 2006 to 2026: 137,000  
 Design speed 10 mph.

## PRELIMINARY INFORMATION SHEET

PROJECT NAME: THETFORD  
 PROJECT NUMBER: BHO 1444 (43)  
 FILE NAME: Structures/s03j036p1.dgn    PLOT DATE: 27-SEP-2006  
 PROJECT LEADER: M. EVANS-MONGEON    DRAWN BY: G. ROKES  
 DESIGNED BY: M. EVANS-MONGEON    CHECKED BY: S. SCRIBNER  
 IPARM: s03j036p1.i    SHEET 2 OF 60



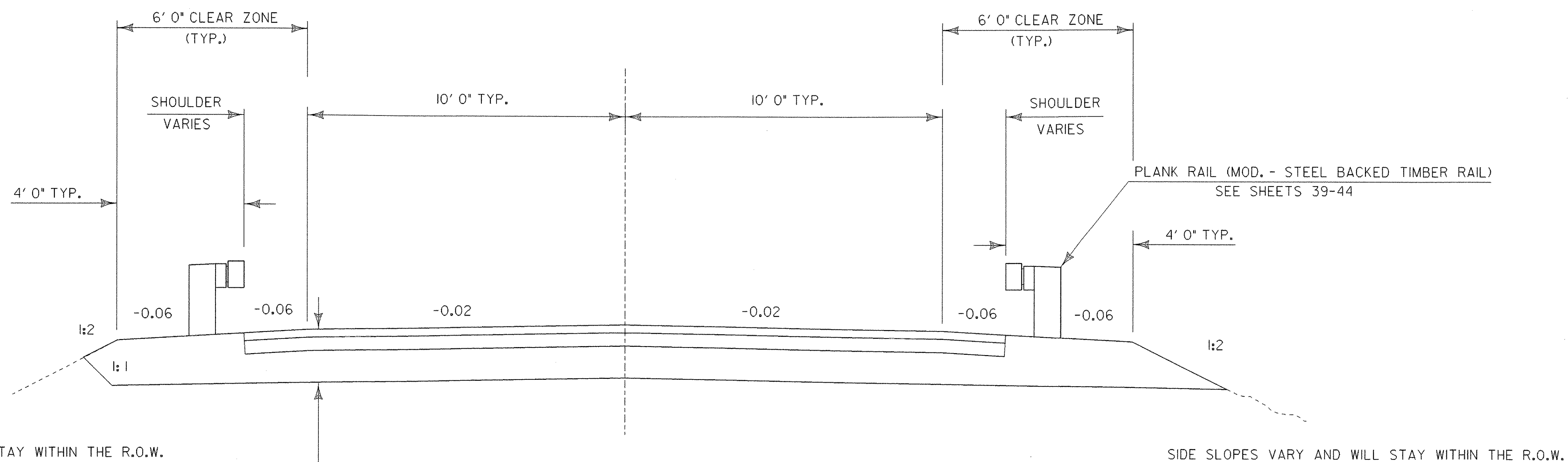
**ABUTMENT NO. 1 TYPICAL AND WINGWALL SECTION**  
 NOT TO SCALE



**REPOINTING STONE ABUTMENTS DETAIL**  
 NOT TO SCALE

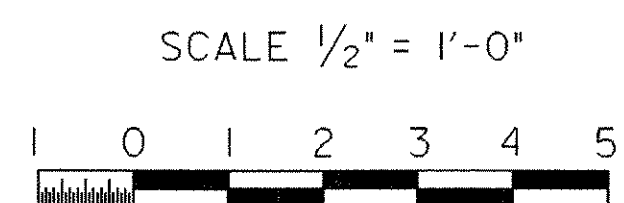
NOTE 1: COMPACT MATERIAL BEHIND AND INTO VOIDS OF STONE ABUTMENTS BY USING TAMPING RODS OR OTHER METHODS ACCEPTABLE TO THE RESIDENT ENGINEER.

MATERIAL ITEM	TOLERANCE
PAVEMENT	± 1/4" TOTAL THICKNESS
BASE COURSE	± 1/2"
SUBBASE	± 1"



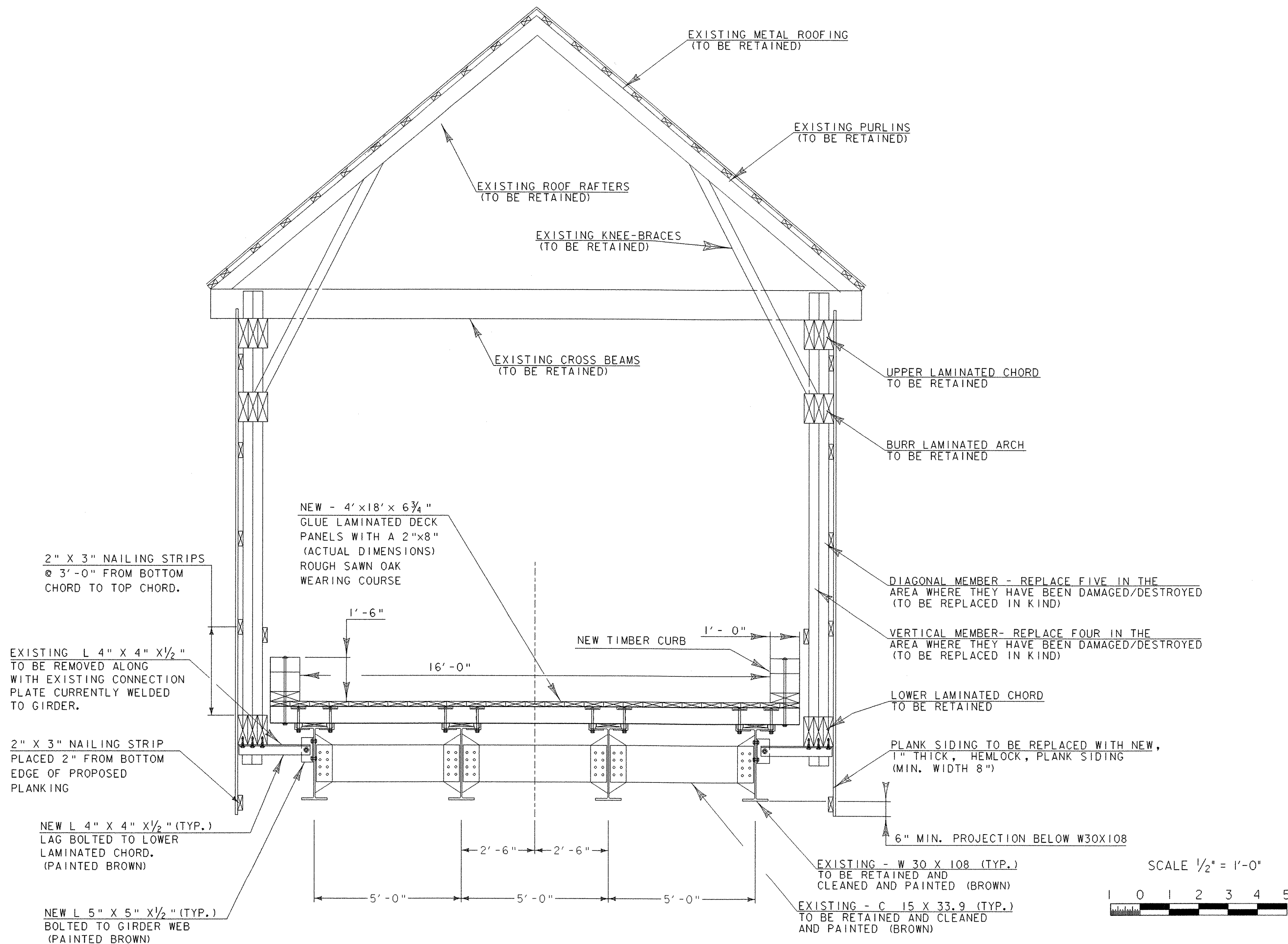
3" B.C.P., TYPE III OR TYPE IV (2 - 1 1/2" LIFTS)  
5" B.C.P., TYPE I OR TYPE II (2 - 2 1/2" LIFTS)  
12" SUBBASE OF GRAVEL

NOTE: THIS TYPICAL IS FOR SUBBASE REPLACEMENT,  
SEE MATERIAL TRANSITION AND CROSS SECTIONS  
FOR AREA NOT REQUIRING SUBBASE REPLACEMENT.



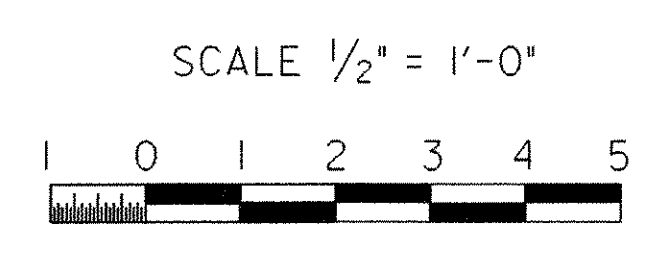
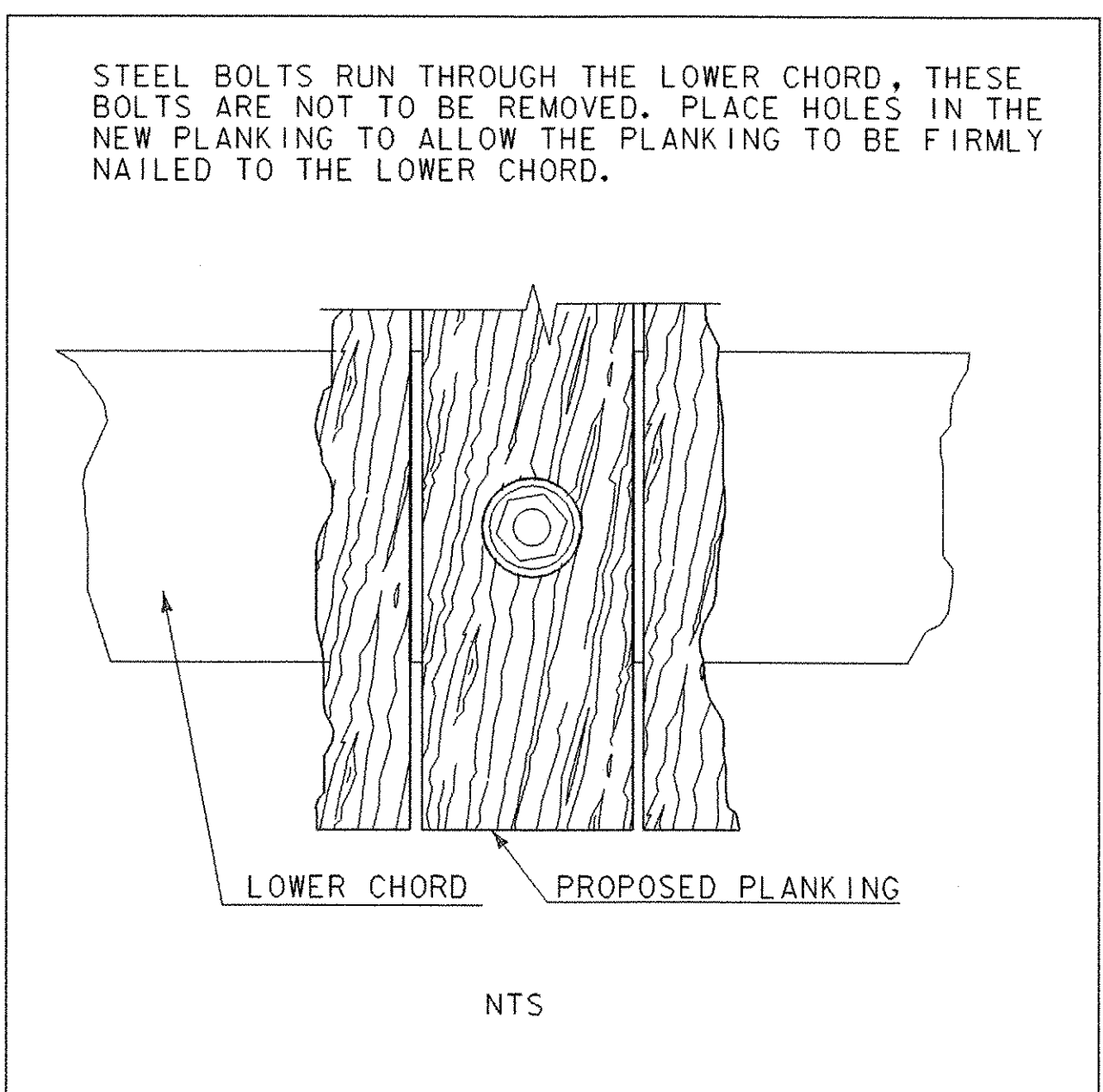
**ROAD TYPICAL**

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G. ROKES
FILE NAME: Structures\s03j036typ.dgn	CHECKED BY: S. SCRIBNER
PROJECT LEADER: M. EVANS-MONGEON	SHEET 3 OF 60
DESIGNED BY: G. ROKES	
IPARM: s03j36tyl1	



**NOTES:**

1. NEW SIDING SHALL CONSIST OF SINGLE VERTICAL PIECES IN ALL LOCATIONS. HORIZONTAL JOINTS IN THE SIDING WILL NOT BE ALLOWED.
2. THE TIMBER PORTION OF THE STRUCTURE WILL BE RAISED IN RELATION TO THE STEEL SUPPORTS, TO MAINTAIN A MINIMUM VERTICAL CLEARANCE OF 12'-6" OVER THE HIGHWAY. CLEARANCE MEASURED AS SHOWN ON "FINISHED PORTAL OPENING" SHEET 7.
3. AT ABUTMENT #1, THE LAID UP STONE PART OF THE ABUTMENT WILL BE REPAIRED AS SHOWN ON SHEET 2 USING ITEM 602.30 REPOINTING MASONRY (MOD.). THE CONCRETE PORTION OF THIS ABUTMENT, BELOW THE LAID UP STONE PORTION WILL BE REPAIRED USING ITEM 580.13, REPAIR OF CONCRETE SUBSTRUCTURE SURFACE, CLASS 1, ITEM 580.14, REPAIR OF CONCRETE SUBSTRUCTURE SURFACE, CLASS 11, AND ITEM 580.15, REPAIR OF CONCRETE SUBSTRUCTURE SURFACE, CLASS 111. THE STEEL BEAMS WILL BE JACKED UP USING ITEM 502.10 SHORING SUPERSTRUCTURE, THE EXISTING CONCRETE CAP WILL BE REMOVED AND A NEW CONCRETE CAP WILL BE CONSTRUCTED AS SHOWN ON SHEETS 36 AND 37.
4. THE BEAMS AT ABUTMENT #2 WILL NOT BE JACKED, THE BEARINGS WILL REMAIN IN PLACE AND BE CLEANED INCIDENTAL TO ITEM 513.41, "SURFACE PREPARATION, FIELD".
5. THE PIER WILL NOT BE JACKED, THE BEARINGS WILL REMAIN IN PLACE AND BE CLEANED INCIDENTAL TO ITEM 513.41, "SURFACE PREPARATION, FIELD".



<b>BRIDGE TYPICAL</b>		<b>Revised Oct. 24, 2006</b>	
PROJECT NAME:	THETFORD		
PROJECT NUMBER:	BHO 1444 (43)		
FILE NAME:	Structures*J036typ.dgn	PLOT DATE:	24-OCT-2006
PROJECT LEADER:	M. EVANS-MONGEON	DRAWN BY:	G. ROKES
DESIGNED BY:	G. ROKES	CHECKED BY:	S. SCRIBNER
IPARM:	s03J036ty2.1	SHEET	4 OF 60

# QUANTITY SHEET

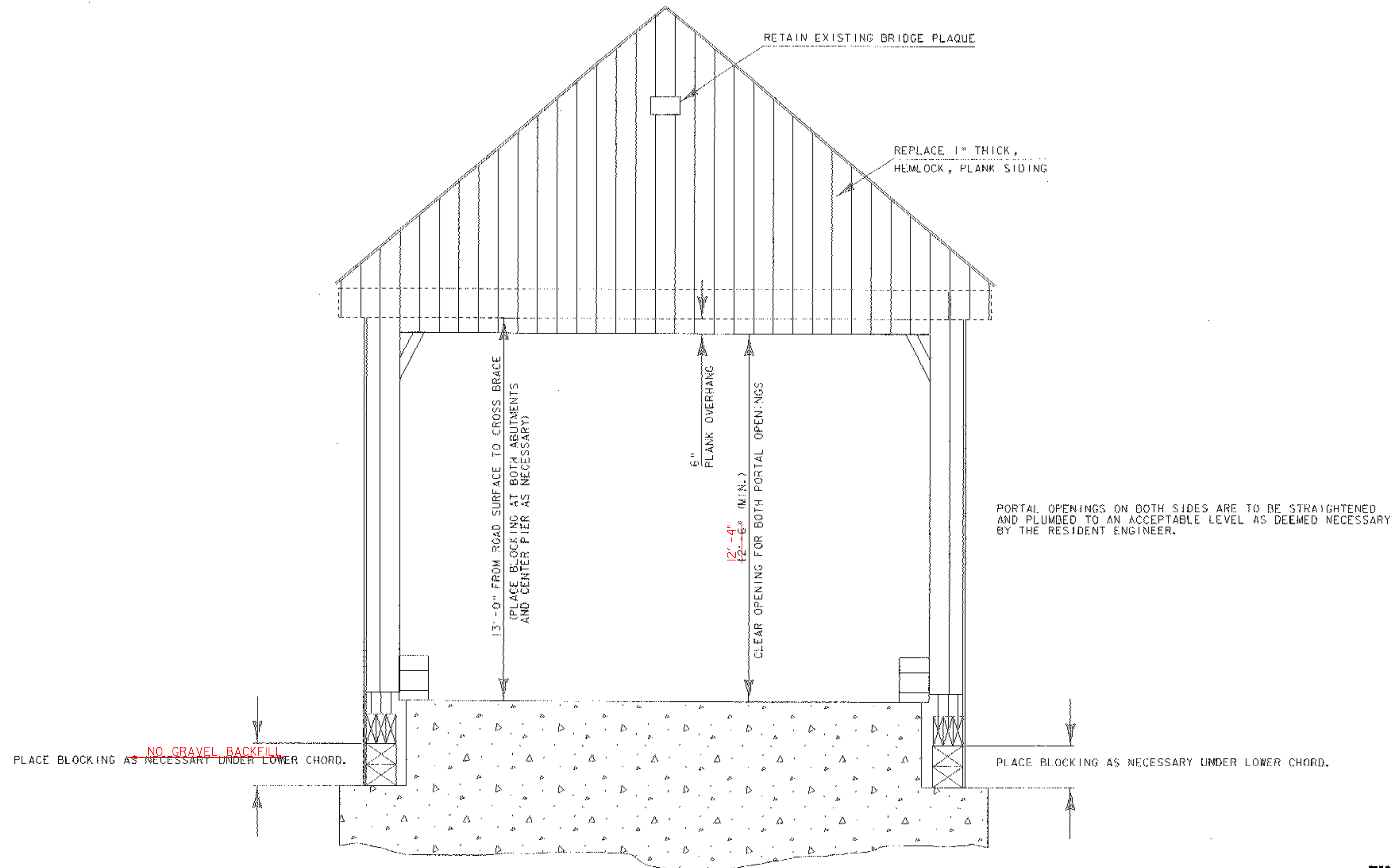
SUMMARY OF ESTIMATED QUANTITIES													TOTALS			DESCRIPTIONS			DETAILED SUMMARY OF QUANTITIES		
FULL E&C ITEMS	ROADWAY	ABUT. 1	ABUT. 2	PIER	DECK	TRUSS	EROSION CONTROL	BRIDGE QUANTITY	ROUND	GRAND TOTAL	FNAL	UNIT	ITEMS	ITEM NUMBER	QUAN TITIES	UNIT	ITEMS				
	1									1		LS	CLEARING AND GRUBBING (INCL. INDV. TREES AND STUMPS)	201.10							
	130									130		CY	COMMON EXCAVATION	203.15							
	225									225		CY	EXCAVATION OF SURFACES AND PAVEMENTS	203.28							
							10			10		CY	TRENCH EXCAVATION OF EARTH	204.20							
		150	30					180		180		CY	STRUCTURE EXCAVATION	204.25							
		120						120		120		CY	GRANULAR BACKFILL FOR STRUCTURES	204.30							
	190						10			200		CY	SUBBASE OF GRAVEL	301.15							
	3									3		CWT	EMULSIFIED ASPHALT	404.65							
	325									325		TON	BITUMINOUS CONCRETE PAVEMENT (PG 58-28)	406.25							
		16						16		16		CY	CONCRETE, HIGH PERFORMANCE CLASS B	501.34							
						1		1		1		LS	SHORING SUPERSTRUCTURE (COVERED BRIDGE)	502.10							
					4540			4540		4540		LB	STRUCTURAL STEEL	506.60							
		1100						1100		1100		LB	EPOXY COATED REINFORCING STEEL	507.17							
					1			1		1		LS	STRUCTURAL PAINTING, SHOP APPLIED (2 TONS)	513.25							
						1		1		1		LS	STRUCTURAL PAINTING, FIELD APPLIED (MOD. - INSECTICIDE AND FIRE RETARDANT)	513.30							
					1			1		1		LS	STRUCTURAL PAINTING, FIELD APPLIED (35 TONS)	513.30							
					1			1		1		LS	CONTAINMENT & ENVIRONMENTAL PROTECTION, FIELD	513.36							
						1		1		1		LS	CONTAINMENT & ENVIRONMENTAL PROTECTION, FIELD (MOD. - INSECTICIDE AND FIRE RETARDANT)	513.36							
					1			1		1		LS	SURFACE PREPARATION, SHOP (2 TONS)	513.40							
					1			1		1		LS	SURFACE PREPARATION, FIELD (35 TONS)	513.41							
		1	1	4				6		6		GAL	WATER REPELLENT (MOD. - SILANE)	514.10							
					4.8			4.8		4.8		MFBM	STRUCTURAL LUMBER AND TIMBER - UNTREATED	522.20							
						7.1		7.1		7.1		MFBM	STRUCTURAL LUMBER AND TIMBER - TREATED	522.25							
						6		6		6		MFBM	NON - STRUCTURAL LUMBER-UNTREATED	522.30							
					1			1		1		LS	STRUCTURAL GLUED LAMNATED TIMBER (DECK)	522.40							
					7			7		7		GAL	JOINT SEALER, COLD POURED	524.12							
	1							1		1		EACH	PARTIAL REMOVAL OF STRUCTURE	529.20							
	4							4		4		EACH	BEARING DEVICE ASSEMBLY (FABRIC)	531.10							
	2							2		2		SY	REPAIR OF CONCRETE SUBSTRUCTURE SURFACE CLASS I	580.13							
	2							2		2		SY	REPAIR OF CONCRETE SUBSTRUCTURE SURFACE CLASS II	580.14							
	2							2		2		CY	REPAIR OF CONCRETE SUBSTRUCTURE SURFACE CLASS III	580.15							
	32							32		32		SY	REPOINTING MASONRY (MOD.)	602.30							
							1			1		HR	BULLDOZER RENTAL, TYPE I	608.10							
	2									2		HR	POWER GRADER RENTAL	608.15							
							1			1		HR	ALL PURPOSE EXCAVATOR RENTAL, TYPE I	608.25							
							10			10		CY	STONE FILL, TYPE I (MOD. - CONSTRUCTION ENTRANCE)	613.10							
							2			2		CY	STONE FILL, TYPE I	613.10							
							1			1		TON	BITUMINOUS CONCRETE SIDEWALK (MOD.)	618.15							
							170			170		LF	SNOW FENCE (MOD. - PDF)	620.70							

PROJECT NAME: **THETFORD**  
 PROJECT NUMBER: **BHO 1444 (43)**  
 FILE NAME: s03j036qty.dgn PLOT DATE: 2/1/2006  
 PROJECT MANAGER: M. EVANS-MONGEON DRAWN BY: G. ROKES  
 DESIGNED BY: G. ROKES CHECKED BY: S. SCRIBNER  
 QUANTITY SHEET #1 SHEET 5 OF 60

# QUANTITY SHEET

SUMMARY OF ESTIMATED QUANTITIES													TOTALS			DESCRIPTIONS			DETAILED SUMMARY OF QUANTITIES		
FULL E&C ITEMS	ROADWAY	ABUT. 1	ABUT. 2	PIER	DECK	TRUSS	EROSION CONTROL	BRIDGE QUANTITY	ROUND	GRAND TOTAL	FINAL	UNIT	ITEMS	ITEM NUMBER	QUANTITIES	UNIT	ITEMS				
							80			80		LF	SNOW FENCE (MOD. - ARCH)	620.70							
	220									220		LF	PLANK RAIL (MOD. - STEEL BACKED TIMBER RAIL)	621.15							
	150									150		LF	REMOVAL AND DISPOSAL OF GUARD RAIL	621.80							
	40									40		LF	TEMPORARY TRAFFIC BARRIER	621.90							
1										1		LS	FIELD OFFICE-ENGINEERS	631.10							
1										1		LS	TESTING EQUIPMENT - CONCRETE	631.16							
1										1		LS	TESTING EQUIPMENT - BITUMINOUS	631.17							
1										1		LU	FIELD OFFICE - TELEPHONE (N.A.B.I.)	631.25							
	1									1		LS	MOBILIZATION / DEMOBILIZATION	635.11							
	1									1		LS	TRAFFIC CONTROL	641.10							
	440									440		LF	4" YELLOW LINE	646.21							
							10			10		SY	GEOTEXTILE UNDER STONE FILL	649.31							
							80			80		SY	GEOTEXTILE FOR SILT FENCE	649.51							
							6			6		LB	SEED	651.15							
							10			10		LB	SEED-WINTER RYE	651.17							
							50			50		LB	FERTILIZER	651.18							
							1			1		TON	AGRICULTURAL LIMESTONE	651.20							
							1			1		TON	HAYMULCH	651.25							
							10			10		CY	TOPSOIL	651.35							
							1			1		LS	EROSION PREVENTION & SEDIMENT CONTROL PLAN	652.10							
							40			40		HR	MONITORING EROSION PREVENTION & SEDIMENT CONTROL PLAN	652.20							
							1			1		LU	MAINTENANCE OF EROSION PREVENTION & SEDIMENT CONTROL PLAN (N.A.B.I.)	652.30							
							10			10		SY	EROSION MATTING	654.10							
	29.5									29.5		SF	TRAFFIC SIGNS, TYPE A	675.20							
	35									35		LF	FLANGED CHANNEL SIGN POST	675.30							
	15									15		EACH	REMOVING SIGNS	675.50							
	3									3		EACH	ERECTING SALVAGED SIGNS	675.60							
	2									2		EACH	SETTING SALVAGED POSTS	675.61							

PROJECT NAME: **THETFORD**  
PROJECT NUMBER: **BHO 1444 (43)**  
FILE NAME: s03j036qty.dgn PLOT DATE: 12/9/98  
PROJECT MANAGER: M. EVANS-MONGEON DRAWN BY: G.ROKES  
DESIGNED BY: G.ROKES CHECKED BY: S. SCRIBNER  
QUANTITY SHEET #2 SHEET 6 OF 60



**FINISHED PORTAL OPENING**

PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures*1036typ.dgn
PROJECT LEADER:	M. EVANS-MONGEON
DESIGNED BY:	G. ROKES
IPARM:	e031036beo.l
PLOT DATE:	27-SEP-2006
DRAWN BY:	G. ROKES
CHECKED BY:	S. SCRIBNER
SHEET	7 OF 60

GPS CONTROL POINTS

HVCTRL #1

STANDARD DISK STAMPED  
 Thetford Az Mk  
 N = 487988.90  
 E = 1707352.54  
 ELEV. = 600.98

To reach from the VT Route 113 bridge over I-91 at exit 14 go northwest along route 113 for 2.3 MI to the intersection of Tucker Hill Road left in Thetford Center. Continue straight ahead and go north along route 113 for 0.4 MI to the intersection of a gravel drive right leading to the Hale Funeral Home of Thetford. Turn right and go southeast up the drive for about 50 m to the funeral home and the site of the mark in a lawn southeast of the funeral home. The mark is set 5 cm below ground surface in the top of a 30 cm diameter concrete monument poured 1.5 m deep. It is 58.6 m east of and about 10.0 m higher than the centerline of route 113, 40.6 m southeast of the southeast corner of the funeral home, 45.2 m east of pole no. 106/10, and 18.1 m west northwest of a 15 cm apple.

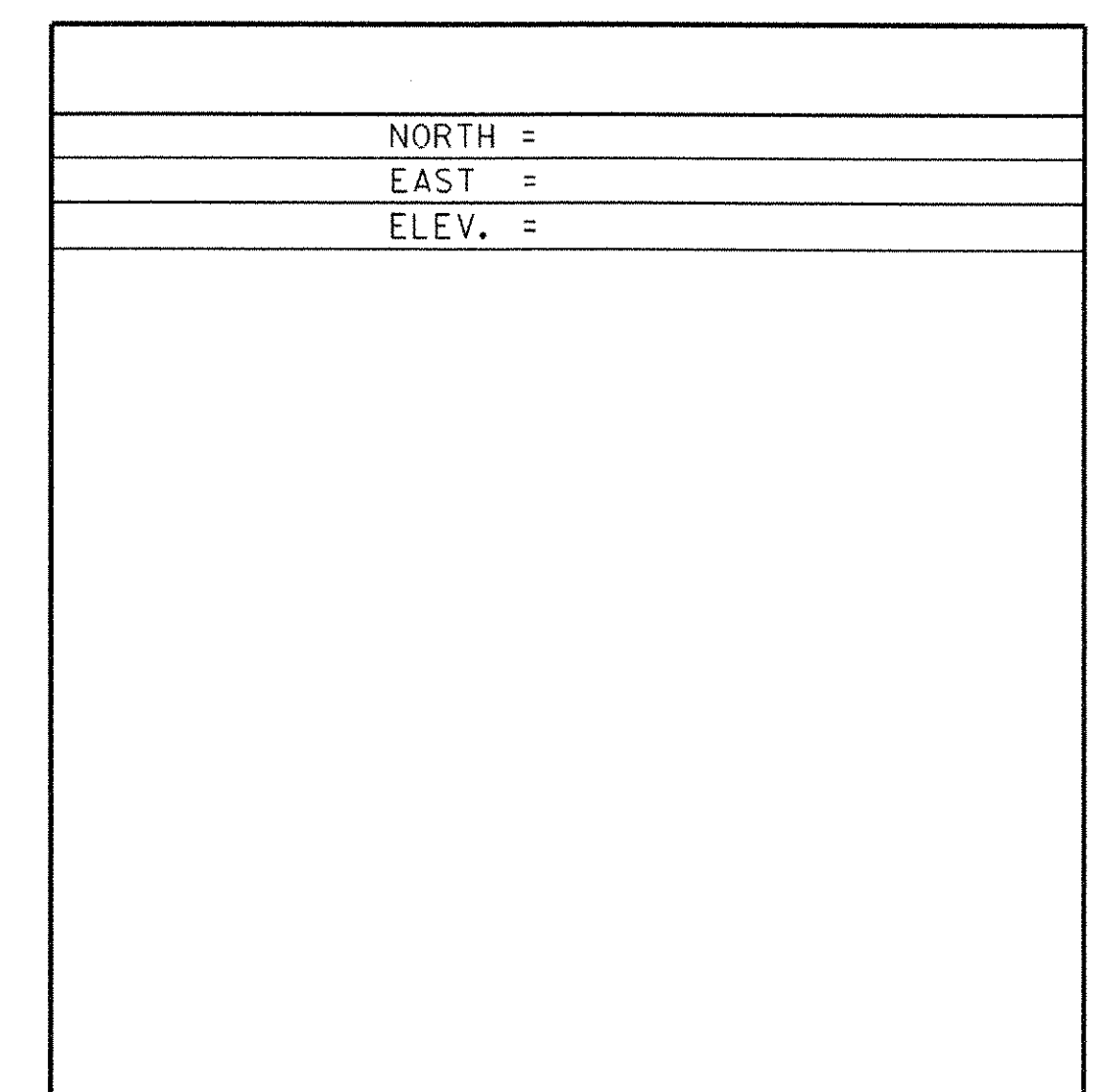
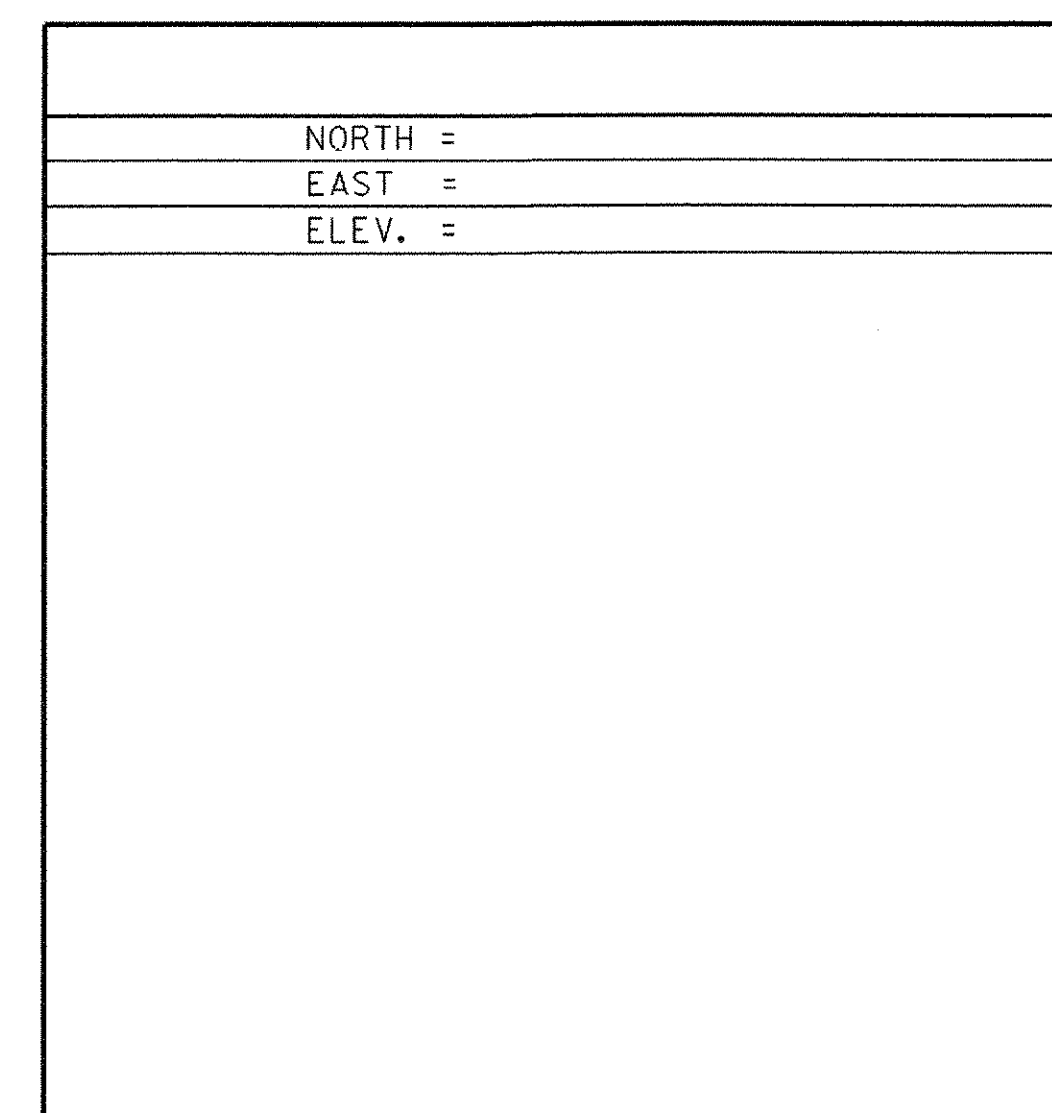
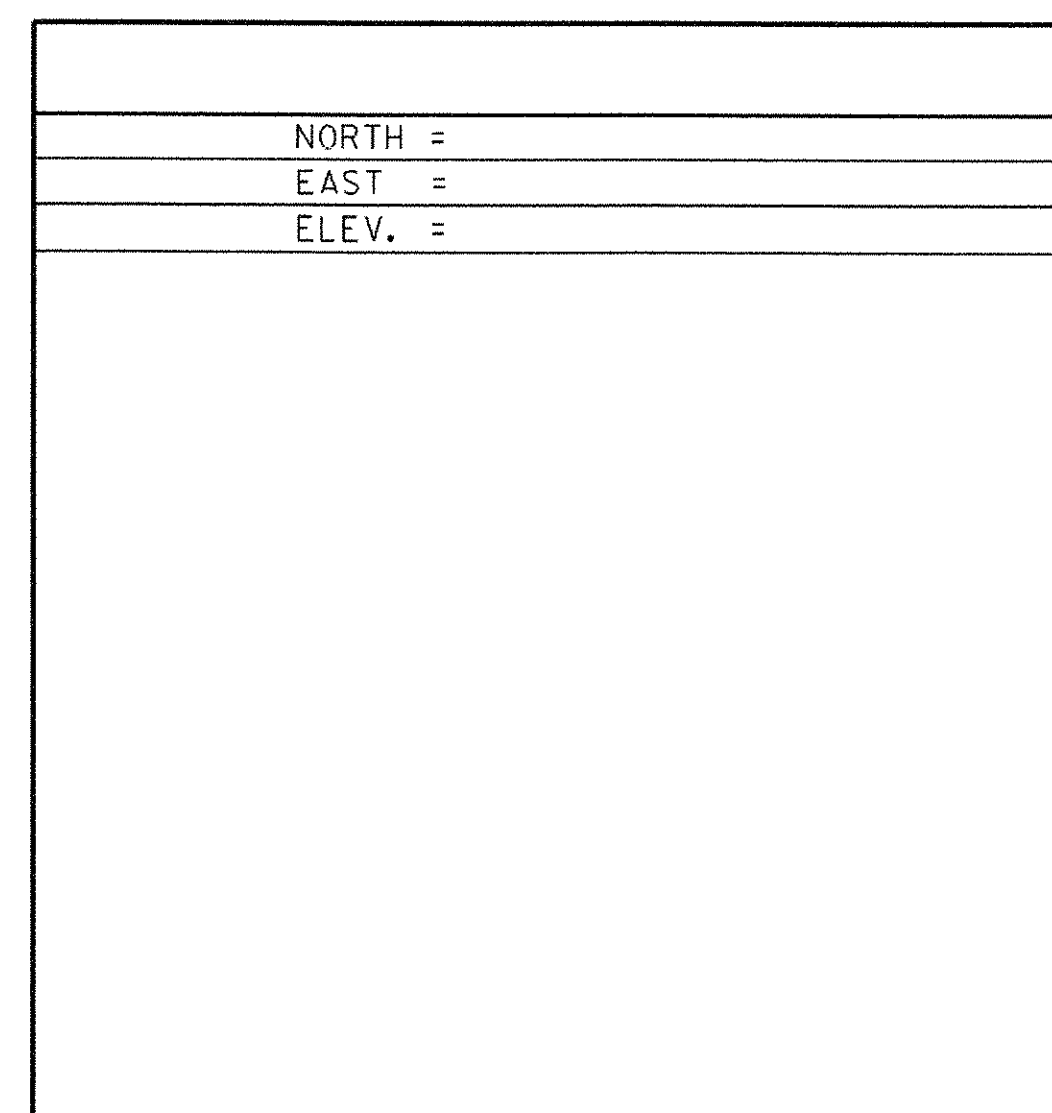
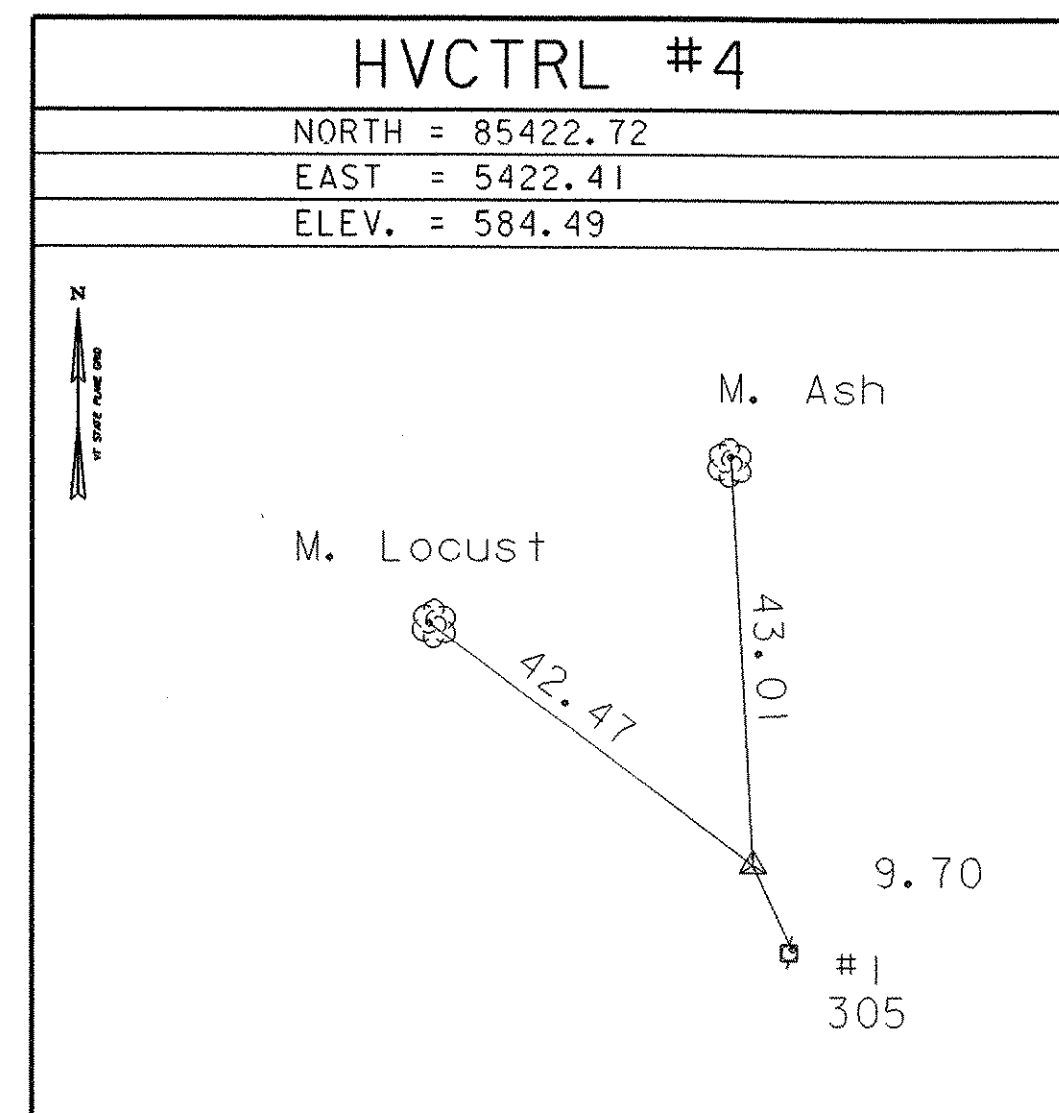
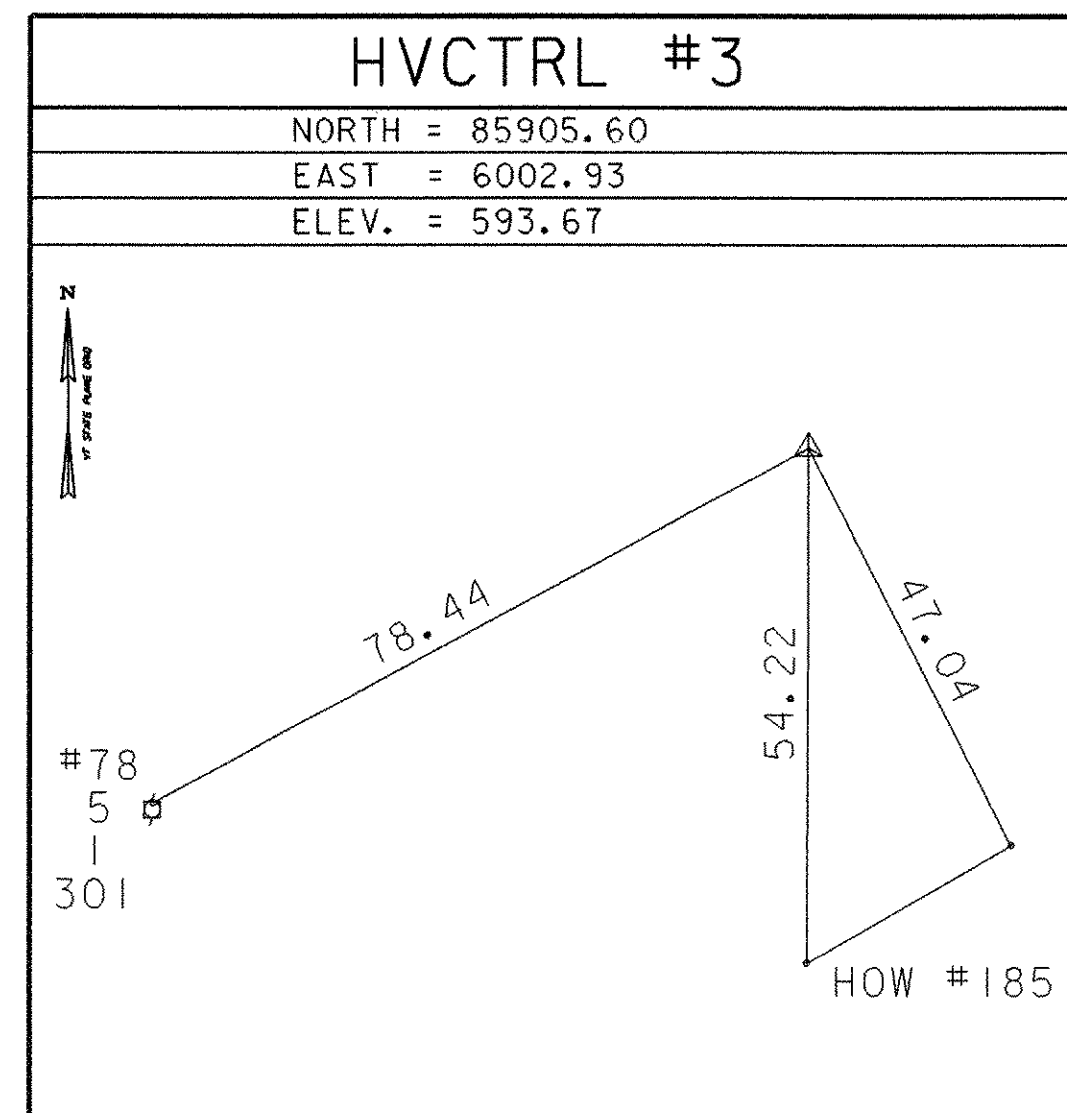
HVCTRL #2

STANDARD DISK STAMPED  
 Thetford  
 N = 485974.59  
 E = 1706229.39  
 ELEV. = 610.89

To reach from the VT Route 113 bridge over I-91 at exit 14 go northwest along route 113 for 2.3 MI to the intersection of Tucker Hill Road left in Thetford Center. Turn left and go west along Tucker Hill Road for 0.15 MI to the intersection of a gravel drive left leading to house no. 139 and the site of the mark on the left in a lawn in the front of a cemetery. The mark is set 7 cm below ground surface in the top of a 30 cm diameter concrete monument poured 1.5 m deep. It is 5.3 m south of and about 0.1 m lower than the centerline of Tucker Hill Road, 4.8 m west of the centerline of a gravel drive, 9.6 m north of the northwest corner of the attached barn on house #139, and 10.2 m northwest of the most easterly gatepost of the gate for the cemetery.

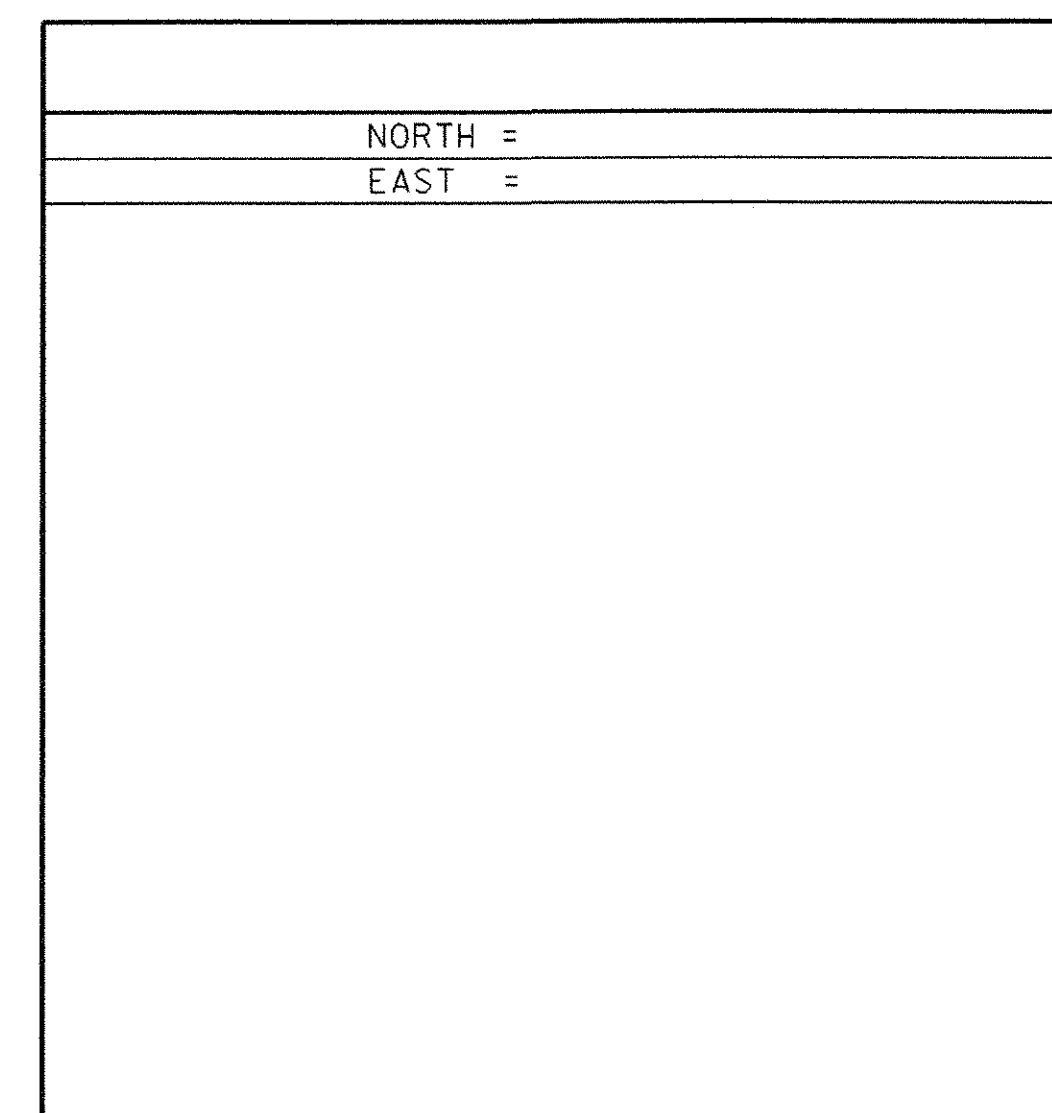
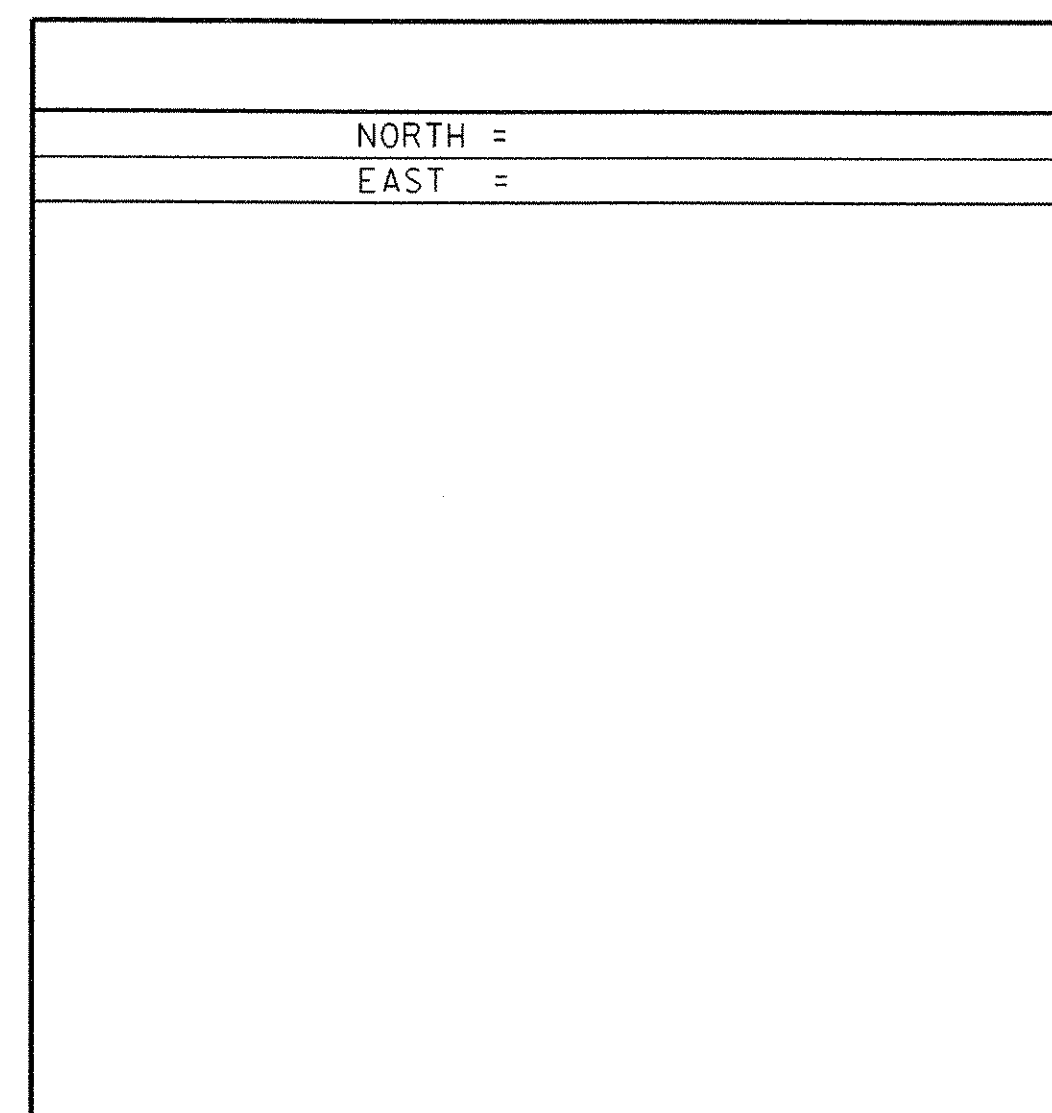
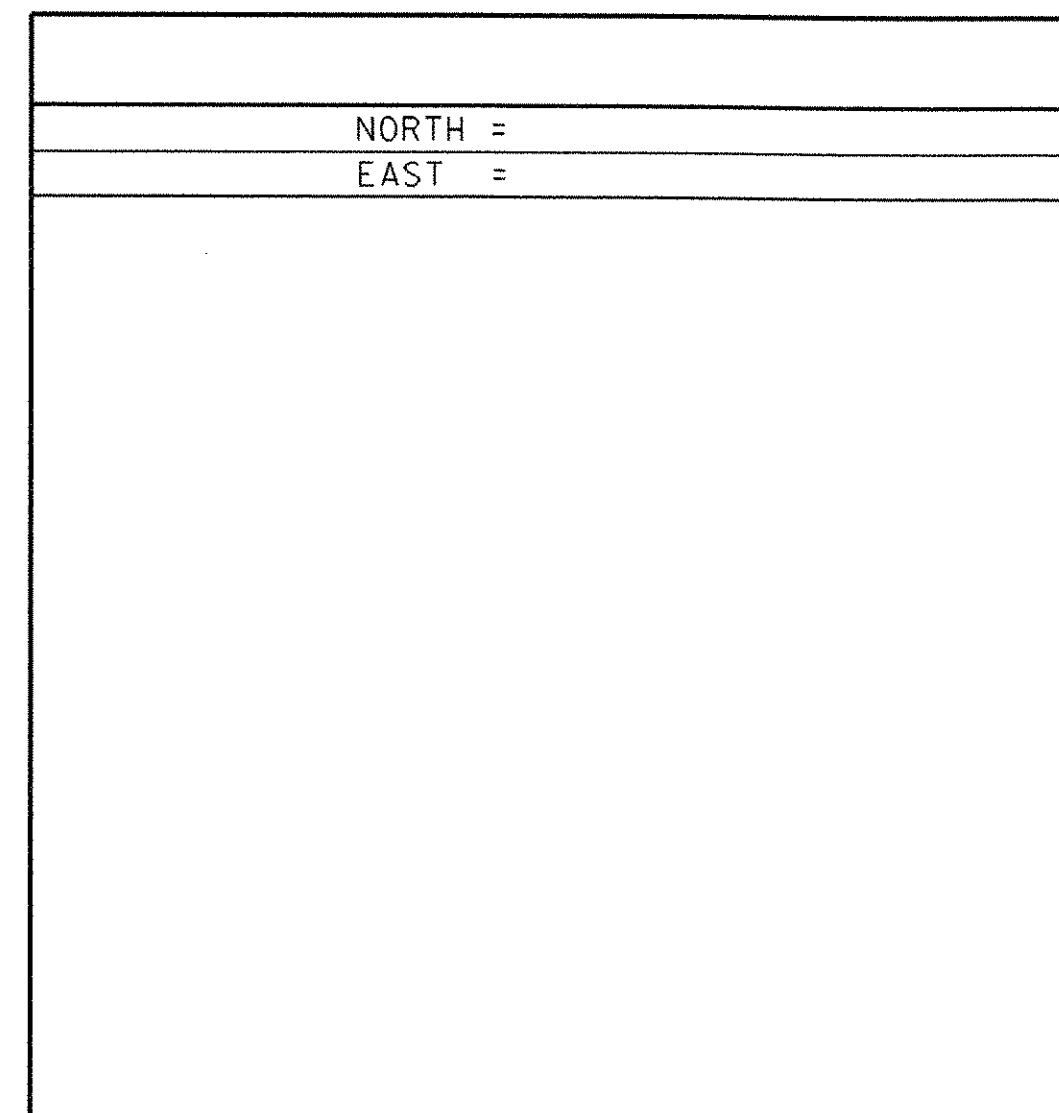
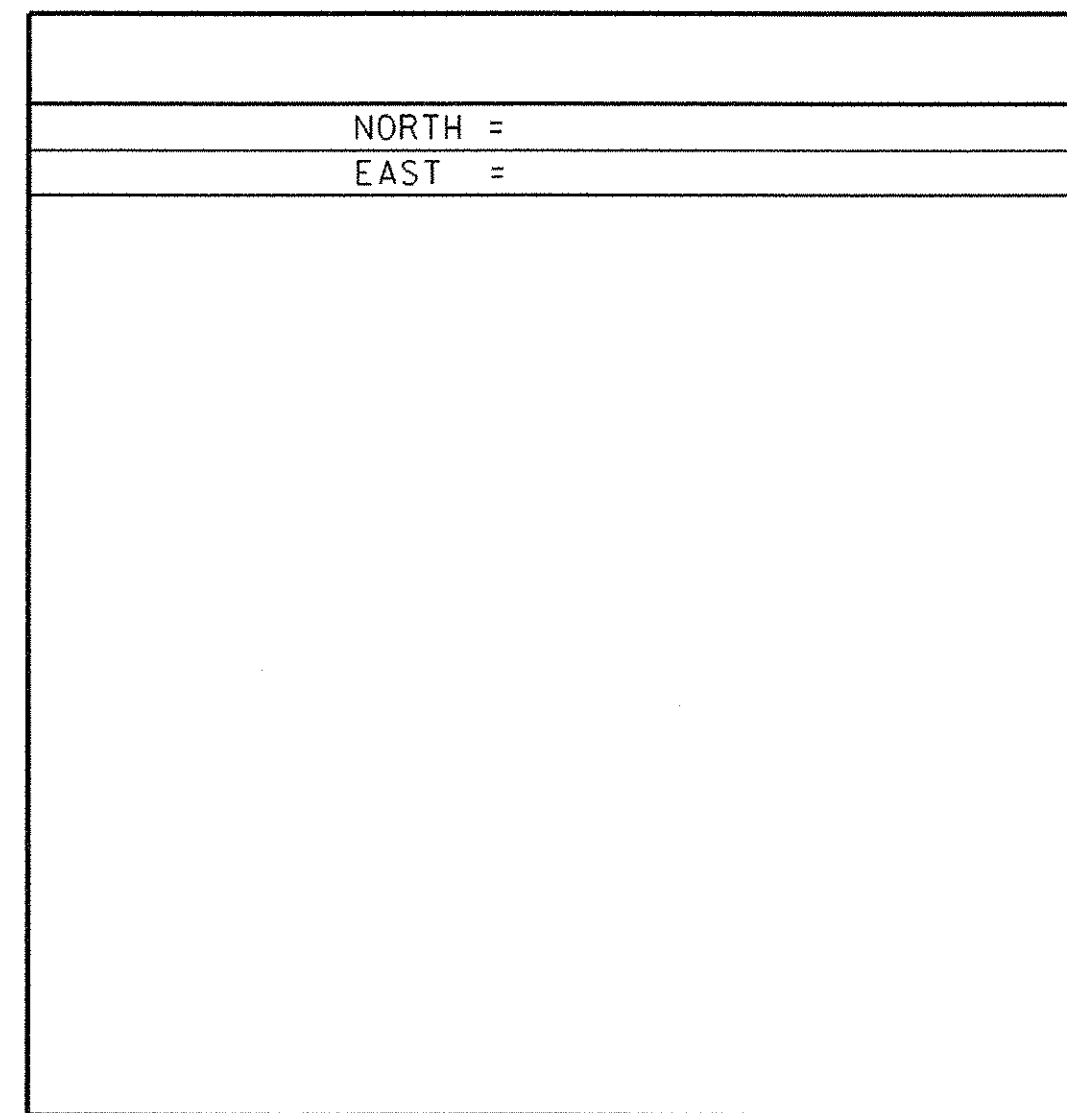
- DESCRIPTION PROVIDED BY VERMONT AGENCY OF TRANSPORTATION GEODETIC SURVEY UNIT
- TO ALLOW THE STATE PLANE COORDINATES TO FIT THE AGENCY DESIGN PLANE, SUBTRACT 400.00 FROM THE NORTHING AND SUBTRACT 1,700,000 FROM THE EASTING

TRAVERSE TIES



• MAIN TRAVERSE COMPLETED 08/13/03 by R. Gilman P.C. & Anthony Somaini

ALIGNMENT TIES

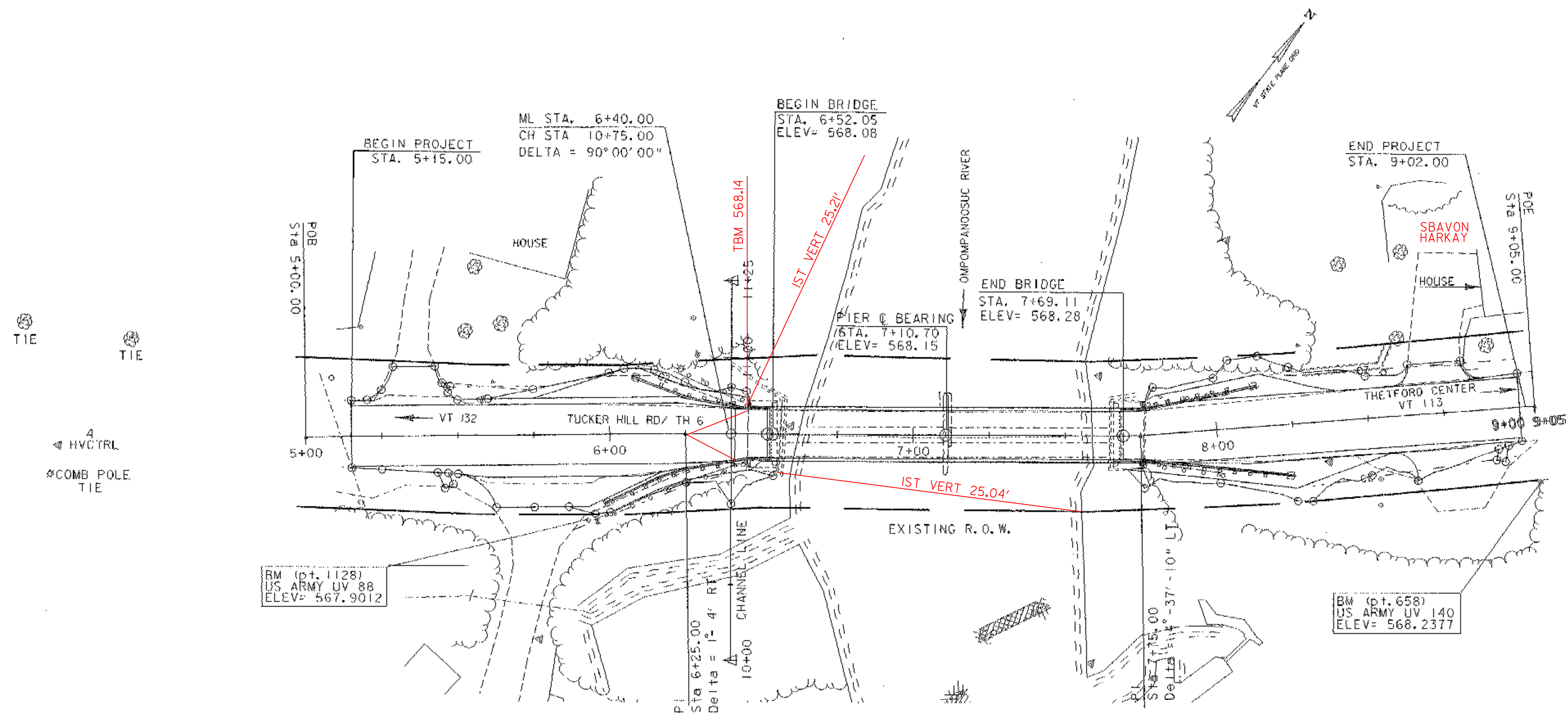


• ALIGNMENT STAKED

DATUM	
VERTICAL	NAVD **
HORIZONTAL	NAD 83 (96)
ADJUSTMENT	Compass

**TIE SHEET**

PROJECT NAME:	Thetford
PROJECT NUMBER:	BHO 1444(43)
FILE NAME:	Structures\s03j036t1.dgn
PROJECT LEADER:	M. EVANS-MONGEON
DESIGNED BY:	M. EVANS-MONGEON
IPARM:	s03j036t1.i
PLOT DATE:	27-SEP-2006
DRAWN BY:	R. Bullock
CHECKED BY:	S. SCRIBNER
SHEET	8 OF 60

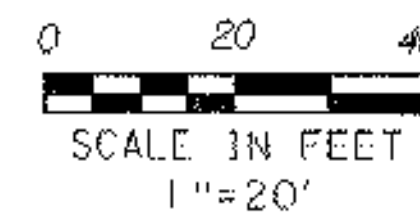


CONSTRUCT DRIVE		STEEL BACKED TIMBER RAIL	
STA.		STA.	TO STA.
5+30.00	LT	5+98.00	RT - 6+55.91 RT
5+75.00	RT	6+07.86	LT - 6+55.91 LT
8+75.00	LT	7+65.50	LT - 8+14.00 LT
		7+65.50	RT - 8+24.50 RT

ITEM 646.21 4" YELLOW LINE			
STA.	TO STA.	LENGTH	
5+15.00	6+25.00	110 ft.	(DOUBLE YELLOW LINE = 220ft.)
7+90.00	9+00.00	110 ft.	(DOUBLE YELLOW LINE = 220ft.)

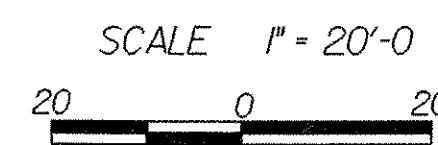
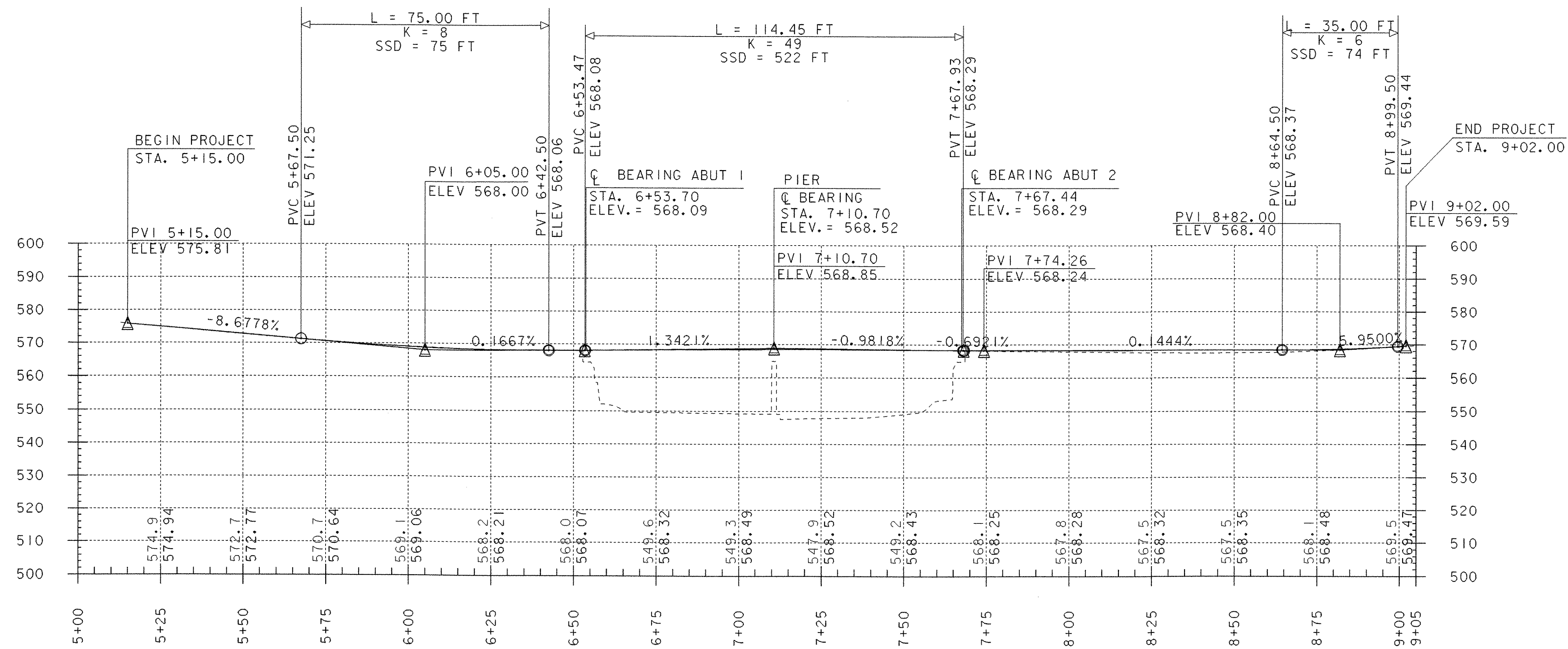
STEEL BACKED TIMBER RAIL ANCHORS	
STA.	
5+99.20	RT
6+08.90	LT
8+12.98	LT
8+23.28	RT

BRIDGE DECK TIMBER RAIL	
STA.	TO STA.
6+53.41	RT - 7+68.00 RT
6+53.41	LT - 7+68.00 LT



**LAYOUT SHEET**

PROJECT NAME:	THETFORD	PLOT DATE:	27-SEP-2006
PROJECT NUMBER:	BHO 1444 (43)	DRAWN BY:	G.ROKES
FILE NAME:	Structures\603j036bdr.dgn	CHECKED BY:	S. SCRIBNER
DESIGNED BY:	G.ROKES		
IPARM:	603j036(a).1	SHEET	9 OF 60

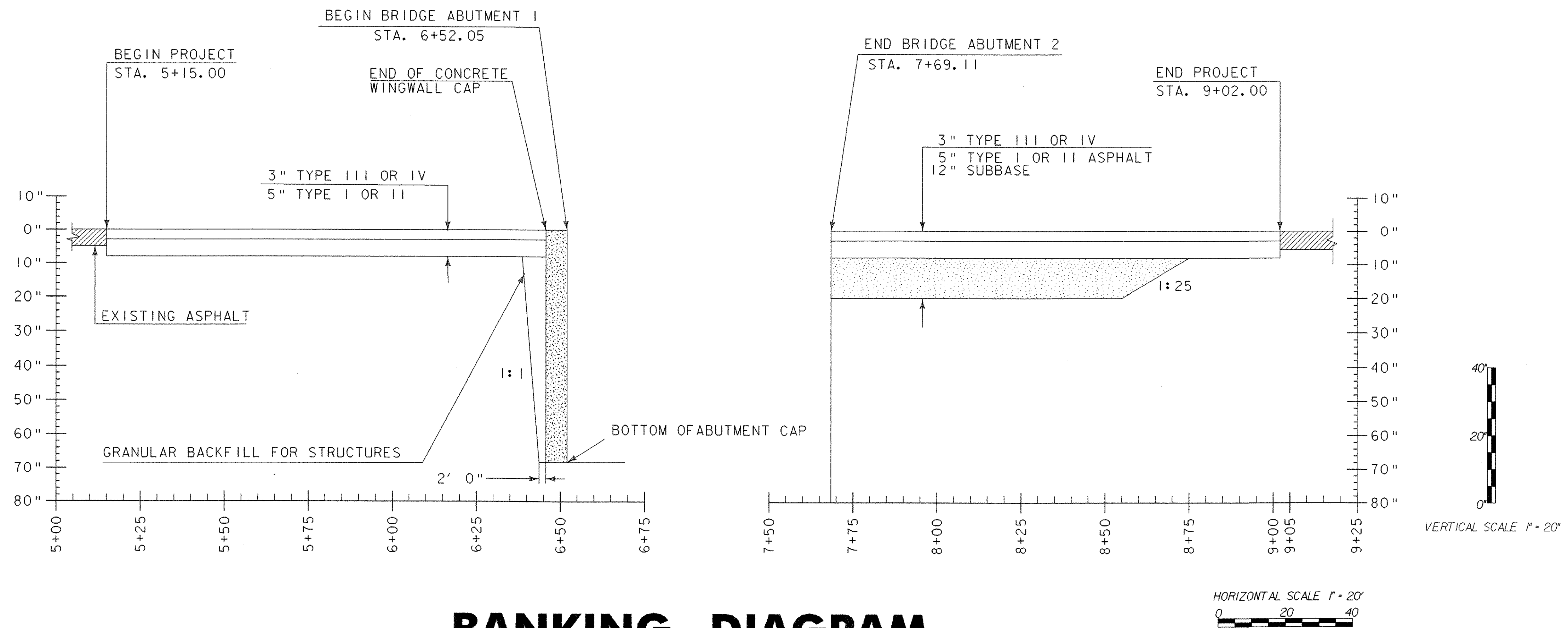


NOTE: ELEVATIONS TO THE "TENTH'S" REPRESENT EXISTING GROUND ELEVATIONS.  
ELEVATIONS TO THE "HUNDREDTH'S" REPRESENT PROPOSED GROUND ELEVATIONS.

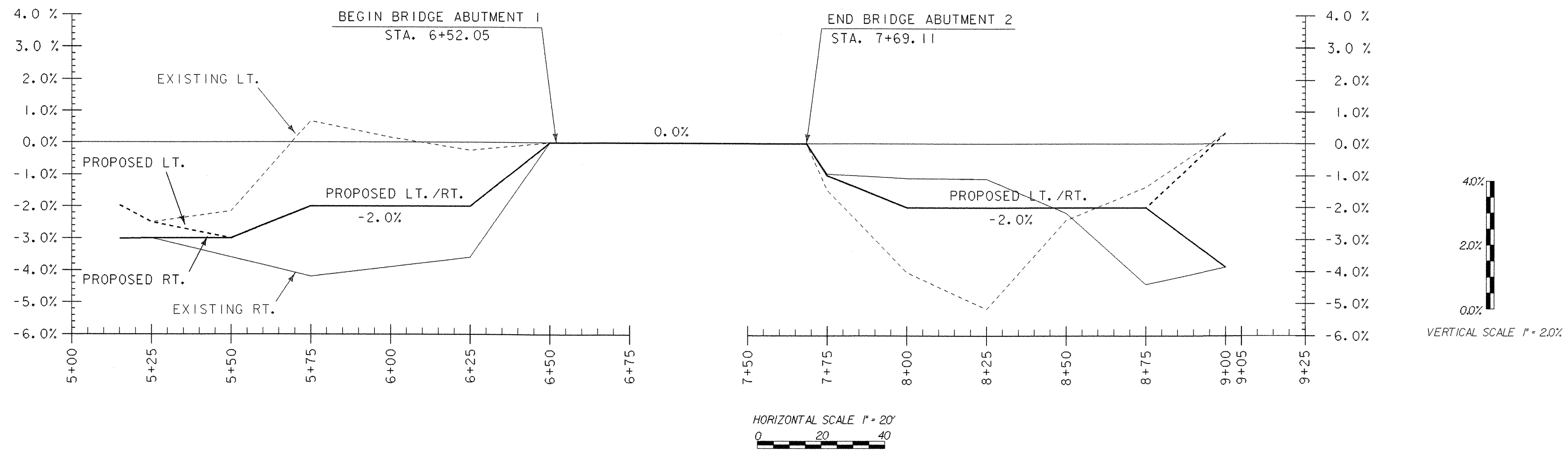
**PROFILE**

PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures\s03j036wk.dgn
PROJECT LEADER:	M.EVANS-MONGEON
DESIGNED BY:	G.ROKES
IPARM:	s03j036prf.I
PLOT DATE:	27-SEP-2006
DRAWN BY:	G.ROKES
CHECKED BY:	S. SCRIBNER
SHEET	10 OF 60

# MATERIAL TRANSITION



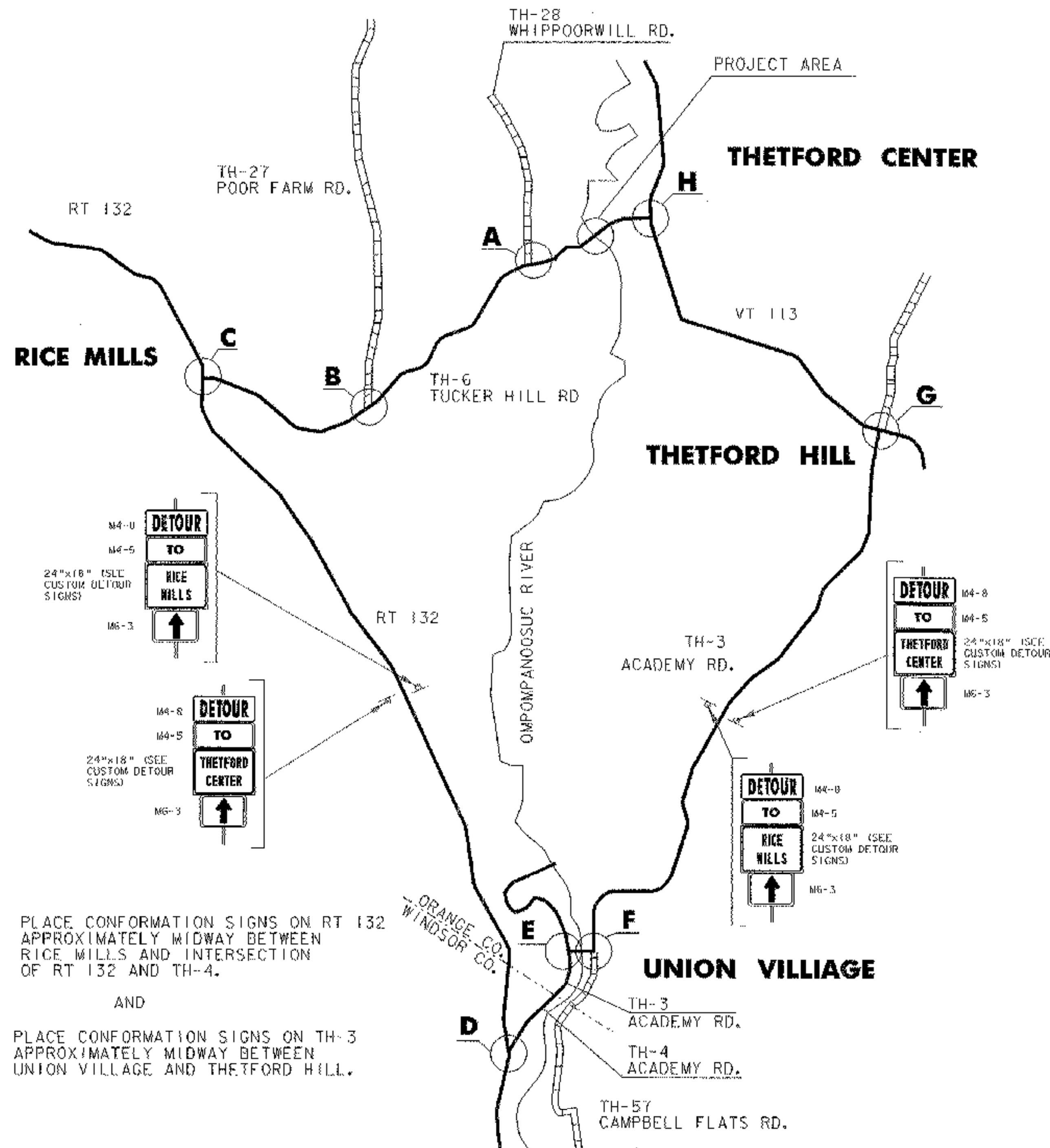
# BANKING DIAGRAM



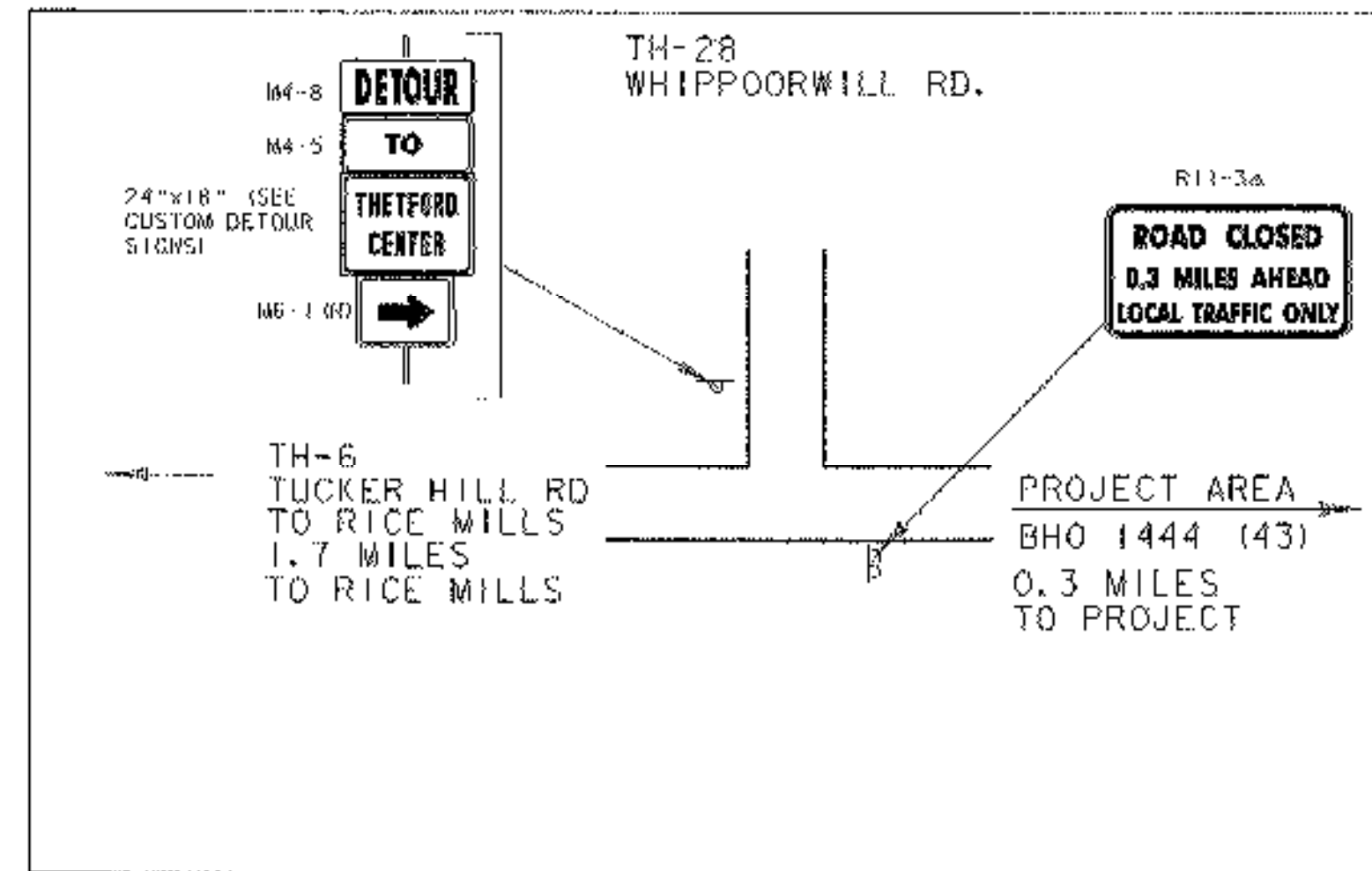
## MATERIAL TRANSITION / BANKING DIAGRAM

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G. ROKES
FILE NAME: Structures\s03j036typ.dgn	CHECKED BY: S. SCRIBNER
PROJECT LEADER: M. EVANS-MONGEON	SHEET II OF 60
DESIGNED BY: G. ROKES	
IPARM: s03j036mat.i	

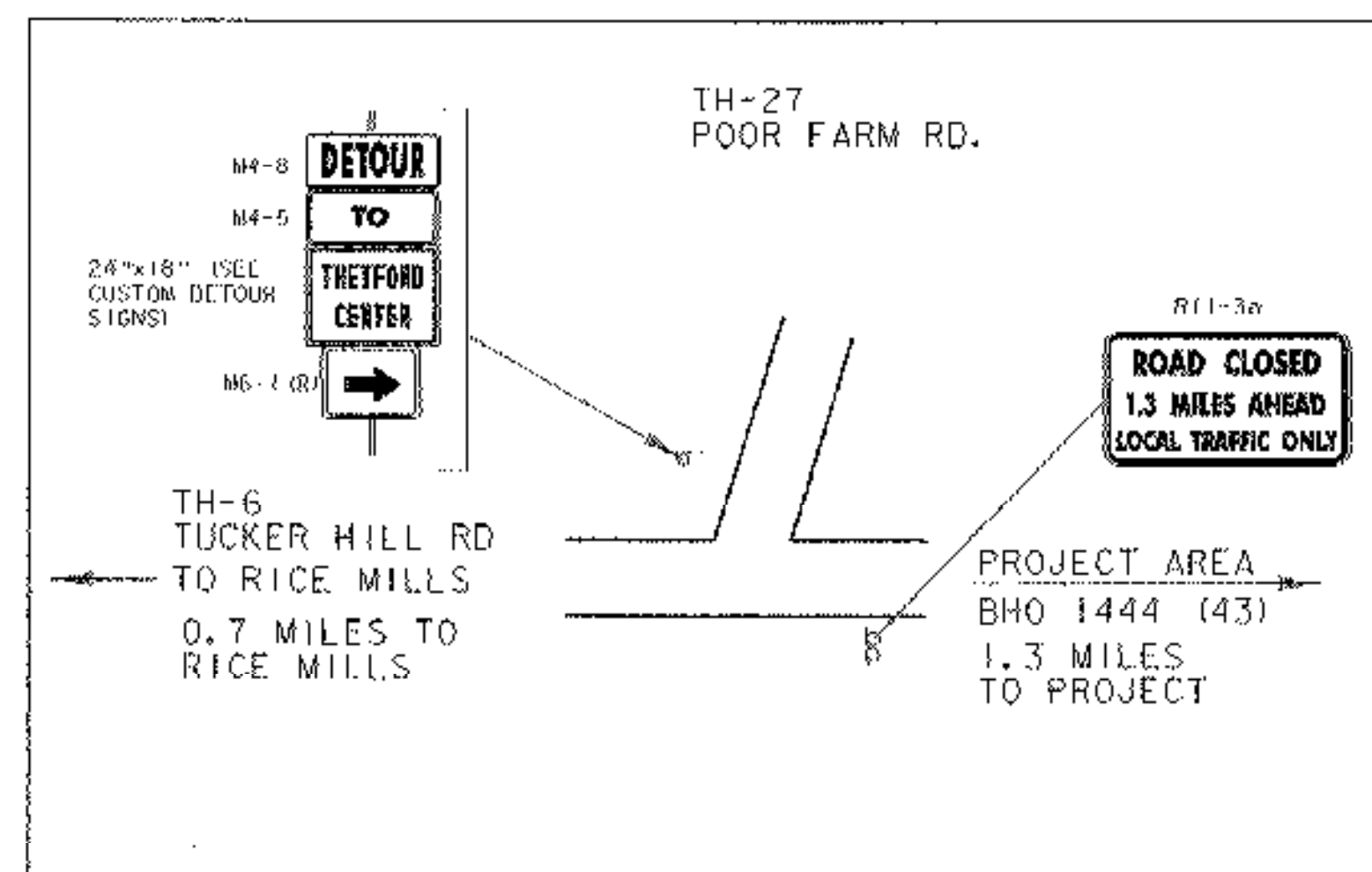
# THETFORD TEMPORARY DETOUR SIGNAGE AND ROUTING



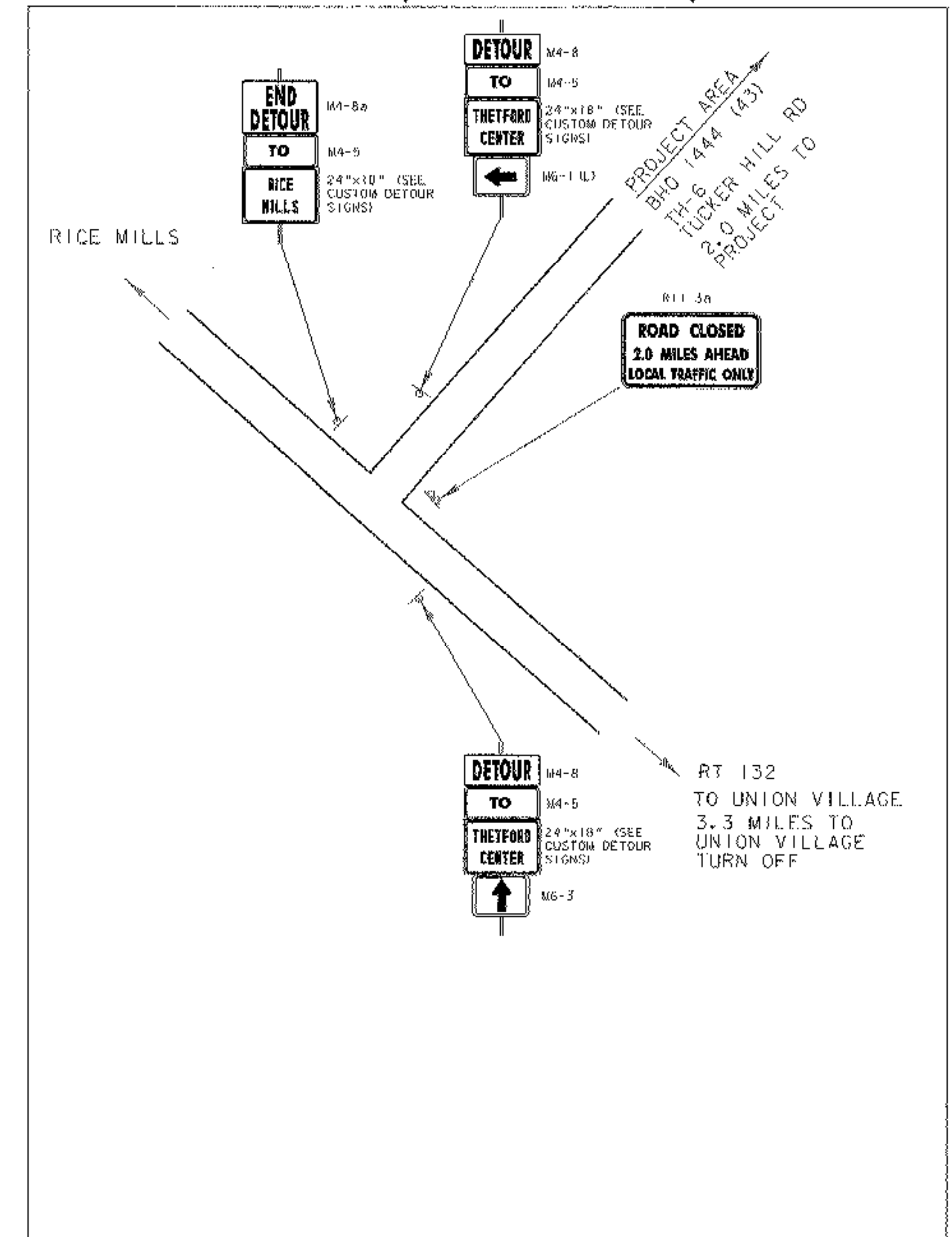
**A**  
INTERSECTION TH-28 (WHIPPOORWILL RD.)  
AND TH-6 (TUCKER HILL RD.)



**B**  
INTERSECTION TH-27 (POOR FARM RD.)  
AND TH-6 (TUCKER HILL RD.)



**C**  
INTERSECTION RT. 132 AND  
TH-6 (TUCKER HILL RD.)



PLACE CONFORMATION SIGNS ON RT 132  
APPROXIMATELY MIDWAY BETWEEN  
RICE MILLS AND INTERSECTION  
OF RT 132 AND TH-4.

AND  
PLACE CONFORMATION SIGNS ON TH-3  
APPROXIMATELY MIDWAY BETWEEN  
UNION VILLAGE AND THETFORD HILL.

FROM PROJECT BEGINNING AND TAKING  
THE DETOUR THRU UNION VILLAGE  
TO PROJECT END IS APPROXIMATELY  
9 MILES

NOTE: INSTALL ALL TEMPORARY DETOUR SIGNS SO AS NOT TO OBSCURE OR  
CONFLICT WITH EXISTING TRAFFIC CONTROL DEVICES.

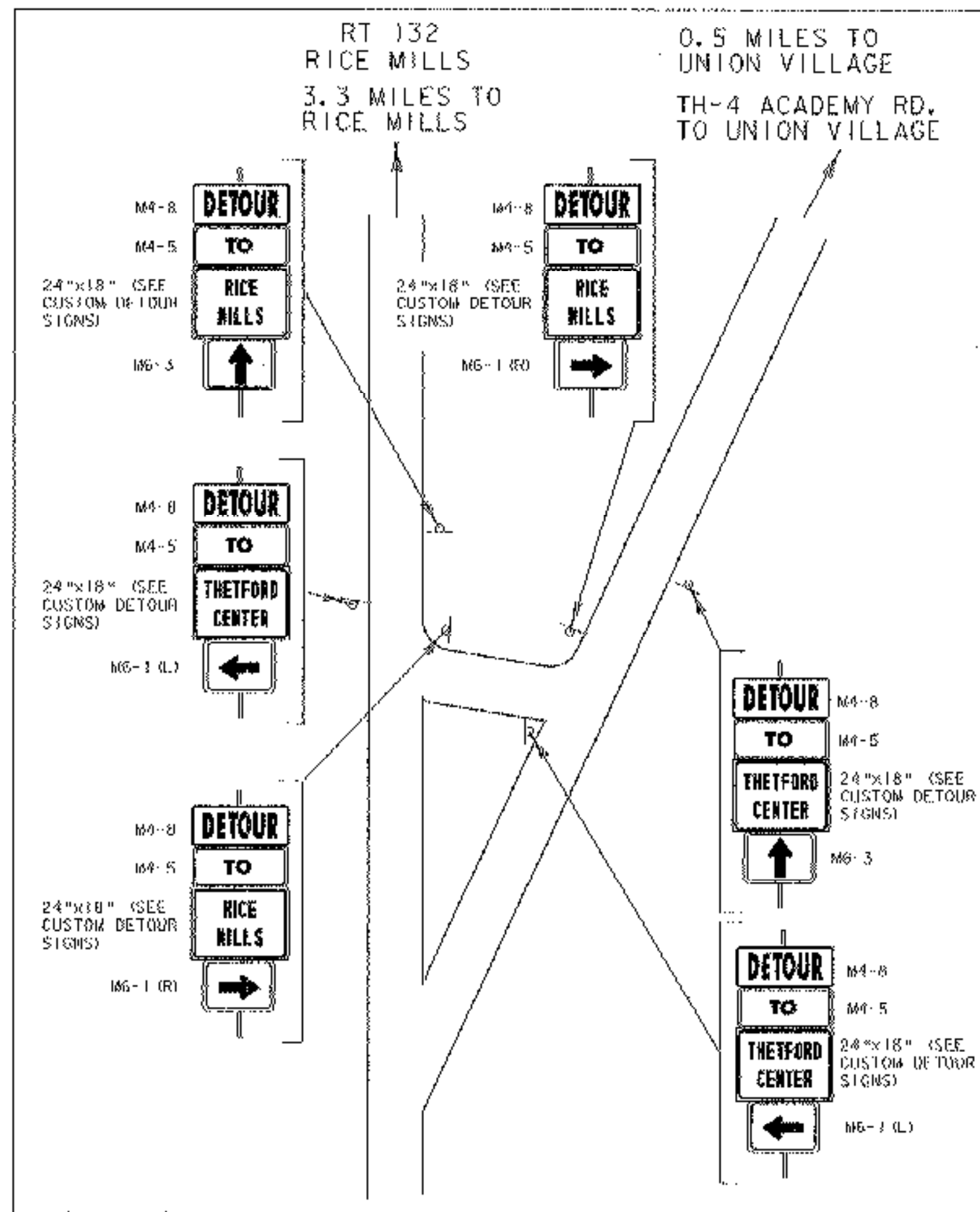
DRAWINGS ON THIS SHEET ARE NOT TO SCALE

## TEMPORARY DETOUR SIGNAGE AND ROUTING (1)

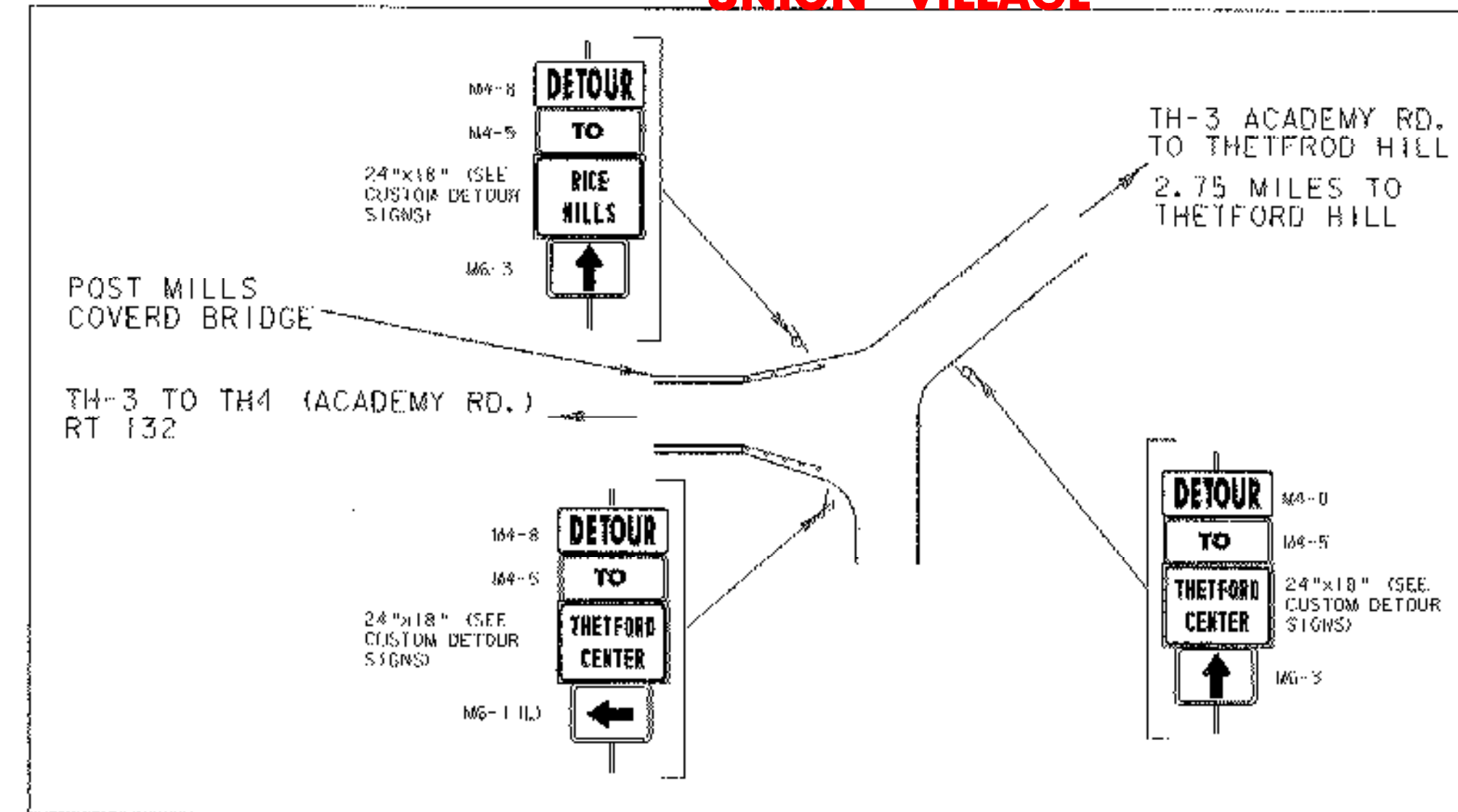
PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures\s03j036det.dgn
PROJECT LEADER:	M.EVANS-MONGEON
DESIGNED BY:	G. ROKES
IPARM:	e03j036d.tl
PLOT DATE:	27-SEP-2006
DRAWN BY:	G. ROKES
CHECKED BY:	S. SCRIBNER
SHEET	12 OF 60

# THETFORD TEMPORARY DETOUR SIGNAGE AND ROUTING

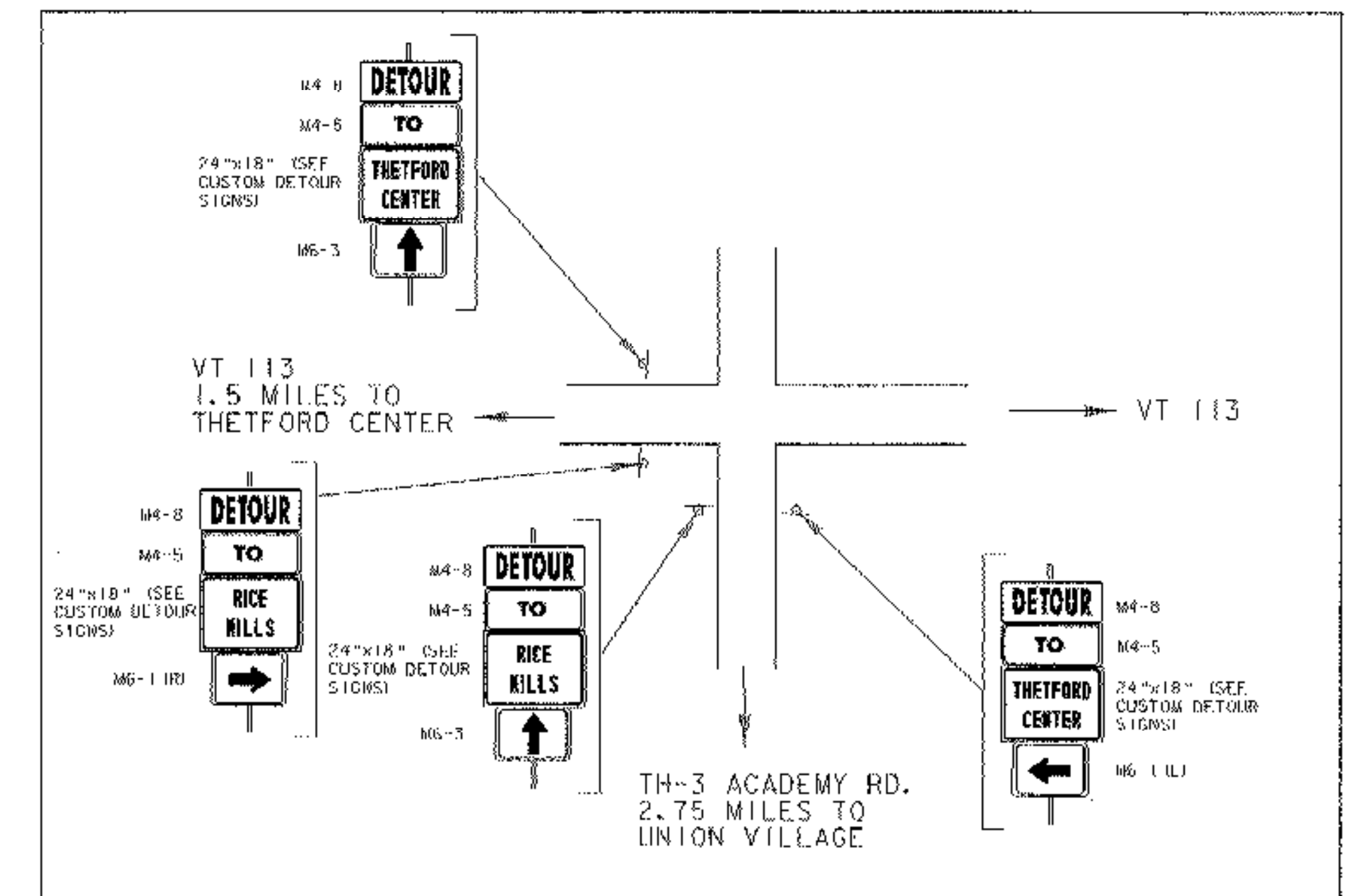
**D**  
INTERSECTION RT. 132 AND TH-3 (ACADEMY RD.)



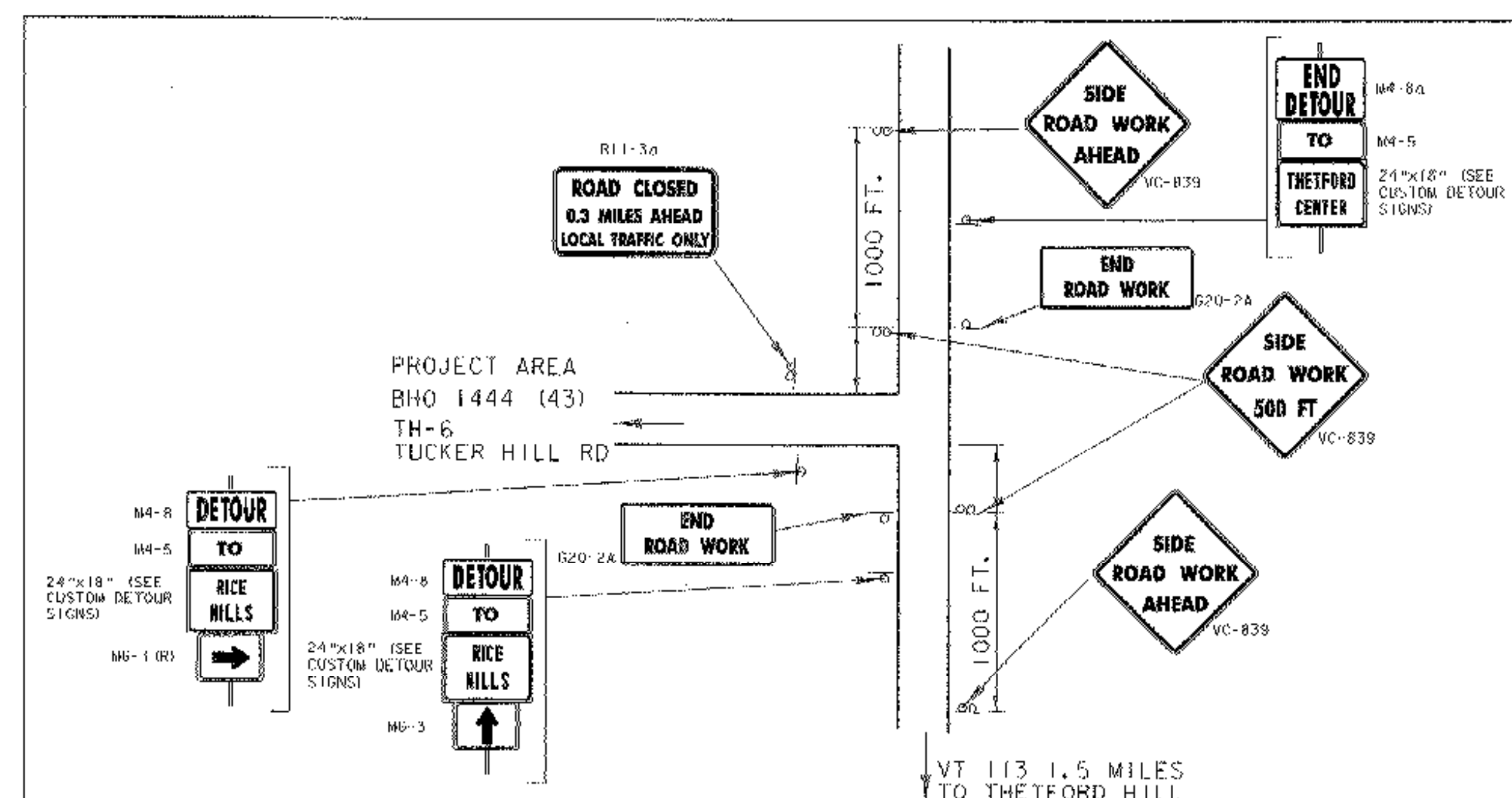
**F**  
TH-3 TO TH-4 (ACADEMY RD.) AND CAMPBELL FLATS RD INTERSECTION AT POST MILLS BRIDGE UNION VILLAGE



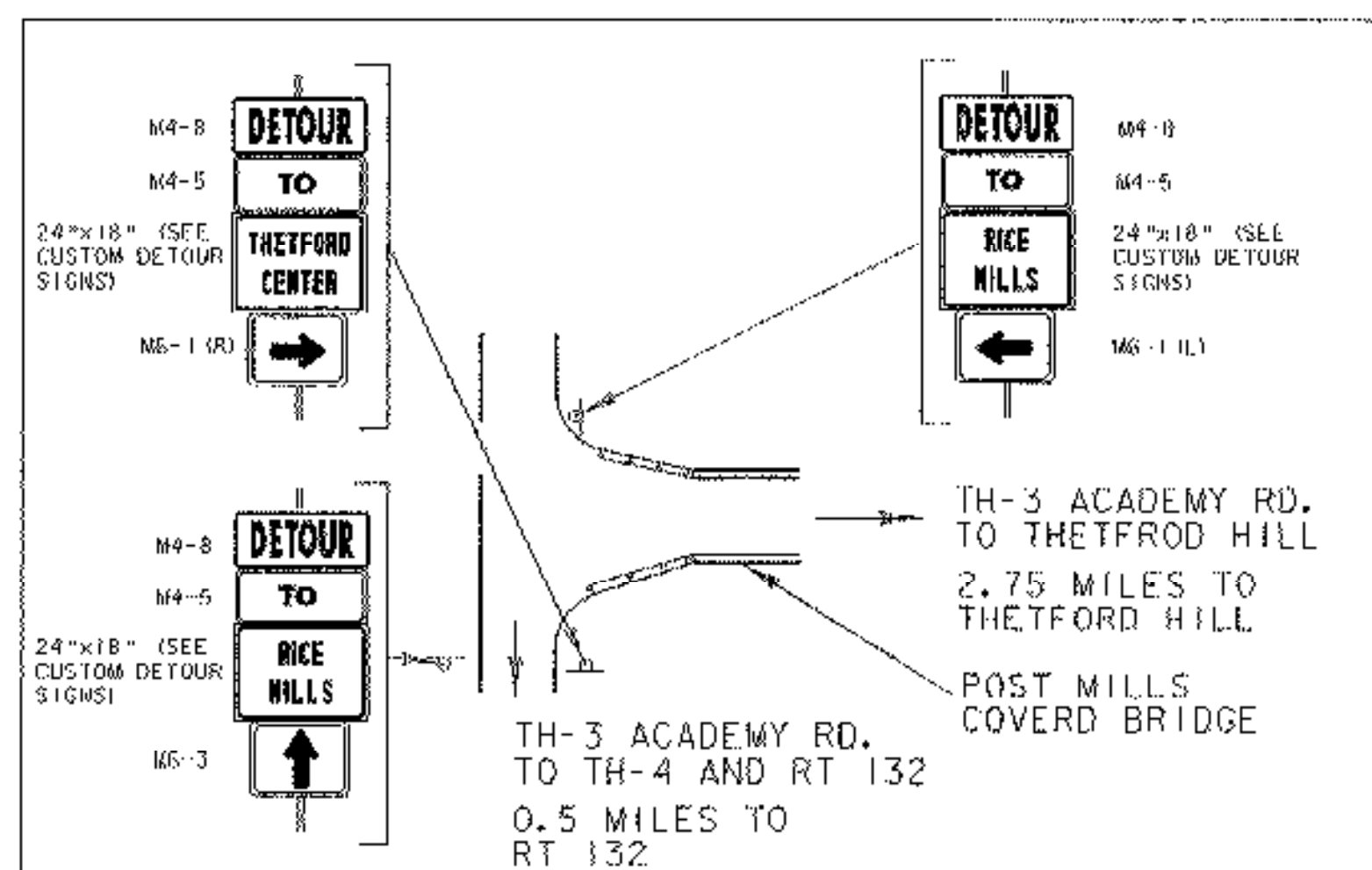
**G**  
INTERSECTION VT. 113 AND TH-3 (ACADEMY RD.) AT THETFORD HILL



**H**  
INTERSECTION VT. 113 AND TH-6 (TUCKER HILL RD.)



**E**  
INTERSECTION TH-4 (ACADEMY RD.) AND TH-3 AT POST MILLS BRIDGE



THE "DETOUR" AND "END DETOUR" SIGNS WILL BE BLACK LETTERS ON ORANGE BACK GROUND, ASTM TYPE VIII OR IX RETRO-REFLECTIVE SHEETING.

THE LEAD SIGNS "SIDE ROAD WORK AHEAD, END ROAD WORK, SIDE ROAD WORK 500 FT." AND "BRIDGE CLOSED AHEAD" WILL BE BLACK LETTERS ON ORANGE BACK GROUND, ASTM TYPE VIII OR IX RETRO-REFLECTIVE SHEETING.

THE "TO" AND "ARROWS" SIGNS WILL BE BLACK LETTERS ON WHITE BACK GROUND, ASTM TYPE III SHEETING.

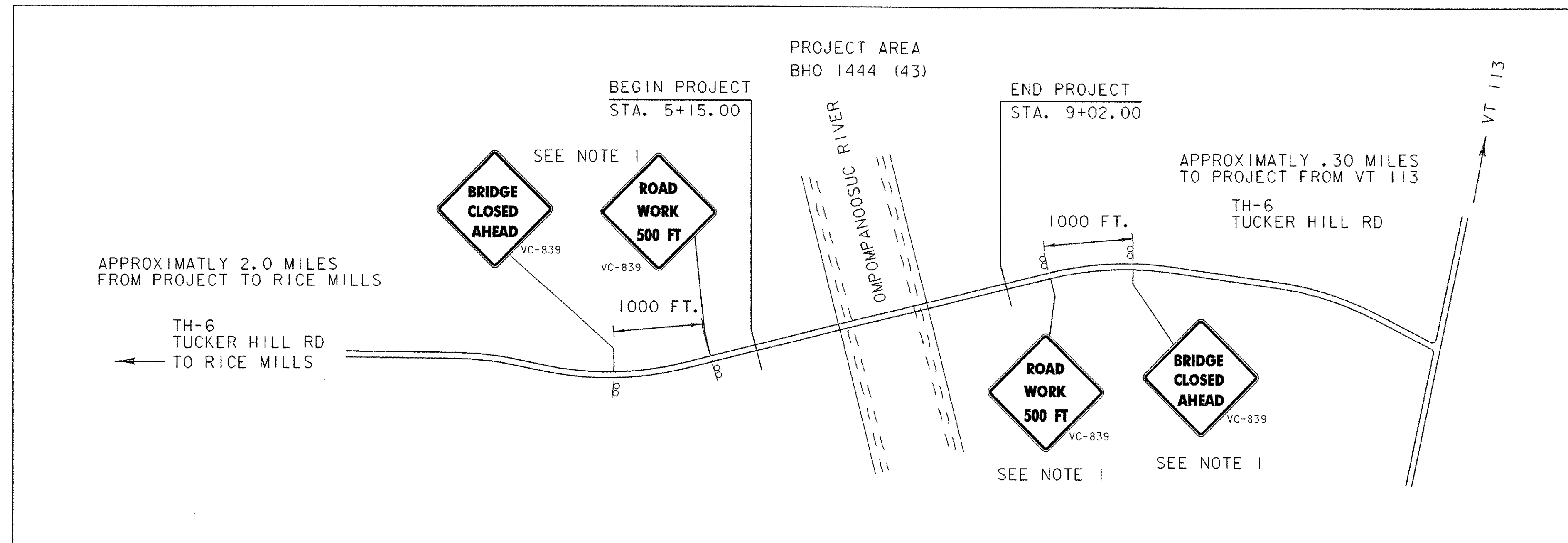
THE "ROAD CLOSED LOCAL TRAFFIC ONLY" AND "BRIDGE CLOSED" SIGNS WILL BE BLACK LETTERS ON WHITE BACK GROUND, ASTM TYPE III SHEETING.

DRAWINGS ON THIS SHEET ARE NOT TO SCALE

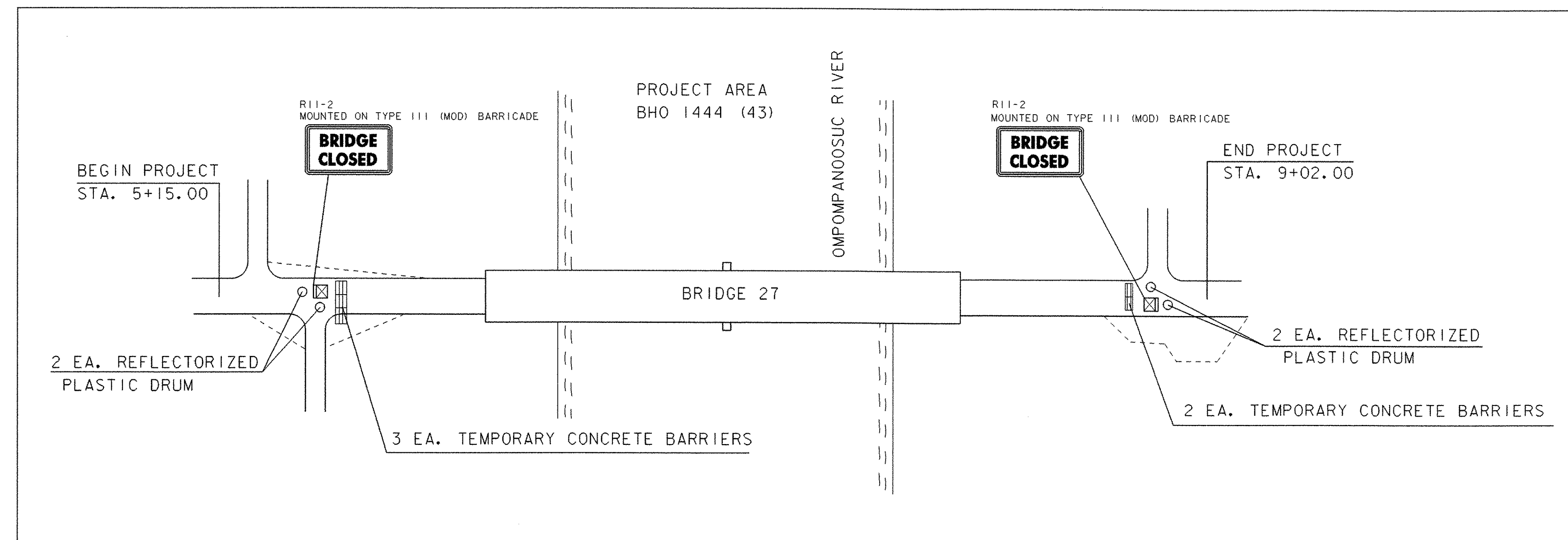
## TEMPORARY DETOUR SIGNAGE AND ROUTING (2)

PROJECT NAME:	THETFORD	PLOT DATE:	27-SEP-2006
PROJECT NUMBER:	BHO 1444 (43)	DRAWN BY:	G.ROKES
FILE NAME:	Structures\03J036det.dgn	CHECKED BY:	S. SCRIBNER
DESIGNED BY:	G.ROKES	SHEET	13 OF 60
IPARM:	03J036d12.1		

# THETFORD TEMPORARY DETOUR SIGNAGE AND ROUTING



**SIGNAGE LEADING TO PROJECT BEGIN STA. AND PROJECT END STA.**

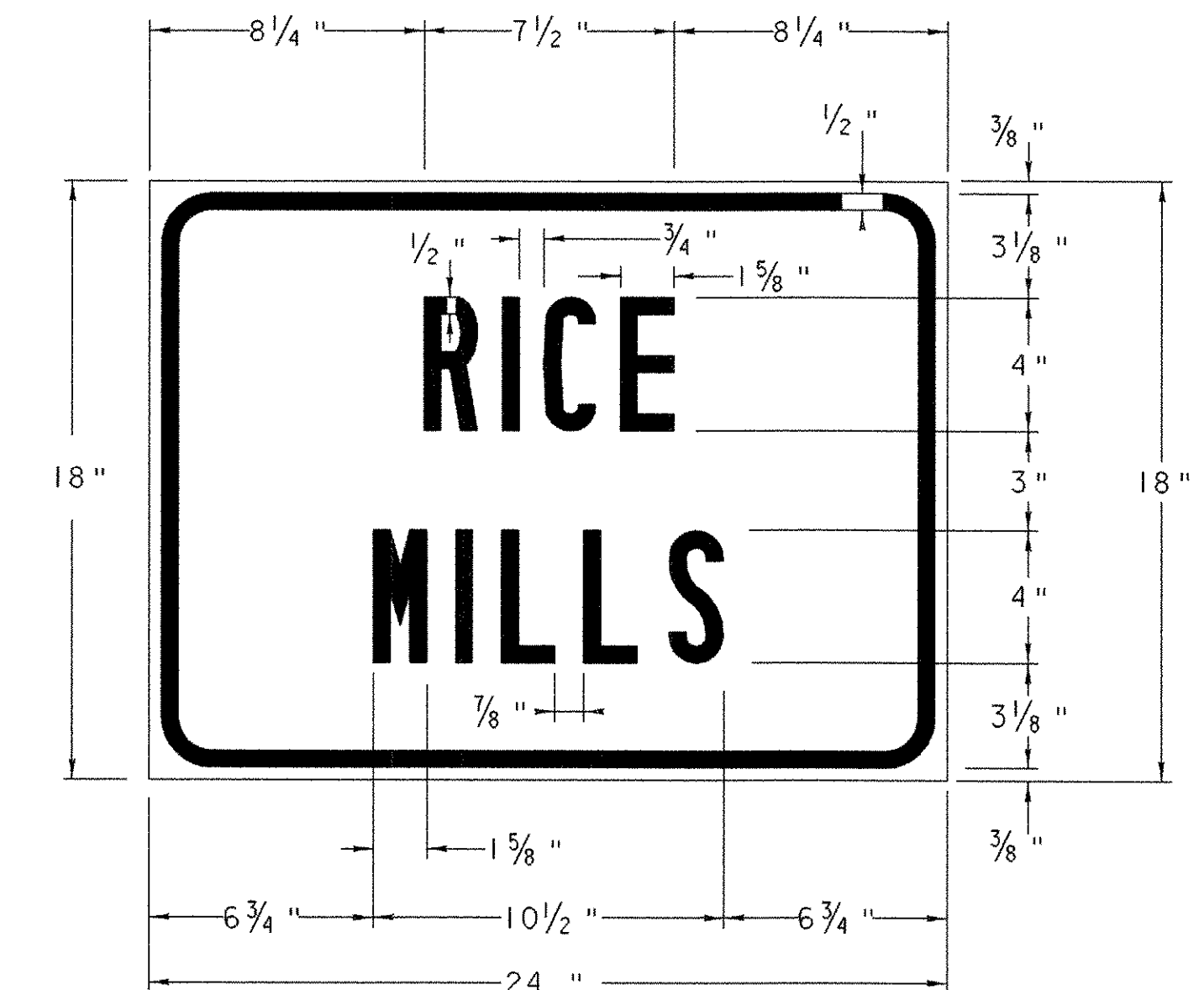
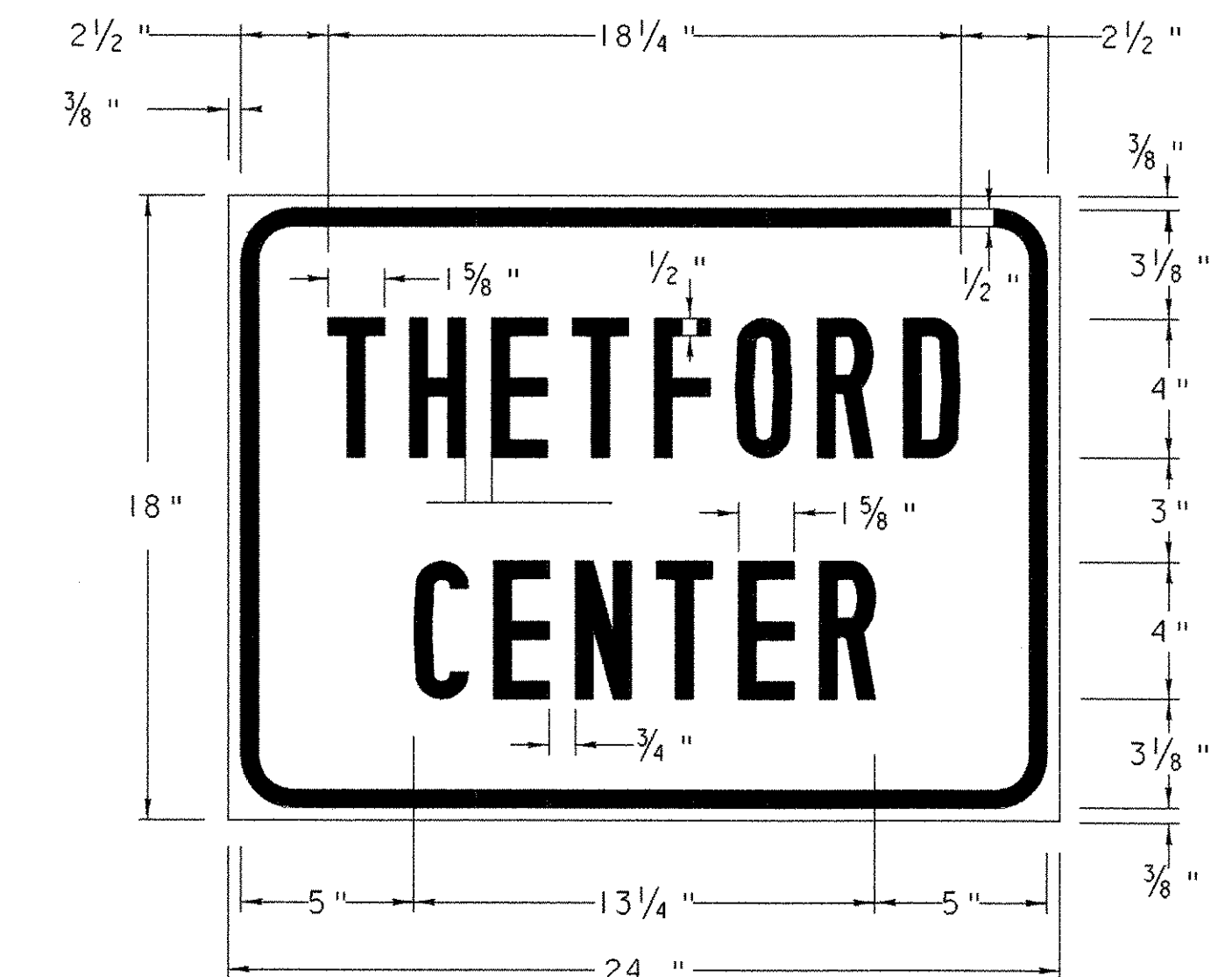


**SIGNAGE AT PROJECT**

DRAWINGS ON THIS SHEET ARE NOT TO SCALE

NOTE 1:

PLACEMENT OF CONSTRUCTION SIGNS IN THE INDICATED LOCATIONS ARE APPROXIMATE AND MAY NEED TO BE REPOSITIONED TO ENSURE MOTORIST WILL HAVE ADEQUATE TIME TO OBSERVE THE INDICATED POSTED SIGNAGE.

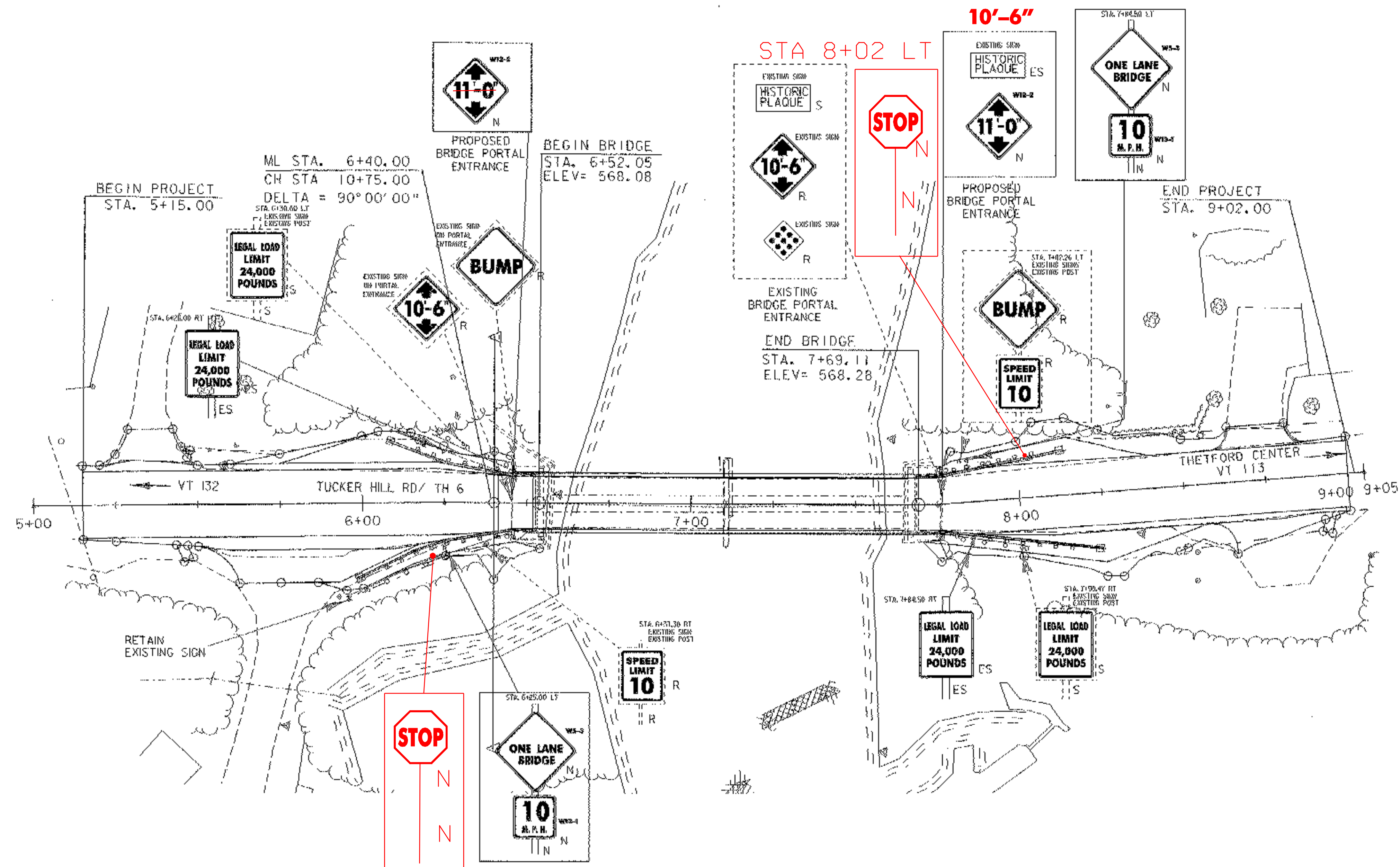


**CUSTOM DETOUR SIGNS**

CUSTOM DETOUR SIGNS "THETFORD CENTER" AND "RICE MILLS" WILL BE BLACK LETTERS ON WHITE BACK GROUND, ASTM TYPE III SHEETING.

**TEMPORARY DETOUR SIGNAGE AND ROUTING (3)**

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036det.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 14 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036dt3.1	



STA 6+20 RT

STA 8+02 LT

10'-6"

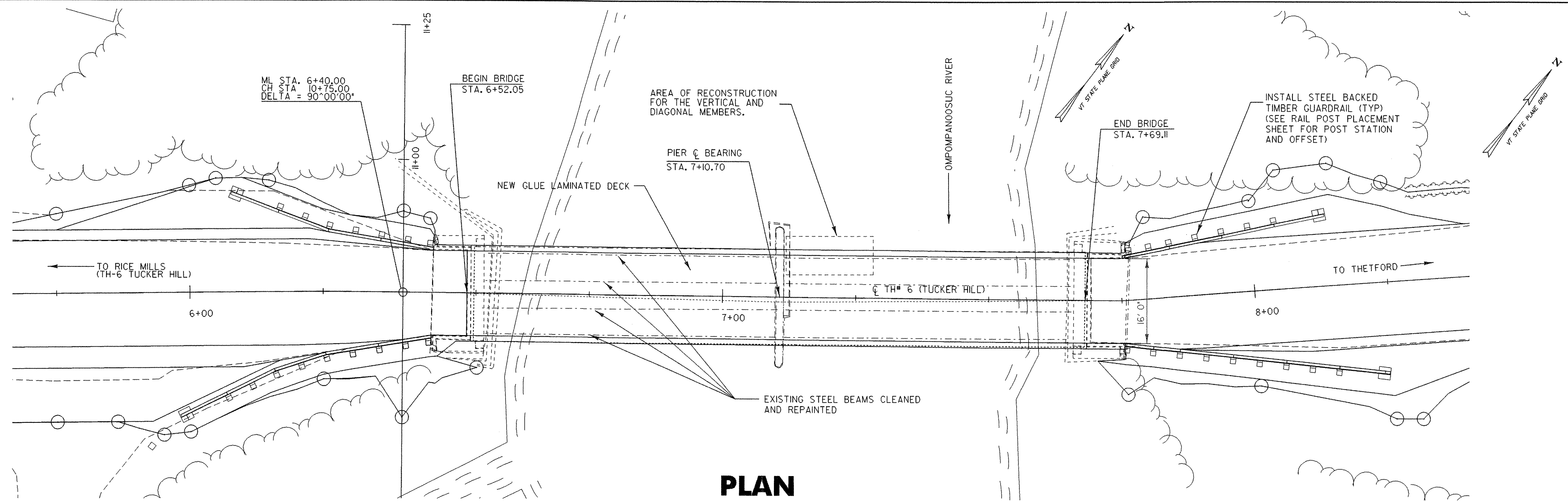


S = SALVAGE  
 ES = ERECT SALVAGE  
 R = REMOVE  
 N = NEW

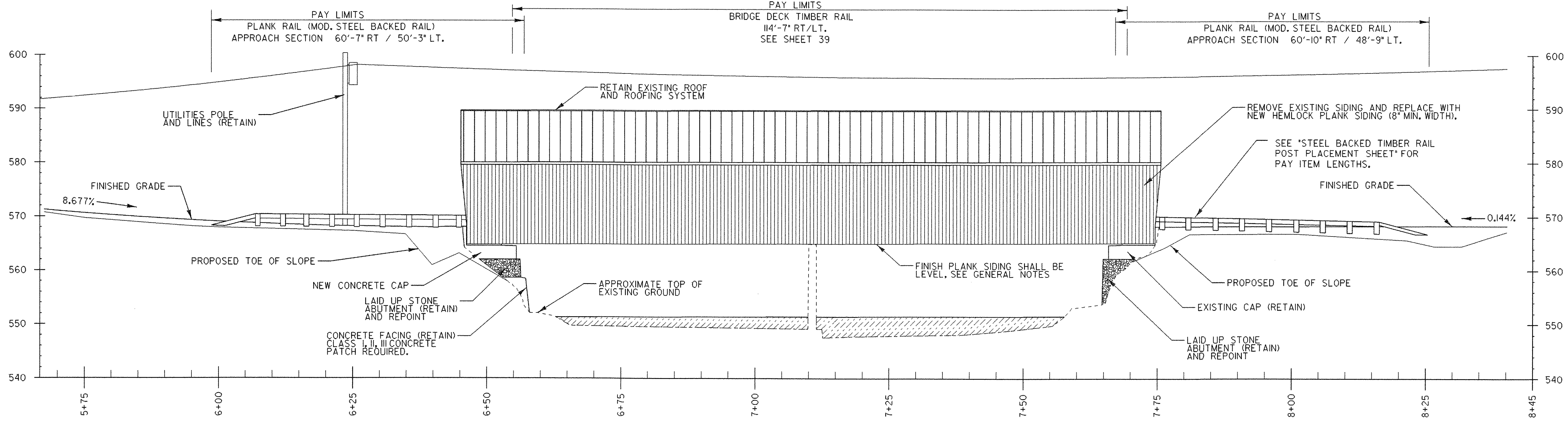
**SIGN LAYOUT SHEET (PERM.)**

PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structure\03\036bdr.dgn
PROJECT LEADER:	M.EVANS-MONGEON
DESIGNED BY:	G. ROKES
IPARM:	s03\036sls.t
PLOT DATE:	27-SEP-2006
DRAWN BY:	G.ROKES
CHECKED BY:	S. SCRIBNER
SHEET	15 OF 60





**PLAN**  
**SCALE 1" = 10'-0"**  
 0 10 20  
 SCALE IN FEET



**ELEVATION**  
**SCALE 1" = 10'-0"**  
 0 10 20  
 SCALE IN FEET

<b>PLAN AND ELEVATION</b>	
PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures\s03j036pe.dgn
PROJECT LEADER:	M.EVANS-MONGEON
DESIGNED BY:	G. ROKES
IPARM:	s03j036pe.l
PLOT DATE:	27-SEP-2006
DRAWN BY:	G. ROKES
CHECKED BY:	S. SCRIBNER
SHEET	17 OF 60

# GENERAL NOTES

1. ALL MATERIALS AND CONSTRUCTION SHALL CONFORM TO THE AGENCY OF TRANSPORTATION'S STANDARD SPECIFICATIONS FOR CONSTRUCTION, DATED 2001, AND ITS LATEST REVISIONS, AND THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SEVENTEENTH EDITION, DATED 2002, AND ITS LATEST REVISIONS. ALL WOOD CONSTRUCTION SHALL COMPLY WITH THE LATEST AASHTO SPECIFICATIONS, THE NATIONAL DESIGN SPECIFICATION (NDS), SUPPLEMENT FOR WOOD CONSTRUCTION, AND THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC) SPECIFICATION.
2. BRIDGE IS DESIGNED FOR H20-44 LIVE LOAD WITH NO ALLOWANCE FOR FUTURE PAVEMENT.
3. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING A SUITABLE STAGING AREA, INCLUDING NECESSARY PERMITS AND CLEARANCES.
4. THE CONTRACTOR SHALL TAKE ALL PRECAUTIONS NECESSARY TO PREVENT SILTATION OR POLLUTION, ESPECIALLY THE DISCHARGE OF RAW CONCRETE, INTO ANY BROOK, STREAM OR RIVER.
5. THE COST OF ANY ON OR OFF PROJECT SIGNING AND BARRICADES, EXCLUDING TEMPORARY TRAFFIC BARRIER, SHALL BE CONSIDERED INCIDENTAL TO CONTRACT ITEM 641.10. TEMPORARY TRAFFIC BARRIERS SHALL BE PAID FOR UNDER CONTRACT ITEM 621.90.
6. ALL WORK SHALL PROCEED IN A CAREFUL, ORDERLY MANNER SO THAT AFFECTED HISTORIC STRUCTURES ARE NOT DAMAGED IN ANY WAY. THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING ALL DAMAGE TO THE STRUCTURE AS A RESULT OF ITS OPERATIONS AT NO COST TO THE STATE. ALL DAMAGE WILL BE REPORTED TO THE PROJECT MANAGER IMMEDIATELY AND NO REPAIRS WILL BE MADE UNTIL APPROVED BY THE AGENCY.
7. THE FOLLOWING TABLE OF ALLOWABLE STRESSES AND WEIGHTS APPLY TO THESE PLANS FOR DESIGN PURPOSES:  
  
STRUCTURAL STEEL, AASHTO M270/M270M GRADE 50  
Fy = 50,000 PSI  
  
CONCRETE, HIGH PERFORMANCE CLASS B:      f'c = 3500 PSI      fc = 1600 PSI  
  
REINFORCING STEEL:      Ft 24,000      PSI GRADE 60  
SOIL: UNIT WEIGHT      140 PCF  
  
CONCRETE
8. ALL CONCRETE SHALL BE CONCRETE, HIGH PERFORMANCE CLASS B AND SHALL BE PAID FOR UNDER ITEM 501.34, CONCRETE HIGH PERFORMANCE CLASS B.
9. ALL REINFORCING STEEL SHALL BE EPOXY COATED AND PAID FOR UNDER ITEM 507.17, EPOXY COATED REINFORCING STEEL. WHEN EPOXY COATED REINFORCING IS TO BE CUT, THE UNCOATED ENDS SHALL BE REPAIRED WITH MATERIALS AND PROCEDURES APPROVED BY THE COATING MANUFACTURER. FLAME CUTTING OF EPOXY COATED REINFORCING STEEL WILL NOT BE PERMITTED.
10. THE KEY IN CONCRETE CONSTRUCTION JOINTS SHALL BE MONOLITHIC AND CONTINUOUS FOR THE FULL LENGTH OF THE JOINT, ANY UPWARD KEY SHALL BE PLACED INTEGRALLY WITH THE CONCRETE BELOW THE JOINT.
11. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 1" BY 1".
12. JOINTS AND SCORE MARKS IN CONCRETE SHALL BE CONSTRUCTED AS INDICATED ON THE PLANS OR AS DIRECTED BY THE ENGINEER.
13. ALL REINFORCING STEEL SHALL BE DETAILED AND FABRICATED USING PROCEDURES AND TOLERANCES IN ACCORDANCE WITH APPLICABLE PUBLICATIONS OF THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI).
14. REINFORCING PLACEMENT TOLERANCES SHALL BE:  
SPACING + - 1"  
CLEARANCE + - 1/4"
15. MINIMUM COVER FOR REINFORCING STEEL SHALL BE TWO (2) INCHES ALONG THE BACK FACES OF WALLS AGAINST EARTH, AND THREE INCHES (3") ELSEWHERE UNLESS OTHERWISE NOTED.
16. SURFACES OF BRIDGE SEATS UNDER BEARING DEVICES SHALL BE LEVEL, OTHER BRIDGE SEAT AREAS SHALL BE SLOPED 1/4" PER FOOT TOWARDS MID-SPAN. THE ENTIRE BRIDGE SEAT SURFACE SHALL BE SMOOTH MAGNESIUM FLOAT FINISHED.
17. WATER REPELLENT (MOD.- SILANE) SHALL BE APPLIED TO ALL EXPOSED CONCRETE SURFACES, PROPOSED AND EXISTING.
18. ABUTMENT #1 WILL HAVE A CONCRETE PATCH APPLIED TO THE CONCRETE BELOW THE LAID-UP-STONE, THE LIMITS OF THE PATCH WILL BE DETERMINED BY THE RESIDENT ENGINEER AND PAID UNDER ITEM 580.13, REPAIR OF CONCRETE SUBSTRUCTURE SURFACE, CLASS I, ITEM 580.14, REPAIR OF CONCRETE SUBSTRUCTURE SURFACE, CLASS II, AND ITEM 580.15, REPAIR OF CONCRETE SUBSTRUCTURE SURFACE, CLASS III.

STRUCTURAL STEEL

19. THE EXISTING STRUCTURAL STEEL IS PAINTED WITH A MATERIAL THAT MAY CONTAIN LEAD. THE CONTRACTOR SHALL FOLLOW ALL APPLICABLE REGULATIONS WHEN HANDLING AND WORKING WITH THIS STEEL. THE REMOVED STRUCTURAL STEEL IS THE PROPERTY OF THE CONTRACTOR. THE CONTRACTOR SHALL INDEMNIFY AND HOLD THE STATE, ITS OFFICERS, AND EMPLOYEES HARMLESS CONCERNING THE CONTRACTORS USE OR DISPOSITION OF THE REMOVED EXISTING STRUCTURAL STEEL.
  20. PAINTING OF NEW STEEL SHALL BE PAID FOR UNDER ITEM 513.25 "STRUCTURAL PAINTING, SHOP APPLIED". ALL EXISTING STEEL RETAINED IN THE REHABILITATED STRUCTURE SHALL BE BLAST CLEANED AND REPAINTED. PAINTING OF EXISTING STEEL SHALL BE PAID FOR UNDER ITEM 513.30 "STRUCTURAL PAINTING, FIELD APPLIED". ALL STRUCTURAL STEEL, EXCEPT THAT WHICH IS SPECIFIED TO BE EITHER GALVANIZED OR METALIZED, SHALL BE PAINTED AS DESCRIBED UNDER ITEM 513.30 STRUCTURAL PAINTING FIELD APPLIED. THE FINAL COAT OF PAINT SHALL BE BROWN AND SHALL CONFORM WITH FEDERAL STANDARD NO. 595, COLOR CHIP NO. 20059. THE CONTRACTOR SHALL ENSURE COMPATIBILITY BETWEEN THE SHOP AND FIELD PAINT SYSTEMS. SEE SECTION 513 OF THE SUPPLEMENTAL SPECIFICATIONS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
  21. ALL NEW STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50 PAINTED UNLESS OTHERWISE NOTED. DIAPHRAGMS (IF NEEDED), CONNECTING ANGLES, CHANNELS AT THE ENDS OF THE DECK PLATES SHALL BE PAID FOR UNDER ITEM 506.60 STRUCTURAL STEEL.
  22. ALL DIMENSIONS ARE HORIZONTAL OR VERTICAL AND ARE GIVEN AT 68 DEGREES F UNLESS OTHERWISE NOTED.
  23. ANY CONNECTIONS THAT ARE NOT DETAILED ON THE PLANS SHALL BE DETAILED BY THE FABRICATOR AND SUBMITTED TO THE STRUCTURES ENGINEER FOR APPROVAL.
  24. ALL WELDING SHALL CONFORM TO THE PROVISIONS OF VERMONT SPECIFICATION 506.10.
  25. ALL FIELD CONNECTIONS SHALL BE MADE WITH 7/8" DIAMETER ASTM DESIGNATION A-325 TYPE I BOLTS IN 15/16" DIAMETER HOLES, UNLESS OTHERWISE NOTED.
  26. ALL NEW BOLTS, LAG SCREWS, SCREWS, NUTS AND WASHERS, OR OTHER METAL ELEMENTS SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO SPECIFICATION M 232. PAYMENT WILL BE CONSIDERED INCIDENTAL TO 522.20 STRUCTURAL LUMBER AND TIMBER - UNTREATED.
  27. ABUTMENT #2 AND PIER BEARINGS TO BE CLEANED IN PLACE, WITH NO REMOVAL OF THE BEARINGS REQUIRED, THIS WILL BE PAID FOR UNDER ITEM 513.41, SURFACE PREPARATION, FIELD.
- TEMPORARY DETOUR
28. TOWN HIGHWAY #6 IS TO BE CLOSED TO THRU TRAFFIC DURING CONSTRUCTION.
  29. THE CONTRACTOR SHALL NOTIFY THE TOWN A MINIMUM OF TWO (2) WEEKS PRIOR TO CLOSING THE ROAD.
  30. BRIDGE NO.27 WILL BE CLOSED TO ALL PEDESTRIAN AND VEHICULAR TRAFFIC DURING CONSTRUCTION. TRAFFIC SHALL BE TEMPORARILY REROUTED AROUND THE SITE. FOR TEMPORARY DETOUR SIGNING AND TRAFFIC CONTROL NOTES SEE TEMPORARY DETOUR SIGNAGE AND ROUTING SHEETS.
  31. THE REMOVAL AND/OR RESETTING OF TRAFFIC SIGNS, AS DEEMED NECESSARY BY THE RESIDENT ENGINEER, WILL BE PAID FOR UNDER ITEM 675.60 ERECTING SALVAGED SIGNS.
  32. ALL EXISTING SIGNS NOT RE-USED SHALL REMAIN THE PROPERTY OF THE TOWN OF THETFORD. THE CONTRACTOR SHALL STOCKPILE THESE ITEMS AT THE PROJECT SITE FOR REMOVAL BY TOWN FORCES. THE CONTRACTOR SHALL CONTACT THE THETFORD TOWN OFFICE WHEN THESE ITEMS ARE READY FOR REMOVAL FROM THE PROJECT SITE.
- TIMBER
33. DIMENSIONS, ANGLES, AND BEARINGS OF THE EXISTING BRIDGE SHOWN ON THESE PLANS WERE OBTAINED FROM LIMITED FIELD INVESTIGATION AND RECORD PLANS AND MAY NOT ACCURATELY REFLECT ACTUAL FIELD CONDITIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAKING FIELD MEASUREMENTS OF ALL EXISTING STRUCTURE COMPONENTS IMPACTED BY THE NEW WORK TO ASSURE CONSISTENCY WITH THE PROPOSED MODIFICATIONS. ANY DISCREPANCIES IN DIMENSIONS, CHARACTER OR EXTENT OF THE EXISTING FEATURES SHALL BE BROUGHT TO THE ATTENTION OF THE RESIDENT ENGINEER BEFORE ADVANCING THE WORK.
  34. THE CONTRACTOR SHALL DEVELOP A METHOD TO SUPPORT BOTH TIMBER AND STEEL AT ITS CURRENT LOCATION THAT WILL ALLOW ALL SUPERSTRUCTURE AND SUBSTRUCTURE WORK TO BE COMPLETED AS REQUIRED IN THE CONTRACT OR AS ORDERED BY THE RESIDENT ENGINEER. THE CONTRACTOR SHALL SUBMIT TO THE STRUCTURES DIVISION FOR APPROVAL CONSTRUCTION DRAWINGS PER SUBSECTION 105.03 OF THE VDOT STANDARD SPECIFICATIONS FOR ALL NECESSARY JACKING, SHORING, AND LIFTING OF THE BRIDGE FOR ALL PHASES OF THE PROJECT. THE CONSTRUCTION DRAWINGS SHALL BE STAMPED BY A PROFESSIONAL ENGINEER AND NO WORK SHALL COMMENCE UNTIL THE DRAWINGS HAVE BEEN APPROVED. THIS WORK SHALL BE PAID AS ITEM 502.10 SHORING SUPERSTRUCTURE.

THIS WORK SHALL BE PAID AS ITEM 502.10 SHORING SUPERSTRUCTURE.

35. THE CONTRACTOR SHALL AVOID UNNECESSARY DISASSEMBLY OF THE TRUSS AND FLOOR SYSTEM. ONLY THE JOINTS THAT MUST BE DISASSEMBLED TO AFFECT THE REPAIRS TO THE TRUSSES SHALL BE DISASSEMBLED.
36. ALL TIMBER MEMBERS AND CONNECTIONS SHALL BE JOINTLY INSPECTED BY THE CONTRACTOR AND RESIDENT ENGINEER TO DETERMINE IF ADDITIONAL REPAIRS ARE NECESSARY. THE FINAL DETERMINATION SHALL BE MADE BY THE RESIDENT ENGINEER.
37. IF THE CONTRACTOR FINDS THAT A SPECIFIC TIMBER ELEMENT IS NOT AVAILABLE, HE MAY SUBMIT AN ALTERNATIVE SPECIES WITH COMPARABLE PROPERTIES FOR CONSIDERATION AND APPROVAL BY THE RESIDENT ENGINEER AT NO ADDITIONAL COST.
38. EXTREME CARE SHALL BE TAKEN TO ENSURE THAT ANY EXISTING HOLES IN MEMBERS TO BE RETAINED SHALL NOT BE ELONGATED OR ENLARGED. THIS INCLUDES, BUT IS NOT LIMITED TO, BOLT HOLES AND TRUNNEL HOLES. THE CONTRACTOR SHALL DEMONSTRATE THEIR METHOD OF INSTALLING NEW MEMBERS WHILE MAINTAINING THE EXISTING HOLE PATTERN IN ADJACENT RETAINED MEMBERS TO THE RESIDENT ENGINEER FOR APPROVAL.
39. ALL EXISTING WOOD TRUNNELS THAT ARE FOUND TO BE DEFICIENT, OR FOUND MISSING, SHALL BE REPLACED WITH NEW WOOD TRUNNELS, AS DEEMED NECESSARY BY THE RESIDENT ENGINEER. NEW TRUNNELS SHALL BE UNTREATED, DRIED (MAXIMUM MOISTURE CONTENT OF 19%), CLEAR, DRY WHITE OAK AND SHALL BE DRIVEN IN A MANNER WHICH AVOIDS SPLITTING OF THE TRUNNELS OR THE ELEMENTS CONNECTED BY THESE TRUNNELS. THE TRUNNELS MAY BE DIPPED IN BOILED LINSEED OIL PRIOR TO DRIVING. PAYMENT FOR TRUNNELS, ALONG WITH REMOVAL AND INSTALLATION COSTS, SHALL BE INCIDENTAL TO ITEM 522.20.
40. AFTER THE BRIDGE SUPERSTRUCTURE IS COMPLETE AND ALL TIMBER HAS BEEN INSTALLED, THE BRIDGE SHALL BE COATED WITH A FIRE RETARDANT COATING AND AN INSECTICIDE/FUNGICIDE COATING. ALL NEW WOOD AND WOOD TO BE RETAINED WILL BE COATED. THE FIRE RETARDANT COATING AND INSECTICIDE/FUNGICIDE COATING SHALL BE PAID UNDER ITEM 513.30, "STRUCTURAL PAINTING, FIELD APPLIED (MOD.)."
41. TREATED LUMBER AND DECK SHALL BE PRESSURE TREATED WITH TYPE II (PENTACHLOROPHENAL) PER SECTION 726 OF THE SUPPLEMENTAL SPECIFICATIONS, UNLESS OTHERWISE NOTED. ALL EXPOSED ENDS AND HOLES DRILLED IN TREATED MEMBERS SHALL BE TREATED BY APPROVED METHODS, PRIOR TO ASSEMBLY.
42. ALL NEW TIMBER CONNECTIONS, INCLUDING NOTCHES, SCARF JOINTS, MORTISE-AND-TENDON JOINTS, AND OTHER CONNECTIONS, SHALL MATCH THE EXISTING JOINTING CONFIGURATION AND SHALL BE CUT TO PROVIDE A TIGHT JOINT. THE RESIDENT ENGINEER SHALL APPROVE ANY DEVIATION FROM THE EXISTING JOINTING TECHNIQUES.
43. ANY EXISTING TRUSS MEMBER (UPPER CHORD, LOWER CHORD, ARCH, DIAGONALS AND VERTICALS) DEEMED TO REMAIN IN PLACE, WHICH IN THE OPINION OF THE RESIDENT ENGINEER NEEDS INSTEAD TO BE REPLACED, WILL BE REPLACED WITH EASTERN SPRUCE, STRUCTURAL GRADE. PAYMENT FOR THIS WORK WILL BE MADE UNDER ITEM 522.20, STRUCTURAL LUMBER AND TIMBER - UNTREATED.
44. ALL NEW STRUCTURAL TIMBER SHALL CONFORM TO SECTION 709.01. NEW ELEMENTS SHALL HAVE A ROUGH FINISH AND MATCH THE EXISTING ELEMENT DIMENSIONS.
45. ALL SIDING BOARDS AND SIDING NAILERS SHALL BE FASTENED WITH 2 1/2" LONG GALVANIZED STEEL SCREWS. COST SHALL BE INCIDENTAL TO ITEM 522.30.
46. ITEM 529.20, PARTIAL REMOVAL OF STRUCTURE, SHALL INCLUDE REMOVING AND DISPOSING OF THE FOLLOWING ITEMS:
  - EXISTING TIMBER DECK AND PAVEMENT
  - EXISTING SIDING
  - STRUCTURAL STEEL CONNECTING TIMBER TRUSS TO FASCIA BEAMS
  - STRUCTURAL STEEL PLATES AT EACH END OF THE DECK AND CHANNELS WHICH SUPPORT THEM.
  - DETERIORATED TIMBER, AS DETERMINED BY THE ENGINEER, (SEE NOTE 37)
  - STEEL RODS AND PLATES AND TIMBER USED TO REPAIR DAMAGED BRIDGE, AREA SHOWN ON SHEET 28.
  - CONCRETE ABUTMENT CAP AT ABUTMENT #1
  - BEARINGS AT ABUTMENT #1
  - HARDWOOD BLOCKS AT ABUTMENTS AND PIERS (IF NECESSARY)
47. THE BRIDGE SHALL BE STRAIGHT ALONG A HORIZONTAL LINE BETWEEN THE ABUTMENTS AND NOT BE RACKED WHEN COMPLETED.

Revised Oct. 24, 2006

PROJECT NAME:	THETFORD BHO 1444 (43)		
PROJECT NUMBER:	03J036		
FILE NAME:	03J036/STR/S03J036	PLOT DATE:	10/24/2006
PROJECT LEADER:	M. EVANS-MONGEON	DRAWN BY:	G. ROKES
DESIGNED BY:	M. EVANS-MONGEON	CHECKED BY:	S. SCRIBNER
GENERAL NOTES SHEET #1		SHEET	18 OF 60

# GENERAL NOTES

48. FULL SAWN STRUCTURAL LUMBER AND TIMBER SHALL INCLUDE THE FOLLOWING MEMBERS:
- A. STRUCTURAL UNTREATED, ITEM 522.20
    - REPLACEMENT AND ADDITIONAL DIAGONALS (EASTERN SPRUCE, STRUCTURAL GRADE)
    - REPLACEMENT CHORD AND ARCH MEMBERS (EASTERN SPRUCE, STRUCTURAL GRADE)
    - REPLACEMENT AND ADDITIONAL VERTICALS (EASTERN SPRUCE, STRUCTURAL GRADE)
    - NEW WEARING FLANKS (WHITE OAK, NO. 1 GRADE)
    - REPLACEMENT TRUNNELS, AS REQUIRED (WHITE OAK, NO. 2 GRADE)
  - B. STRUCTURAL TREATED, ITEM 522.25.
    - NEW BLOCKING, BEARING BLOCKS (SOUTHERN YELLOW PINE, STRUCTURAL GRADE)
    - NEW CURBS (SOUTHERN YELLOW PINE, NO. 1 GRADE)
    - NEW SPACER STRIP (SOUTHERN YELLOW PINE, NO. 1 GRADE)
    - GLUED LAMINATED TIMBER DECK (20F-V5)
49. FULL SAWN NON-STRUCTURAL LUMBER SHALL INCLUDE THE FOLLOWING MEMBERS:
- NON-STRUCTURAL LUMBER UNTREATED, ITEM 522.30
    - NEW SIDING (HEMLOCK, NO. 1 GRADE)
    - NEW NAILING STRIPS 2" X 3" (HEMLOCK, NO. 1 GRADE)
50. THE BOLTS CONNECTING THE DECK TO THE BEAMS SHALL BE RECESSED INTO THE DECK AND AND PLUGGED AND SEALED USING ITEM 524.12.
51. THE EXISTING COVERED BRIDGE TRUSSES ARE NOT PERFECTLY SYMMETRICAL AND THE BEAM SPACING MAY VARY SLIGHTLY. TO HELP FACILITATE CONNECTING THE GLUED LAMINATED DECK PANELS TO THE BEAMS, THE LEAD HOLES FOR THE BOLTS WILL BE DRILLED IN THE FIELD.
52. LEAD HOLES FOR THE LAG SCREWS WILL BE DOUBLE DRILLED. THE DIAMETER OF THE LEAD HOLE FOR THE THREADED PORTION OF THE LAG SCREW WILL BE 1/4 INCHES FOR THE 1/2 INCH DIAMETER LAG SCREW AND 7/16 INCHES FOR THE 3/4 INCH LAG SCREW. THE LEAD HOLE FOR THE UNTHREADED SHANK MUST BE THE SAME DIAMETER AND EXTEND TO THE SAME DEPTH AS THE UNTHREADED PORTION.
53. TRUNNELS IN EXISTING MEMBERS SHALL BE RETAINED EXCEPT WHERE THE RESIDENT ENGINEER DEEMS IT NECESSARY TO REPLACE THEM. NEW TRUNNELS WILL BE THE SAME SIZE AS THE ONE BEING REPLACED.

Revised Oct. 24, 2006

PROJECT NAME:	THETFORD BHO 1444 (43)		
PROJECT NUMBER:	03J036		
FILE NAME:	03J036/STR/S03J036	PLOT DATE:	10/24/2006
PROJECT LEADER:	M. EVANS-MONGEON	DRAWN BY:	G. ROKES
DESIGNED BY:	M. EVANS-MONGEON	CHECKED BY:	S. SCRIBNER
GENERAL NOTES SHEET #2		SHEET	19 OF 60

# EROSION CONTROL NARRATIVE

## DESCRIPTION OF PROJECT

This project involves the rehabilitation of the covered bridge (No. 27) over the Ompompanoosuc River. The project is on TH-6 (Tucker Hill Rd.), a paved town road that is narrow at the project site. The bridge is also situated at the bottom of a vertical sag.

Traffic will be detoured around the project and the bridge will be closed for rehabilitation. The rehabilitation will consist of: removal and replacement of the concrete abutment cap at abutment #1, Removal and replacement of bridge bearings (at abutment #1), Cleaning and repainting of the steel girders. Replace nail laminated deck with a glue laminated deck. Place timber wearing surface on the deck. Placement of a wooden guard rail through the structure. The timber truss will have all broken/damaged members replaced in a manner consistent with the original construction methods. The approaches will have new pavement, wooden rail, and side slopes. All of this work will take place within the existing R.O.W.

There is no channel work needed with this project. Existing Side slopes are steep and all new slopes will be steep (especially near R.O.W. lines).

It is anticipated that this project will last one construction season.

## SITE INVENTORY & ANALYSIS

### OFF SITE DRAINAGE CHARACTERISTICS:

The property surrounding the project site consists of well established vegetation with moderate to steeply sloping banks. The surrounding area consists of mixed softwood and hardwood forest with defined drainage ways. Due to the nature of the surrounding terrain, runoff water entering the project site will be primarily limited to that which is conveyed along roadway ditches. Because the bridge is in a vertical sag, temporary asphalt diversionary berms should be placed in roadway to control offsite water from entering the project site.

### DRAINAGE, WATERWAYS, BODIES OF WATER:

The Ompompanoosuc River is located in the project area. There are no other water bodies or wetlands within the project area. The Ompompanoosuc River, at normal water levels, is a slow velocity river. The river bed is shallow to deep with a sandy silt bottom. Around the bridge abutments there are sandy silt beaches. At ordinary high water these beaches are submerged.

### TOPOGRAPHY, EXISTING ROADS, BUILDINGS, UTILITIES:

The topography of the project site is mountainous with various sized wooded growth around toe of project slopes. The Ompompanoosuc River runs perpendicular to the structure and has medium to steep banks with dense, varied growth overhanging the river. There are three driveways with two all season residences near the project. There are overhead utilities that will be maintained and the need for relocation of the utility poles is unlikely.

### VEGETATION:

A mix of hardwood and softwood trees of all sizes exist along Tucker Hill Rd. (TH-6). The two residences near the bridge site have small areas of lawn and landscape plantings. The one land owner at Stations 8+25 thru 8+64 has hedges within the existing R.O.W. and these hedges will be retained. During the bridge rehabilitation, the original concrete cap at abutment #1 is to be replaced. This will require a small amount of soil excavation around the old cap. During this time, the entire project may not have all the erosion controls in place. Placement of proper erosion controls around the abutment cap will be needed at this time. The rest of the erosion controls may be placed once the project proceeds to the approach work and guard rail slopes phase. During the approach work, slopes greater than 3:1 will require erosion control matting. Slopes will be vegetated/reestablished with standard seed & mulching practices.

### SOILS:

The Soil Conservation Service has mapped the soils throughout Orange County. The soil type identified for this project site is as follows:

(NnB) Ninigret-fine sandy loam, 0 to 8% slopes. This soil type is described as "...Level to gently sloping soil is depression areas on stream terraces and are irregular in shape. This soil is considered to be potentially highly erodible. The soil is somewhat poorly drained to moderately well drained.

The erodibility coefficient for this soil is rated (k value) is 0.32.

The other soil found at this site is: (HdD) Hartland silt loam 15 to 25%

(HdD) Hartland silt loam 15 to 25% slopes. This type of soil is described as "...Moderately steep soil on strongly dissected terraces, also included are areas of soil that have a surface layer of very fine sandy loam, steep spots, out crop of bedrock, and small eroded spots. This soil is considered to be highly erodible in areas of no vegetative cover. This soil is somewhat poorly drained to moderately well drained.

The erodibility coefficient for this soil is rated (k value) of 0.49.

Generally, K-values indicate the following: 0.0 - 0.23 = low erodibility; 0.24 - 0.36 = moderate erodibility; 0.37 and higher = higher erodibility.

### SENSITIVE RESOURCE AREAS:

No 'Threatened & Endangered Species' have been identified within the project limits. There is a Historic site and archeologically sensitive areas in the vicinity of the project, but not within the project limits. There are wetlands in the vicinity of the project, these also are not within the project limits. The only resource within the project limits is the Ompompanoosuc river.

### PROXIMITY TO NATURAL OR MAN-MADE WATER FEATURES:

Disturbance of soils near natural or man-made waterways consists of that which is necessary to construct two new concrete abutment caps and applicable roadway approaches as well as guard rail flares with slopes. No stabilization of stream banks will be included with this project since this project does not have any channel work associated with it.

## TEMPORARY EROSION PREVENTION & SEDIMENT CONTROL

Temporary erosion prevention measures to be utilized include:

"Project Demarcation Fencing," denoted -PDF- on the plans, to delineate the limits the contractor can access. This measure limits the area that the contractor may, but not necessarily will, disturb and expose soils to erosion.

Seeding, mulching and biodegradable erosion control matting, or an equivalent product, will be utilized on all slopes steeper than 3:1.

These slopes shall be stabilized within 48 hours of reaching final grade or during intermittent phases of construction activity.

Temporary mulching and or matting will be utilized on a regular basis. Any slopes to be exposed for several days prior to final grading shall be mulched and or matted. The forecast of rainfall events shall also trigger protection of exposed slopes and matting of slopes greater than 3:1.

Temporary asphalt diversionary berms will be placed on the existing asphalt surfaces to channel off site roadway water flow from entering the project and thus reduce the potential for erosion. Placed at the end of the asphalt diversionary berms will be flow decelerators and silt fencing to slow the water flow and trap any sediment. See 'Erosion Controls Details' sheet. The diversionary berms may be removed once the all slopes (including slopes greater than 3:1) and surrounding areas are stabilized.

Temporary measures to control sediment transport include:

Silt fence will be installed and maintained as indicated in "erosion & sediment control plan". The contractor may need to adjust locations as indicated on the plans to better suit their construction needs. Proposed and or alternate silt fence locations will prevent sediment transport to down gradient areas. Each line of silt fence will be placed along the contour with ends turned slightly uphill to create a ponding effect should water try to run along the fencing and around the ends. Because of narrow width of the project, short effective runs of silt fencing should be installed. Silt fence shall be installed prior to any upslope earthwork.

Measures such as temporary asphalt diversionary berms, silt fence, and diversionary flow decelerators shall be checked regularly for accumulation of sediment. Sediment build-up shall be removed when the level of sediment reaches one-half the height of the control measure or if sediment renders the erosion control device ineffective for its intended purpose. Sediments shall be disposed of in an approved area such that they will not be subject to erosion.

Temporary sediment settling basins may not be utilized on this project.

## PERMANENT EROSION CONTROL MEASURES

Grass, or other suitable ground cover will be established outside of the roadway limits. Specifically, 3:1 and greater slopes shall be matted and seeded promptly upon achieving final grade.

## GENERAL EROSION & SEDIMENT CONTROL GUIDELINES

The Erosion Control Plans are meant as a guideline for preventing erosion and controlling sediment transport. The work outlined in this narrative consists of applying measures throughout the life of the project to control erosion and minimize the sedimentation of receiving waters. The measures include stabilization and structural practices, storm water controls and other pollution prevention controls.

Coordinate the installation, use, and removal of erosion and sediment control measures with construction activities to ensure economical, effective and continuous erosion and sediment control. Employ temporary stabilization practices in incremental stages as construction proceeds. The contractor will use additional erosion control measures as necessitated by the sequence of construction and as directed by the Engineer. See section 105.23 of the Vermont AOT Standard Specifications for Construction, dated 2001.

Install all erosion and sediment control measures as shown in the Erosion Control Plan or as directed by the Engineer. Do not modify the type, size or location of any control or practice without approval of the Engineer. Any changes shall be noted on the plans, in the weekly inspection report, and reported to the appropriate authority in a timely manner. Inspect all control measures weekly and after each rainfall event. Repair measures promptly once damage is discovered.

Preventing initial soil erosion is much more effective than treating eroded sediment. Therefore, stabilize all disturbed areas promptly after construction activity has temporarily or permanently ceased. Temporary vegetation shall be established if the area is to be without construction activity for a period of 14 days. Perimeter control measures shall be installed following clearing, but prior to the start of any grubbing or grading activity, install other temporary controls in incremental stages as construction proceeds.

Maintaining vegetated buffers along stream banks, wetlands or other sensitive areas is a crucial erosion and sediment control measure that should be utilized wherever possible.

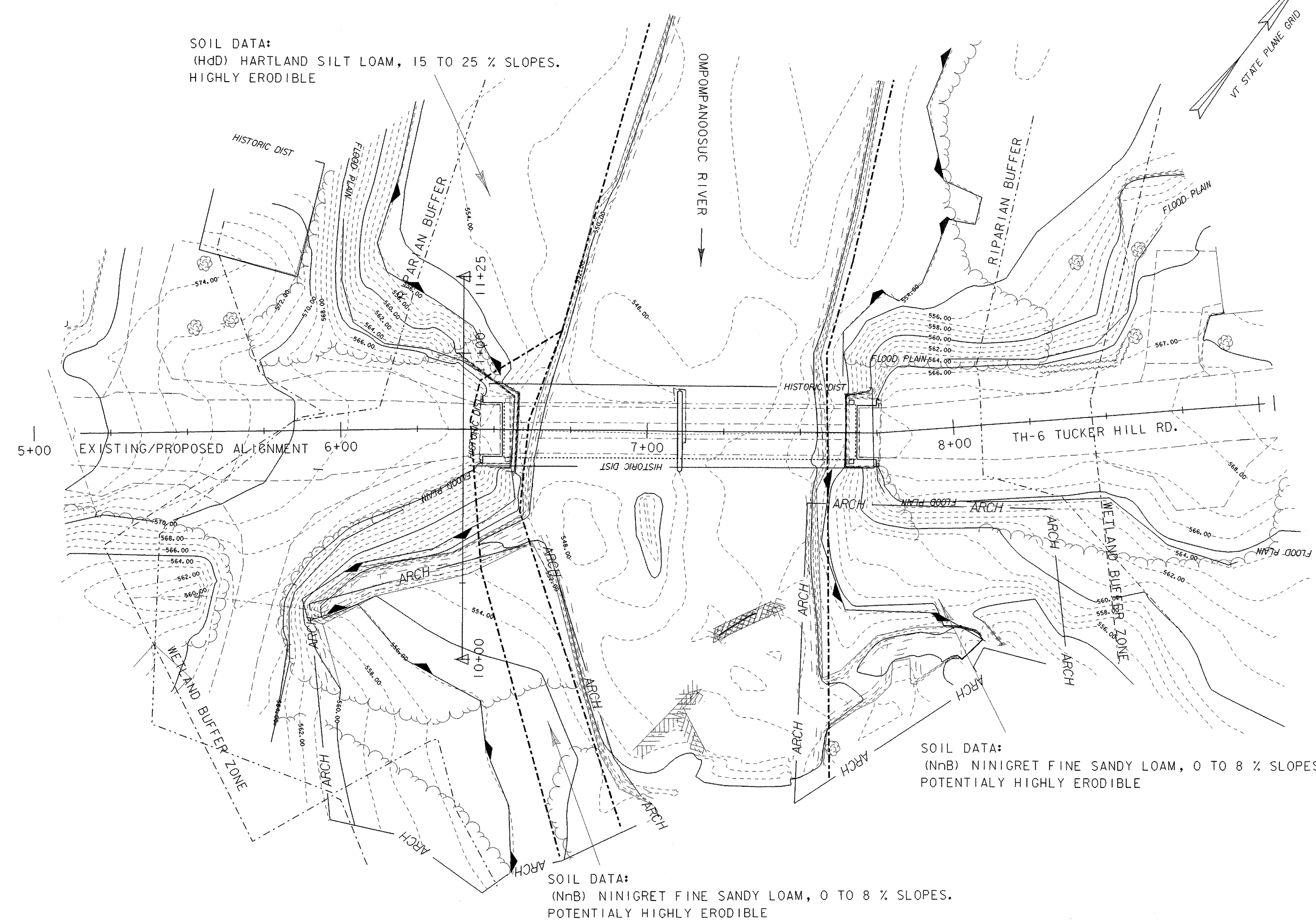
Control only sediment-laden runoff generated by the project site. Collect and route clean offsite runoff around or through the project site using diversion berms, diversion channels, culverts and/or temporary pipes.

Do not allow construction equipment to operate on the down slope side of perimeter control measures.

## EROSION CONTROL NARRATIVE

PROJECT NAME:	THETFORD	PLOT DATE:	1/31/2006
PROJECT NUMBER:	BHO 1444 (43)	DRAWN BY:	G.ROKES
FILE NAME:	structures/sj036ero.dgn	CHECKED BY:	M. E. MONGEON
PROJECT LEADER:	M. EVANS-MONGEON	SHEET	20 OF 60
DESIGNED BY:	G.ROKES		
IPARM	s03j036ena.i		

LEGEND	
-----	WETLAND BUFFER ZONE
-----	RIPARIAN BUFFER
////	LEDGE
ARCH	ARCHEOLOGICAL LIMITS
FLOOD PLAIN	ESTIMATED FLOOD PLAIN
HISTORIC DIST	HISTORIC DISTRICT
▲	WETLAND BOUNDARY
-----	SOIL TYPE BOUNDARY

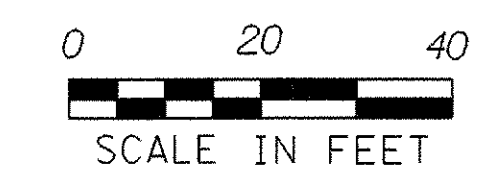


SOIL DATA:  
(HdD) HARTLAND SILT LOAM, 15 TO 25 % SLOPES.  
HIGHLY ERODIBLE

SOIL DATA: (NnB) GROUP  
NINIGRET FINE SANDY LOAM, 0 TO 8 % SLOPES.  
THE SOIL CONSERVATION SERVICE HAS DESCRIBED THIS SOIL AS FOLLOWS:  
THIS LEVEL TO GENTLY SLOPING SOIL IS IN DEPRESSIONAL AREAS ON STREAM TERRACES AND AREAS ARE IRREGULAR IN SHAPE.  
THIS SOIL IS CONSIDERED TO BE POTENTIALLY HIGHLY ERODABLE. THIS SOIL IS CLASSIFIED AS A TYPE B HYDROGROUP. THIS INDICATES THAT THE SOIL IS SOMEWHAT POORLY DRAINED TO MODERATELY WELL DRAINED.  
THE SOIL ERODIABILITY COEFFICIENT (Kw-VALUE) FOR THIS SOIL TYPE IS 0.32.

SOIL DATA: (HdD) GROUP  
HARTLAND SILT LOAM, 15 TO 25 % SLOPES.  
THE SOIL CONSERVATION SERVICE HAS DESCRIBED THIS SOIL AS FOLLOWS:  
THIS MODERATELY STEEP SOIL IS ON STRONGLY DISSECTED TERRACES. ALSO INCLUDED ARE AREAS OF SOILS THAT HAVE A SURFACE LAYER OF VERY FINE SANDY LOAM, STEEP SPOTS, OUTCROPS OF BEDROCK, AND SMALL ERODED SPOTS.  
THIS SOIL IS CONSIDERED TO BE HIGHLY ERODABLE IN AREAS OF NO VEGETATIVE COVER. THIS SOIL IS CLASSIFIED AS A TYPE B HYDROGROUP THIS INDICATES THAT THE SOIL IS SOMEWHAT POORLY DRAINED TO MODERATELY WELL DRAINED.  
THE SOIL ERODAILITY COEFFICIENT (Kw-VALUE) FOR THIS SOIL TYPE IS 0.49

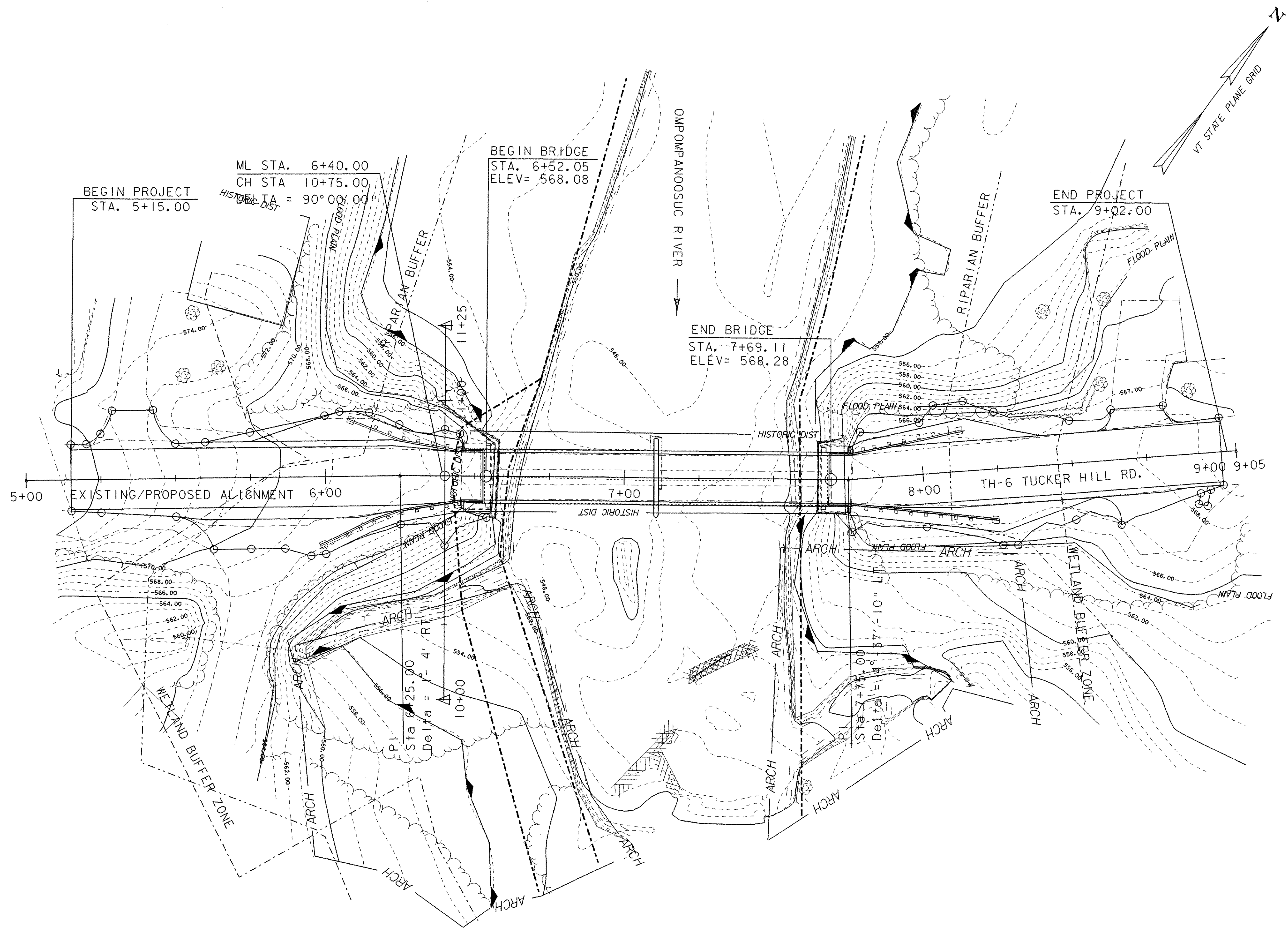
SOIL DATA:  
(NnB) NINIGRET FINE SANDY LOAM, 0 TO 8 % SLOPES.  
POTENTIALLY HIGHLY ERODIBLE



**EROSION AND SEDIMENT CONTROL PLAN  
EXISTING CONDITIONS**

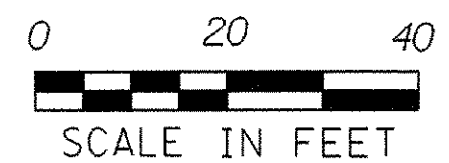
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PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures\s03j036ero.dgn
PROJECT LEADER:	M.EVANS-MONGEON
DESIGNED BY:	G. ROKES
IPARM:	s03j036er1.i
PLOT DATE:	27-SEP-2006
DRAWN BY:	G.ROKES
CHECKED BY:	S. SCRIBNER
SHEET	21 OF 60





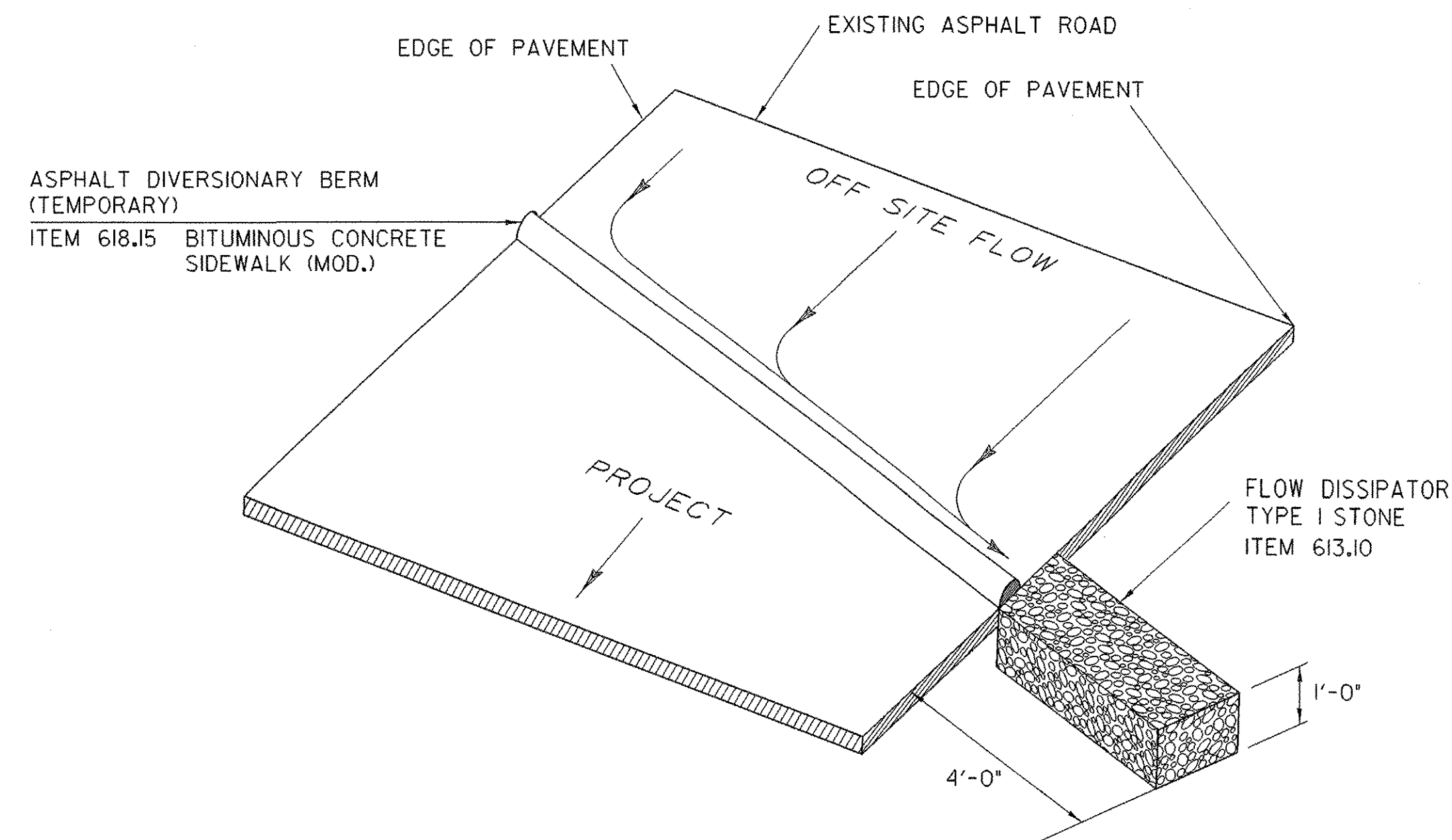
LEGEND	
-----	WETLAND BUFFER ZONE
-----	RIPARIAN BUFFER
////	LEDGE
ARCH	ARCHEOLOGICAL LIMITS
FLOOD PLAIN	ESTIMATED FLOOD PLAIN
HISTORIC DIST	HISTORIC DISTRICT
▲	WETLAND BOUNDARY
-----	SOIL TYPE BOUNDARY

NOTE:  
 CONTOURS SHOWN ARE THE EXISTING SITE CONTOURS.  
 SEE CROSS SECTIONS AND PLAN VIEW FOR GRADE CHANGES.

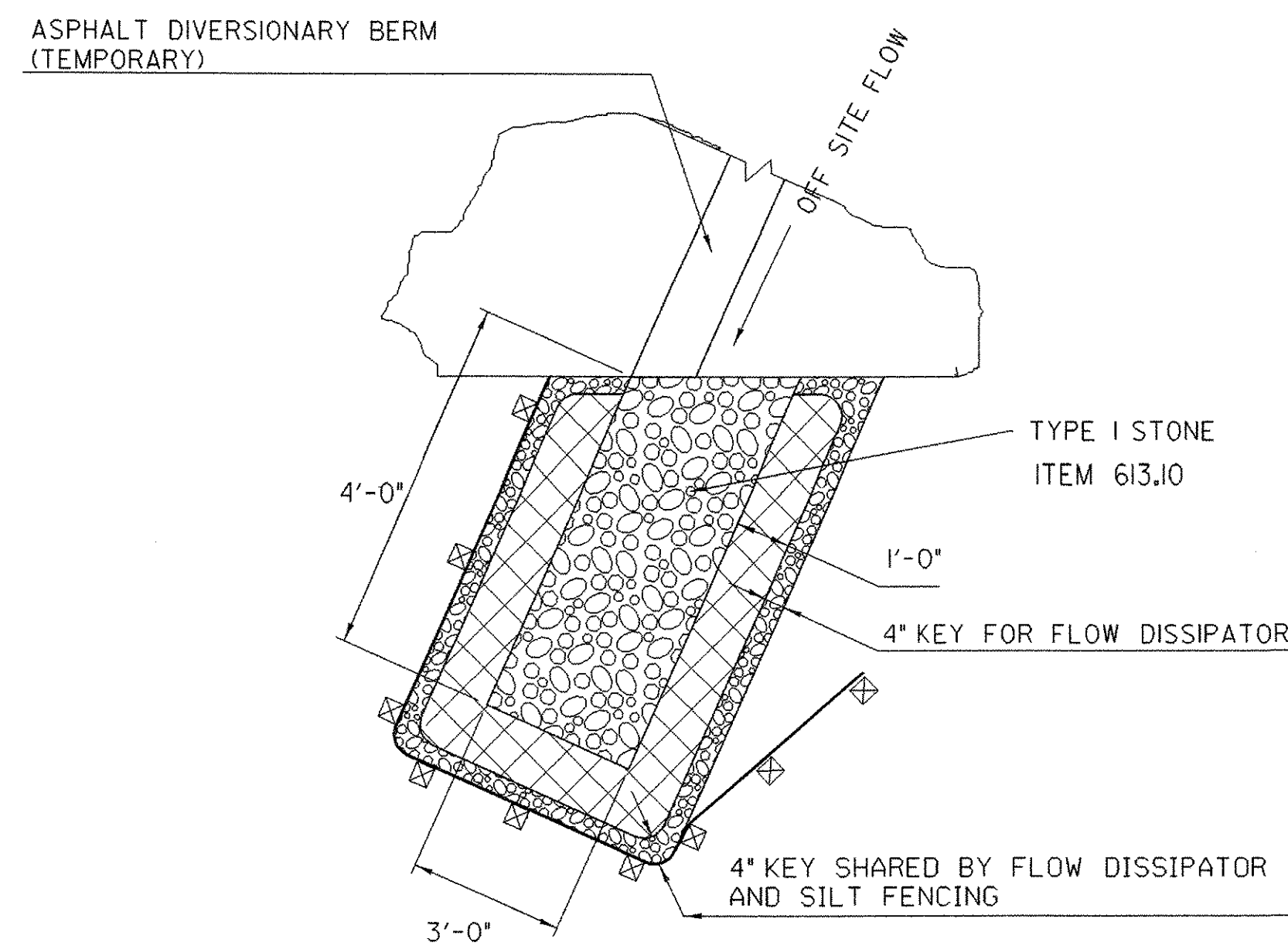


**EROSION AND SEDIMENT CONTROL PLAN  
 FINAL SITE CONDITIONS**

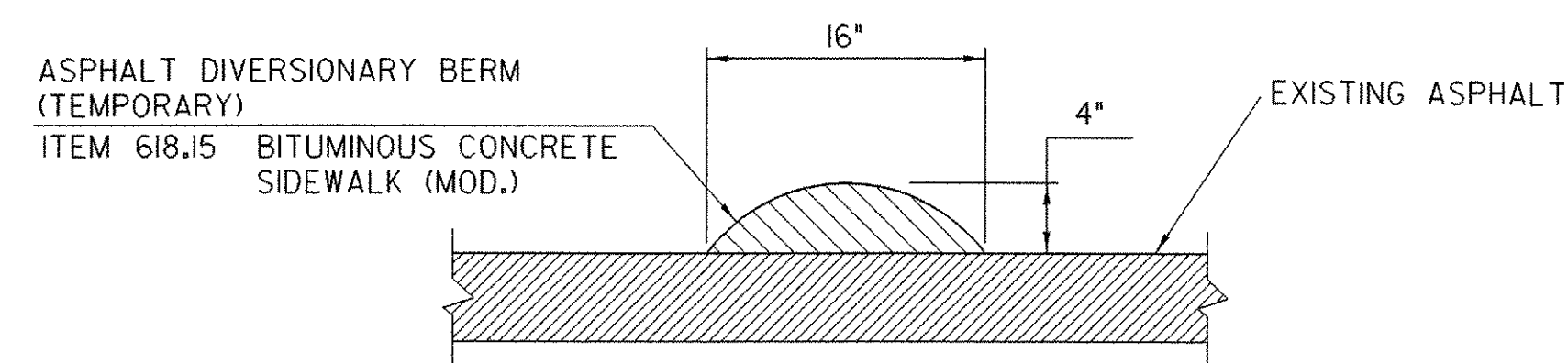
PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures\s03j036ero.dgn
PROJECT LEADER:	M.EVANS-MONGEON
DESIGNED BY:	G. ROKES
IPARM:	s03j036er3.1
PLOT DATE:	27-SEP-2006
DRAWN BY:	G.ROKES
CHECKED BY:	S. SCRIBNER
SHEET	23 OF 60



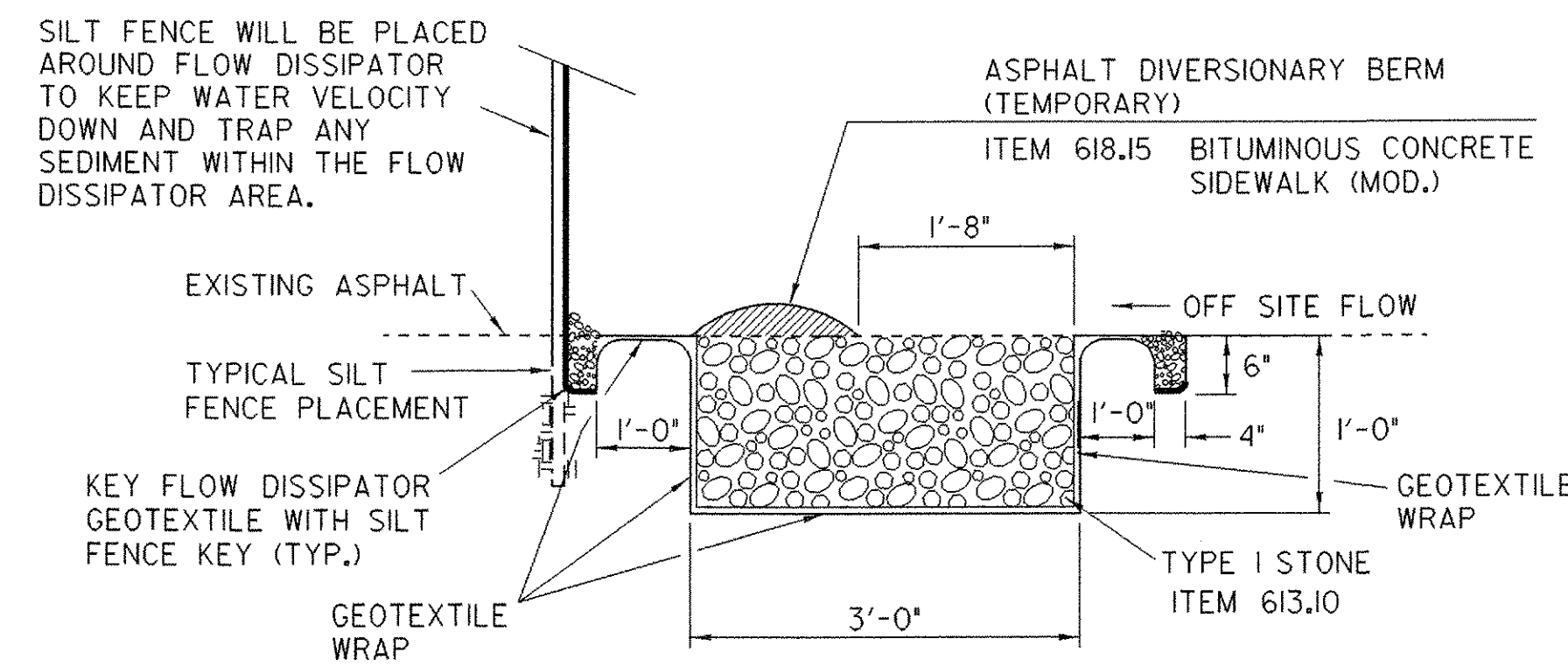
DIVERSIONARY BERM AND FLOW DISSIPATOR SETUP



PLAN VIEW OF FLOW DISSIPATOR



ASPHALT DIVERSIONARY BERM



CROSS SECTION OF FLOW DISSIPATOR

**SEEDING FORMULA  
RURAL AREAS**

% WT.	kg/ha	NAME	PUR %	GERM %
37.5	26.0	CREeping RED FESCUE	98	85
37.5	26.0	TALL FESCUE	95	90
5.0	4.0	RED TOP	95	90
15.0	10.0	BIRDSFOOT TREFOIL	98	85
5.0	4.0	ANNUAL RYE GRASS	95	85
100.0	70.0			

**GENERAL NOTES**

- SEED MIXTURE: SHALL NOT HAVE A WEED CONTENT EXCEEDING 0.40% BY WEIGHT AND SHALL BE FREE OF ALL NOXIOUS SEED.
- SEED: TO BE APPLIED PER SEEDING FORMULAS OR AS DIRECTED BY THE ENGINEER.
- FERTILIZER: FORMULA 10-20-10, TO BE USED WITH SEED, APPLIED AT THE RATE OF 560 kg/ha. (HYDRO SEEDERS MAY USE 19-19-19 FORMULA).
- AGRICULTURAL LIMESTONE: TO BE APPLIED AT THE RATE OF 4500 kg/ha, OR AS DIRECTED BY THE ENGINEER.
- HAY MULCH: TO BE PLACED ON EARTH SLOPES AT THE RATE OF 4500 kg/ha, OR AS DIRECTED BY THE ENGINEER.
- TOPSOIL: TO BE USED WITH SEED AS INDICATED ON THE PLANS, OR AS DIRECTED BY THE ENGINEER.

**NOTES:**

- PAYMENT FOR ASPHALT DIVERSIONARY BERM SHALL BE MADE UNDER ITEM 618.15, BITUMINOUS CONCRETE SIDEWALK (MOD.)
- PAYMENT FOR FLOW DISSIPATOR SHALL BE MADE UNDER ITEMS 613.10, STONE FILL, TYPE I, 649.31, GEOTEXTILE UNDER STONE FILL, AND 649.51, GEOTEXTILE FOR SILT FENCE.

**EROSION PREVENTION &  
SEDIMENT CONTROL DETAILS  
DIVERSIONARY BERM AND FLOW DISSIPATOR**

**EROSION & SEDIMENT CONTROL DETAILS 1**

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036ero.dgn	DESIGNED BY: M. EVANS-MONGEON
PROJECT LEADER: M. EVANS-MONGEON	CHECKED BY: S. SCRIBNER
IPARM: s03j036ecd.1	SHEET 24 OF 60

NOTE: ALL EROSION DETAILS NOT TO SCALE

## SILT FENCE

### APPLICATION NOTES:

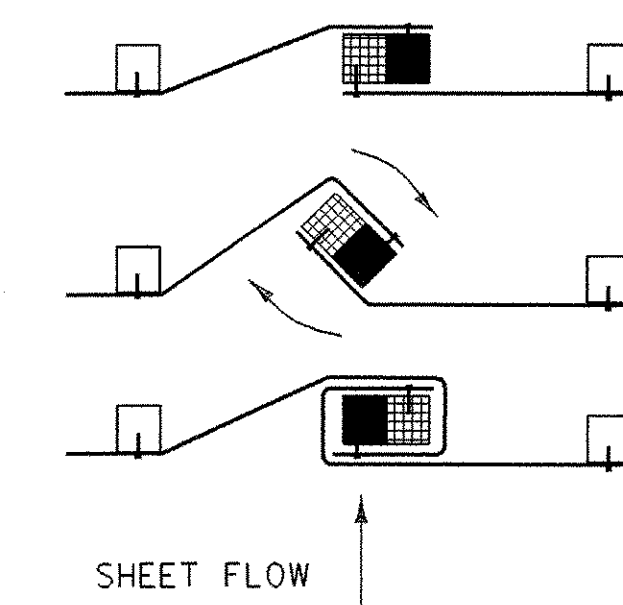
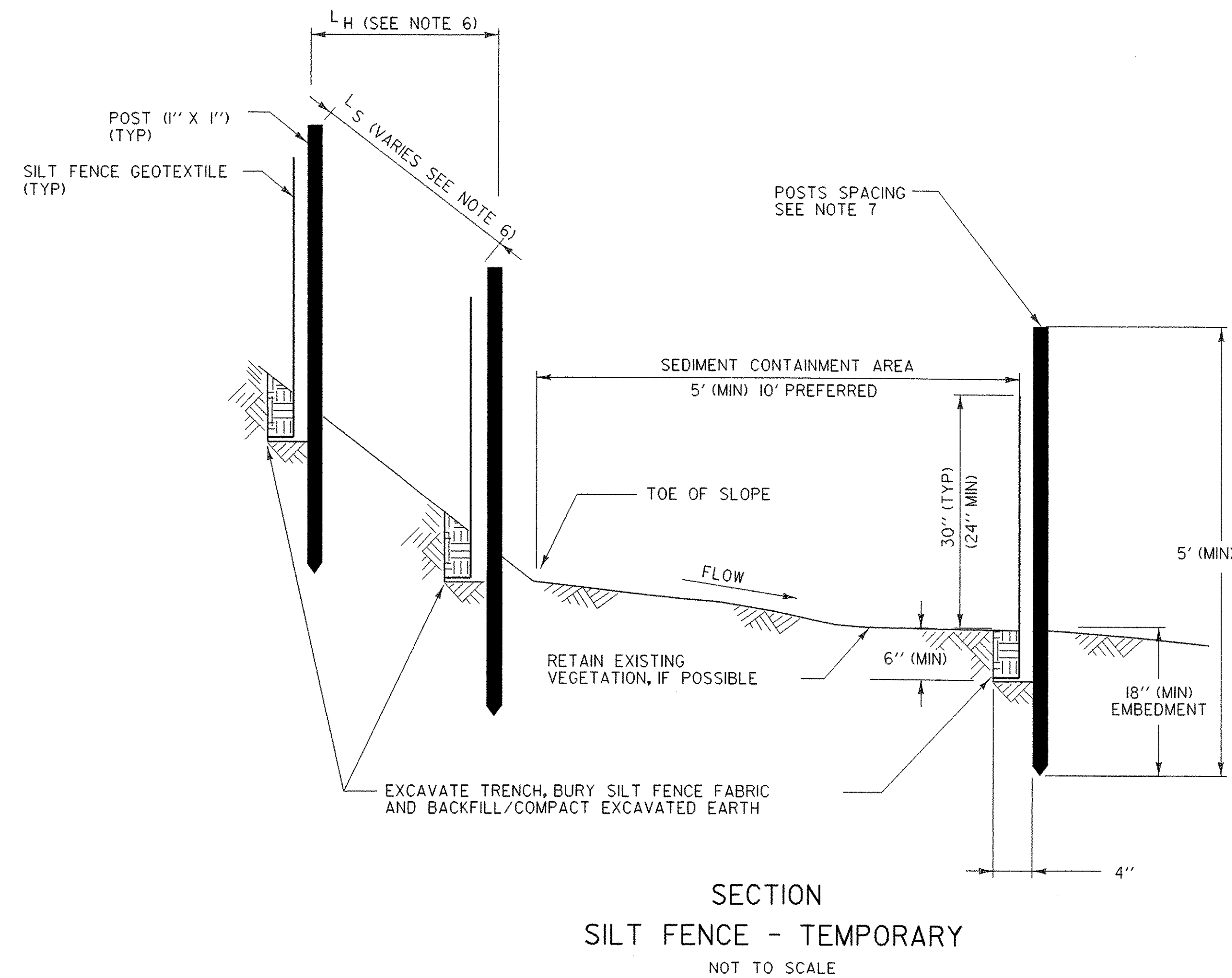
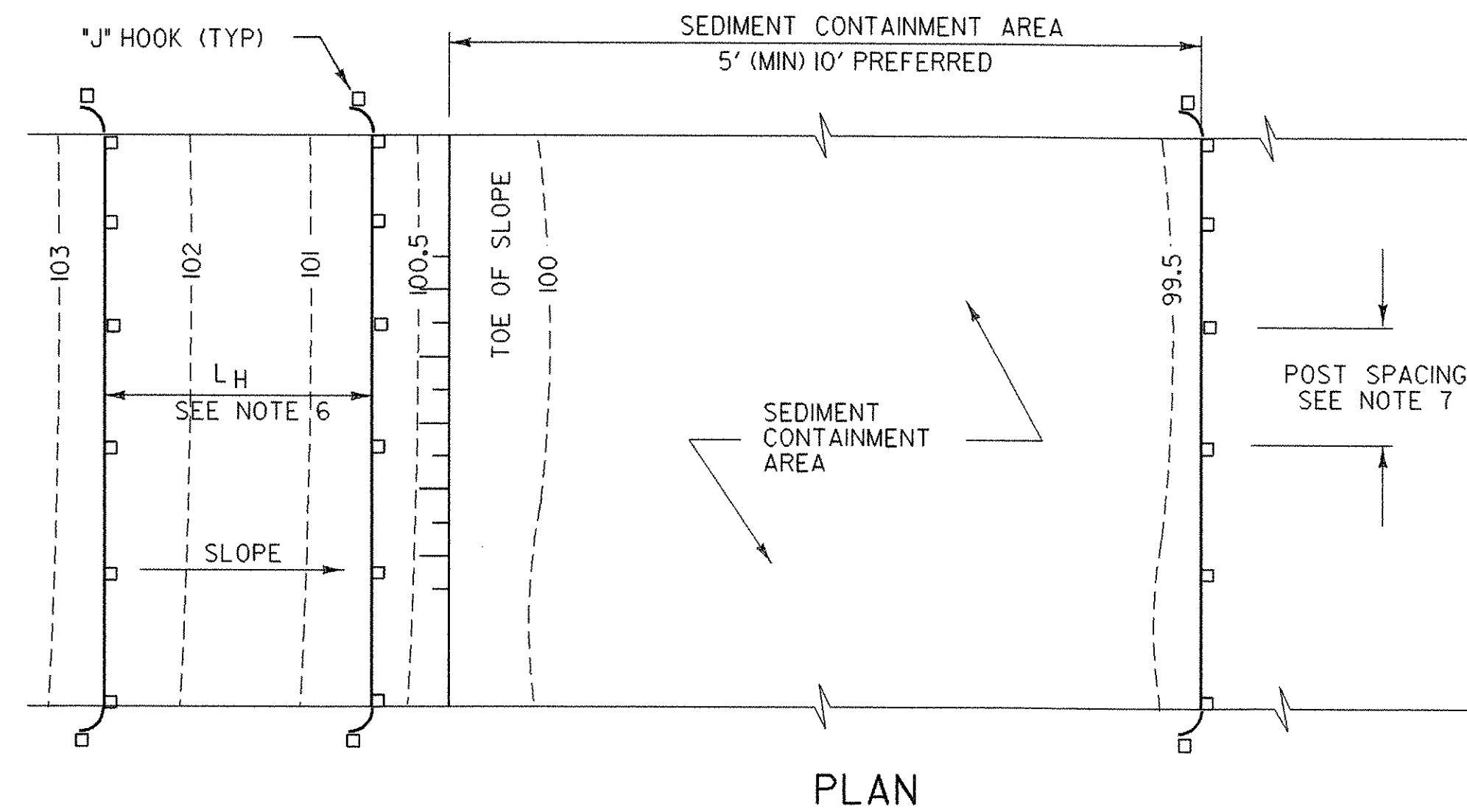
- A. THE PRIMARY PURPOSE OF SILT FENCE IS TO REDUCE RUNOFF VELOCITY AND TRAP SEDIMENT. VELOCITY IS REDUCED, WATER IS IMPOUNDED BEHIND THE MEASURE, AND SEDIMENT FALLS OUT OF SUSPENSION.
- B. SILT FENCE SHALL NOT BE USED ACROSS CONCENTRATED FLOW.

### GENERAL NOTES:

1. SILT FENCE SHALL GENERALLY BE PLACED A MINIMUM OF 5 FEET BEYOND TOE OF SLOPE, 10 FEET PREFERRED, TO PROVIDE ADEQUATE AREA FOR SEDIMENT STORAGE AND FACILITATE MAINTENANCE OF SEDIMENT CONTAINMENT AREA.
2. SILT FENCE SHALL BE INSTALLED ON A LINE OF EQUAL ELEVATION (CONTOUR). IT MAY BE INSTALLED AT INTERMEDIATE POINTS UP SLOPES AS WELL AS AT THE BOTTOM, AS SHOWN IN THE DETAIL.
3. ALL ENDS SHALL BE "J" HOOKED TO TRAP SEDIMENT.
4. IN AREAS WITH TWO SLOPES, SILT FENCE SHALL BE USED TO ERECT A DAM AND TRAP SEDIMENT AT THE BASE OF THE STEEPER SLOPE.
5. THE BOTTOM EDGE OF SILT FENCE SHALL BE BURIED A MINIMUM OF 6 INCHES BELOW GROUND, AND KEYED IN 4 INCHES. THE FENCE SHALL BE INSTALLED WITH THE POSTS ON THE DOWNSTREAM SIDE OF THE FABRIC.
6. MAXIMUM DRAINAGE AREA TRIBUTARY TO 100 FEET OF SILT FENCE SHALL BE 0.25 ACRES.
7. THE FOLLOWING ARE MAXIMUM LENGTHS FOR SILT FENCE INSTALATIONS:

CONSTRUCTED SLOPE	SLOPE LENGTH (LS) FT	HORIZONTAL LENGTH (LH) FT
3H : 4V	80	75
4H : 4V	130	125
5H : 4V	200	200
> 5H : 4V	250	250

8. WHERE ELONGATION IS >50%, POST SPACING SHALL NOT EXCEED 4 FEET. WHERE ELONGATION IS <50%, POST SPACING SHALL NOT EXCEED 6 FEET.
9. SILT FENCE SHALL BE INSPECTED EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF A STORM EVENT GREAT ENOUGH TO CAUSE WATER TO LEAVE THE CONSTRUCTION SITE.
10. SILT FENCE SHALL BE CLEANED AND REPAIRED AS NEEDED. SEDIMENT SHALL BE REMOVED WHEN ACCUMULATION REACHES ONE-HALF OF THE MEASURE HEIGHT. SEDIMENT SHALL BE DISPOSED OF AT AN APPROVED WASTE SITE.
11. SILT FENCE SHALL BE REMOVED WHEN THE AREA HAS BEEN STABILIZED. AT TIME OF REMOVAL OF THE SILT FENCE, THE DISTURBED AREA SHALL BE REPAIRED AND STABILIZED.



1. PLACE THE END POST OF ONE FENCE INSIDE THE END POST OF THE OTHER FENCE.
2. ROTATE BOTH POSTS AT LEAST 180 DEGREES IN A CLOCKWISE DIRECTION TO CREATE A TIGHT SEAL WITH THE FABRIC MATERIAL.
3. DRIVE BOTH POSTS 18 INCHES INTO THE GROUND AND BURY THE FLAP IN THE TRENCH.

### SPlicing DETAIL

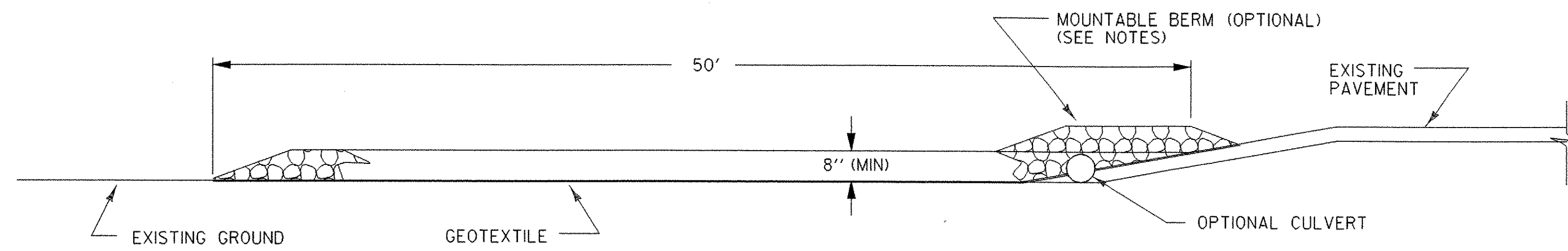
### NOT TO SCALE EROSION & SEDIMENT CONTROL DETAILS 2

PROJECT NAME: THETFORD  
PROJECT NUMBER: BHO 1444 (43)

FILE NAME: Structures\s03j036ero.dgn PLOT DATE: 27-SEP-2006  
PROJECT LEADER: M.EVANS-MONGEON DRAWN BY: G.ROKES  
DESIGNED BY: M. EVANS-MONGEON CHECKED BY: S. SCRIBNER  
IPARM: s03j036ecd.l SHEET 25 OF 60

## EROSION PREVENTION & SEDIMENT CONTROL DETAILS SILT FENCE

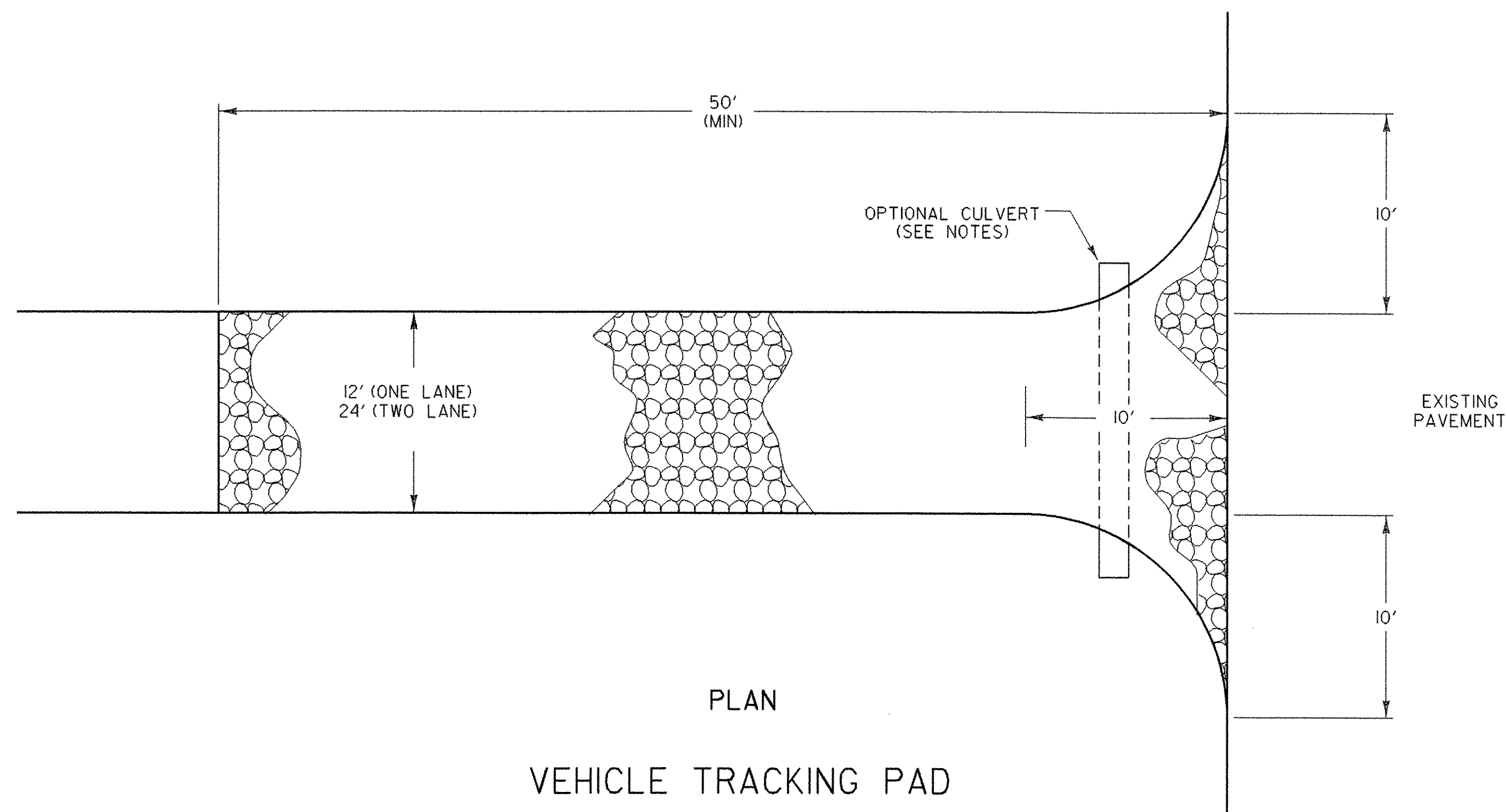
## VEHICLE TRACKING PAD



PROFILE

### VEHICLE TRACKING PAD

NOT TO SCALE



PLAN

### VEHICLE TRACKING PAD

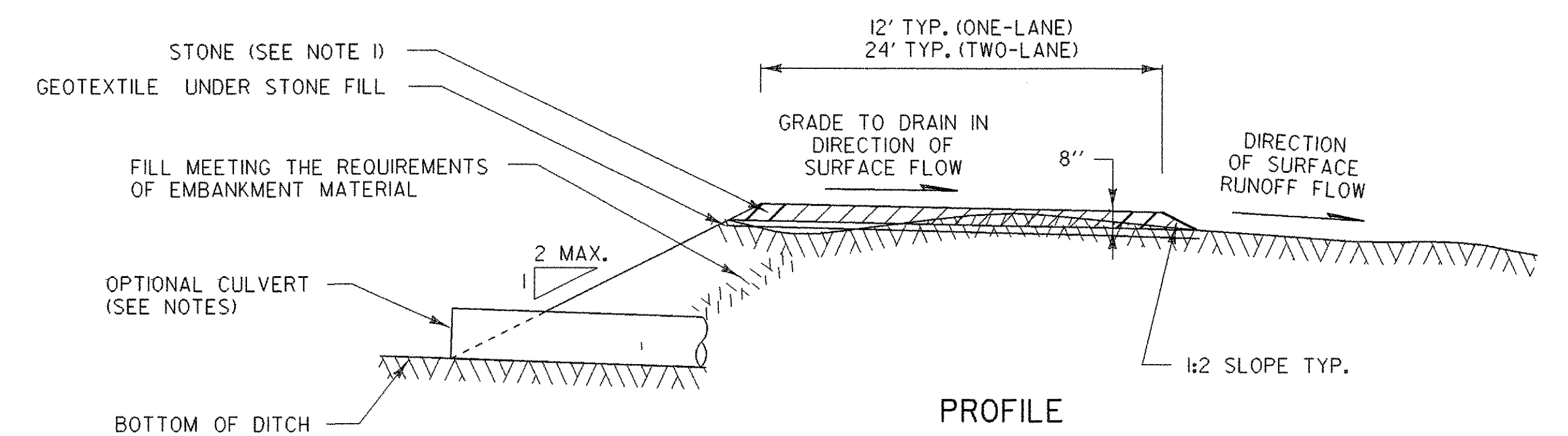
NOT TO SCALE

#### APPLICATION NOTES:

A. THE PURPOSE OF A VEHICLE TRACKING PAD IS TO REDUCE OR ELIMINATE THE TRACKING OF SEDIMENT ONTO PUBLIC RIGHTS OF WAY OR STREETS.

#### GENERAL NOTES:

1. STONE SIZE - USE CLEAN STONE THAT MEETS THE GRADATION REQUIREMENTS SPECIFIED IN THE CONTRACT DOCUMENTS.
2. LENGTH - 50 FEET (MIN.)
3. THICKNESS - 8 INCHES (MIN.)
4. WIDTH - 12 FEET (MIN.)
5. GEOTEXTILE UNDER STONE SHALL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING OF STONE.
6. SURFACE WATER - ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE AS DIRECTED BY THE ENGINEER. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 5:1 SLOPES WILL BE PERMITTED.
7. PROPOSED DRAINAGE PIPES SHALL BE SIZED WITH SUFFICIENT CAPACITY TO CARRY DITCH FLOWS. ALTERNATIVE WAYS OF TRANSPORTING DITCH DRAINAGE ACROSS CONSTRUCTION ENTRANCES MAY BE PROPOSED BY THE CONTRACTOR FOR APPROVAL BY THE ENGINEER.
8. WHEN A VEHICLE TRACKING PAD ALONE IS NOT CAPABLE OF PREVENTING TRACKING OF SEDIMENT ONTO THE ROAD SURFACE THE CONTRACTOR SHALL TAKE ADDITIONAL STEPS BEFORE VEHICLES LEAVE THE CONSTRUCTION AREA.
9. VEHICLE TRACKING PAD SHALL BE INSPECTED EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF A STORM EVENT GREAT ENOUGH TO CAUSE WATER TO LEAVE THE CONSTRUCTION SITE.
10. VEHICLE TRACKING PAD SHALL BE MAINTAINED WHEN THE AGGREGATE BECOMES CLOGGED AND NO LONGER PREVENTS TRACKING OF SEDIMENT ONTO THE PUBLIC RIGHT-OF-WAY. ADDITIONAL AGGREGATE MAY BE ADDED ON TOP OF EXISTING AGGREGATE ONLY TO A POINT WHICH ALLOWS A SMOOTH TRANSITION BETWEEN THE ROAD SURFACE AND CONSTRUCTION AREA.
11. AT THE TIME OF REMOVAL OF THE VEHICLE TRACKING PAD, THE DISTURBED AREA SHALL BE REPAIRED AND STABILIZED.



PROFILE

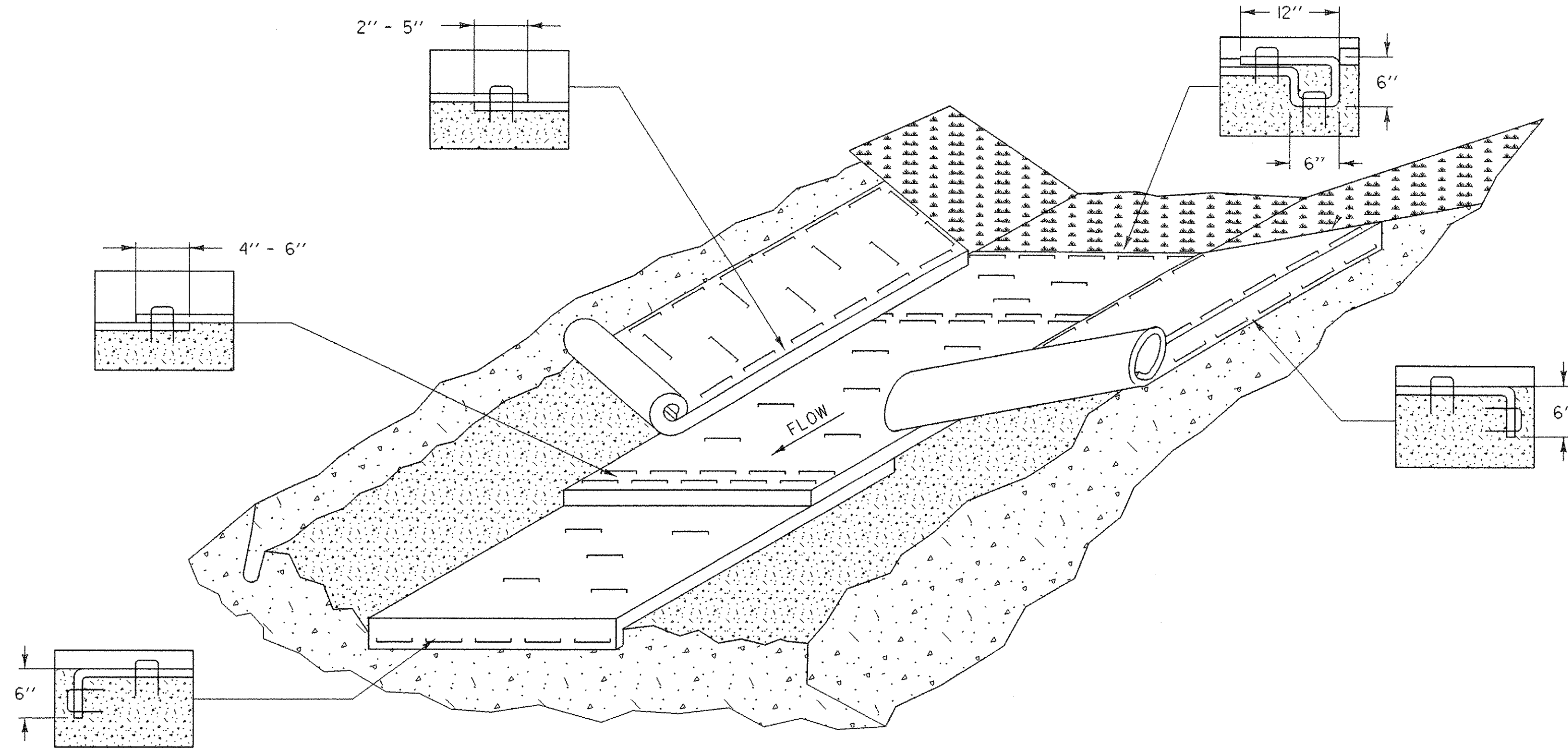
### VEHICLE TRACKING PAD

NOT TO SCALE

## EROSION PREVENTION & SEDIMENT CONTROL DETAILS CONSTRUCTION ENTRANCE

### EROSION & SEDIMENT CONTROL DETAILS 3

PROJECT NAME: THETFORD	
PROJECT NUMBER: BHO 1444 (43)	
FILE NAME: Structures\s03j036ero.dgn	PLOT DATE: 27-SEP-2006
PROJECT LEADER: M.EVANS-MONGEON	DRAWN BY: G.ROKES
DESIGNED BY: M. EVANS-MONGEON	CHECKED BY: S. SCRIBNER
IPARM: s03j036ecd2.j	SHEET 26 OF 60



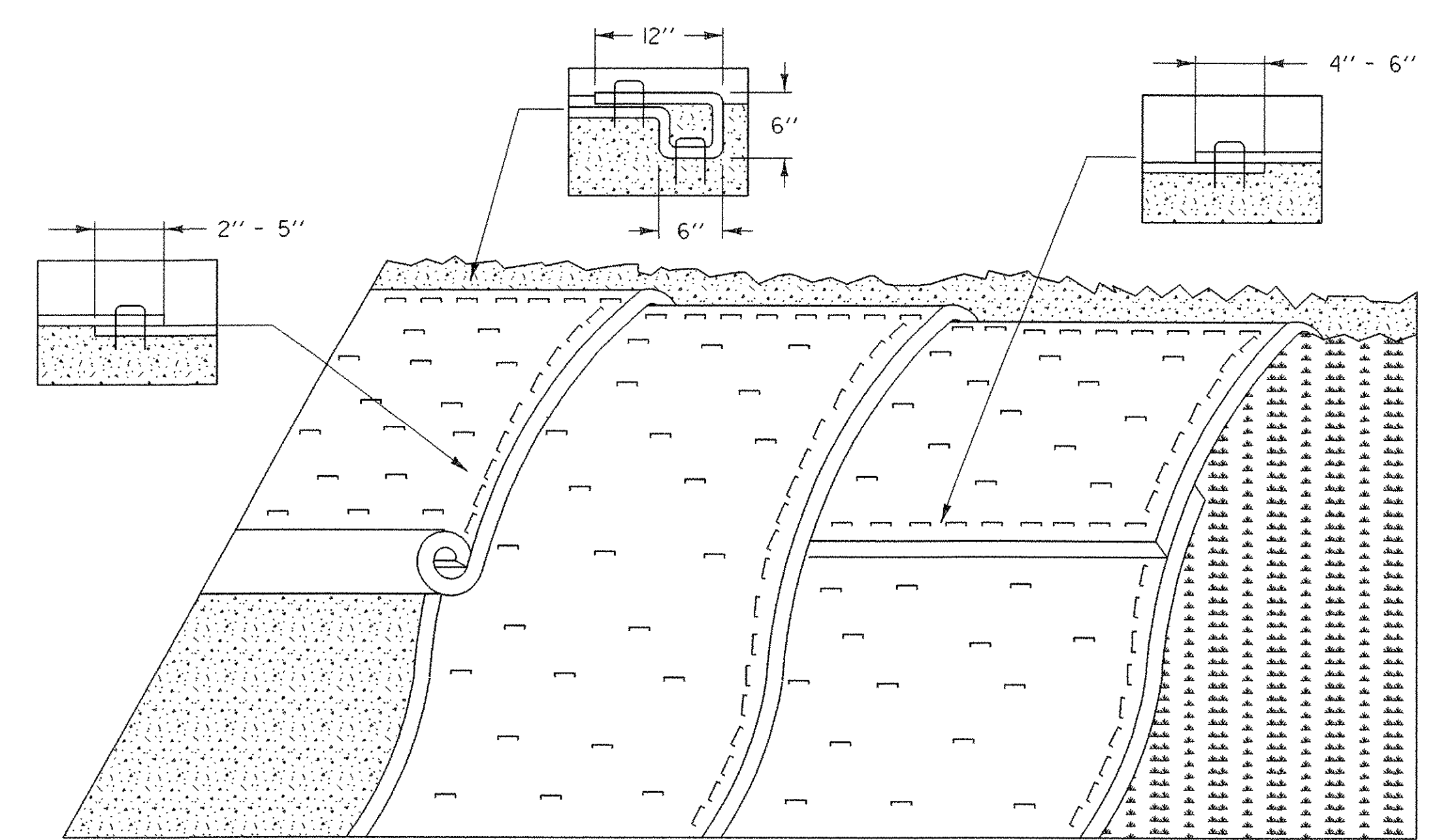
### EROSION MATTING FOR DITCHES

#### APPLICATION NOTES:

- A. THE PURPOSE OF LINING THE DITCH WITH EROSION MATTING IS TO REDUCE EROSION AND AID THE ESTABLISHMENT OF VEGETATION AT LOW VELOCITIES.
- B. TYPE OF EROSION MATTING TO BE USED SHOULD BE BASED ON FACTORS SPECIFIC TO EACH APPLICATION. SEE SPECIFICATIONS AND PRODUCT RECOMMENDATIONS FOR SUITABILITY.

#### GENERAL NOTES:

1. WATER MAY NEED TO BE DIVERTED TO ALLOW PROPER MATTING INSTALLATION.
2. GRADE AND SMOOTH CHANNEL TO PROVIDE GOOD MATTING TO SOIL SURFACE CONTACT.
3. APPLY FERTILIZER, LIME, AND SEED PRIOR TO PLACING MATTING.
4. INSTALL MATTING IN THE CENTER OF THE CHANNEL, IN THE DIRECTION OF THE WATER FLOW.
5. INSTALL MATTING ON THE SIDE SLOPES OF THE CHANNEL, OVERLAPPING THE CENTER MAT.
6. ANCHOR MATTING AS SHOWN, UTILIZING ANCHOR STAPLES. STAPLE PLACEMENT SHALL BE DETERMINED BY THE MANUFACTURER'S INSTALLATION INSTRUCTIONS.
7. EROSION MATTING SHALL BE INSPECTED EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF A STORM EVENT GREAT ENOUGH TO CAUSE WATER TO LEAVE THE CONSTRUCTION SITE.
8. EROSION MATTING SHALL BE REPAIRED AND RESTAPLED AS NECESSARY TO ENSURE PROPER FUNCTION.



### EROSION MATTING FOR SLOPES

#### APPLICATION NOTES:

- A. THE PURPOSE OF EROSION MATTING ON SLOPES IS TO REDUCE EROSION AND AID THE ESTABLISHMENT OF VEGETATION
- B. EROSION CONTROL MATTING SHALL BE USED FOR THE FOLLOWING REASONS:
  - SLOPES > 3H:1V
  - AREAS WHERE SEED AND MULCH WILL NOT STAY IN PLACE ALONE
  - WHERE SEEDING IS OUTSIDE THE GROWING SEASON.

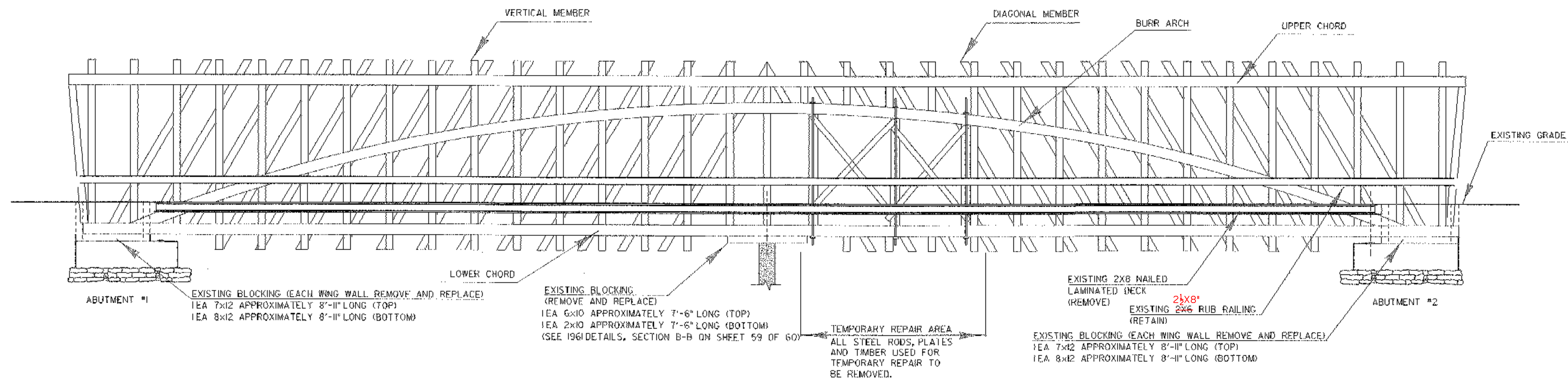
#### GENERAL NOTES:

1. GRADE AND SMOOTH THE SLOPE TO PROVIDE GOOD MATTING TO SOIL SURFACE CONTACT.
2. APPLY FERTILIZER, LIME, AND SEED PRIOR TO PLACING MATTING.
3. ANCHOR MATTING AS SHOWN, UTILIZING ANCHOR STAPLES. STAPLE PLACEMENT SHALL BE DETERMINED BY THE MANUFACTURER'S INSTALLATION INSTRUCTIONS.
4. UNROLL EROSION MATTING VERTICALLY DOWN SLOPE IN THE DIRECTION OF WATER FLOW.
5. OVERLAP UPPER MATTING OVER LOWER MATTING AS SHOWN.
6. OVERLAP ADJACENT MATTING AS SHOWN.
7. CUT EXCESS MATTING AT END OF SLOPE AND ANCHOR THE END.
8. EROSION MATTING SHALL BE INSPECTED EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF A STORM EVENT GREAT ENOUGH TO CAUSE WATER TO LEAVE THE CONSTRUCTION SITE.
9. EROSION MATTING SHALL BE REPAIRED AND RESTAPLED AS NECESSARY TO ENSURE PROPER FUNCTION.

## EROSION PREVENTION & SEDIMENT CONTROL DETAILS DITCH & SLOPE PROTECTION

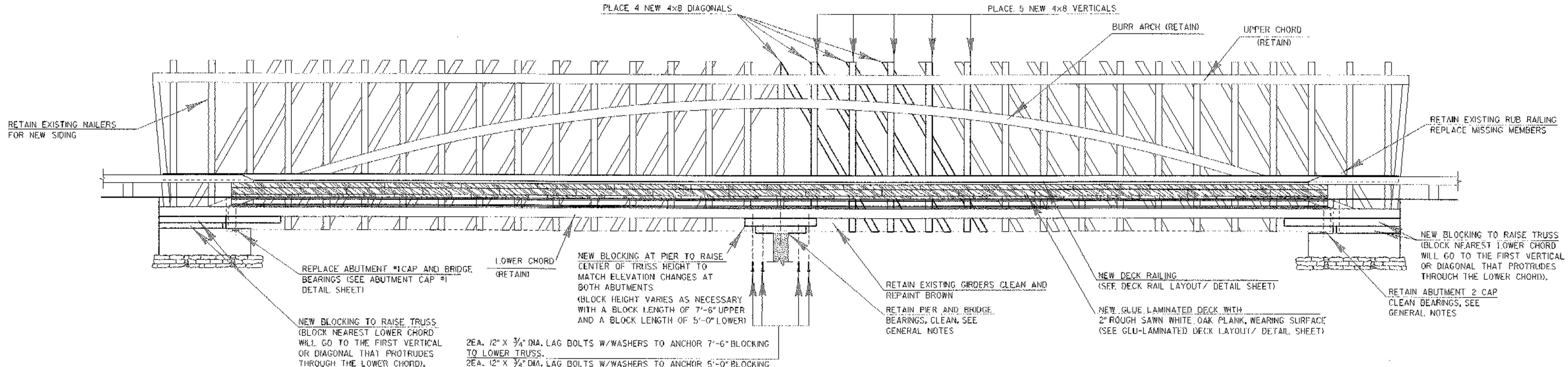
### EROSION & SEDIMENT CONTROL DETAILS 4

PROJECT NAME: THETFORD	
PROJECT NUMBER: BHO 1444 (43)	
FILE NAME: Structures\s03j036ero.dgn	PLOT DATE: 27-SEP-2006
PROJECT LEADER: M.EVANS-MONGEON	DRAWN BY: G.ROKES
DESIGNED BY: M. EVANS-MONGEON	CHECKED BY: S. SCRIBNER
IPARM: s03j036ecd3.i	SHEET 27 OF 60



**EXISTING BRIDGE TRUSSING**

NTS



**PROPOSED REPAIRS AND MODIFICATIONS**

NTS

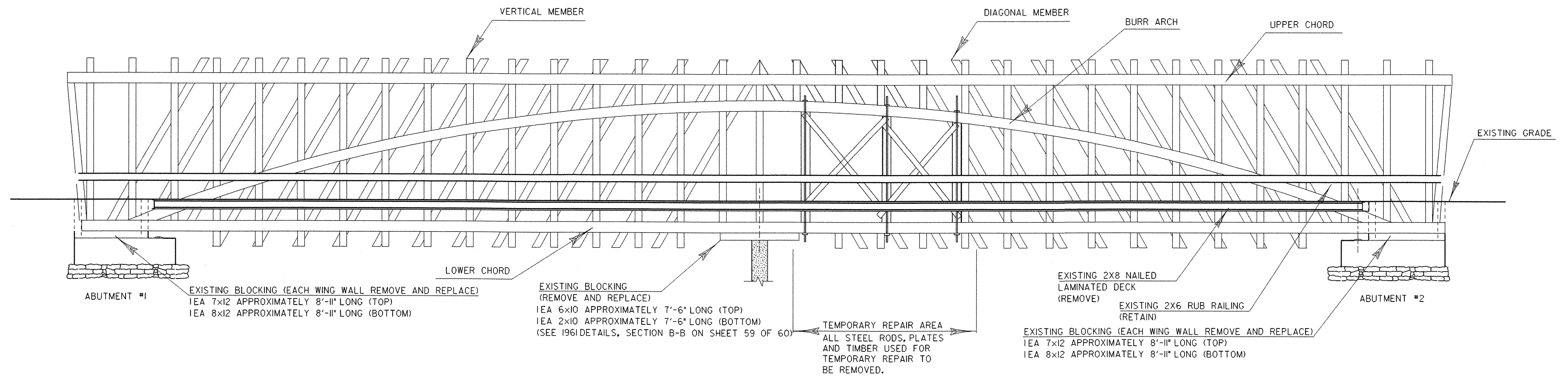
TRUSSING TIMBERS  
 5 EA 4x8 AVERAGE LENGTH 18'-0" (VERTICAL MEMBERS)  
 4 EA 4x8 AVERAGE LENGTH 21'-0" (DIAGONAL MEMBERS)

NEW BLOCKING AT PIER TO RAISE CENTER OF TRUSS HEIGHT TO MATCH ELEVATION CHANGES AT BOTH ABUTMENTS (BLOCK HEIGHT VARIES AS NECESSARY WITH A BLOCK LENGTH OF 7'-6" UPPER AND A BLOCK LENGTH OF 5'-0" LOWER)  
 2EA. 12" X 3/4" DIA. LAG BOLTS W/WASHERS TO ANCHOR 7'-6" BLOCKING TO LOWER TRUSS.  
 2EA. 12" X 3/4" DIA. LAG BOLTS W/WASHERS TO ANCHOR 5'-0" BLOCKING TO THE 7'-6" BLOCKING.  
 BOLT HOLES ARE TO BE LOCATED 1'-0" MINIMUM FROM EACH END OF THE BLOCKS. RECESS THE BOLT HOLES IN EACH BLOCK TO ACHIEVE A 4" MINIMUM LAG BOLT THREAD LENGTH INTO THE LOWER CHORD AND INTO THE 7'-6" BLOCK. (SEE "LOWER CHORD ATTACHMENT NEAR PIER BLOCKING DETAIL" SHEET 34 OF 60)

**NORTH TRUSS REPLACEMENT OF EXISTING / PROPOSED**

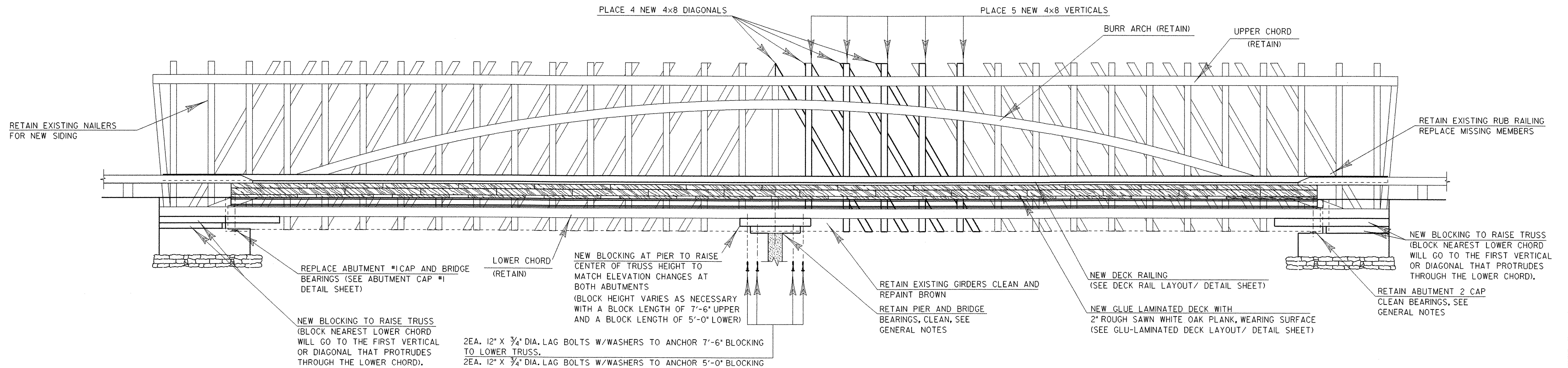
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PROJECT NUMBER:	BHO 1444 (43)	DRAWN BY:	G.ROKES
FILE NAME:	Structures\s03j036abr.dgn	CHECKED BY:	S. SCRIBNER
PROJECT LEADER:	M.EVANS-MONGEON	SHEET	28 OF 60
DESIGNED BY:	G.ROKES		
IPARM:	s03j036br.d		

REVISED OCT. 24, 2006



**EXISTING BRIDGE TRUSSING**

NTS



**PROPOSED REPAIRS AND MODIFICATIONS**

NTS

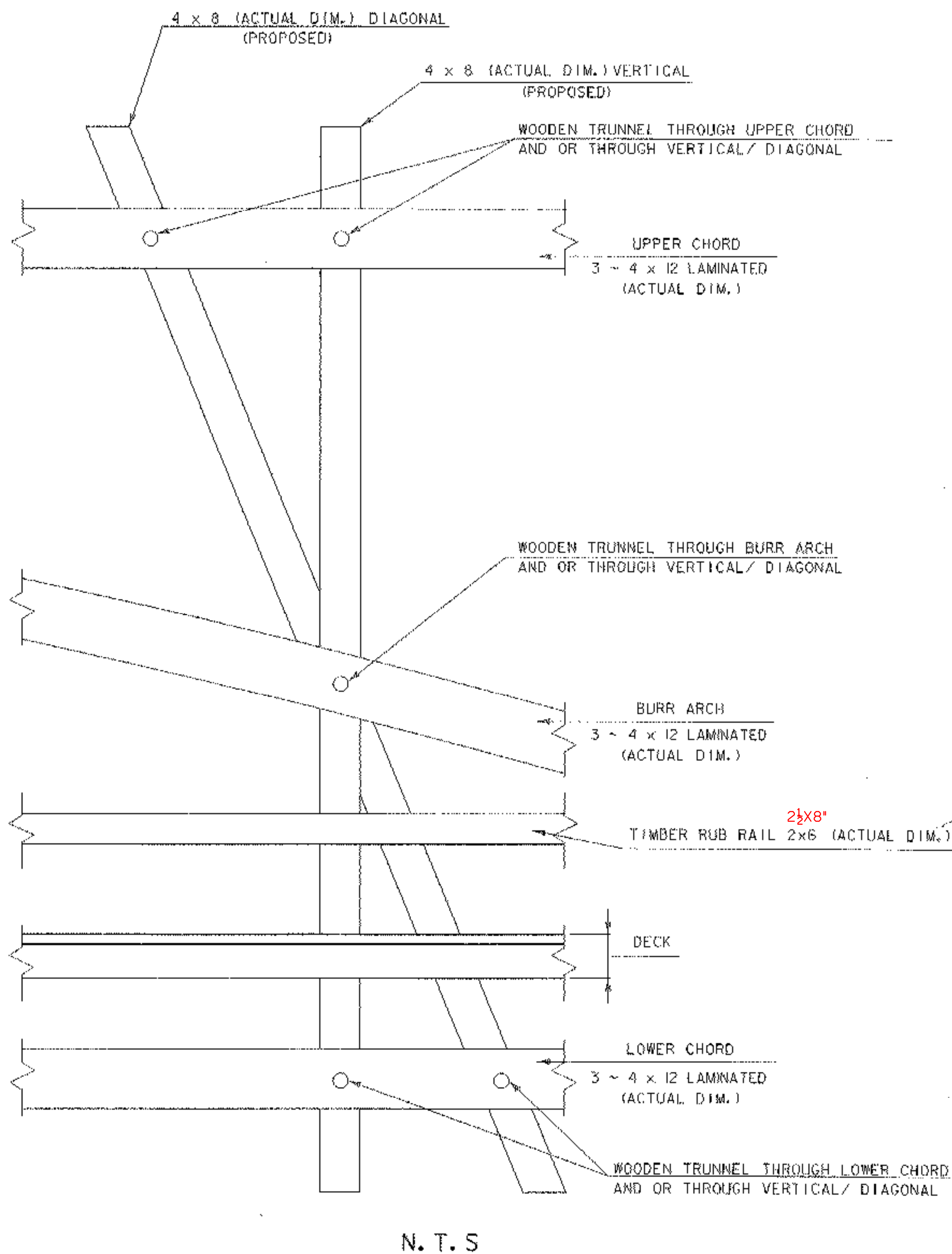
TRUSSING TIMBERS  
 5 EA 4x8 AVERAGE LENGTH 18'-0 (VERTICAL MEMBERS)  
 4 EA 4x8 AVERAGE LENGTH 21'-0 (DIAGONAL MEMBERS)

2EA. 12" X 3/4" DIA. LAG BOLTS W/WASHERS TO ANCHOR 7'-6" BLOCKING TO LOWER TRUSS.  
 2EA. 12" X 3/4" DIA. LAG BOLTS W/WASHERS TO ANCHOR 5'-0" BLOCKING TO THE 7'-6" BLOCKING.  
 BOLTS HOLES ARE TO BE LOCATED 1'-0" MINIMUM FROM EACH END OF THE BLOCKS. RECESS THE BOLT HOLES IN EACH BLOCK TO ACHIEVE A 4" MINIMUM LAG BOLT THREAD LENGTH INTO THE LOWER CHORD AND INTO THE 7'-6" BLOCK. (SEE "LOWER CHORD ATTACHMENT NEAR PIER BLOCKING DETAIL" SHEET 34 OF 60)

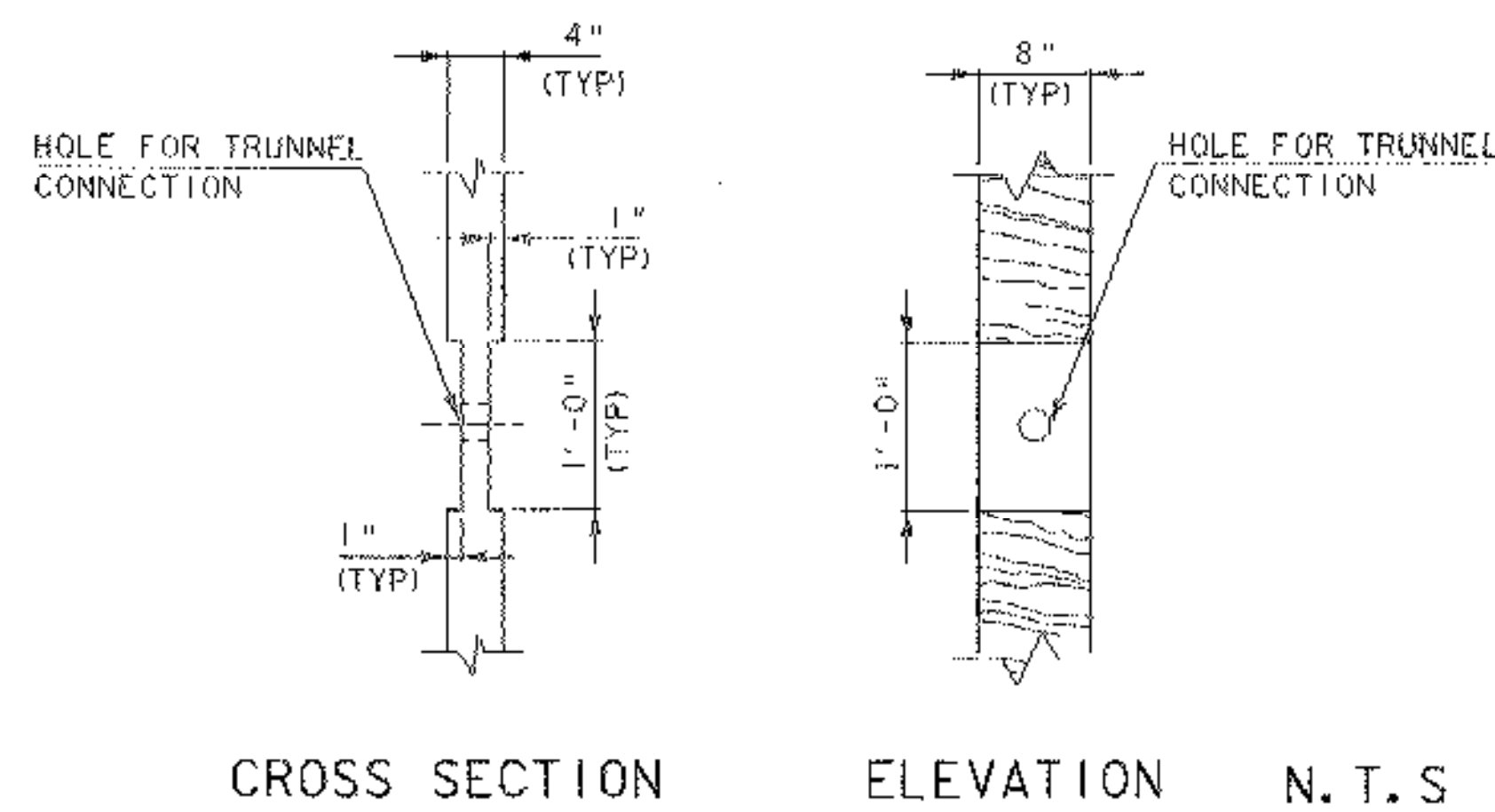
**NORTH TRUSS REPLACEMENT OF EXISTING / PROPOSED**

PROJECT NAME:	THETFORD	FILE NAME:	Structures\s03j036abt.dgn	PLOT DATE:	24-OCT-2006
PROJECT NUMBER:	BHO 1444 (43)	PROJECT LEADER:	M.EVANS-MONGEON	DRAWN BY:	G.ROKES
		DESIGNED BY:	G.ROKES	CHECKED BY:	S. SCRIBNER
		IPARM:	s03j036brd.i	SHEET	28 OF 60

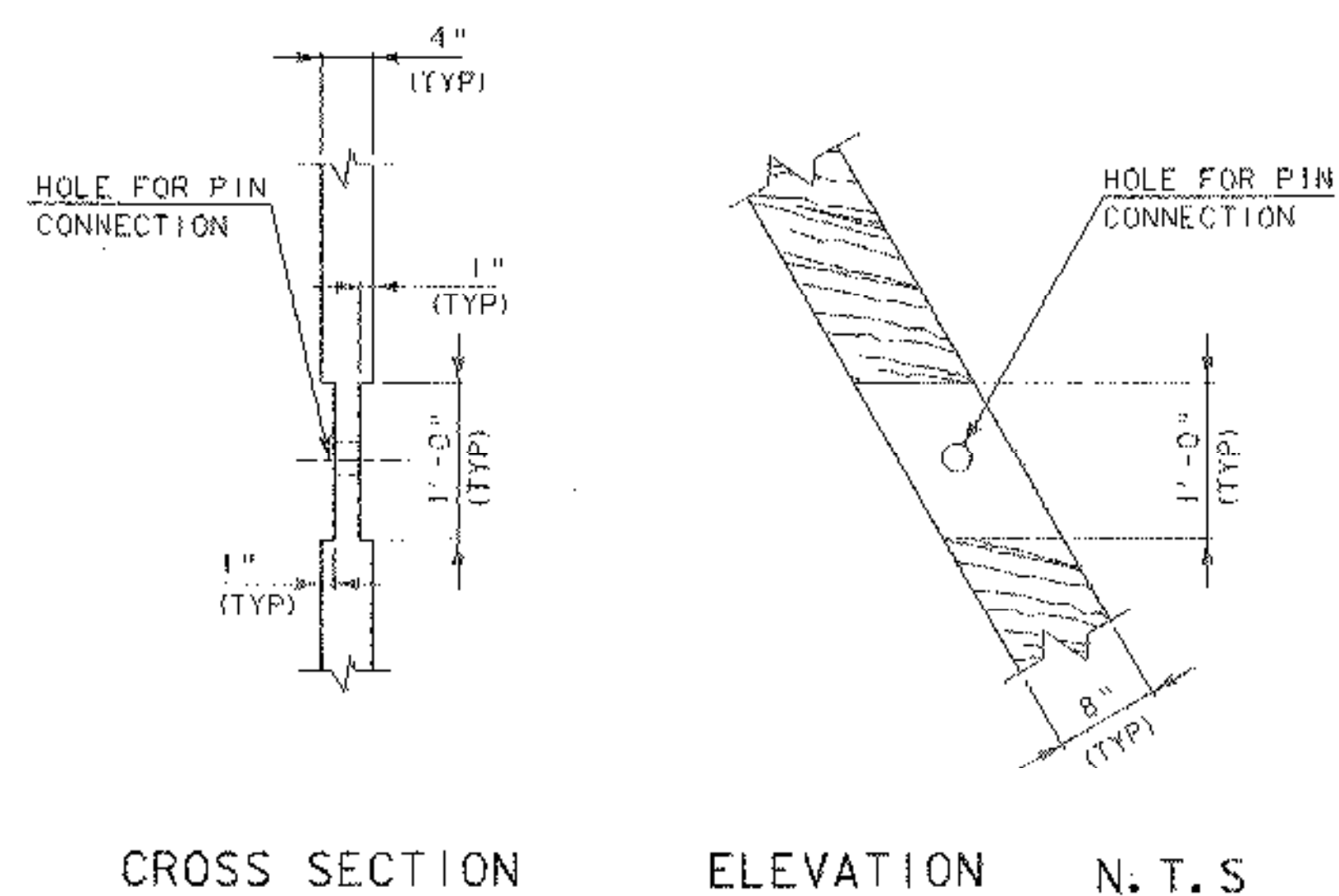
**REVISED OCT. 24, 2006**



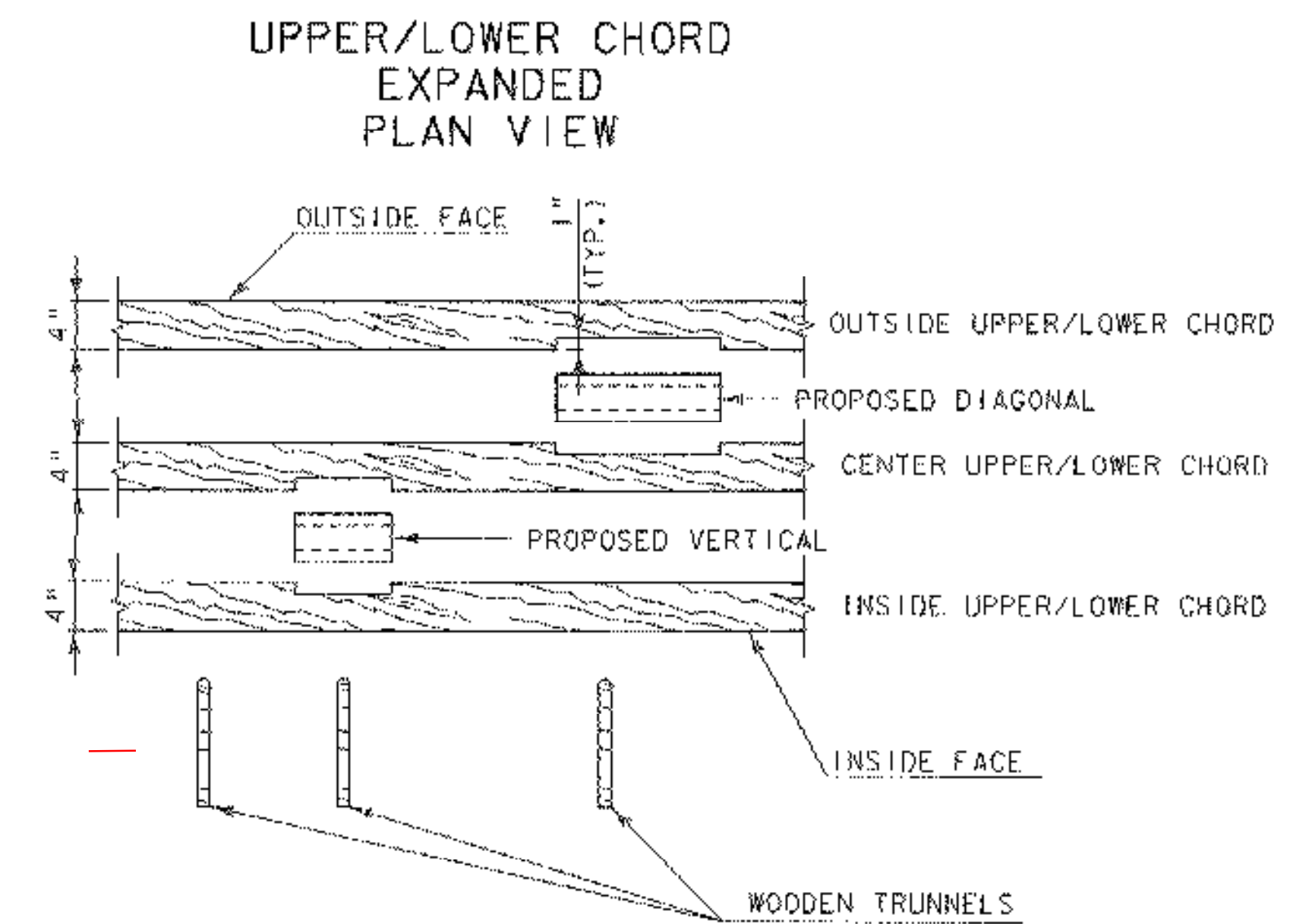
**TYPICAL BRIDGE TRUSS CONFIGURATION ELEVATION VIEW**



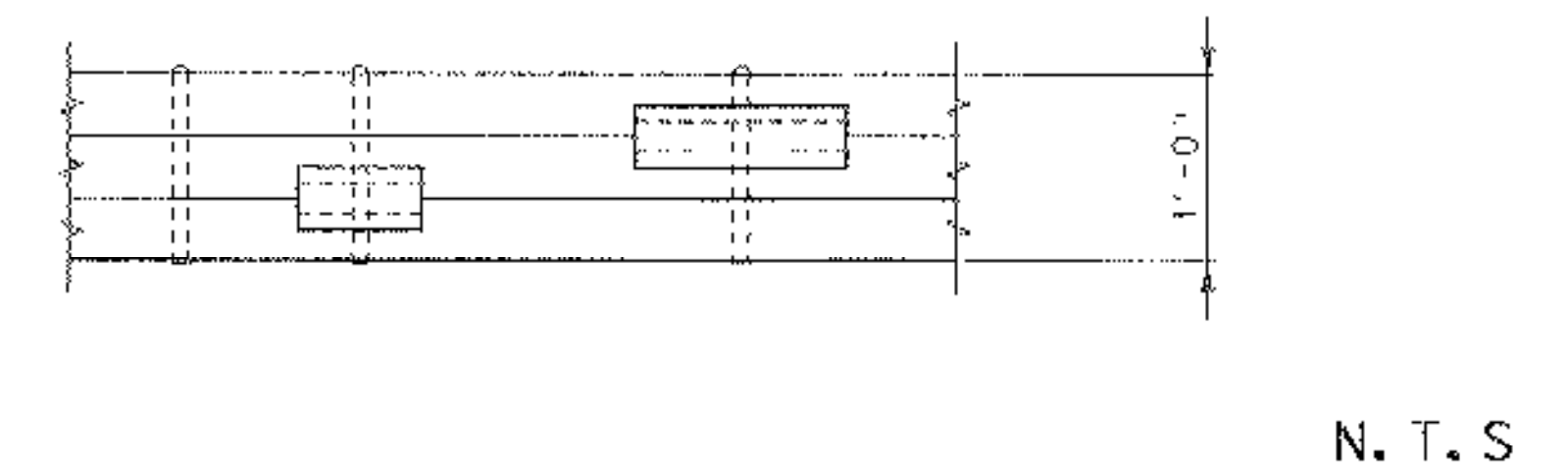
**TYPICAL NOTCHING FOR PROPOSED VERTICAL MEMBERS**



**TYPICAL NOTCHING FOR PROPOSED DIAGONAL MEMBERS**



UPPER/LOWER CHORD ASSEMBLED AND PINNED PLAN VIEW



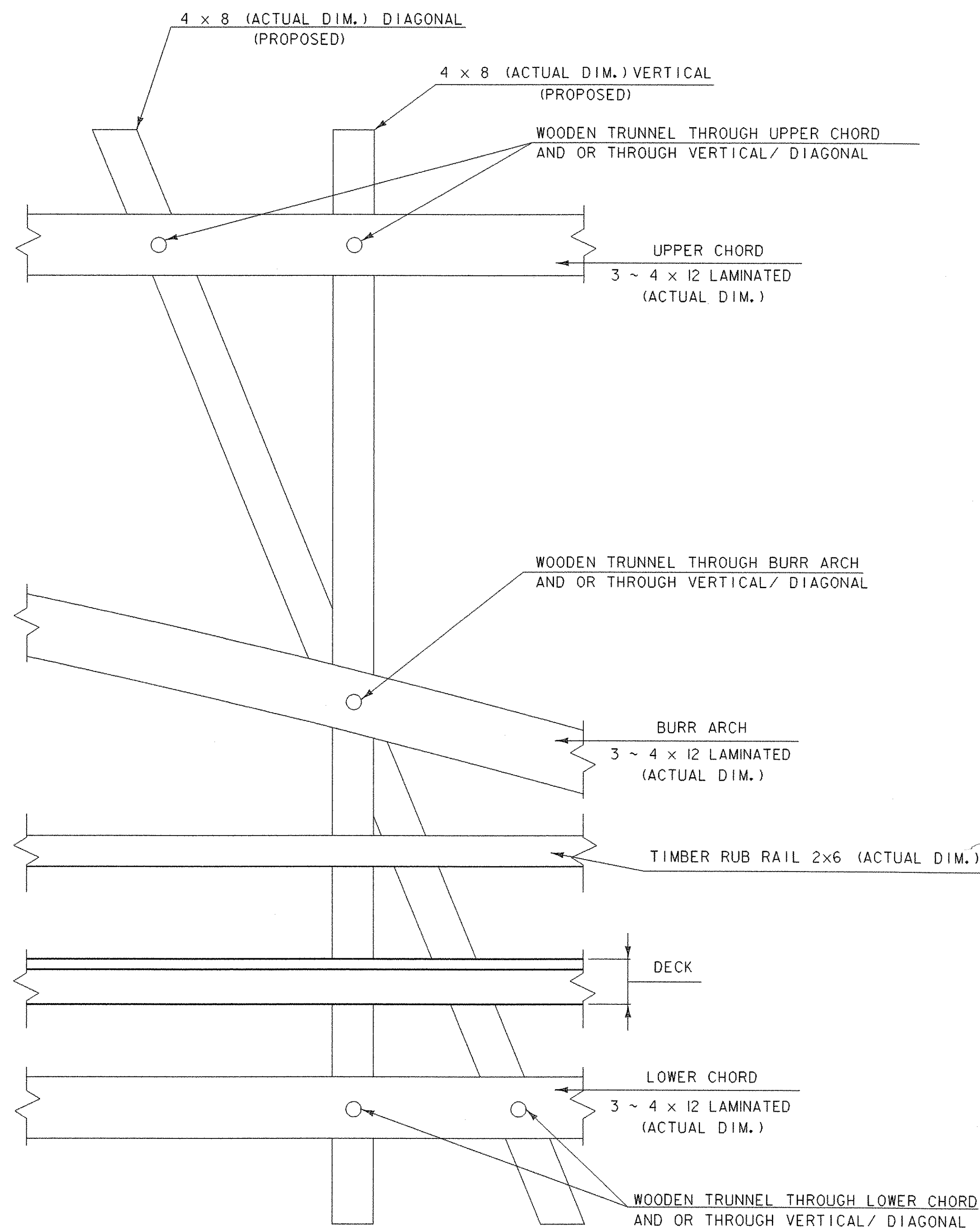
**UPPER/LOWER CHORD ASSEMBLY**

NOTE:

THE BURR ARCH ASSEMBLY IS SIMILAR TO THE UPPER AND LOWER CHORD. THE ASSEMBLY THROUGH THE ARCH VARIES IN MEASUREMENTS. TYPICAL NOTCHING FOR VERTICAL AND DIAGONAL THROUGH THE ARCH WAS NOT DETAILED. THESE SECTIONS WILL NEED TO BE FIT IN FIELD.

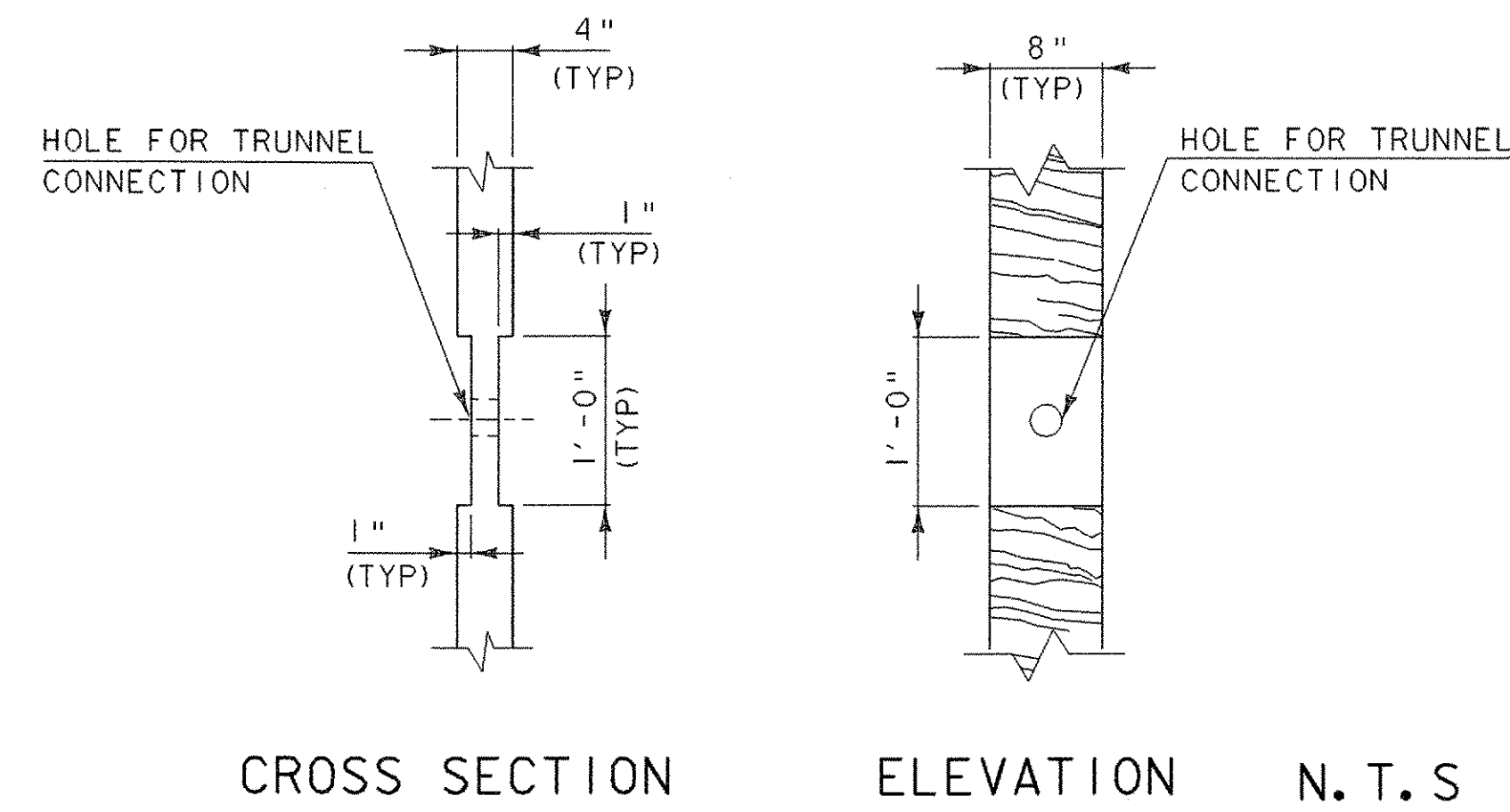
**TIMBER TRUSS ASSEMBLY AND CONFIGURATION SHEET**

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036abt.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MUNGEON	SHEET 29 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036bt.d.f	

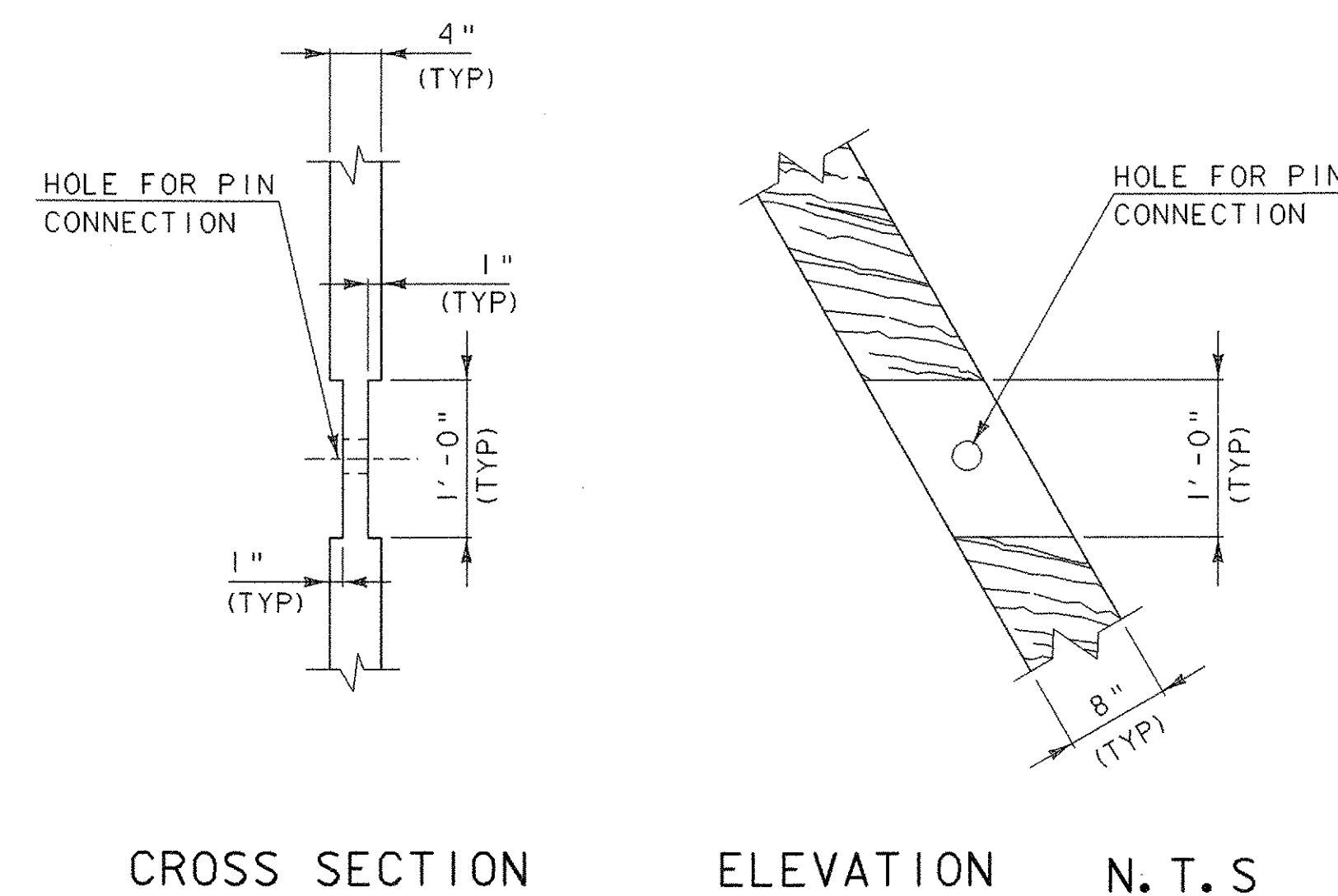


N. T. S

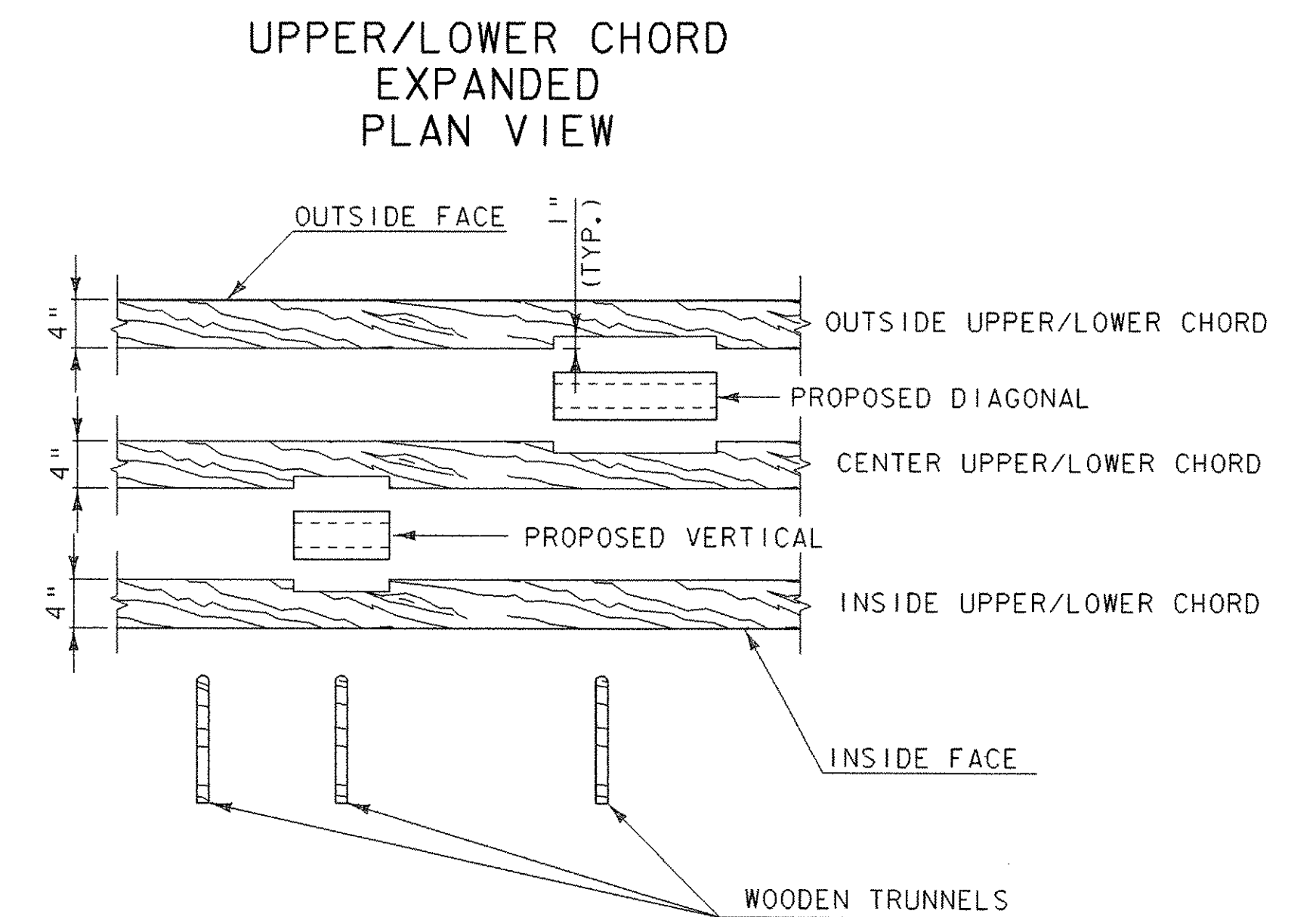
### TYPICAL BRIDGE TRUSS CONFIGURATION ELEVATION VIEW



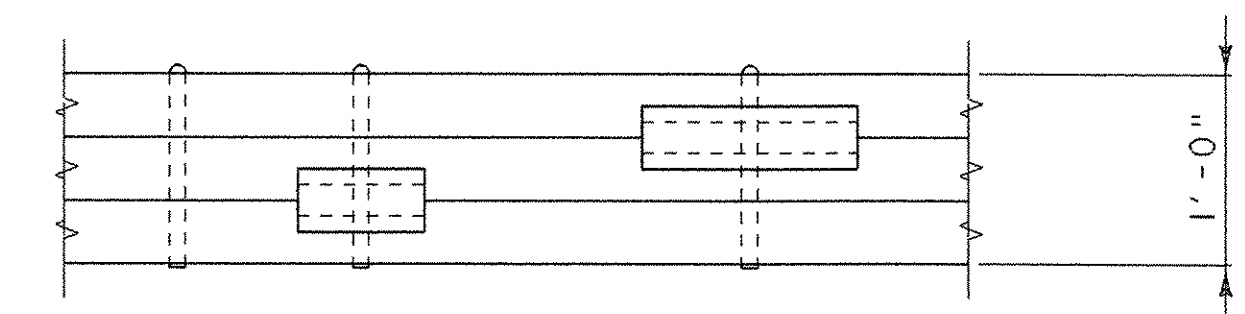
### TYPICAL NOTCHING FOR PROPOSED VERTICAL MEMBERS



### TYPICAL NOTCHING FOR PROPOSED DIAGONAL MEMBERS



### UPPER/LOWER CHORD ASSEMBLED AND PINNED PLAN VIEW



N. T. S

### UPPER/LOWER CHORD ASSEMBLY

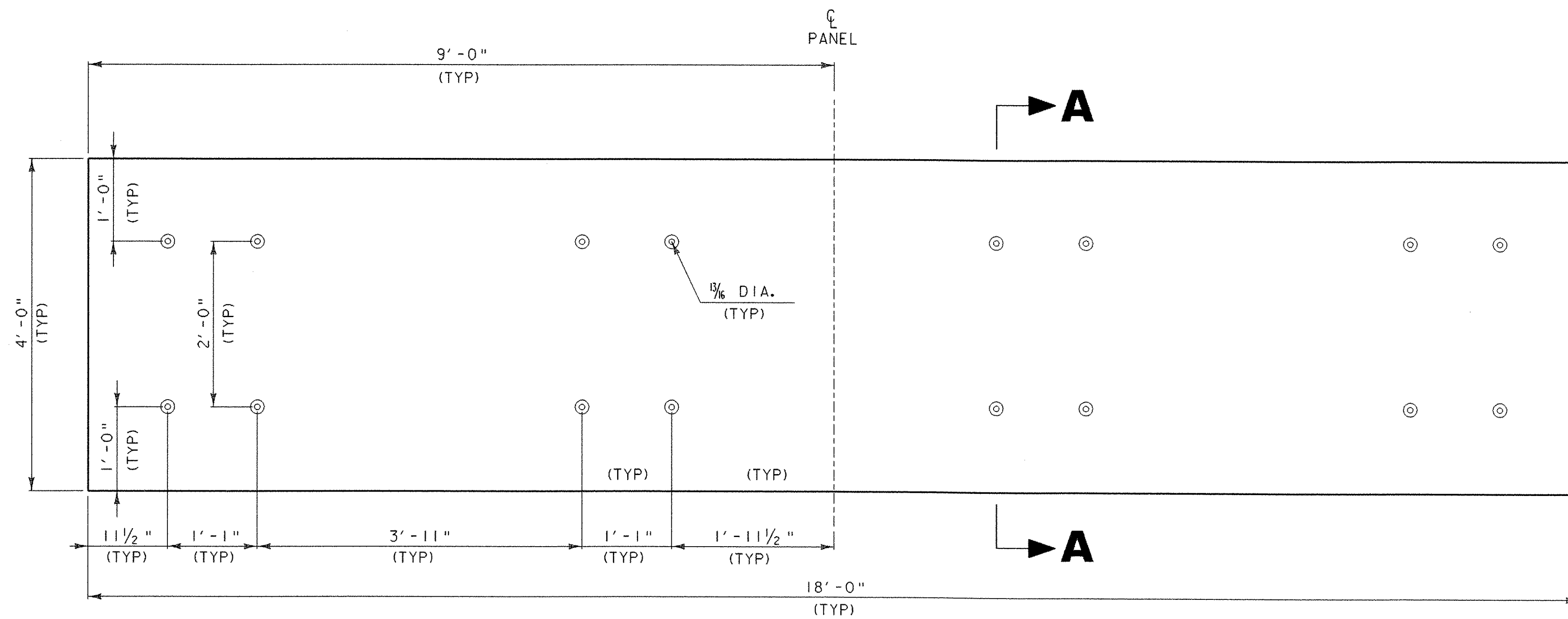
NOTE:

THE BURR ARCH ASSEMBLY IS SIMILAR TO THE UPPER AND LOWER CHORD. THE ASSEMBLY THROUGH THE ARCH VARIES IN MEASUREMENTS. TYPICAL NOTCHING FOR VERTICAL AND DIAGONAL THROUGH THE ARCH WAS NOT DETAILED. THESE SECTIONS WILL NEED TO BE FIT IN FIELD.

### TIMBER TRUSS ASSEMBLY AND CONFIGURATION SHEET

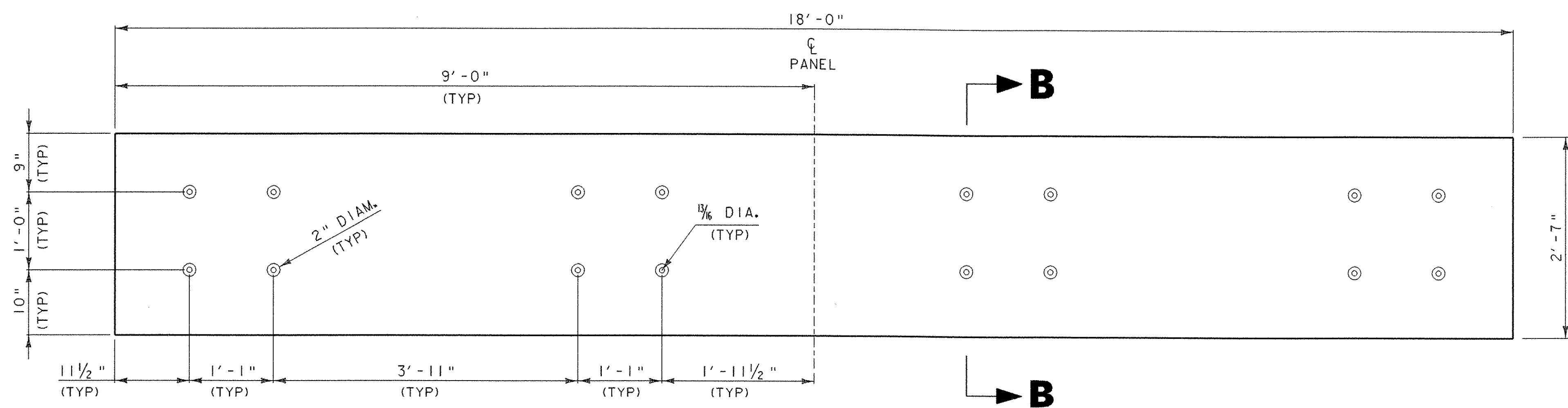
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PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036abt.dgn	CHECKED BY: S. SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 29 OF 60
DESIGNED BY: G. ROKES	
IPARM: s03j036b+d.i	

HOLE LOCATIONS INDICATED BELOW WILL BE VERIFIED AND ADJUSTED AS NECESSARY FOR A PROPER/ SECURE FIT TO EXISTING GIRDERS. ALL HOLE LOCATIONS ARE TO BE DRILLED IN THE FIELD. ONCE ALL DECK PANELS ARE SECURED TO GIRDERS RECESSED HOLES WILL BE FILLED WITH JOINT SEALER COLD Poured (ITEM 524.12) FLUSH TO TOP OF GLUE LAMINATED PANEL



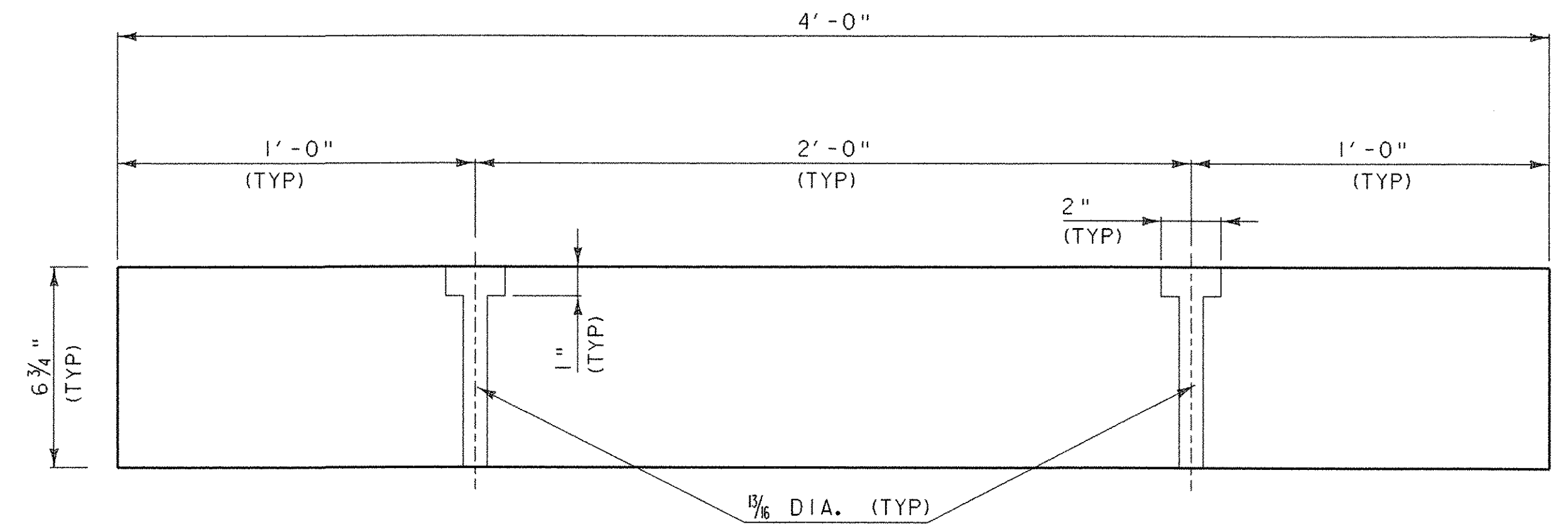
**4' x 18' GLUE LAMINATED DECK DETAIL PLAN VIEW**

SCALE 1" = 1'



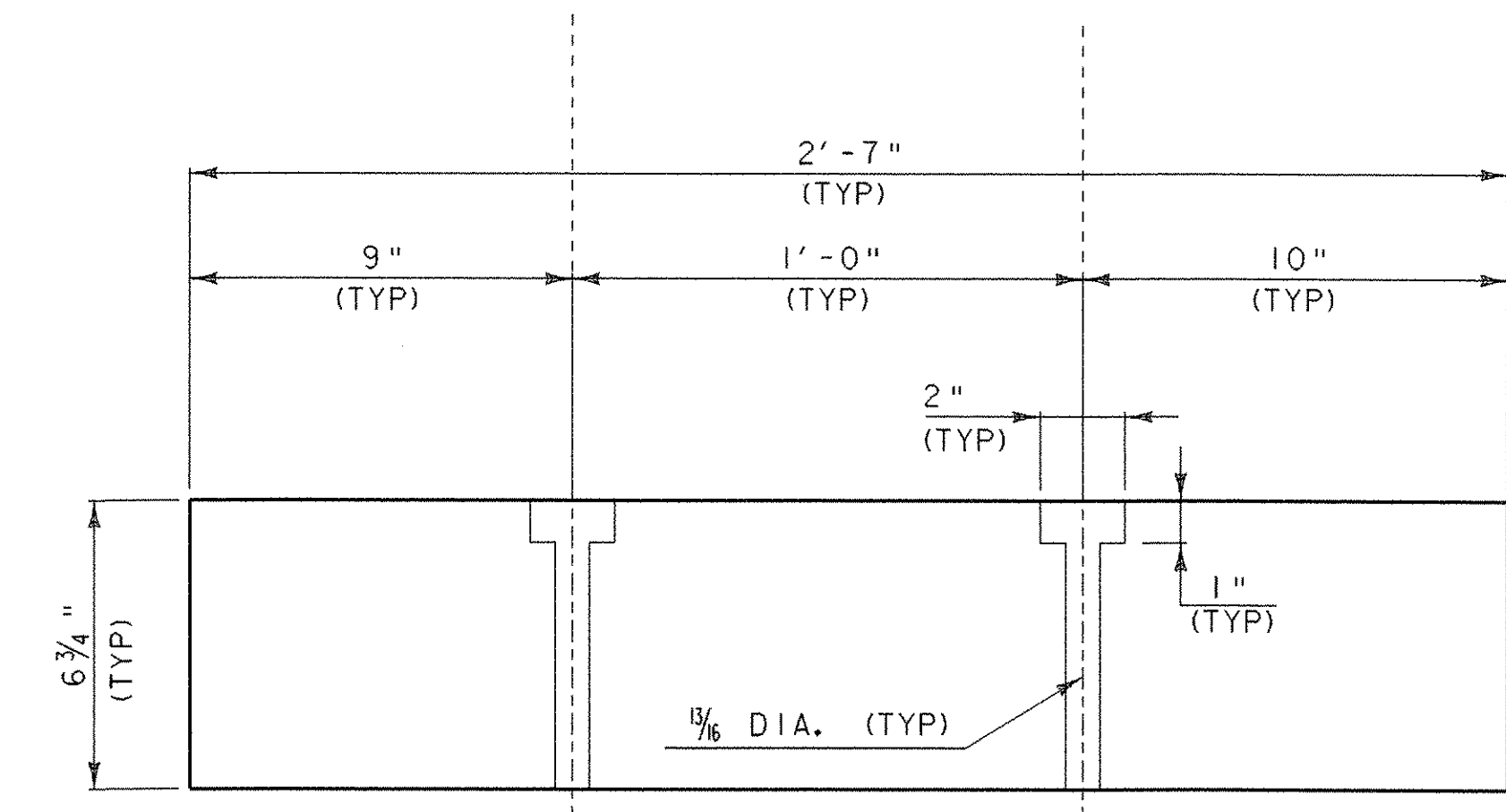
**2'-7" x 18' GLUE LAMINATED DECK DETAIL PLAN VIEW**

SCALE 1" = 1'



**SECTION A-A**

SCALE 1/4" = 1"

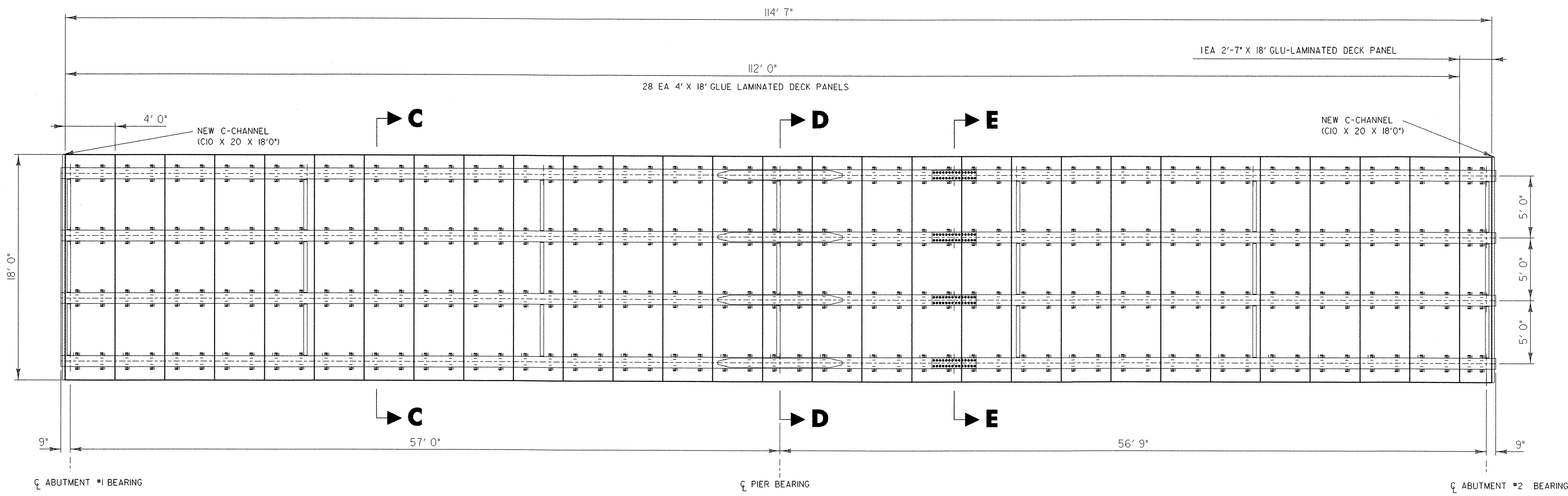


**SECTION B-B**

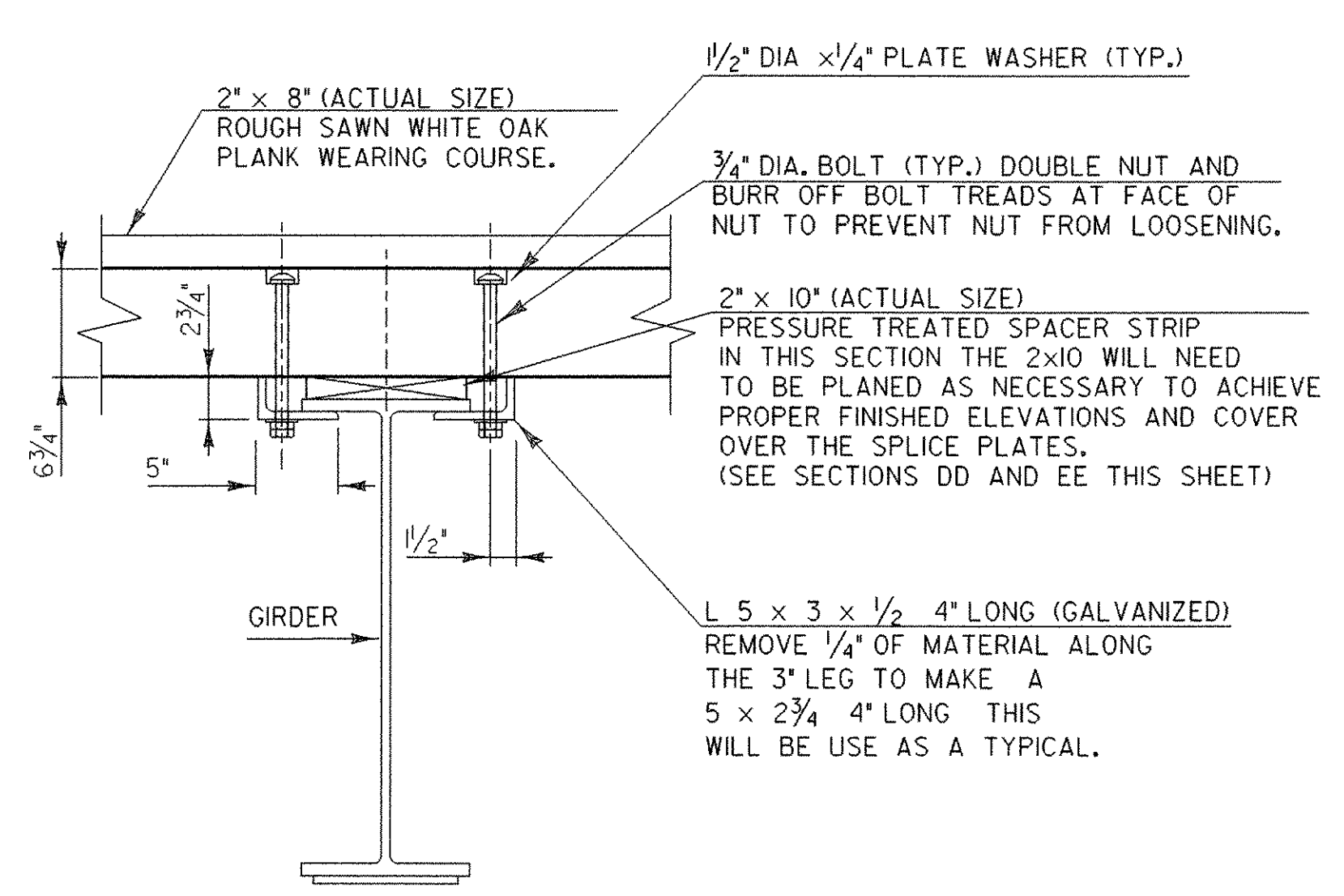
SCALE 1/4" = 1"

**GLUE LAMINATED DECK SECTION DETAILS**

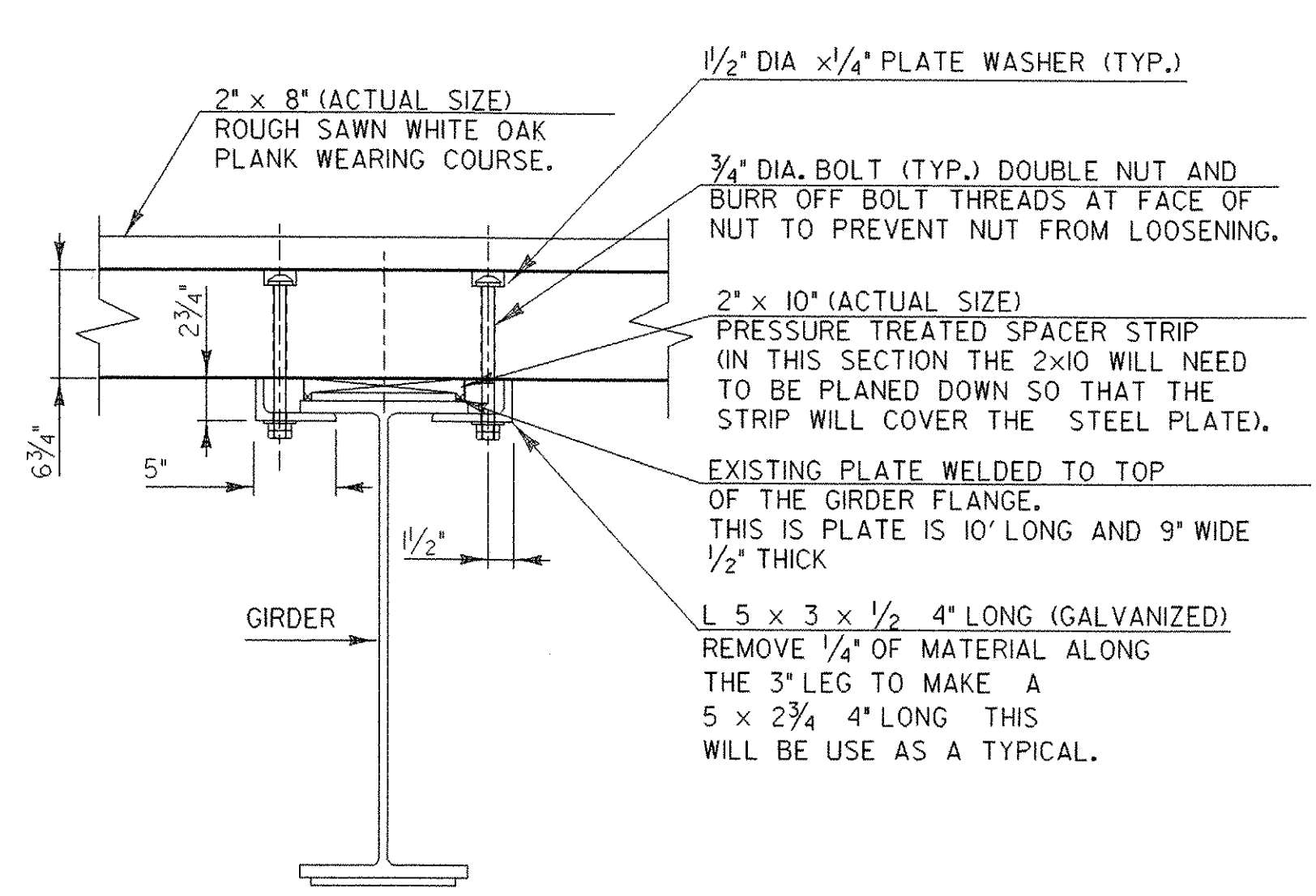
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PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036ab1.dgn	CHECKED BY: S. SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 30 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036bld.l	



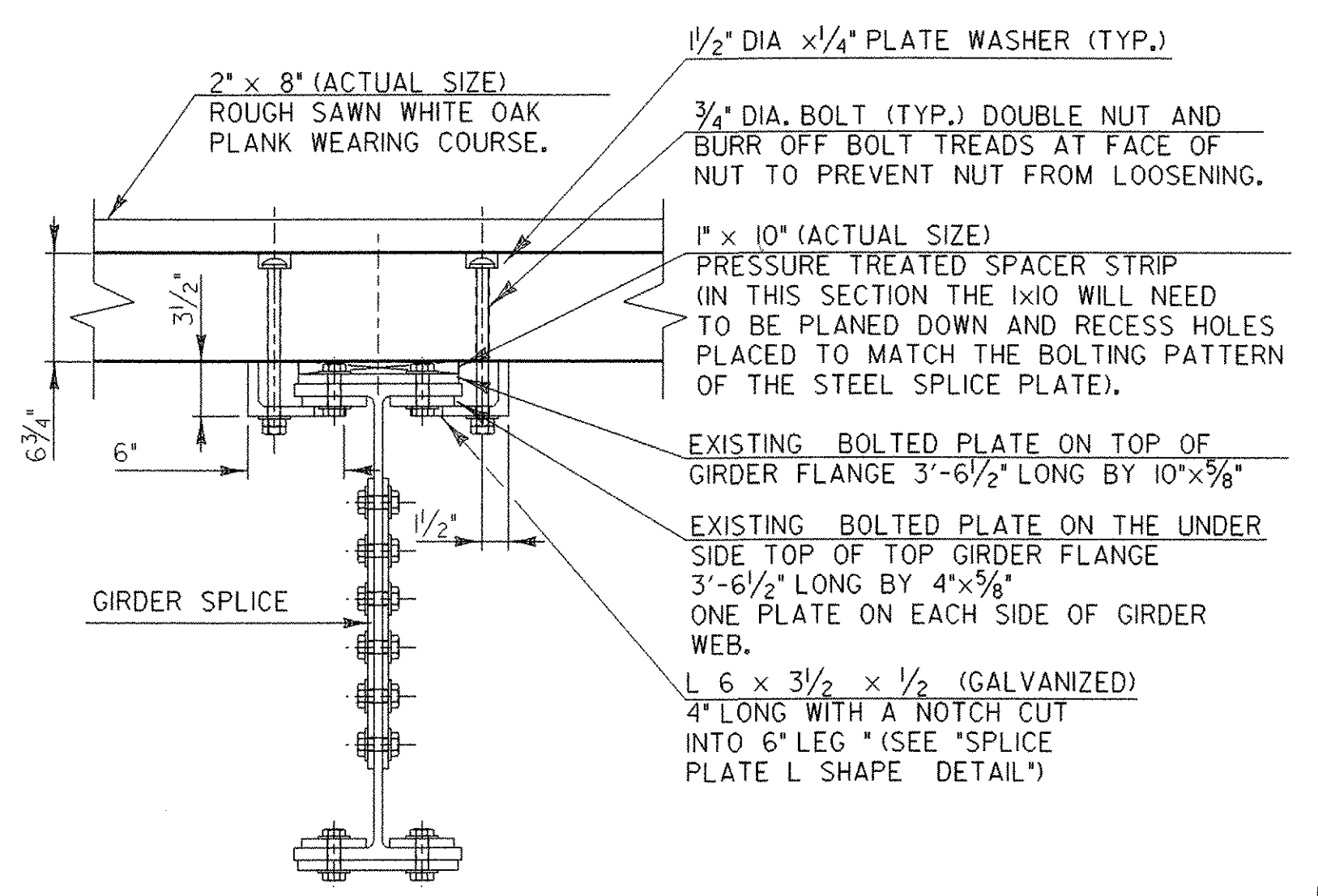
ALL HOLE LOCATIONS ARE TO BE DRILLED IN FIELD SCALE 1/4"=1'



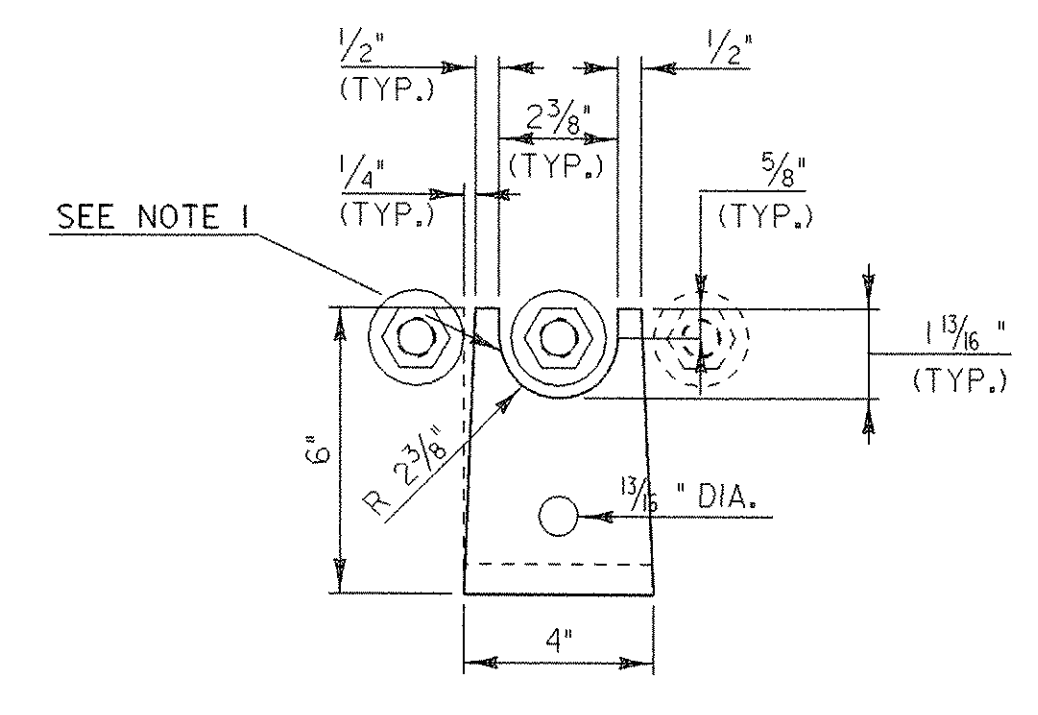
**SECTION C-C**  
NTS



**SECTION D-D**  
NTS



**SECTION E-E**  
NTS



NOTE 1: THE BOLT NOTCH LOCATIONS ARE ESTIMATED. THE ACTUAL BOLT NOTCHES WILL NEED TO BE FIT IN FIELD.

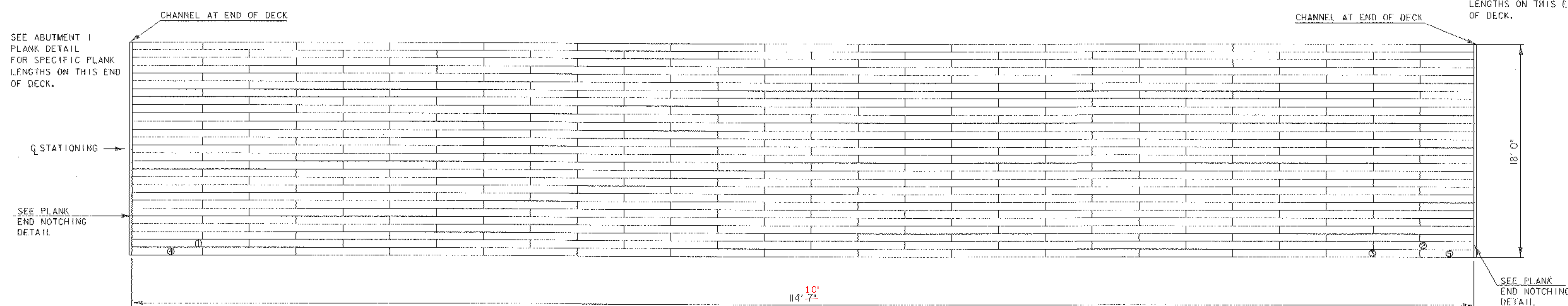
**SPlice PLATE L SHAPE DETAIL** NTS

**GLUE LAMINATED DECK LAYOUT SHEET**

PROJECT NAME: THETFORD	PLOT DATE: 24-OCT-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036ab+.dgn	CHECKED BY: S. SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 31 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036bd.l	

**REVISED OCT. 24, 2006**

SEE ABUTMENT 2  
PLANK DETAIL  
FOR SPECIFIC PLANK  
LENGTHS ON THIS END  
OF DECK.

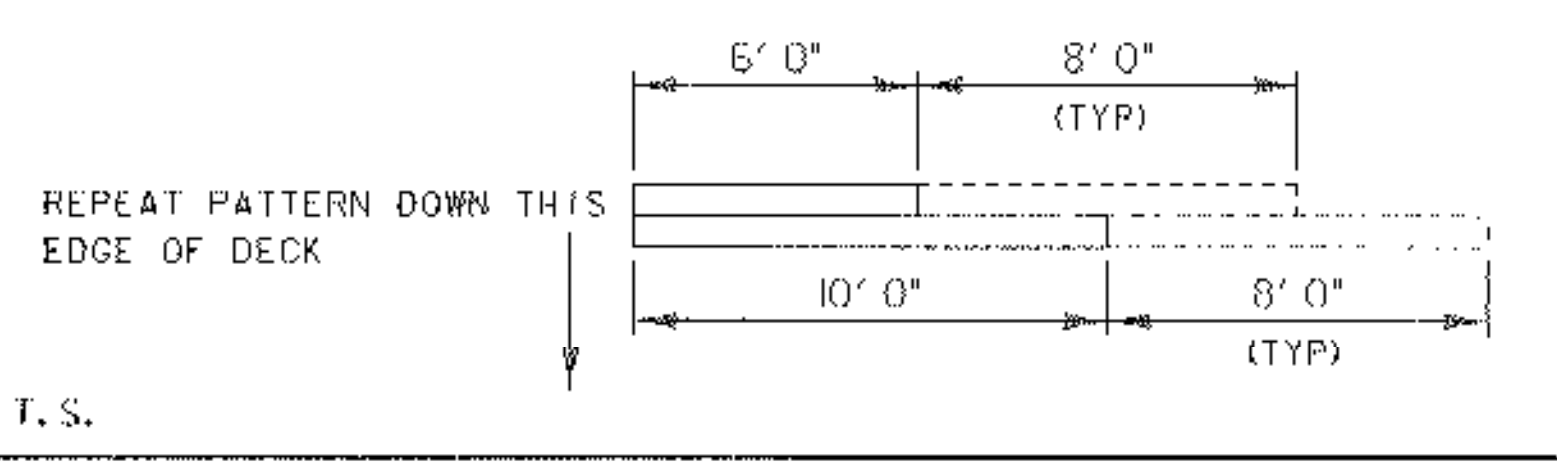


### PLANK LAYOUT

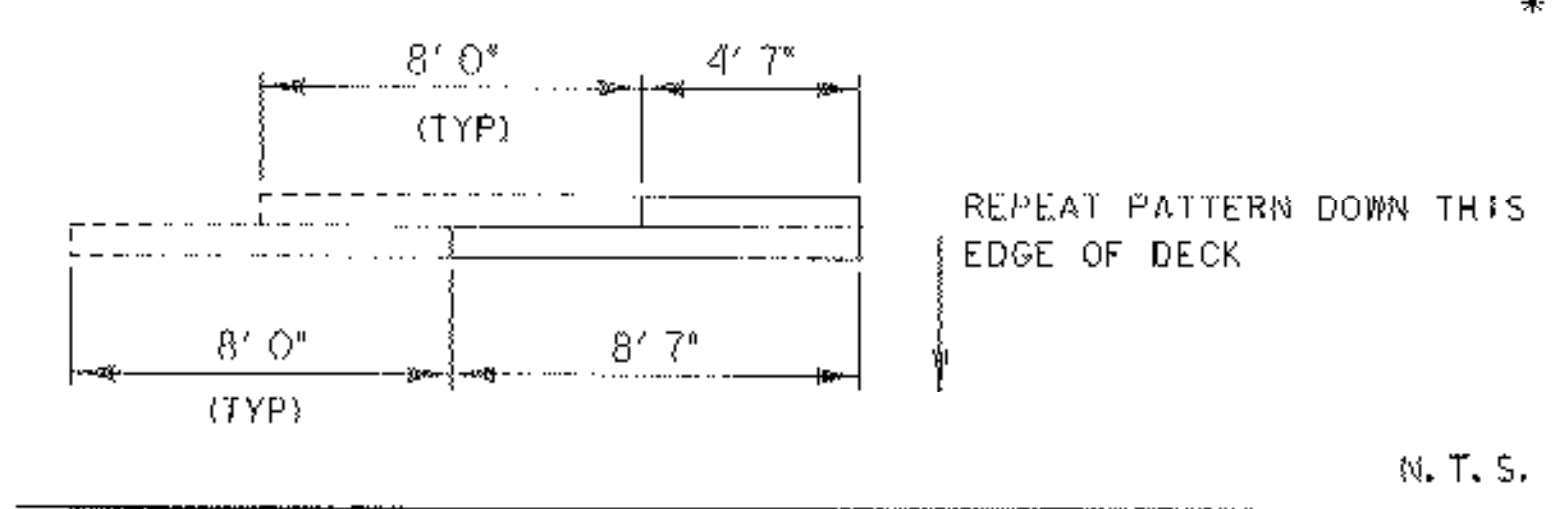
SCALE 1/4"=1'

PLANK DESCRIPTION	
PLANK LOCATION	PLANK LENGTH
1	10'-0"
2	8'-7"
3	8'-0"
4	6'-0"
5	4'-7"

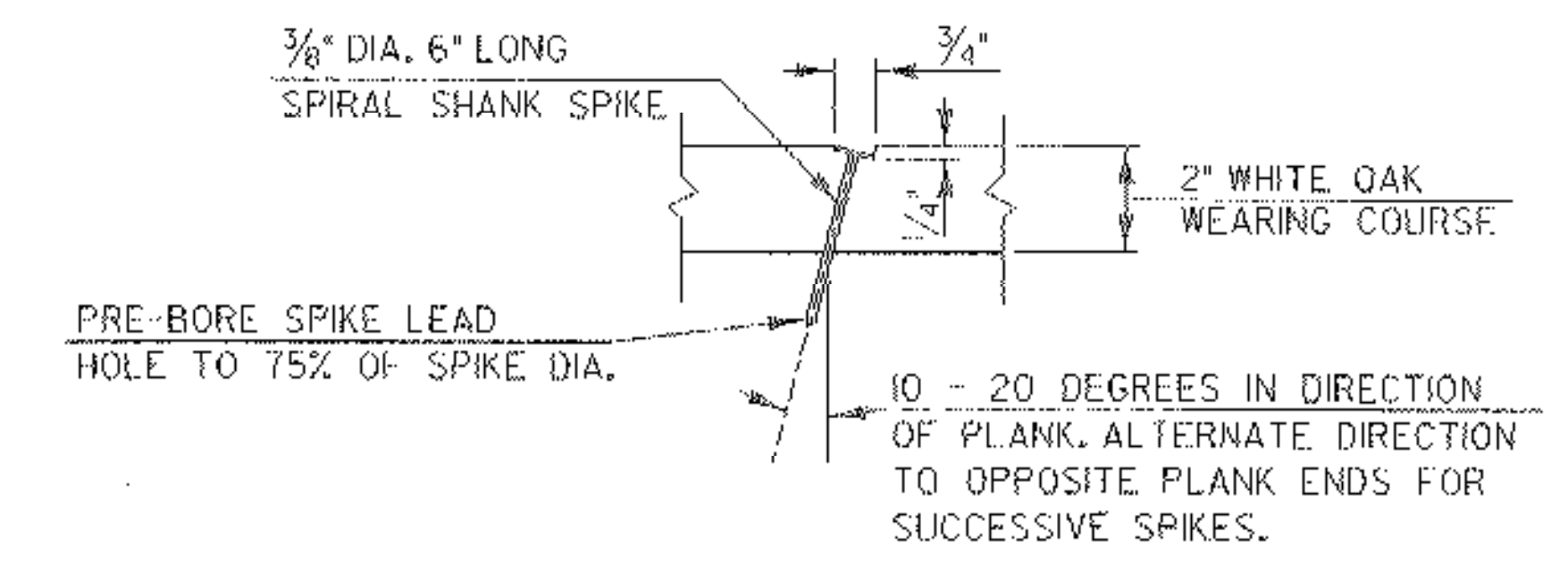
\* PLANK LOCATIONS SHOWN ON PLANK LAYOUT DETAIL



### ABUTMENT 1 PLANK DETAIL

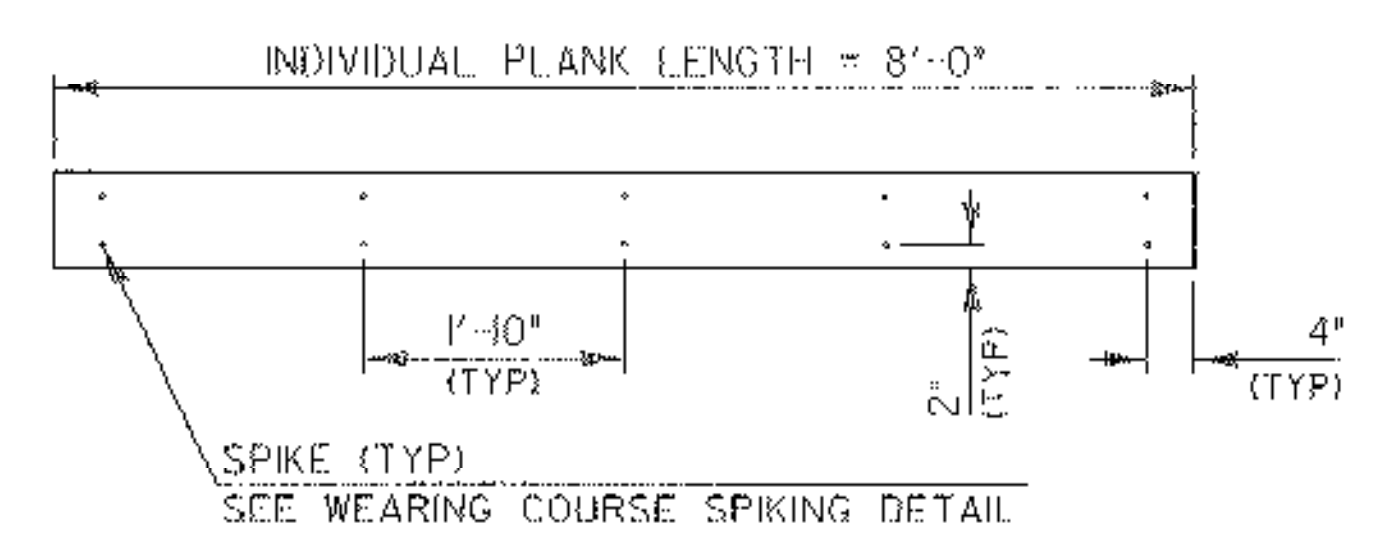


### ABUTMENT 2 PLANK DETAIL

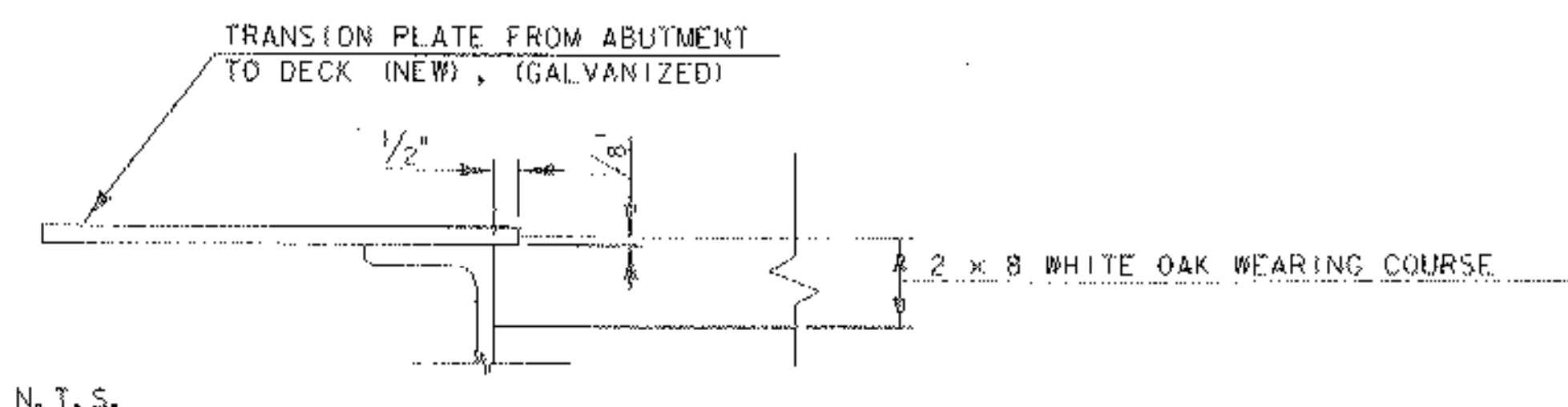


### WEARING COURSE SPIKING DETAIL

GENERAL NOTE:  
THE TERM PLANKING AND OR WEARING COURSE INDICATES THE USE OF 2 x 8 (ACTUAL DIMENSIONS) ROUGH SAWN WHITE OAK PLANKS.



### DECK WEARING SURFACE SPIKE LAYOUT

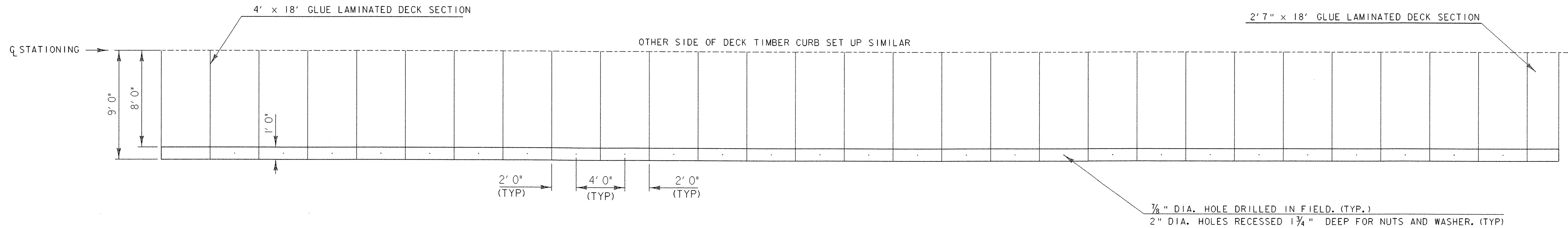


### PLANK END NOTCHING DETAIL

### PLANK WEARING COURSE LAYOUT

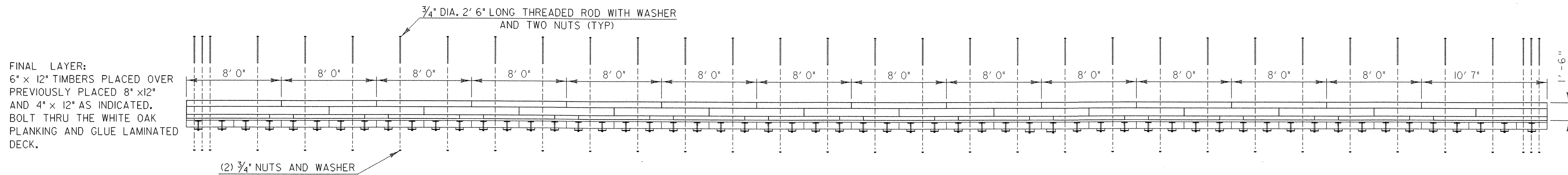
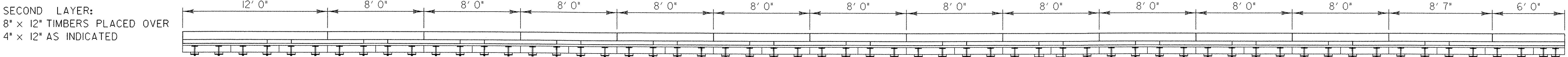
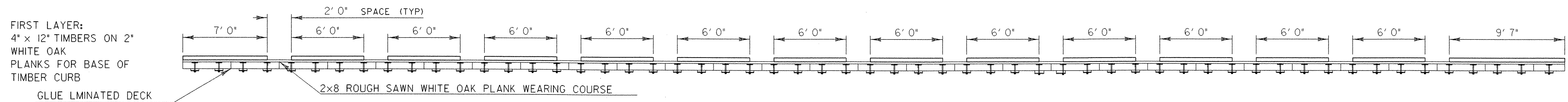
PROJECT NAME:	THETFORD	PLOT DATE:	24-OCT-2006
PROJECT NUMBER:	BHQ 1444 (43)	DRAWN BY:	G.ROKES
FILE NAME:	Structures\s03j036abt.dgn	CHECKED BY:	S. SCRIBNER
DESIGNED BY:	G.ROKES	SHEET	32 OF 60
IPARM:	s03j036bpl1		

REVISED OCT. 24, 2006



**TIMBER CURB TO DECK PLACEMENT  
PLAN VIEW**

SCALE 1/4"=1'



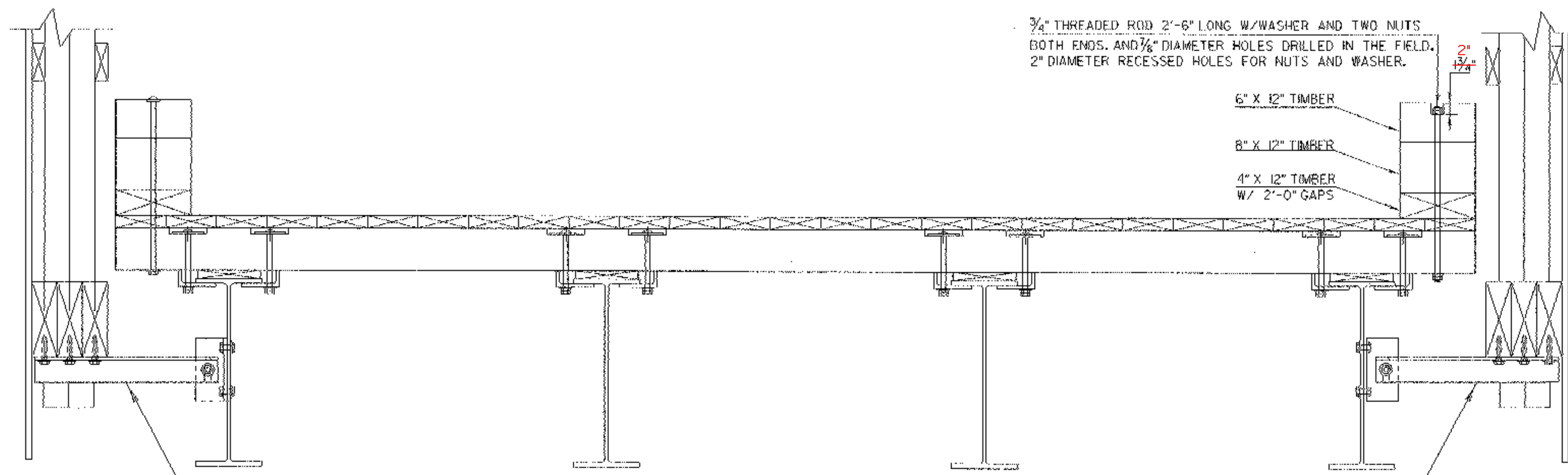
**TIMBER CURB ASSEMBLY  
ELEVATION VIEW**

SCALE 1/4"=1'

**TIMBER CURB ATTACHMENT TO  
DECK LAYOUT SHEET**

PROJECT NAME:	THETFORD	FILE NAME:	Structures\s03j036abt.dgn	PLOT DATE:	24-OCT-2006
PROJECT NUMBER:	BHO 1444 (43)	PROJECT LEADER:	M.EVANS-MONGEON	DRAWN BY:	G.ROKES
		DESIGNED BY:	M. EVANS-MONGEON	CHECKED BY:	S. SCRIBNER
		IPARM:	s03j036tra.j	SHEET	33 OF 60

**REVISED OCT. 24, 2006**

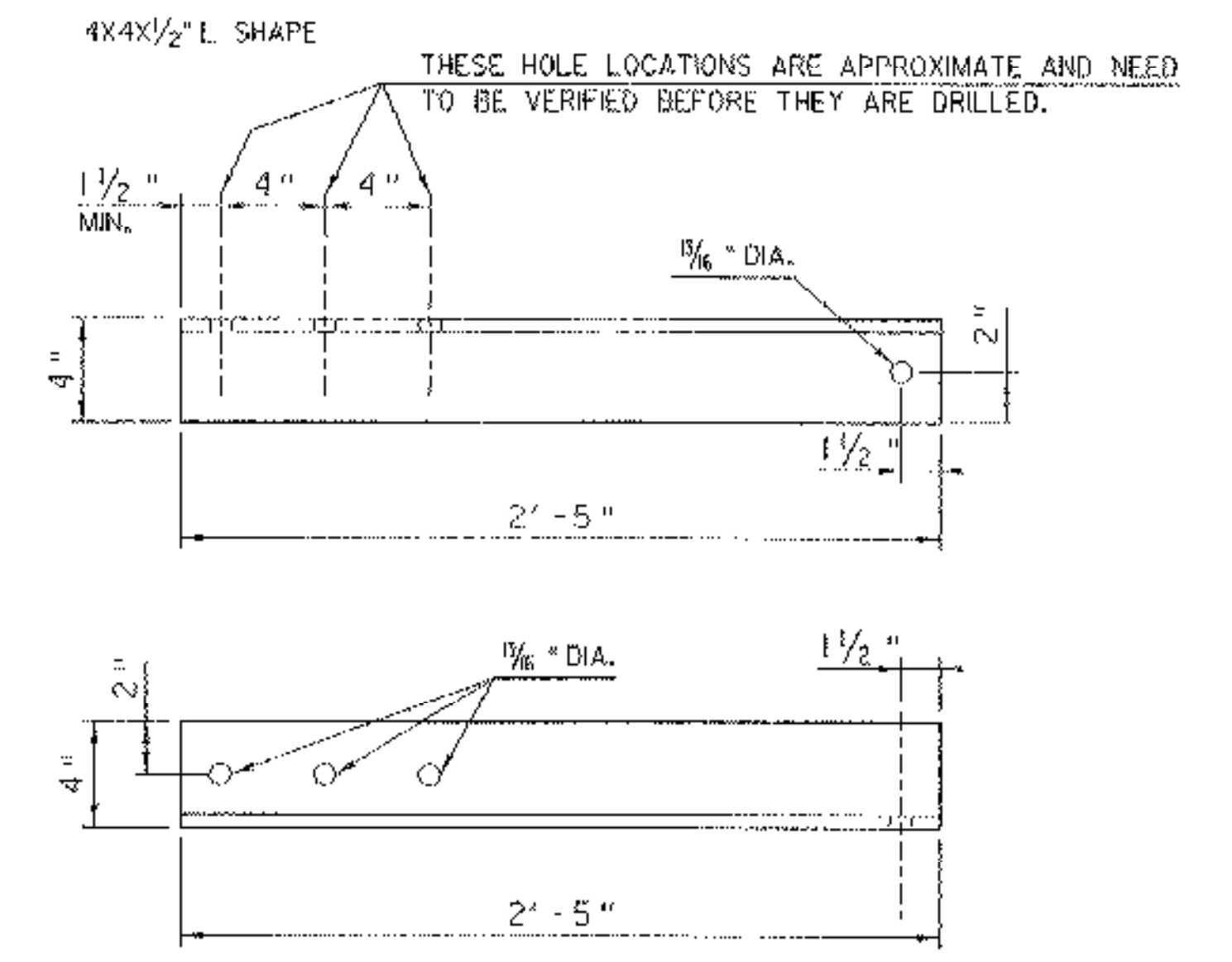


EXISTING EXTERIOR GIRDER TO TIMBER TRUSS CONNECTION WILL BE REMOVED TO WITHIN  $\frac{1}{2}$ " OF THE GIRDER WEB. PAYMENT FOR THIS WORK IS INCIDENTAL TO ITEM 506.60 STRUCTURAL STEEL.

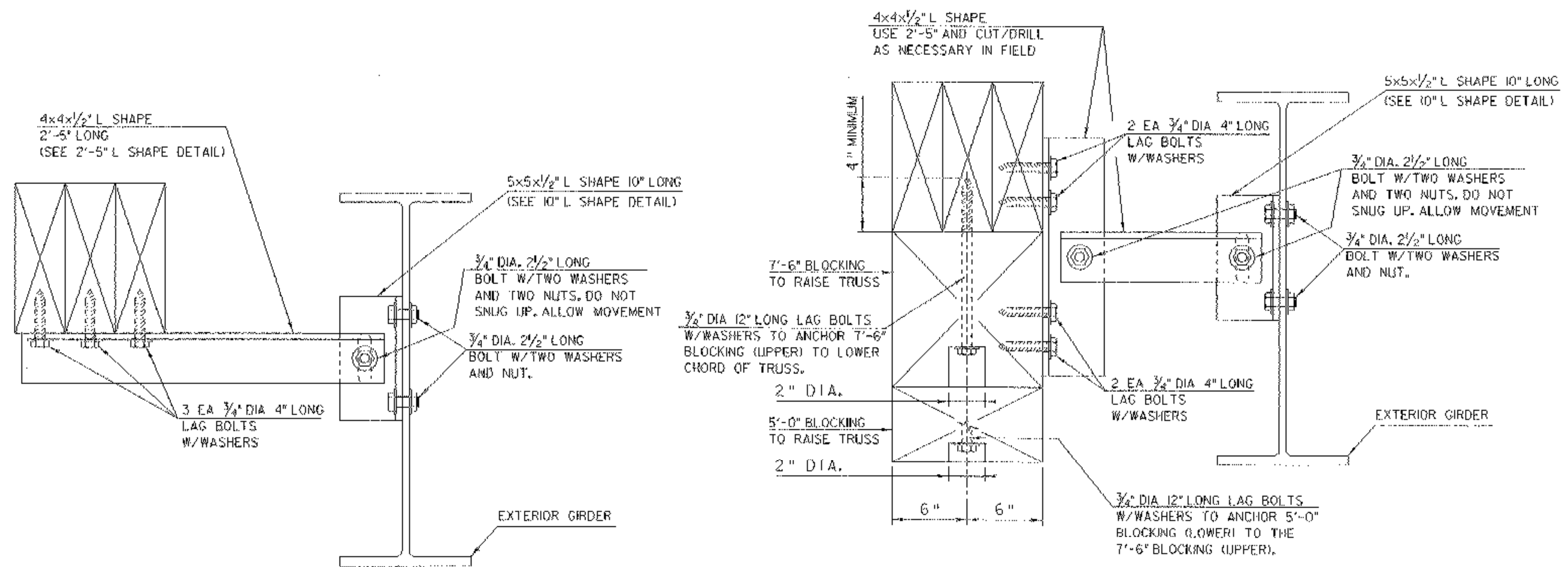
LOWER CHORD TO EXTERIOR GIRDER ATTACHMENT. PLACE THIS ASSEMBLY NEAR EACH DIAPHRAM (DO NOT DRILL WITHIN 6" OF DIAPHRAM WELDS) AND PLACE THESE ASSEMBLIES AT MID POINTS BETWEEN DIAPHRAMS. (SEE LOWER CHORD ATTACHMENT DETAIL) OTHER SIDE SIMILAR

SCALE 1" = 1'

### LOWER CHORD TO EXTERIOR GIRDER ATTACHMENT



### 2'-5" L SHAPE DETAIL



NOTE: THREE PILOT HOLES ARE TO BE DRILLED INTO THE LOWER CHORD FOR LAG BOLT PLACEMENT. THESE HOLES WILL BE  $\frac{1}{2}$ " DIAMETER.

PAYMENT FOR THIS DETAIL IS INCIDENTAL TO PAY ITEM 506.60 STRUCTURAL STEEL.

NTS

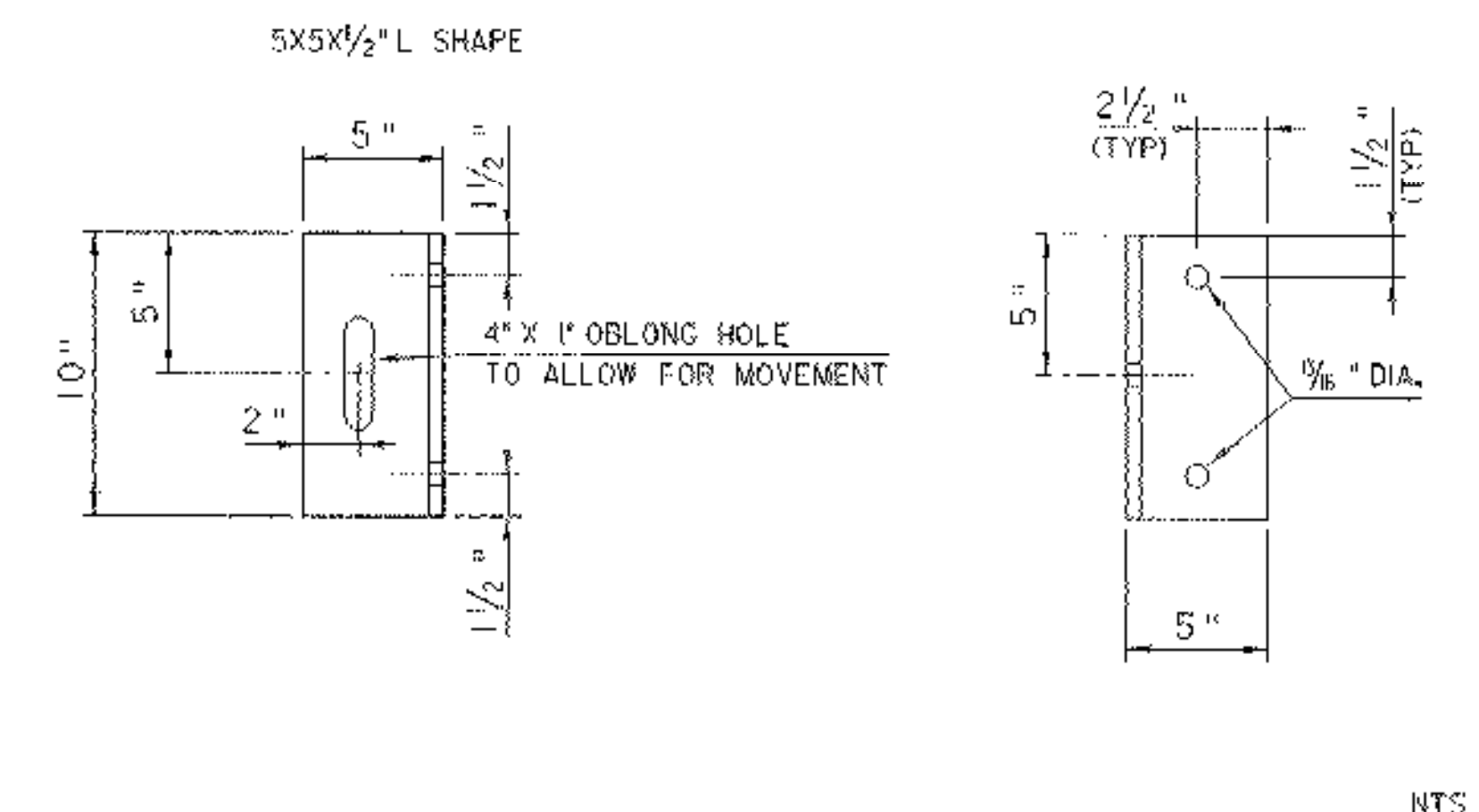
### LOWER CHORD ATTACHMENT DETAIL

NOTE: FOUR PILOT HOLES ARE TO BE DRILLED INTO THE LOWER CHORD FOR LAG BOLT PLACEMENT. THESE HOLES WILL BE  $\frac{1}{2}$ " DIAMETER.

PAYMENT FOR THIS DETAIL IS INCIDENTAL TO PAY ITEM 506.60 STRUCTURAL STEEL.

NTS

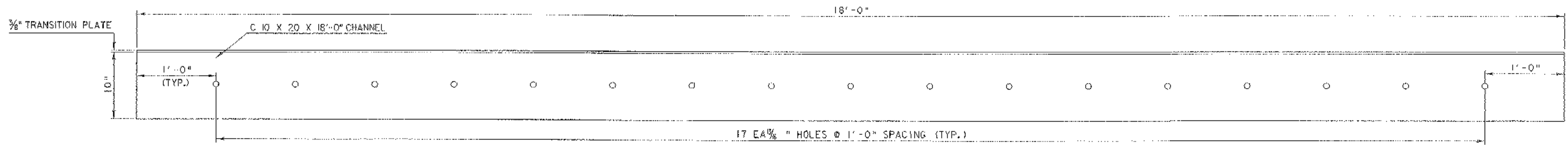
### LOWER CHORD ATTACHMENT NEAR PIER BLOCKING DETAIL



### 10" L SHAPE DETAIL

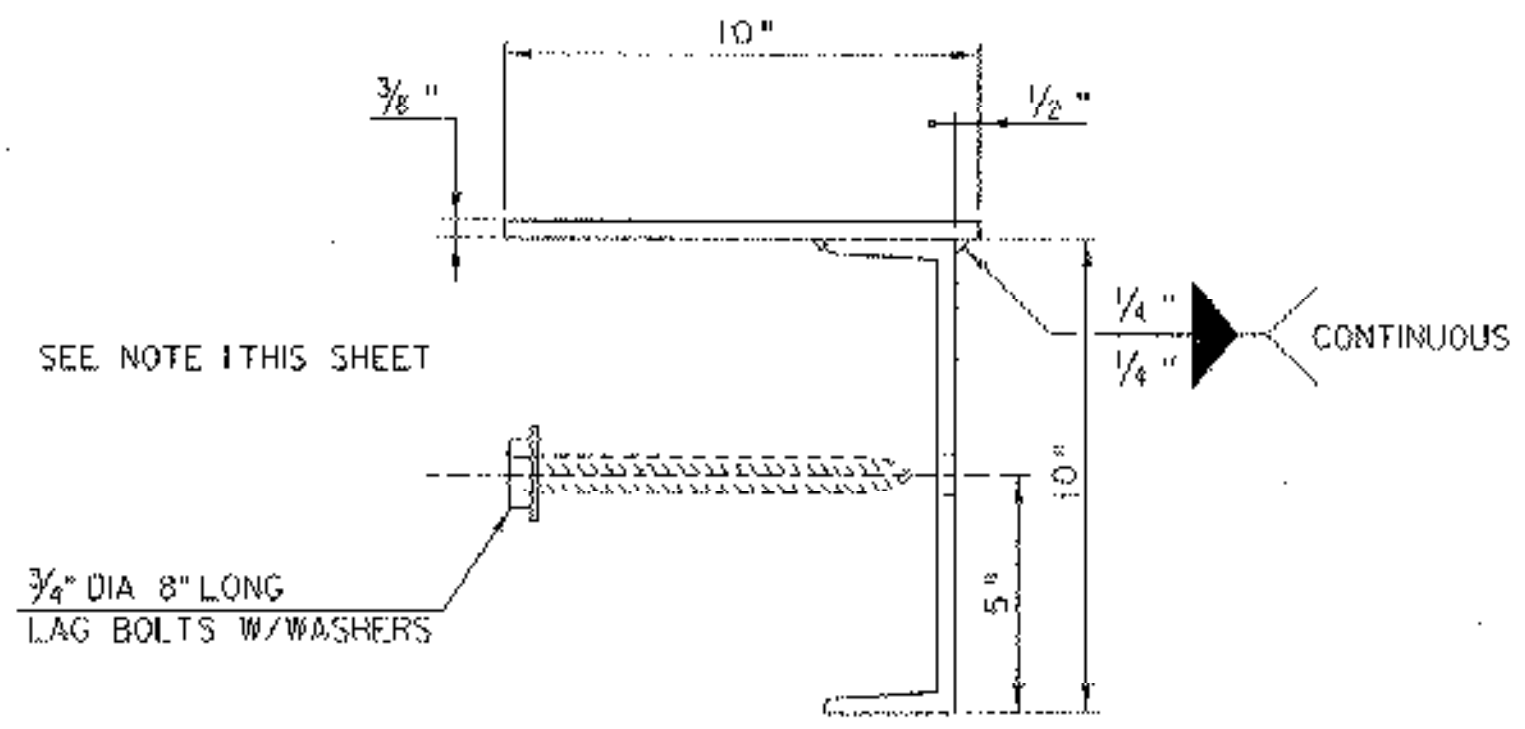
### LOWER CHORD TO EXTERIOR GIRDER ATTACHMENT DETAILS

PROJECT NAME:	THETFORD	PLOT DATE:	27-SEP-2006
PROJECT NUMBER:	BHO 1444 (43)	DRAWN BY:	GROKES
FILE NAME:	Structures\s03j036ab1.dgn	CHECKED BY:	S. SCRIBNER
DESIGNED BY:	M. EVANS-MONGEON	IPARM:	s03j036b1.q1
		SHEET	34 OF 60

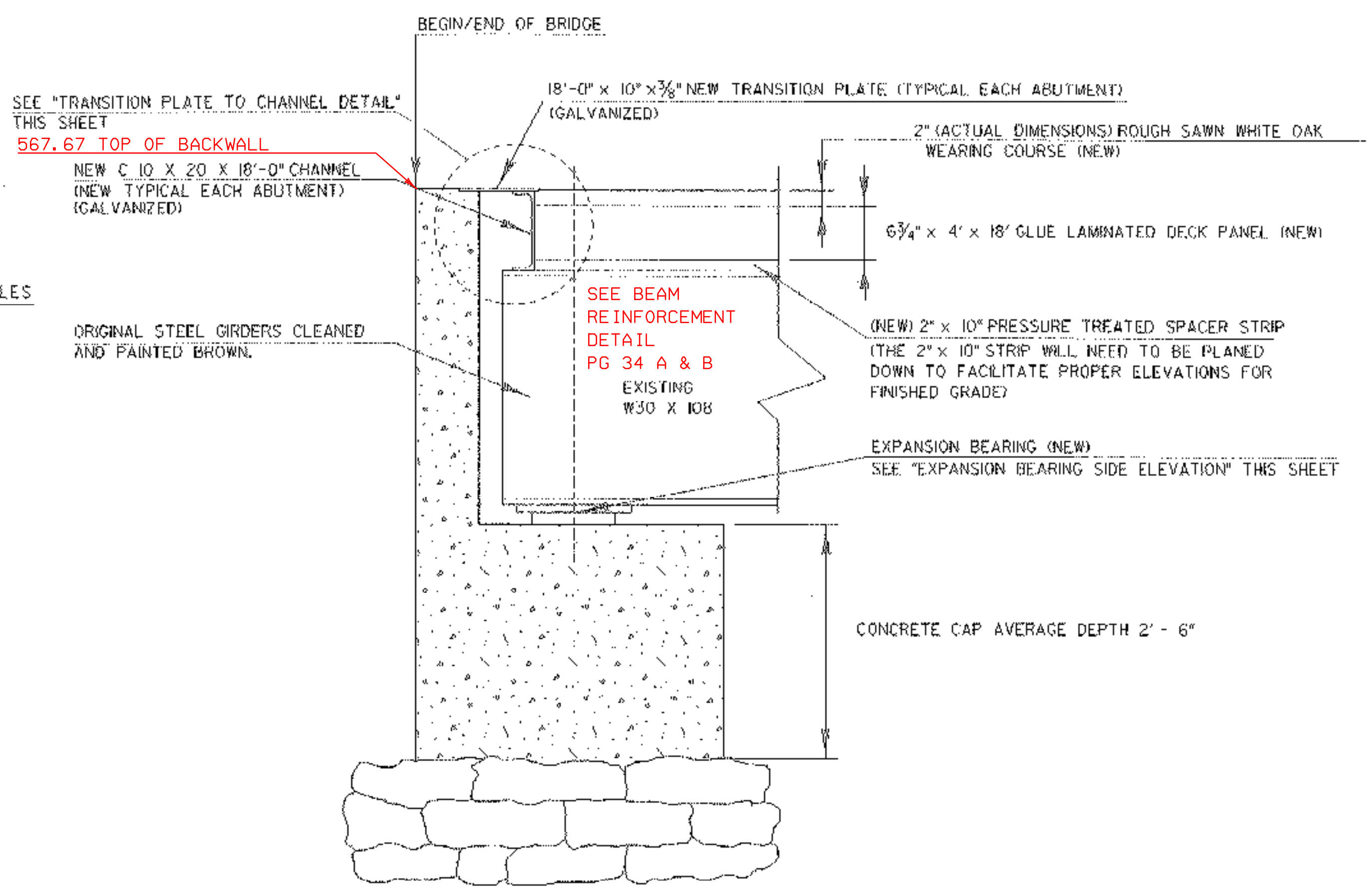


**TRANSITION PLATE /CHANNEL HOLE LOCATIONS**  
SCALE 1/8" = 1"

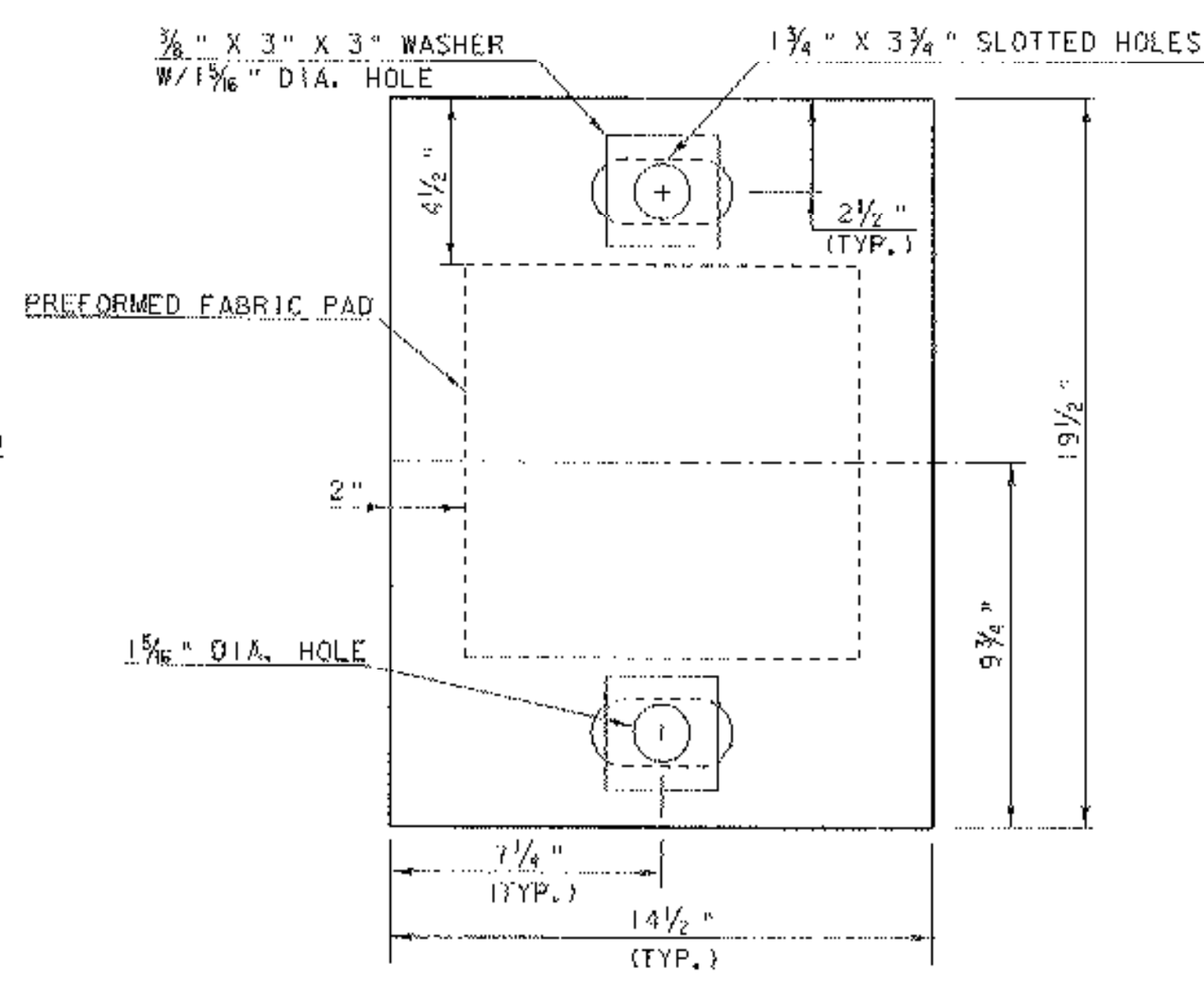
NOTE 1: TRANSITION PLATE WILL BE WELDED TO CHANNEL AS INDICATED IN DETAIL. 17 EA. 1/8" DIA. HOLES WILL BE DRILLED IN TO CHANNEL AS INDICATED. THIS WELDED AND DRILLED UNIT WILL BE GALVANIZED. 1/2" DIA. PILOT HOLES WILL BE DRILLED IN TO LAMINATED DECK FOR PROPER TRANSITION PLATE/CHANNEL CONNECTION. THE TRANSITION PLATE / CHANNEL WILL BE BOLTED TO THE DECK PANEL PRIOR TO ATTACHING THE DECK PANEL TO THE BEAMS. A GAP OF 1/4" BETWEEN THE LIP OF THE CONCRETE IN THE BACK WALL AND THE TRANSITION PLATE / CHANNEL IS EXPECTED AT 75 DEGREES FAHRENHEIT.



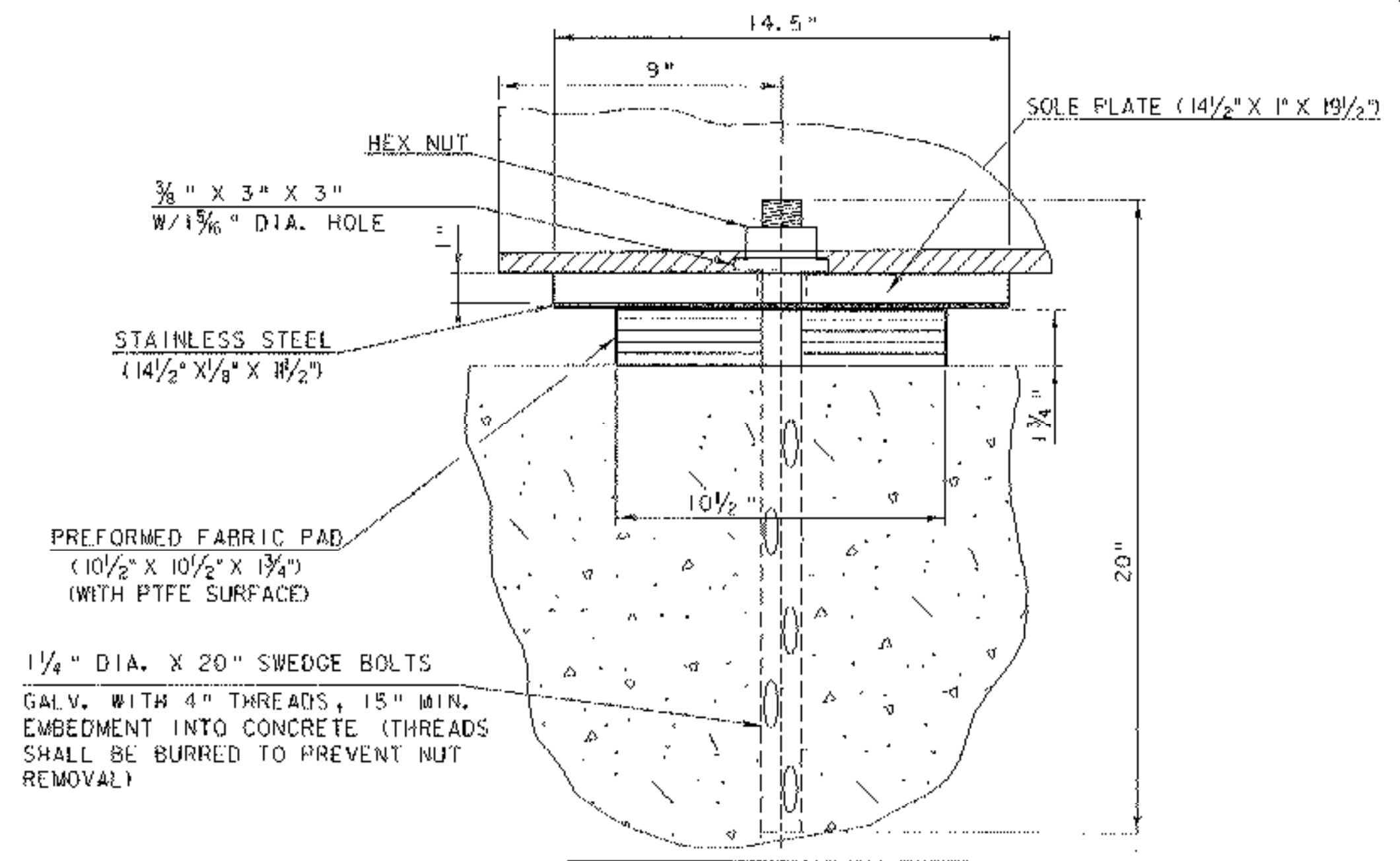
**TRANSITION PLATE TO CHANNEL DETAIL**  
SCALE 1/4" = 1"



**ABUTMENT 1 TYPICAL**  
SCALE 1" = 1'



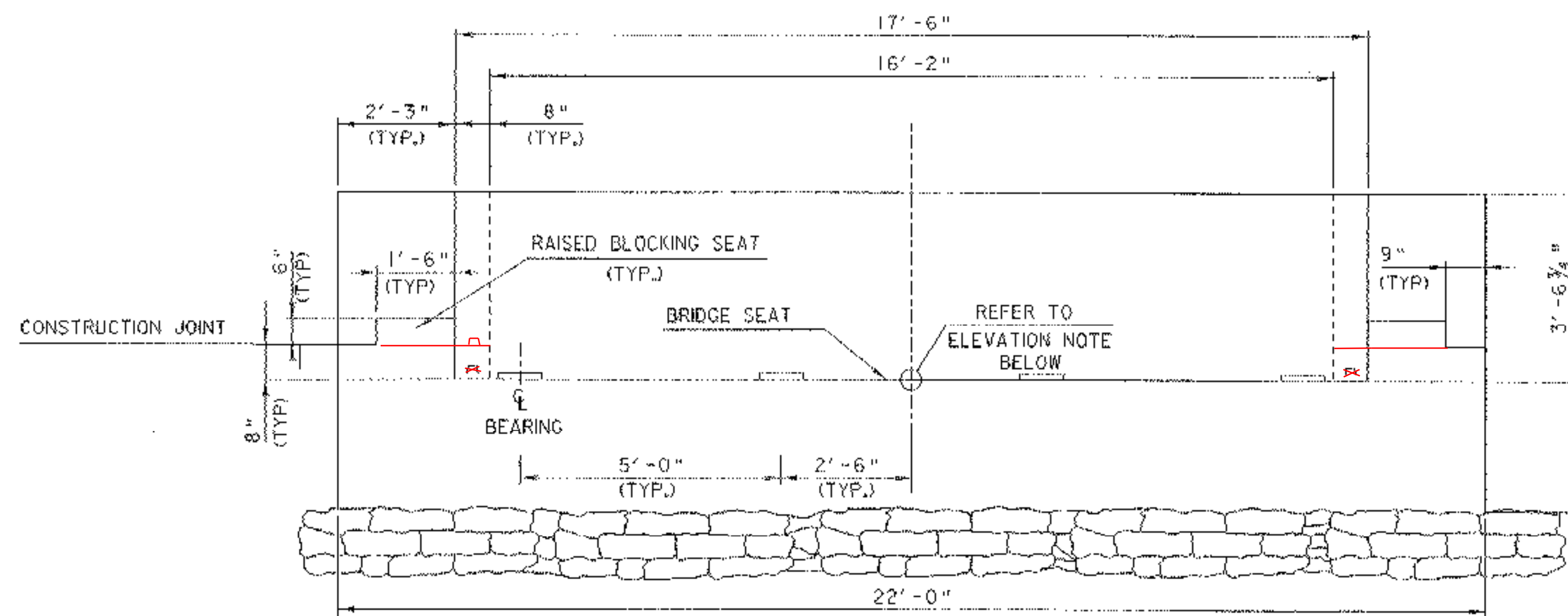
**SOLE PLATE DETAIL**  
SCALE 1/4" = 1"



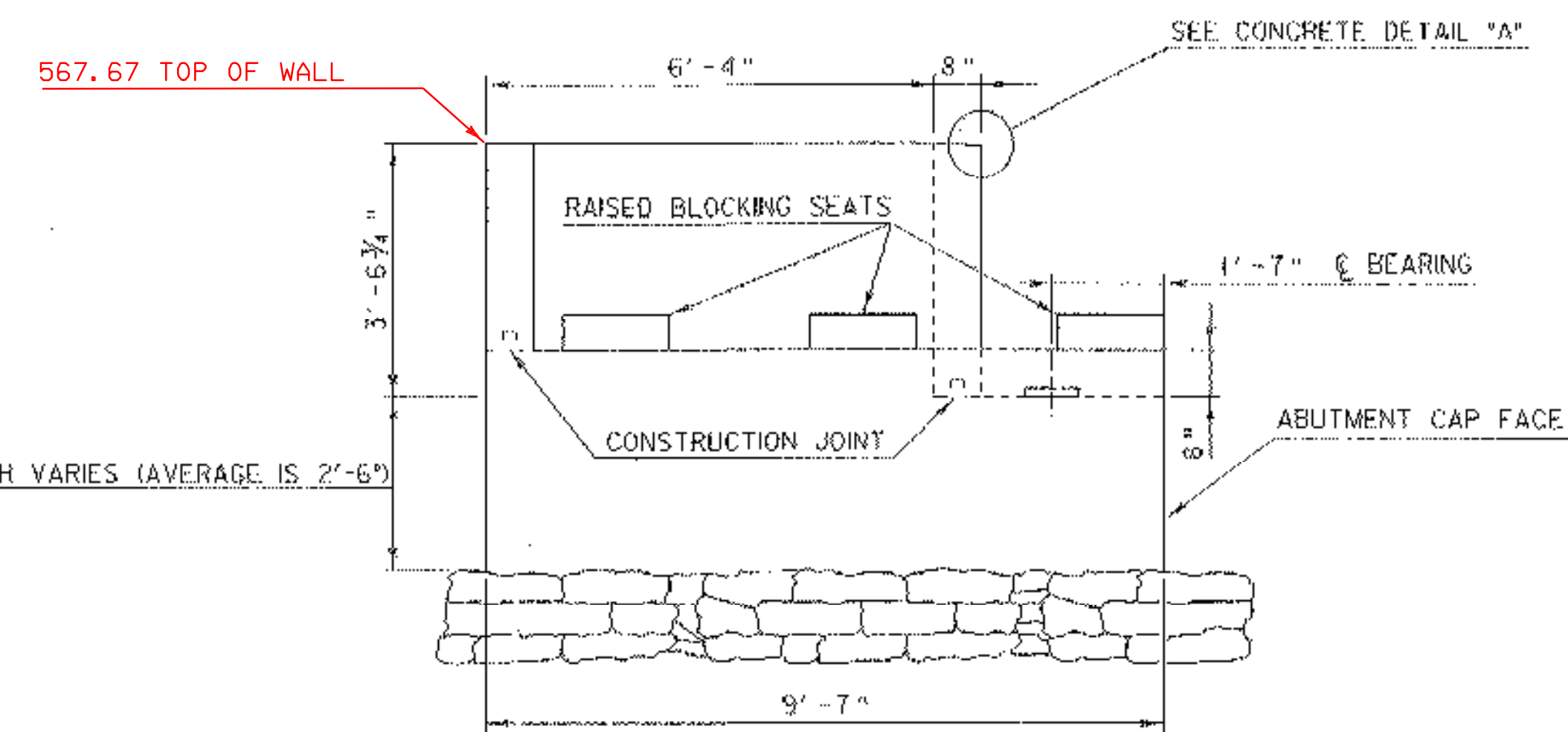
**EXPANSION BEARING SIDE ELEVATION**  
SCALE 1/4" = 1"

ABUTMENT 1 TYPICAL	
PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures\s03j036abt.dgn
PROJECT LEADER:	M.EVANS-MONGEON
DESIGNED BY:	G.ROKES
IPARM:	s03j036brq.j
PLGT DATE:	24-OCT-2006
DRAWN BY:	G.ROKES
CHECKED BY:	S.SCRIBNER
SHEET:	35 OF 60

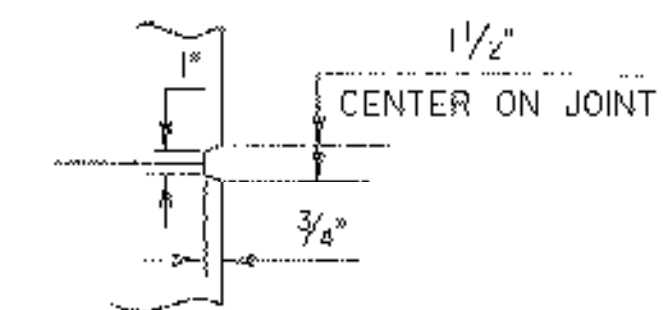
**REVISED OCT. 24, 2006**



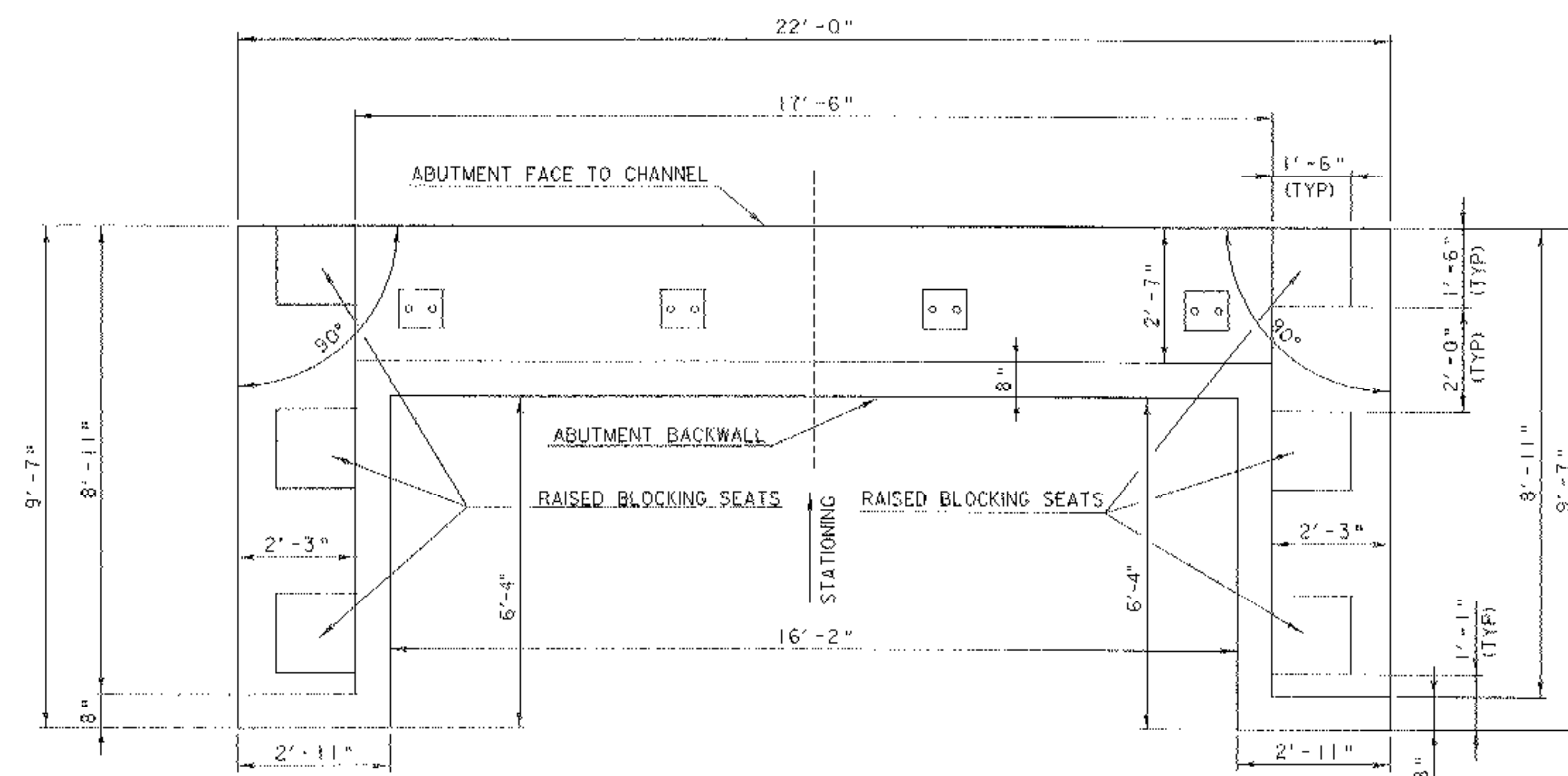
**ABUTMENT #1 FRONT ELEVATION VIEW**



**ABUTMENT #1 SIDE ELEVATION VIEW**

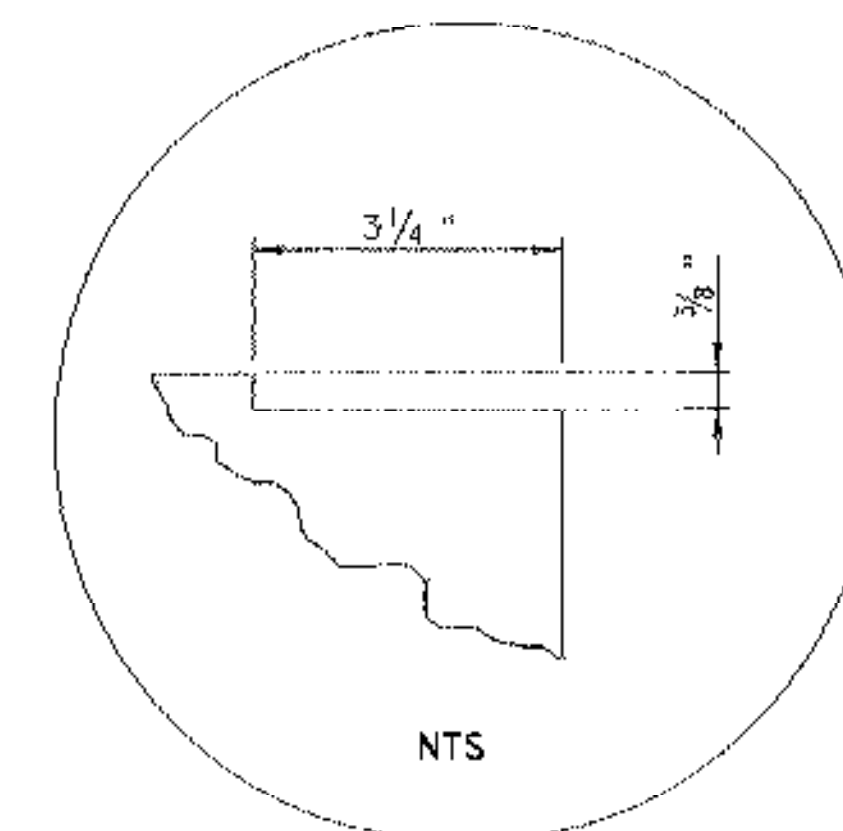


**SCORE MARK DETAIL**

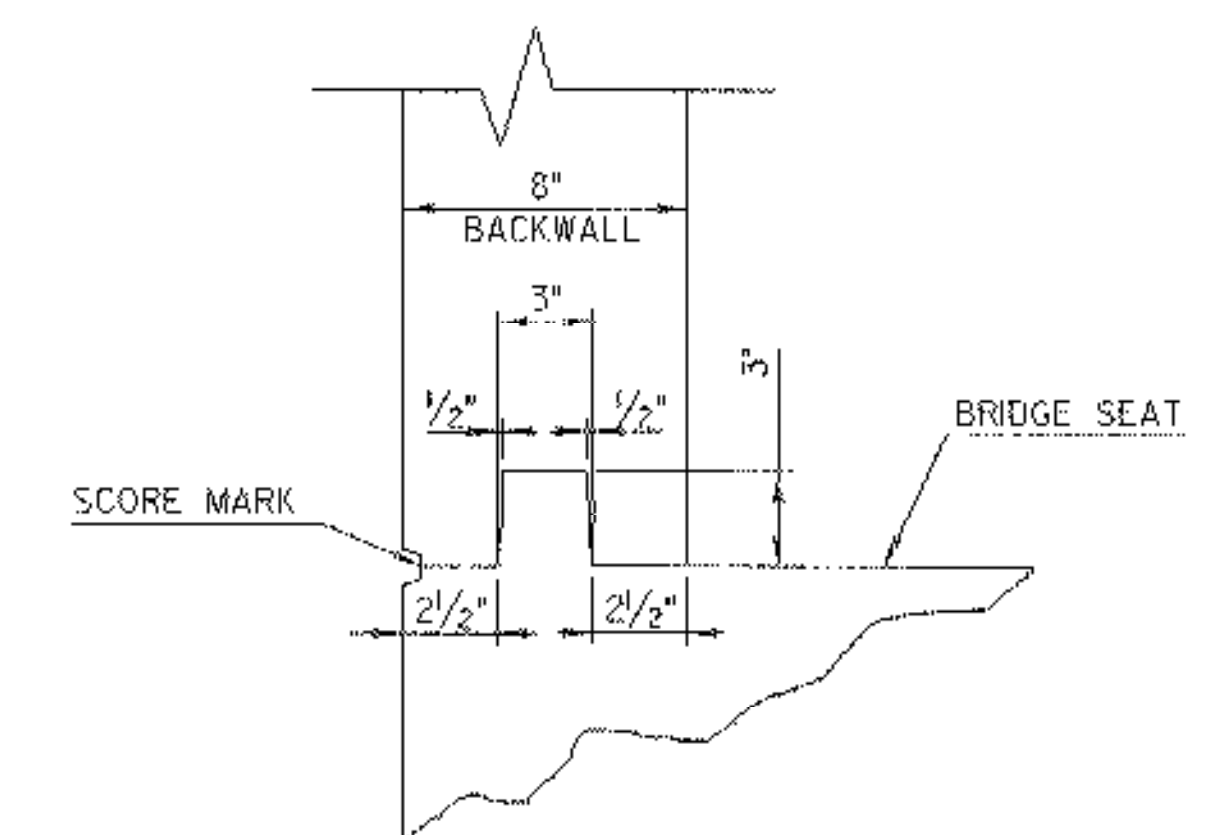


**ABUTMENT #1 PLAN VIEW**

ELEVATION NOTE: PROPOSED BRIDGE SEAT ELEVATION WILL BE SET  $\frac{7}{16}$ " BELOW THE EXISTING BRIDGE SEAT ELEVATION. EXISTING BRIDGE SEAT ELEVATION TO BE MEASURED IN THE FIELD.



**CONCRETE DETAIL "A"**



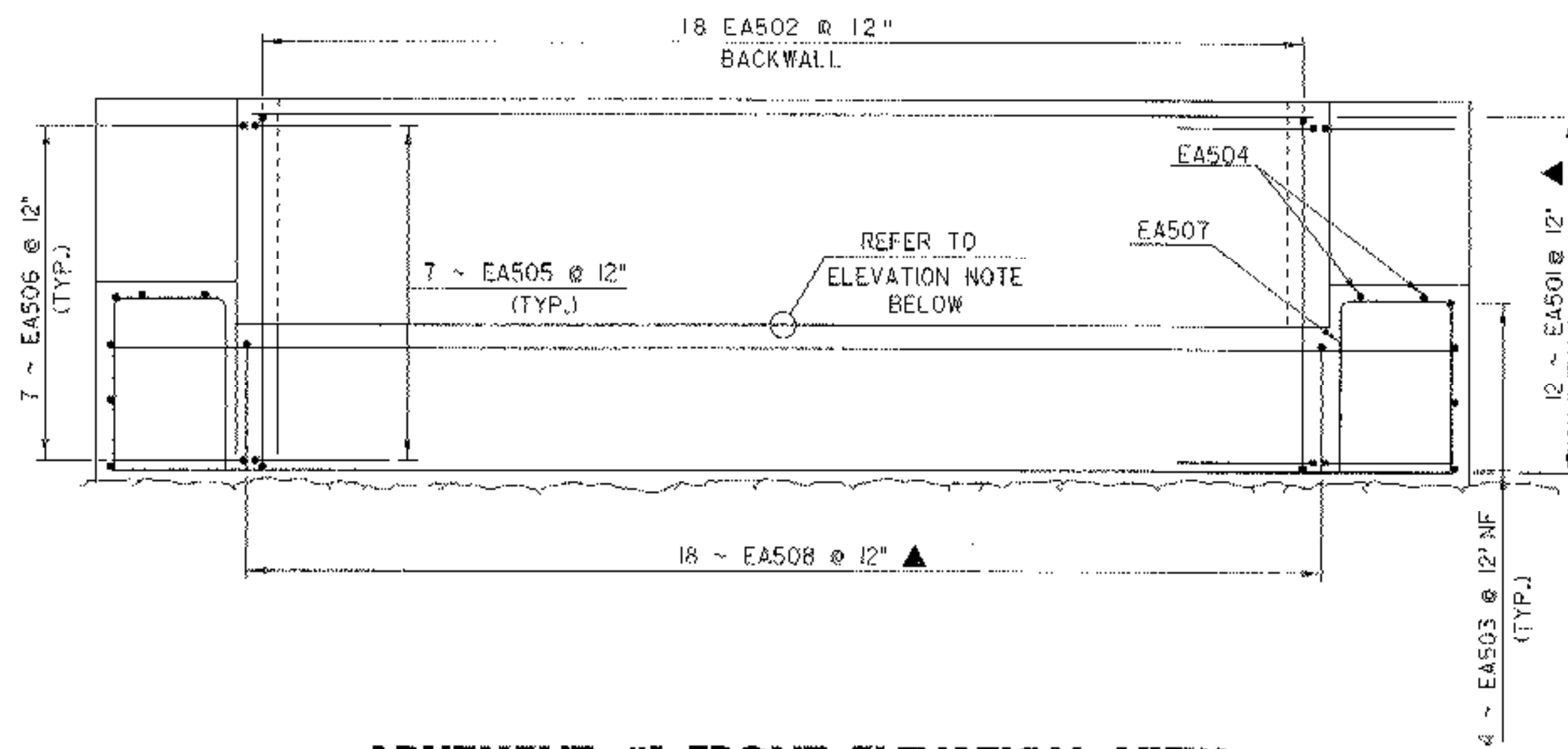
**CONCRETE CONSTRUCTION JOINT**

SCALE  $\frac{1}{2}$ " = 1'-0"

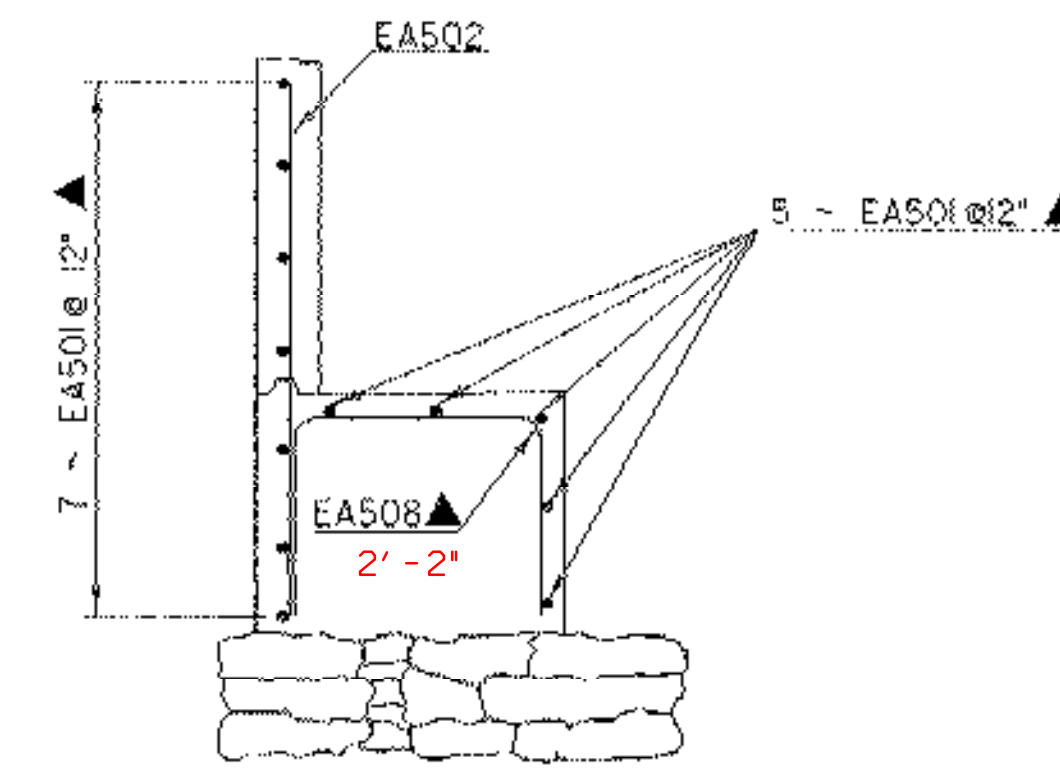
\* NOTE: ALL DIMENSIONS TO BE VERIFIED BY THE CONTRACTOR

**ABUTMENT CAP #1 DETAIL SHEET**

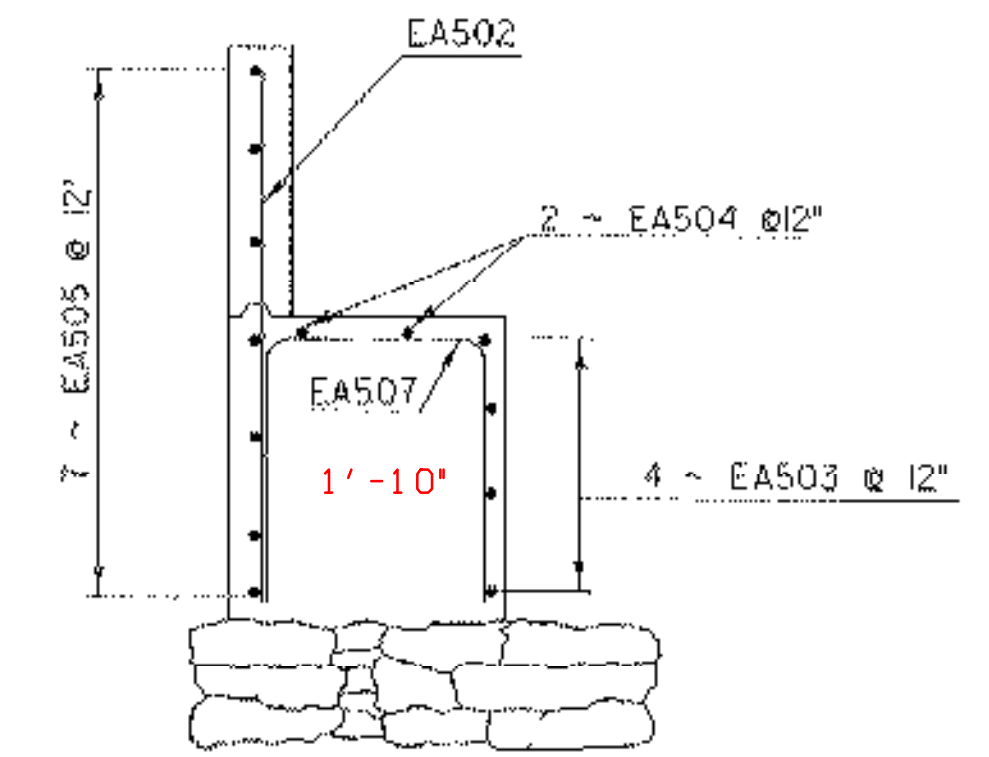
PROJECT NAME:	THETFORD	PLOT DATE:	27-SEP-2006
PROJECT NUMBER:	BHO 1444 (43)	DRAWN BY:	G.ROKES
FILE NAME:	Structures\03J036ab1.dgn	CHECKED BY:	S.SCRIBNER
DESIGNED BY:	G.ROKES	SHEET	36 OF 60
IPARM:	s03j036ab1		



**ABUTMENT #1 FRONT ELEVATION VIEW**

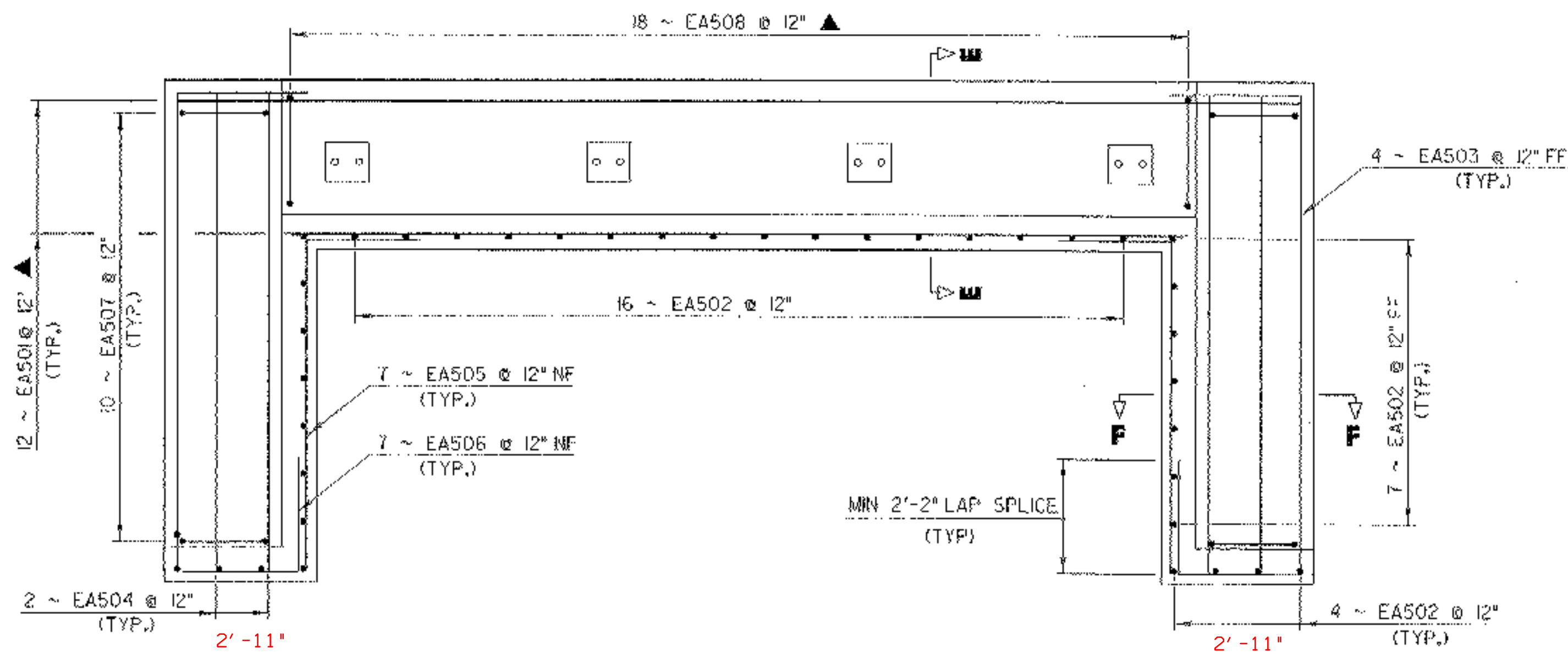


**E-E SECTION**



**F-F SECTION**

ELEVATION NOTE: PROPOSED BRIDGE SEAT ELEVATION WILL BE SET  $\frac{1}{16}$ " BELOW THE EXISTING BRIDGE SEAT ELEVATION. EXISTING BRIDGE SEAT ELEVATION TO BE MEASURED IN THE FIELD.



**ABUTMENT #1 PLAN VIEW**

**NOTE:**

NF = NEAR FACE  
FF = FAR FACE  
EF = EACH FACE

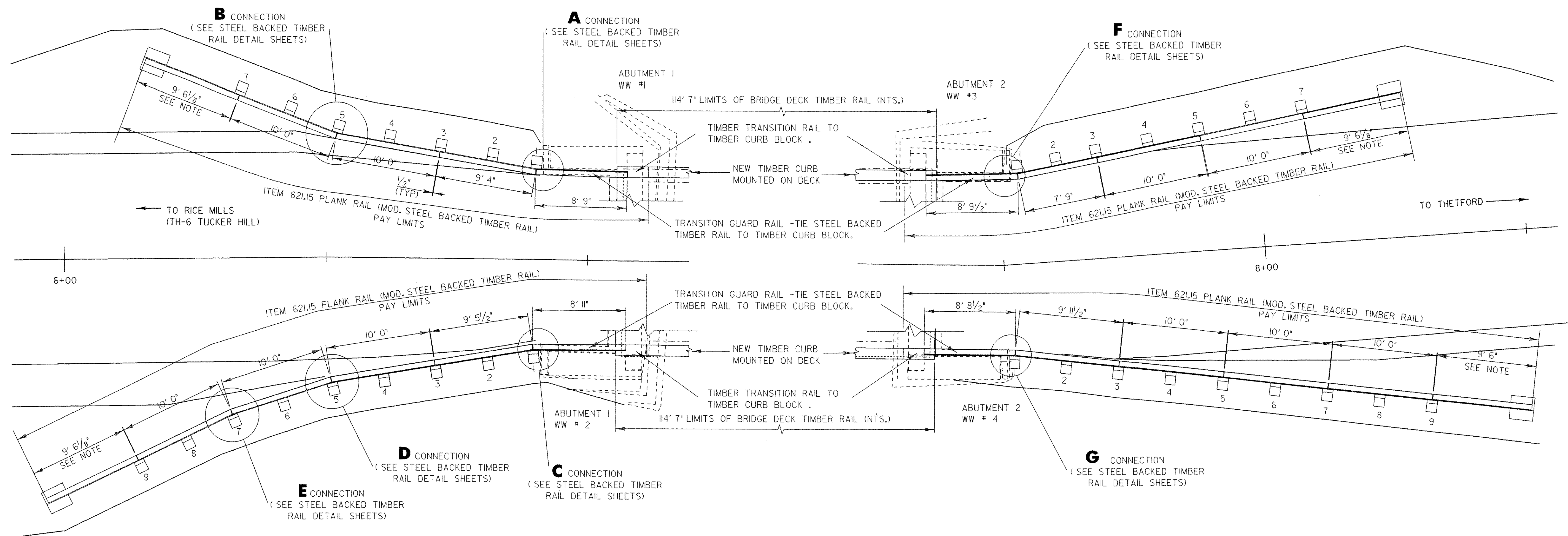
▲ = A CUT TO FIT IN THE FIELD. EPOXY COATED STEEL MUST BE PROPERLY SEALED AFTER THE CUT. REFER TO SPEC. 507.04 3 INCHES CLEAR UNLESS OTHERWISE SPECIFIED ON THE PLANS

**ABUTMENT CAP #1 REBAR LAYOUT SHEET**

PROJECT NAME:	THETFORD		
PROJECT NUMBER:	BHO 1444 (43)		
FILE NAME:	Structures\03j036abt.dgn	PLOT DATE:	27-SEP-2006
PROJECT LEADER:	M. EVANS-MONGEON	DRAWN BY:	G. ROKES
DESIGNED BY:	M. EVANS-MONGEON	CHECKED BY:	S. SCRIBNER
IPARM:	03j036ar-1	SHEET	37 OF 60

SCALE  $\frac{1}{2}$ " = 1'-0"





**STEEL BACKED TIMBER RAIL POST SPACING  
(MEASUREMENTS ARE TO CENTER OF POST)**

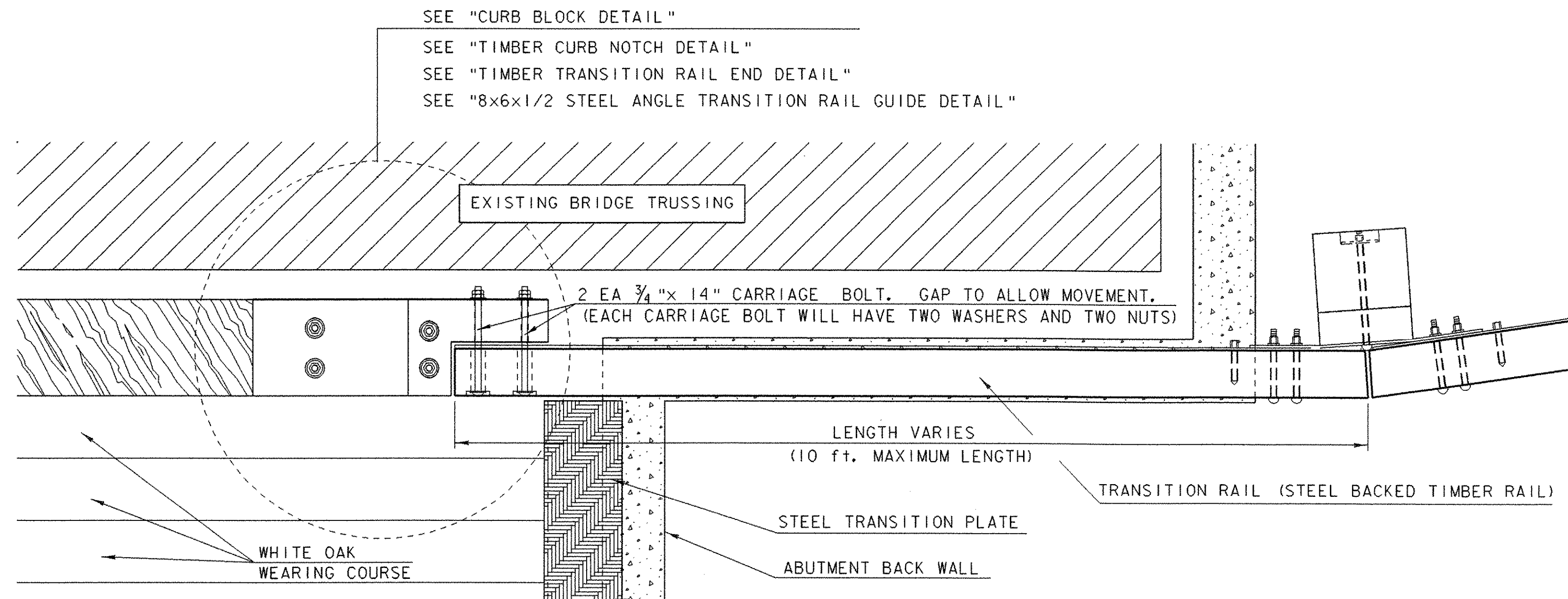
POST	ABUTMENT 1		ABUTMENT 2	
	WW# 1	WW#2	WW#3	WW#4
1	STA. 6+45.07 LT 9.41	STA. 6+44.91 RT 9.48	STA. 7+76.88 LT 9.45	STA. 7+75.33 RT 9.39
2	STA. 6+40.89 LT 10.12	STA. 6+40.62 RT 10.14	STA. 7+80.60 LT 9.98	STA. 7+80.17 RT 10.17
3	STA. 6+35.94 LT 10.95	STA. 6+35.68 RT 10.98	STA. 7+84.47 LT 10.45	STA. 7+85.13 RT 11.15
4	STA. 6+31.05 LT 11.75	STA. 6+30.73 RT 11.81	STA. 7+89.45 LT 11.11	STA. 7+90.07 RT 12.04
5	STA. 6+26.17 LT 12.62	STA. 6+25.91 RT 12.67	STA. 7+94.42 LT 11.77	STA. 7+95.02 RT 12.90
6	STA. 6+21.84 LT 14.35	STA. 6+20.95 RT 14.13	STA. 7+99.39 LT 12.47	STA. 7+99.97 RT 13.78
7	STA. 6+17.17 LT 16.20	STA. 6+16.22 RT 15.65	STA. 8+04.36 LT 13.17	STA. 8+04.91 RT 14.65
8		STA. 6+11.81 RT 17.72		STA. 8+09.85 RT 15.54
9		STA. 6+07.28 RT 19.87		STA. 8+14.80 RT 16.41
10				
11				
GUARD RAIL END ANCHOR	STA. 6+08.90 LT 18.66	STA. 5+99.20 RT 22.89	STA. 8+12.98 LT 13.62	STA. 8+23.28 RT 17.16

NOTE: THIS IS A TERMINAL END THAT TRANSITIONS INTO THE GROUND. THE LENGTH SHOWN IS A PLAN VIEW MEASUREMENT. FOR THE ACTUAL TIMBER LENGTH SEE THE ELEVATION VIEW ON SHEET 44.

**SCALE 1" = 5'-0"**

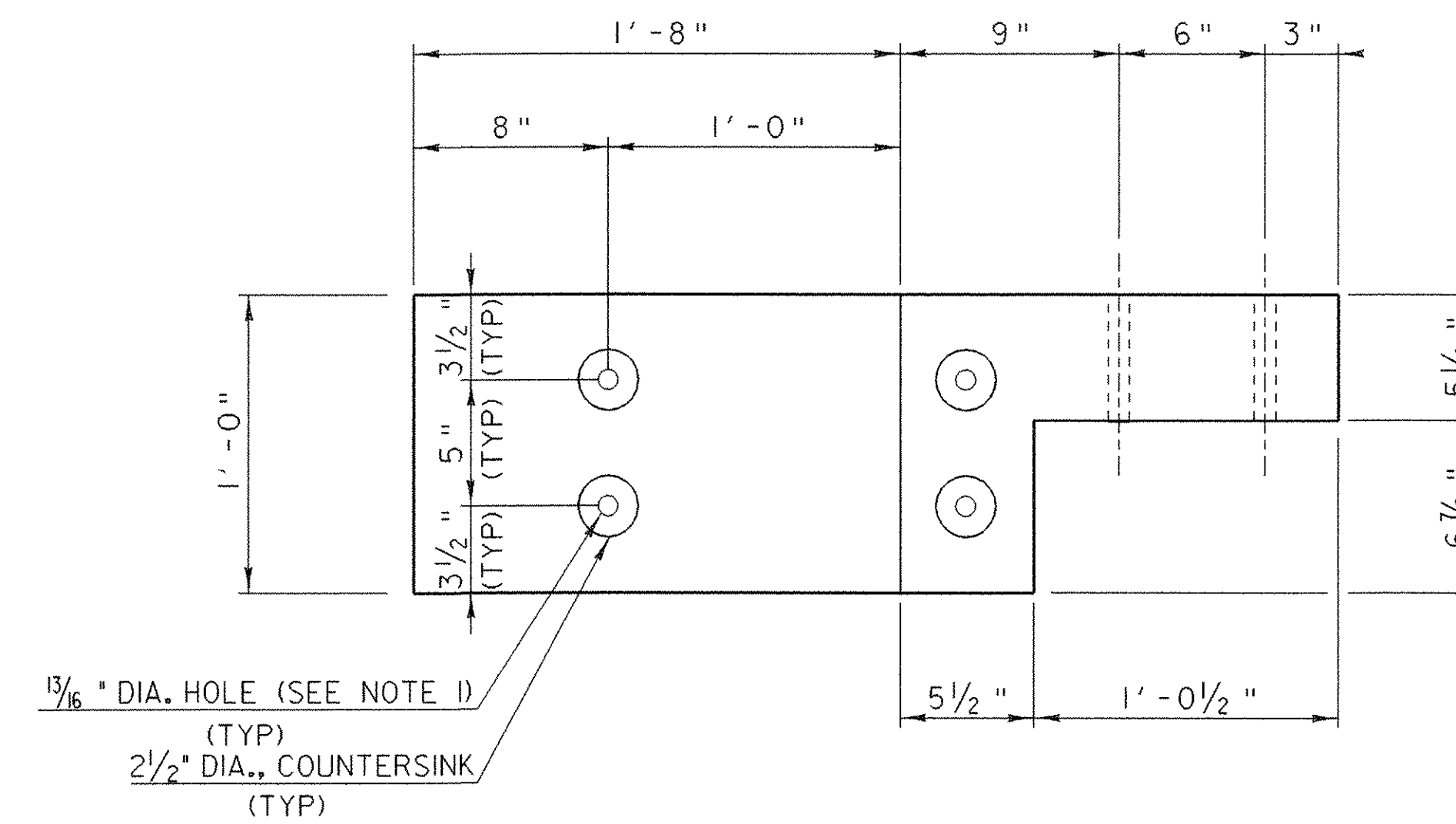
**STEEL BACKED TIMBER RAIL POST PLACEMENT SHEET**

PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures\s03j036ab+.dgn
PROJECT LEADER:	M.EVANS-MONGEON
DESIGNED BY:	G.ROKES
IPARM:	s03j036apr.1
PLOT DATE:	27-SEP-2006
DRAWN BY:	G.ROKES
CHECKED BY:	S.SCRIBNER
SHEET	39 OF 60



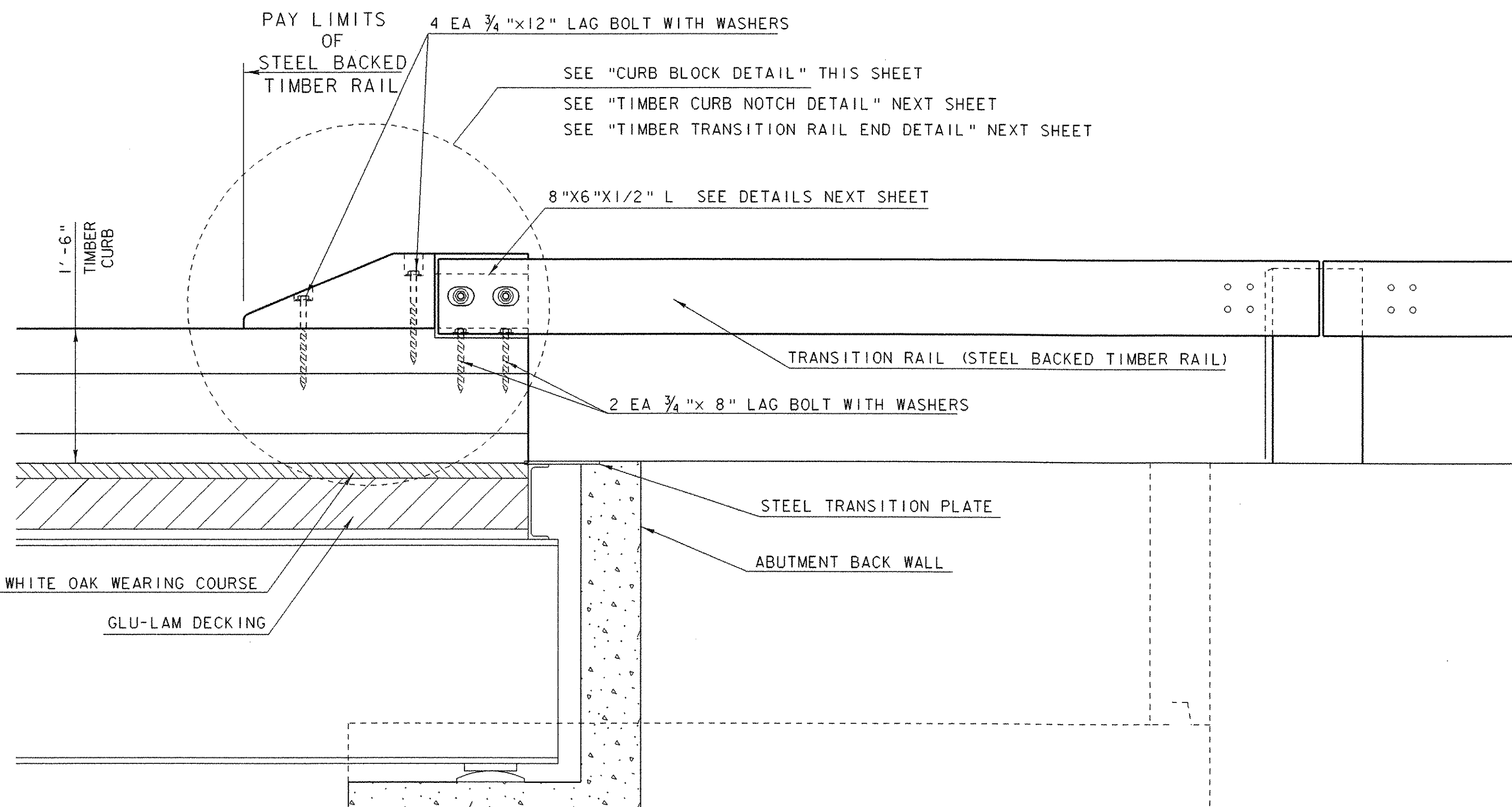
**TRANSITION RAIL TO DECK CURB PLAN VIEW**

SCALE 1"=1'



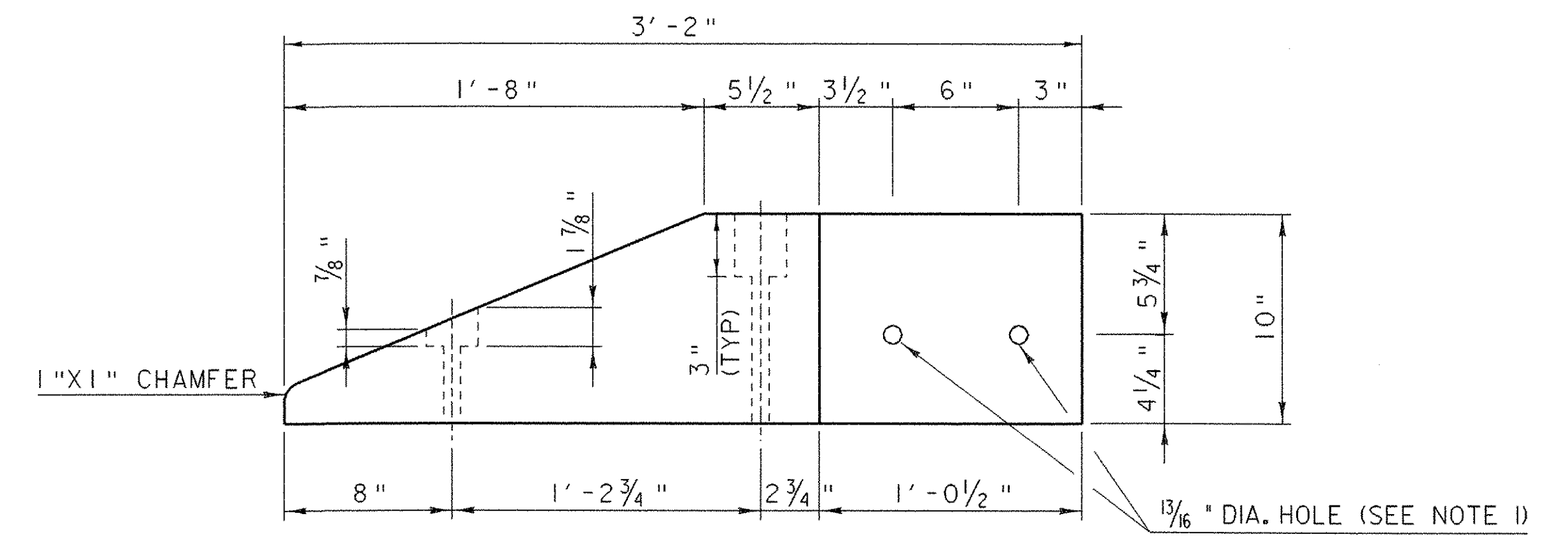
**CURB BLOCK PLAN DETAIL**

SCALE 2"= 1'-0"



**TRANSITION RAIL TO DECK CURB ELEVATION VIEW**

SCALE 1"=1'



**CURB BLOCK ELEVATION DETAIL**

SCALE 2"=1'-0"

**CURB BLOCK DETAILS**

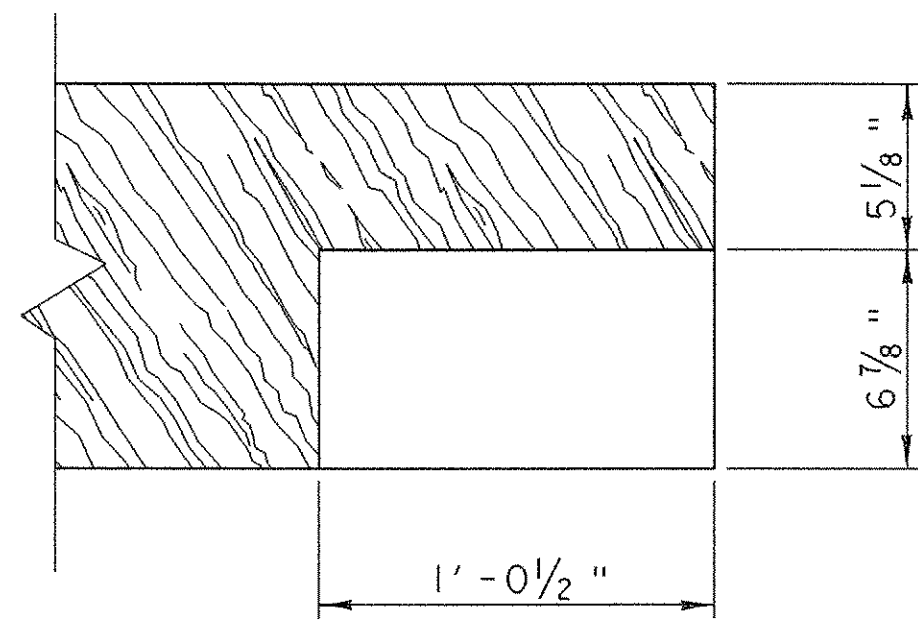
NOTE:  
PILOT HOLES ARE TO BE DRILLED INTO THE TIMBER CURB FOR ATTACHMENT OF THE CURB BLOCK AND 8"X6"X1/2" L SHAPE. THESE PILOT HOLES WILL BE 1/2" DIAMETER HOLES.

NOTE 1:  
ALL INDICATED DRILLED HOLE DIAMETERS OF 3/4" WILL BE 13/16", UNLESS OTHERWISE NOTED.

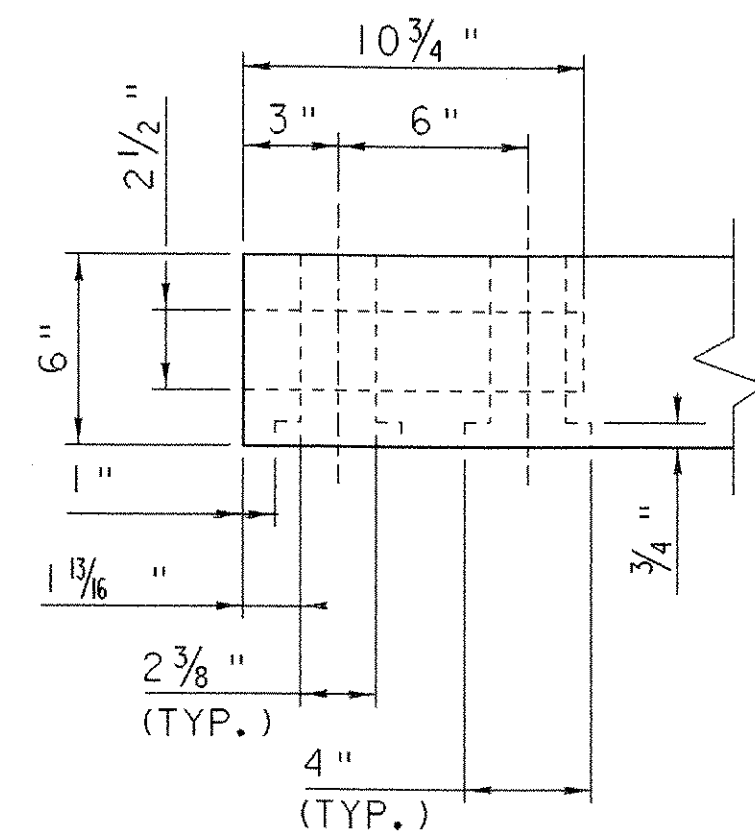
**STEEL BACKED TIMBER TRANSITION RAIL TO TIMBER BRIDGE CURB**

PROJECT NAME: THETFORD	PLOT DATE: 24-OCT-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G. ROKES
FILE NAME: Structures\s03j036abt.dgn	CHECKED BY: S. SCRIBNER
PROJECT LEADER: M. EVANS-MONGEON	SHEET 40 OF 60
DESIGNED BY: G. ROKES	
IPARM: s03j036gbkl1	

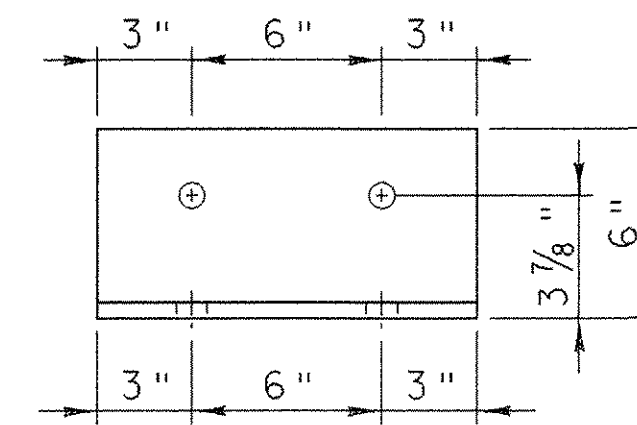
**REVISED OCT. 24, 2006**



**TIMBER CURB NOTCH  
PLAN VIEW**

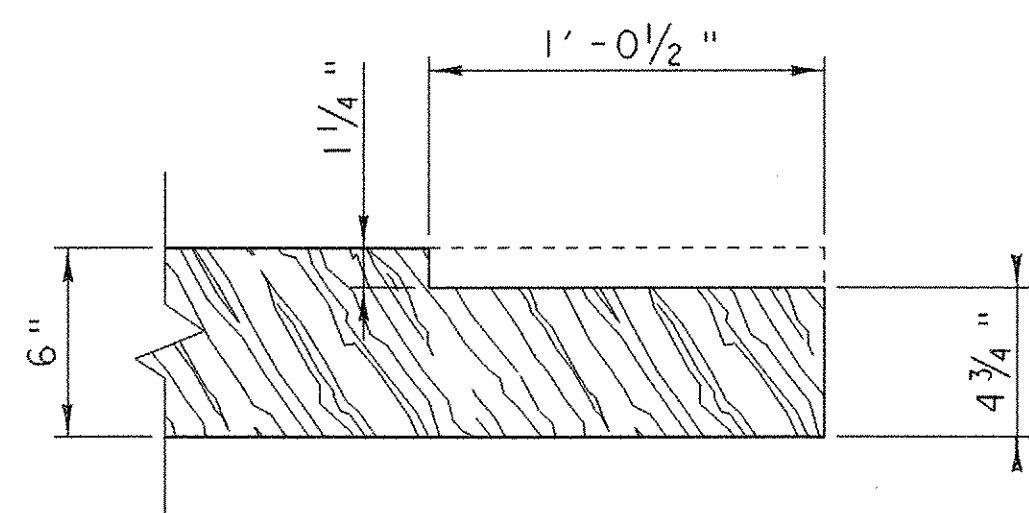


**TIMBER TRANSITION RAIL END  
PLAN VIEW**

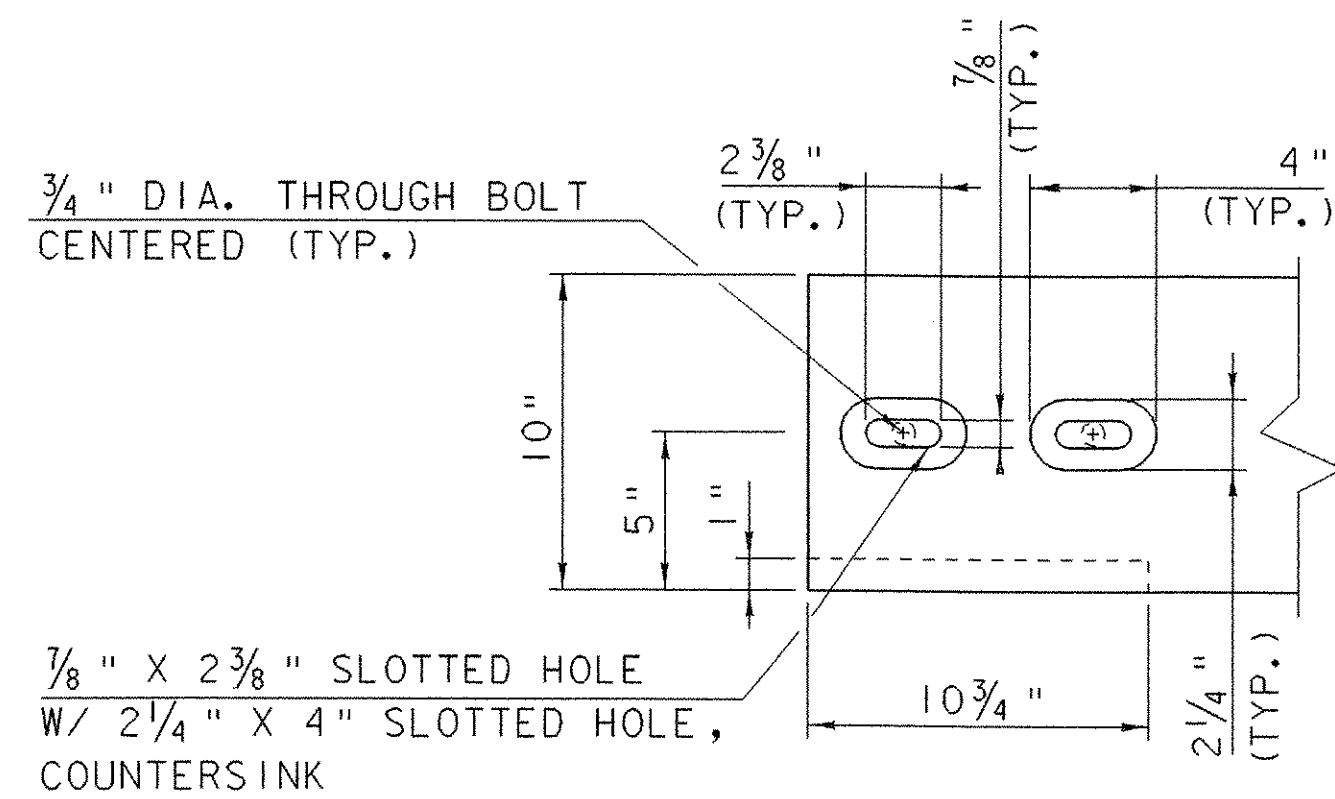


**8x6x1/2 STEEL ANGLE  
TRANSITION RAIL GUIDE  
PLAN VIEW**

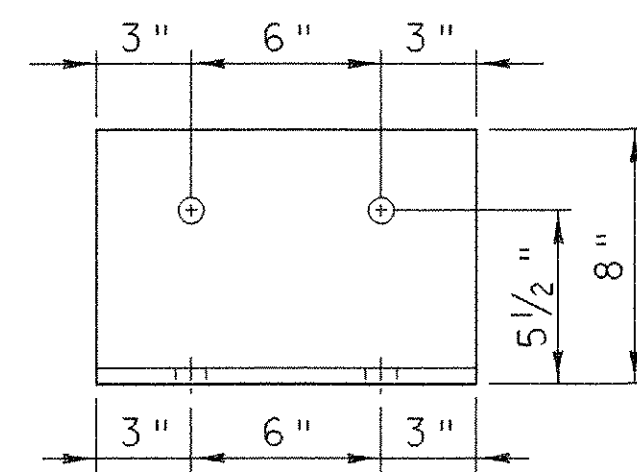
NOTE:  
PILOT HOLES ARE TO BE DRILLED INTO THE  
TIMBER CURB FOR ATTACHMENT OF THE CURB  
BLOCK AND 8"X6"X1/2" L SHAPE. THESE PILOT  
HOLES WILL BE 1/2" DIAMETER.



**TIMBER CURB NOTCH  
ELEVATION VIEW**

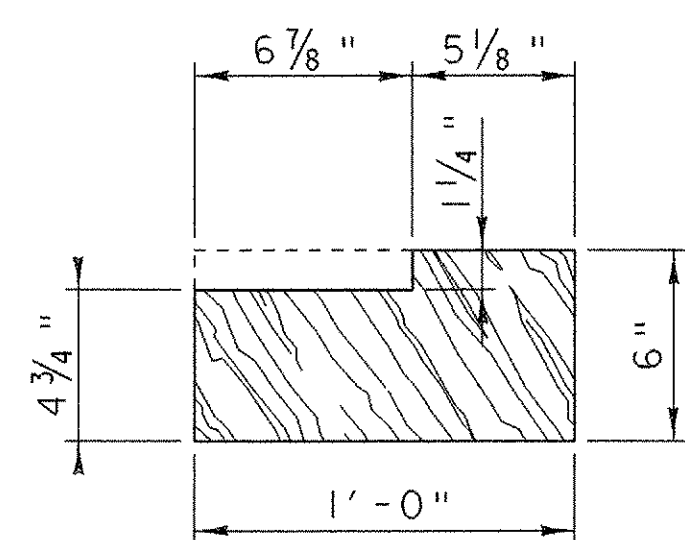


**TIMBER TRANSITION RAIL END  
ELEVATION VIEW**

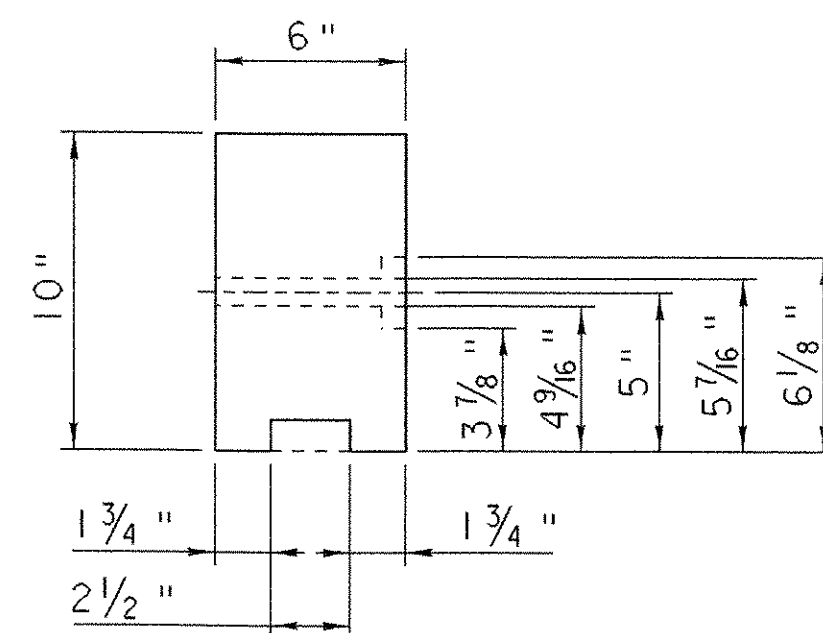


**8x6x1/2 STEEL ANGLE  
TRANSITION RAIL GUIDE  
ELEVATION VIEW**

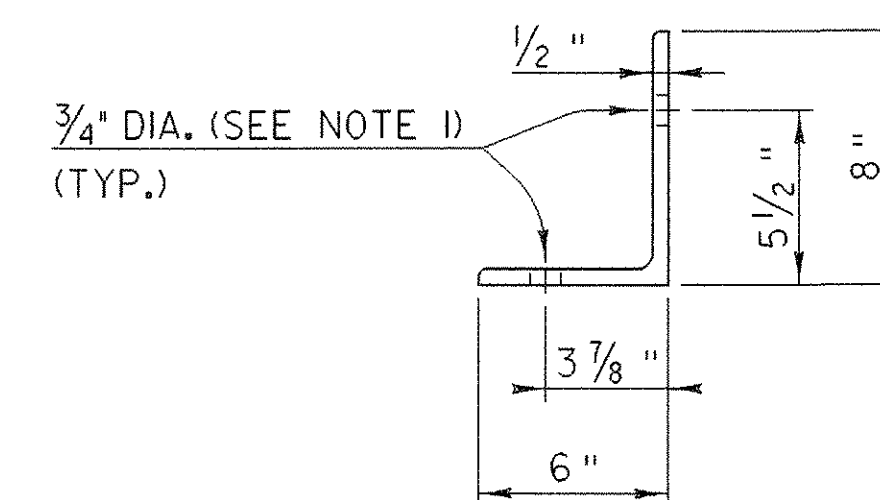
NOTE 1:  
ALL INDICATED DRILLED HOLE DIAMETERS  
WILL BE 5/16", UNLESS OTHERWISE NOTED.



**TIMBER CURB NOTCH  
END VIEW**



**TIMBER TRANSITION RAIL END  
END VIEW**



**8x6x1/2 STEEL ANGLE  
TRANSITION RAIL GUIDE  
END VIEW**

**TIMBER CURB  
NOTCH DETAILS**

**TIMBER TRANSITION RAIL  
END DETAILS**

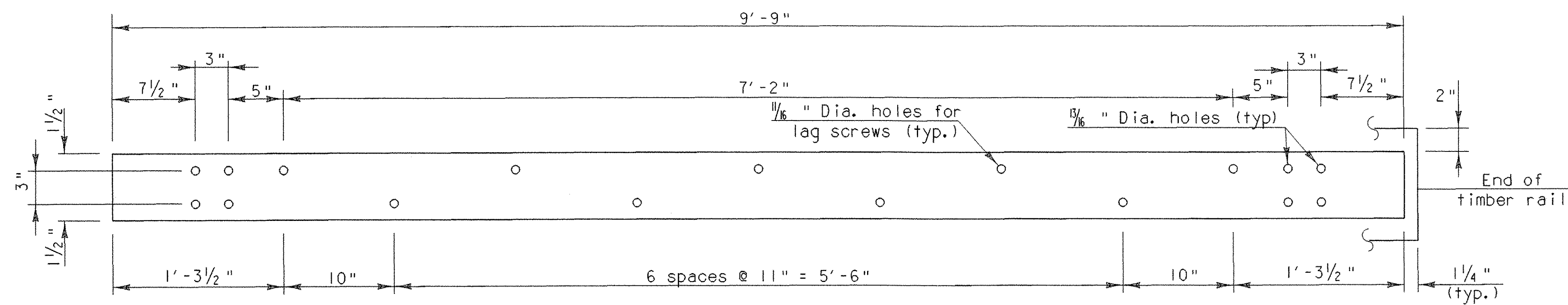
**8x6x1/2 STEEL ANGLE TRANSITION  
RAIL GUIDE DETAILS**

**STEEL BACKED TIMBER TRANSITION RAIL  
TO TIMBER BRIDGE CURB DETAILS**

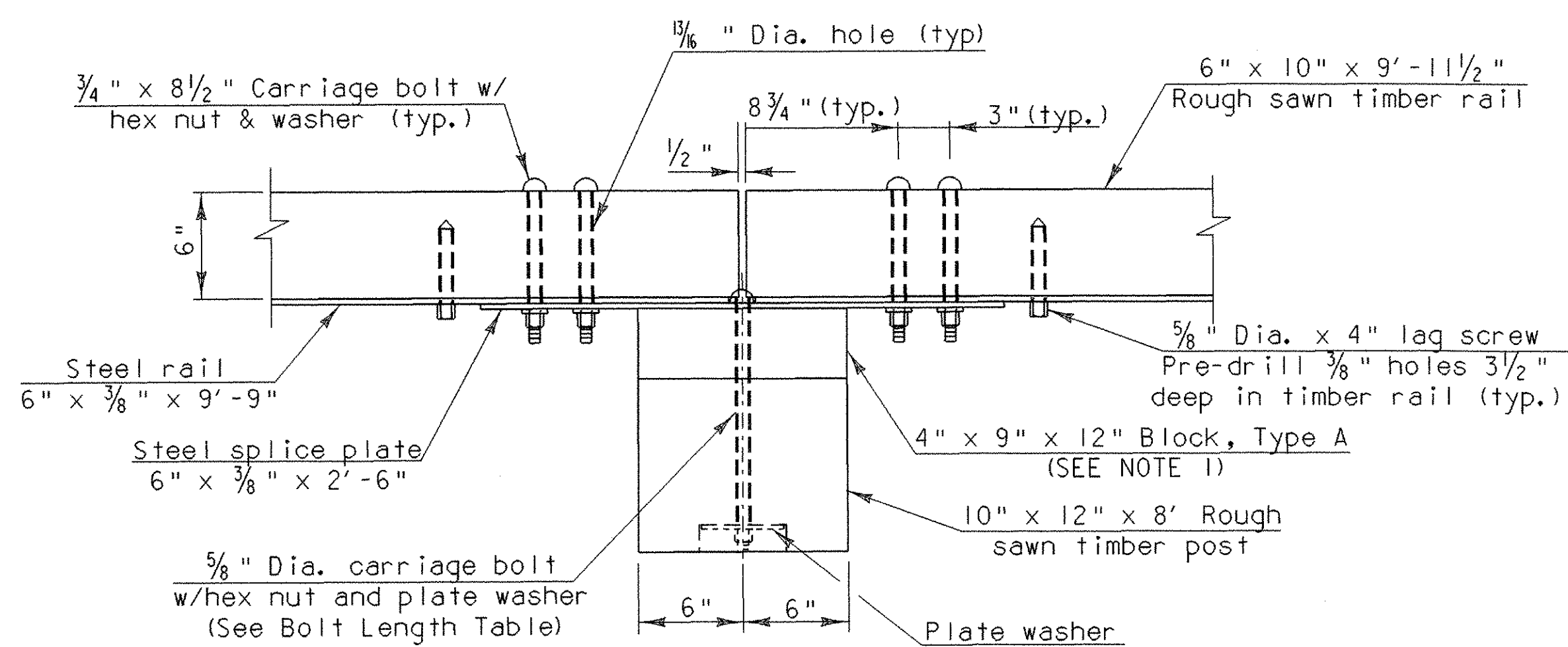
PROJECT NAME: THETFORD  
PROJECT NUMBER: BHO 1444 (43)

FILE NAME: Structures\s03j036abt.dgn PLOT DATE: 27-SEP-2006  
PROJECT LEADER: M. EVANS-MONGEON DRAWN BY: G. ROKES  
DESIGNED BY: G. ROKES CHECKED BY: S. SCRIBNER  
IPARM: s03j036blk2.1 SHEET 41 OF 60

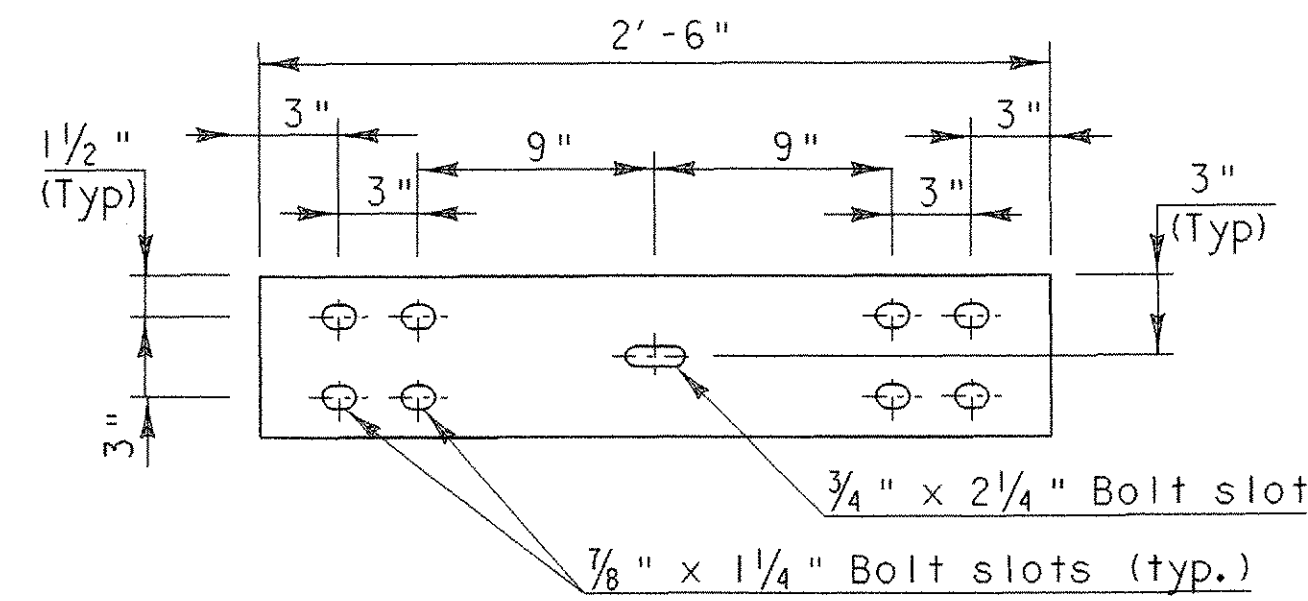
ALL DETAILS ON THIS SHEET ARE NOT TO SCALE



**STEEL RAIL DETAIL**  
6" x 3/8" x 9'-9"



**POST CONNECTION PLAN 180°**



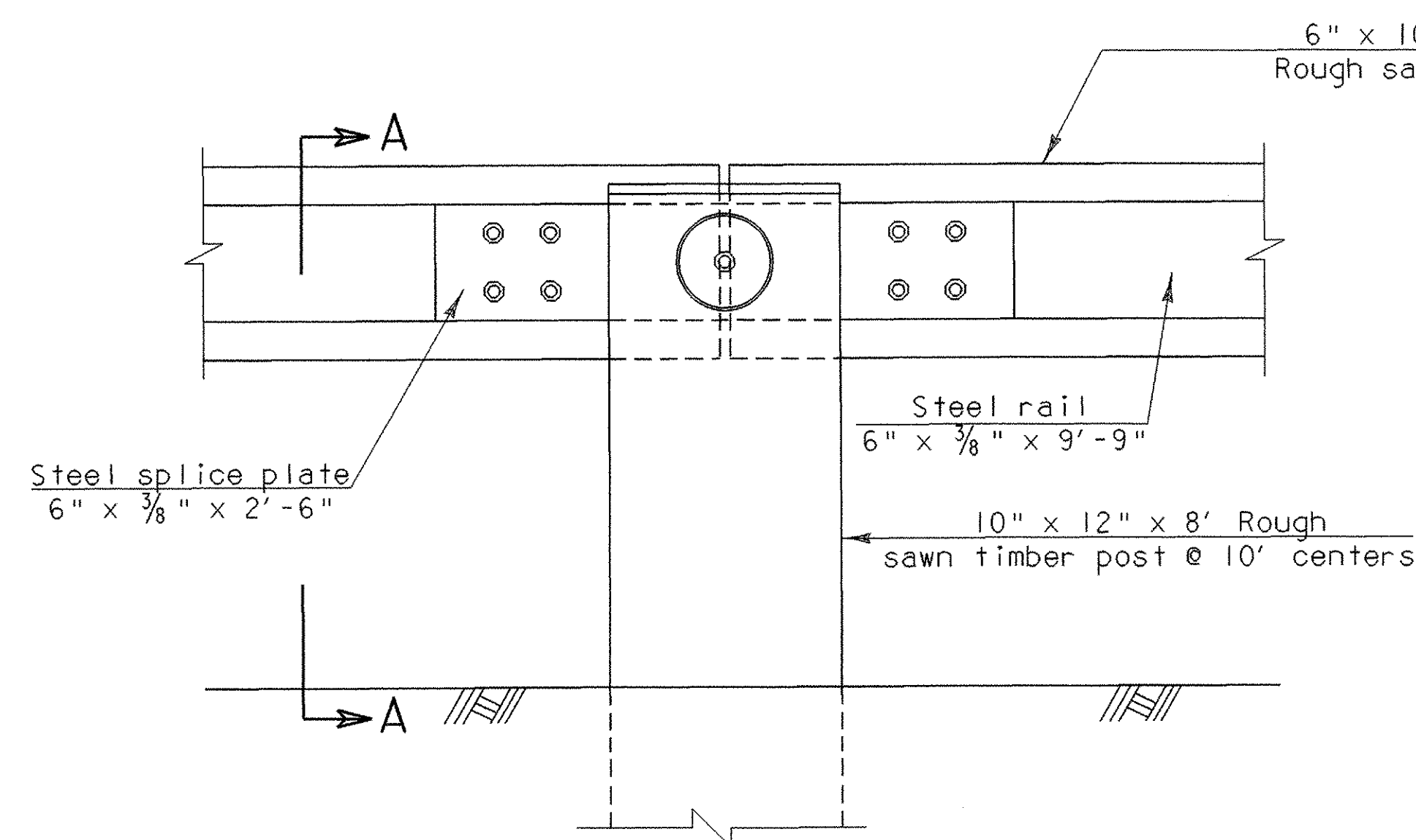
**STEEL SPLICE PLATE DETAIL**  
6" x 3/8" x 2'-6"

**STEEL BACKED TIMBER GUARDRAIL NOTES:**

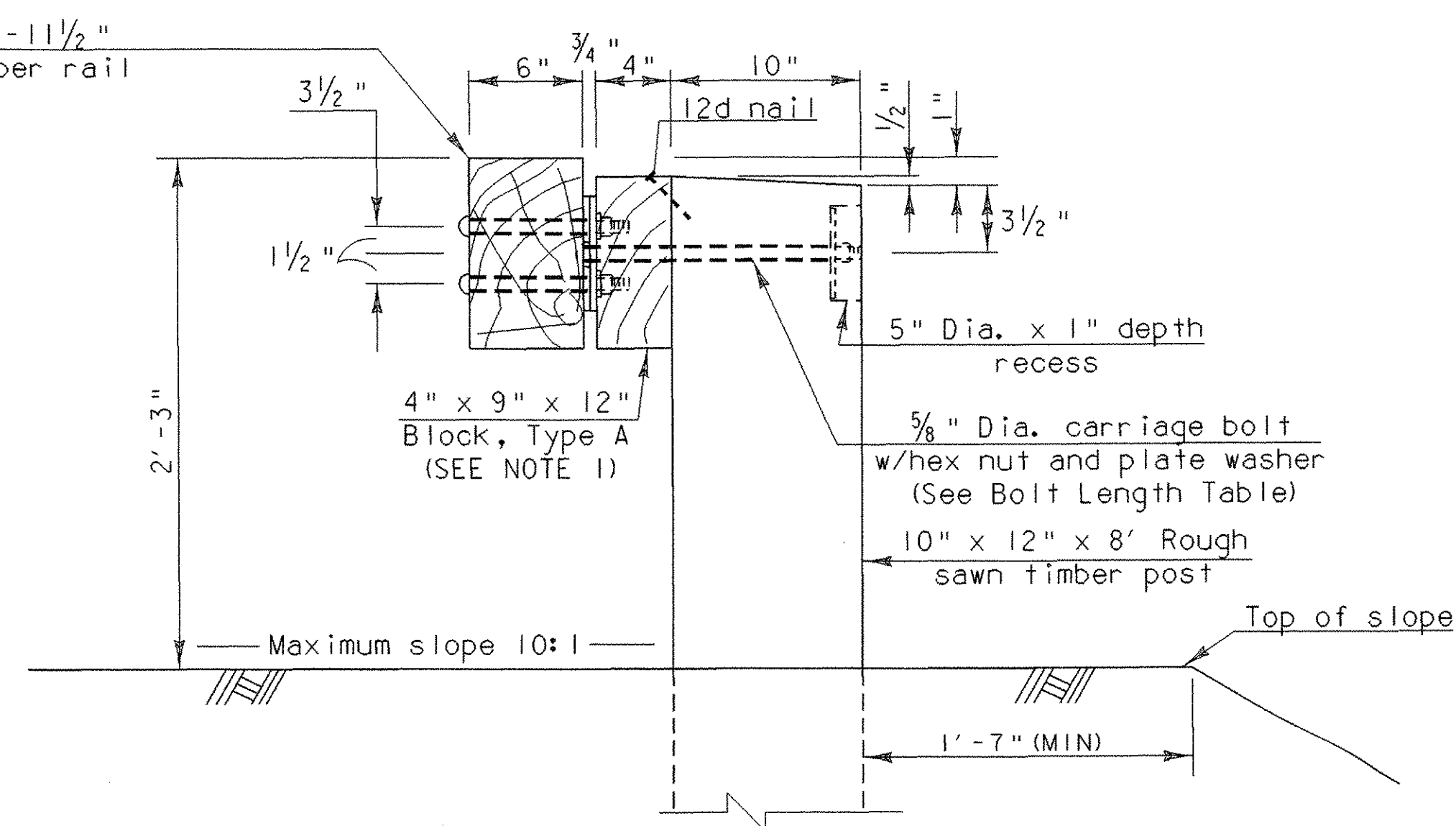
1. RAILING SHALL BE THE TYPE A BLOCK-OUT SYSTEM
2. THE STEEL BACKED TIMBER GUARDRAIL SHALL BE PAID FOR UNDER THE ITEM 621.15 "PLANK RAIL" (MOD. - STEEL BACKED TIMBER RAIL). THE QUANTITY MEASURED FOR PAYMENT SHALL BE ALONG THE FACE OF THE RAIL INCLUDING TERMINAL SECTIONS. THE POST SPACING SHALL BE AS SHOWN ON SHEET 39.
3. THE POSTS AND THE TIMBER RAIL ELEMENTS SHALL CONFORM TO VAOT STANDARD SPECIFICATIONS SECTION 728.01 AND 728.02. IN ADDITION ALL TIMBER USED IN THE RAIL SHALL HAVE A MINIMUM ALLOWABLE BENDING STRESS OF 1450 PSI.
4. THE STEEL RAILS AND SPLICE PLATES SHALL CONFORM TO AASHTO M 222M/M 222 (WEATHERING STEEL).
5. ALL BOLTS AND LAG SCREWS SHALL CONFORM TO ASTM A 307 GR. A. ALL WASHERS SHALL CONFORM TO ASTM F 844. ALL NUTS SHALL CONFORM TO ASTM A 563. ALL FASTENER HARDWARE SHALL BE GALVANIZED ACCORDING TO AASHTO M232.
6. IF AN IMPENETRABLE HOLE IS ENCOUNTERED WHILE PLACING POSTS, ENLARGE THE HOLE TO PROVIDE NOT LESS THAN 6 INCHES CLEARANCE ON ALL SIDES, AND TO A MINIMUM DEPTH OF 2'-6". SET THE POST IN CONCRETE TO WITHIN 6 INCHES OF THE TOP OF THE HOLE. BACKFILL AND COMPACT THE REMAINING 6 INCHES WITH AN ACCEPTABLE MATERIAL.
7. DRIVE POSTS INTO PILOT HOLES THAT ARE PUNCHED OR DRILLED. THE DIMENSIONS OF THE PILOT HOLE SHALL NOT EXCEED THE DIMENSIONS OF THE POST BY MORE THAN 1 INCH. SET THE POSTS PLUMB, BACKFILL AND COMPACT.
8. FIELD CUT TIMBER RAILS TO PRODUCE A CLOSE FIT AT ALL JOINTS. TREAT FIELD CUTS WITH 2 COATS OF CHROMATED COPPER ARSENATE.
9. WHERE CONCRETE ANCHORS ARE INSTALLED IN THE TERMINAL SECTIONS, DO NOT CONNECT THE GUARDRAIL TO THE CAST IN PLACE ANCHORS UNTIL THE CONCRETE HAS CURED 7 DAYS.

Type A (Block-out)	Type B (No Block-out)
1'-3"	11"

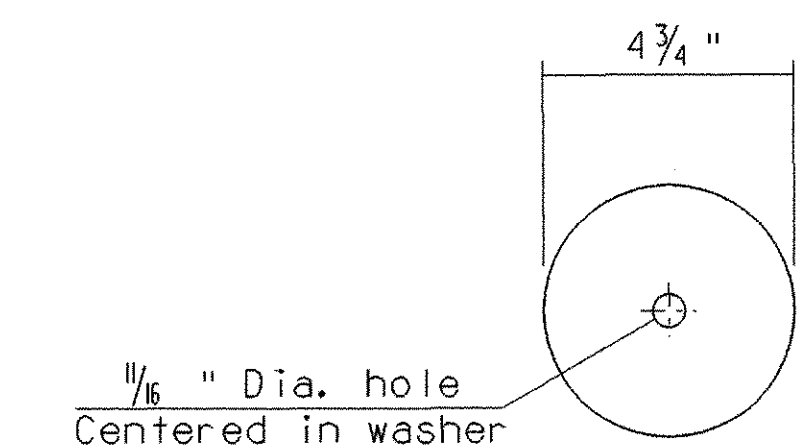
**BOLT LENGTHS  
FOR RAIL/POST CONNECTION**



**POST CONNECTION ELEVATION**



**SECTION A-A**

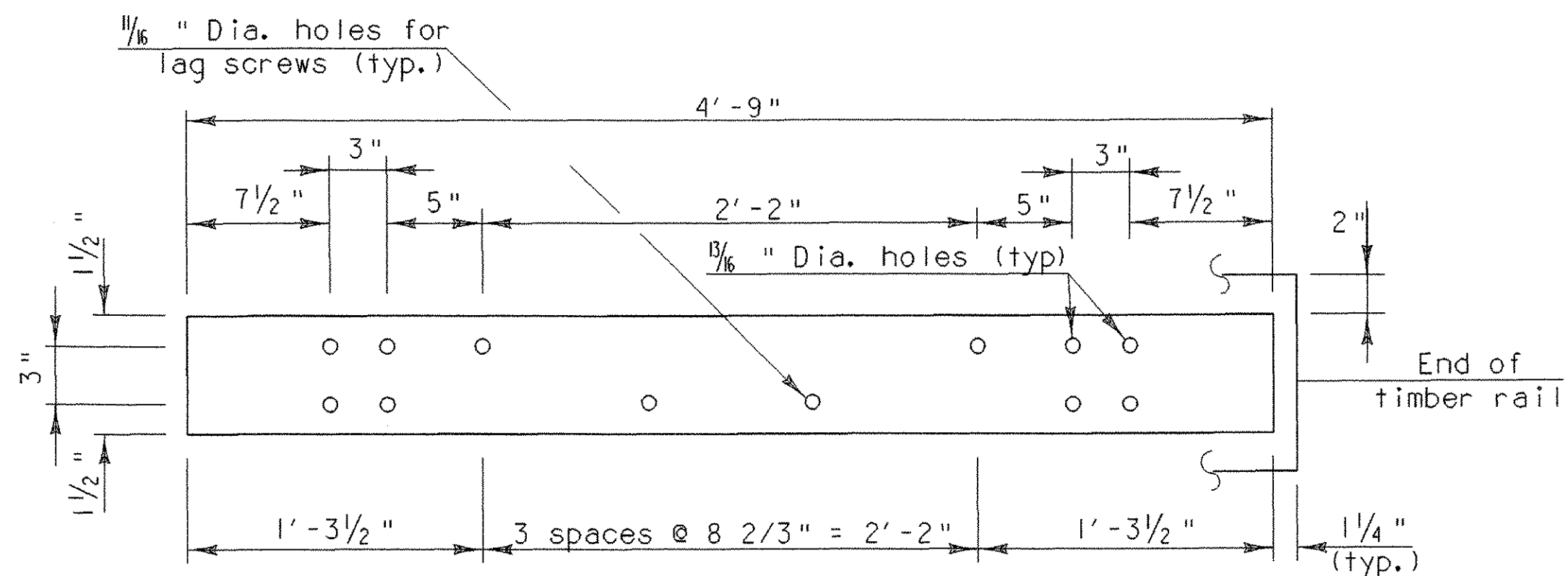


**PLATE WASHER DETAIL**  
4 3/4" Dia. x 1/4"

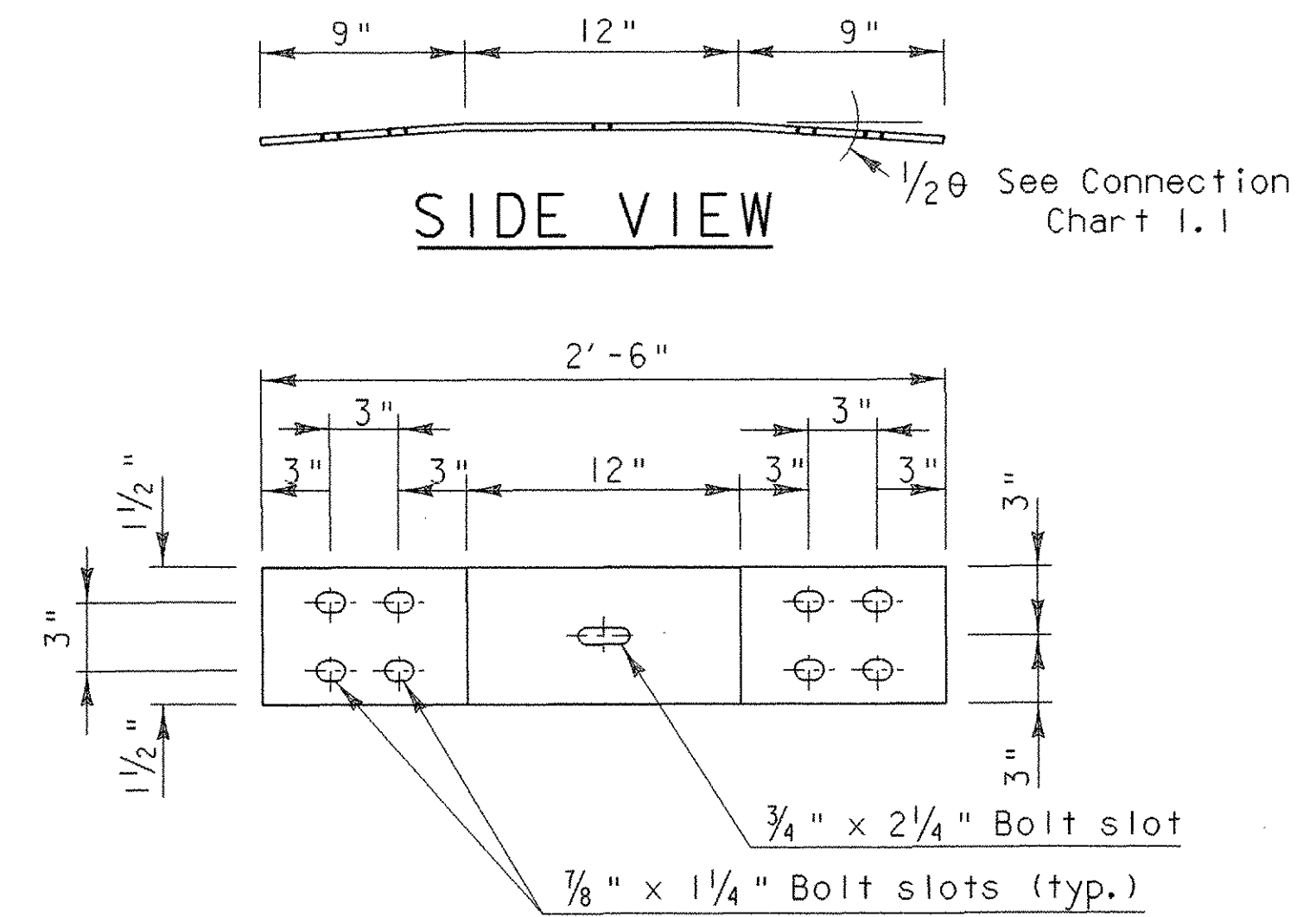
**STEEL BACKED TIMBER RAIL  
STRAIGHT SPLICE PLATE DETAILS**

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G. ROKES
FILE NAME: Structures\s03j036abt.dgn	DESIGNED BY: M. EVANS-MONGEON
PROJECT LEADER: M. EVANS-MONGEON	CHECKED BY: S. SCRIBNER
IPARM: s03j036tr.i	SHEET 42 OF 60

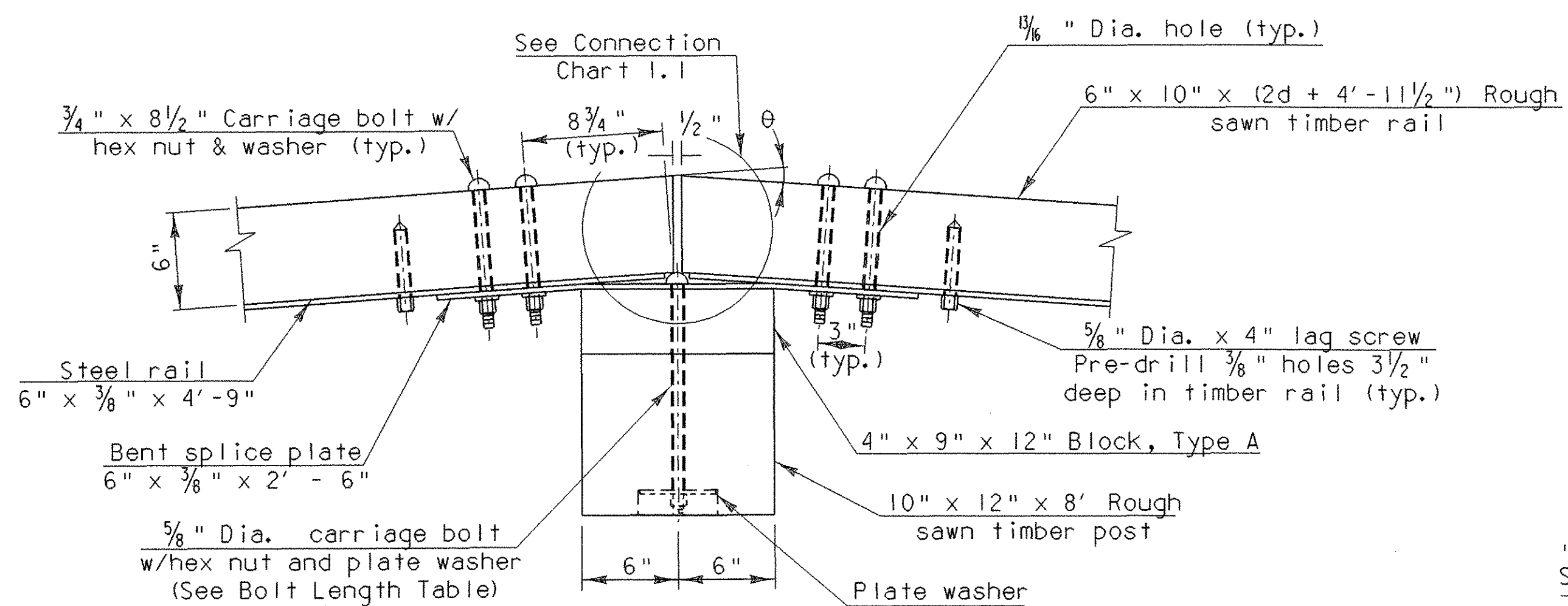
ALL DETAILS ON THIS SHEET ARE NTS



**STEEL RAIL DETAIL**  
6" x 3/8" x 4'-9"



**BENT SPLICE PLATE DETAIL**  
6" x 3/8" x 2'-6"



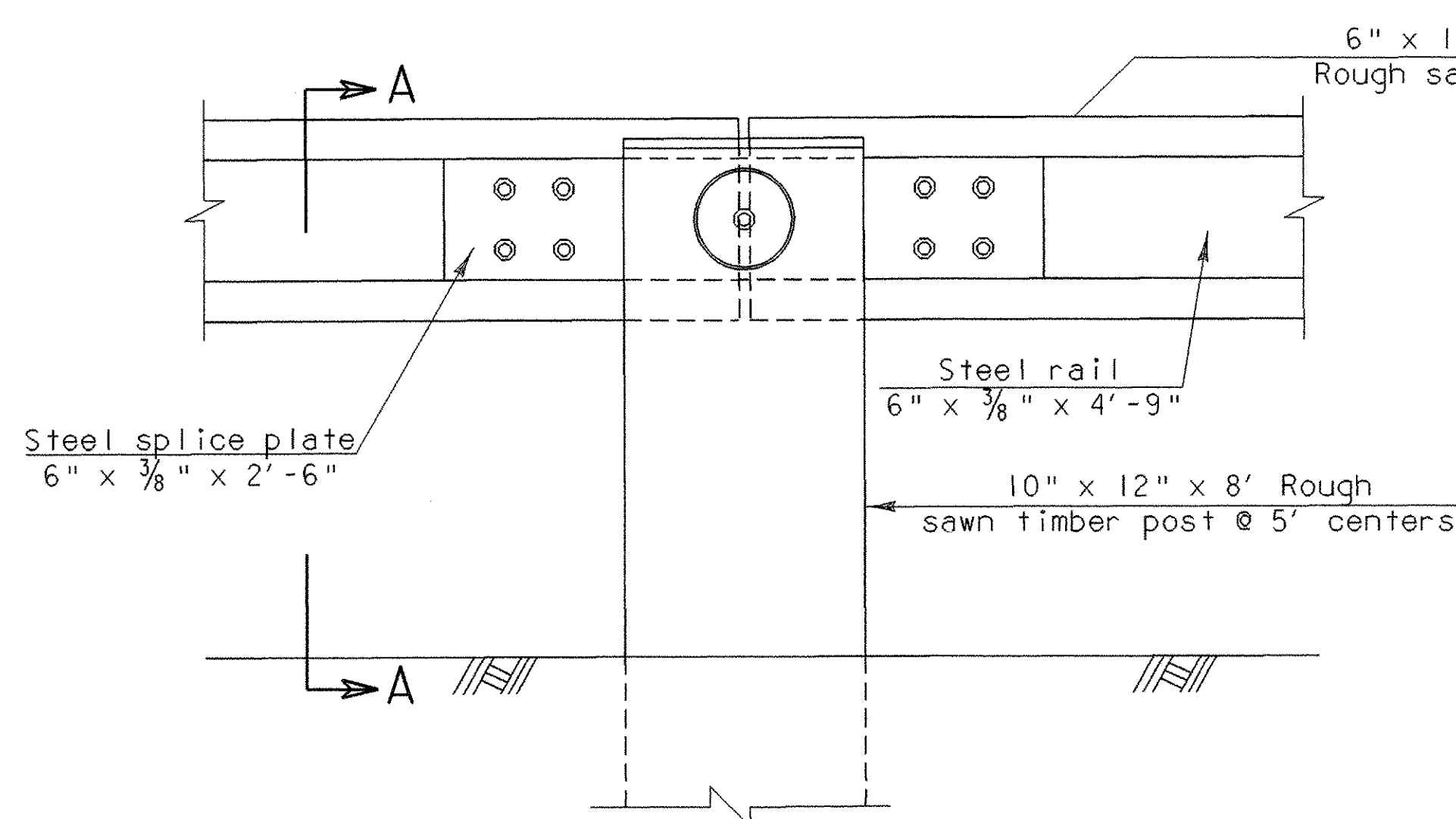
**POST CONNECTION PLAN A-G**

"X" DISTANCE  
SEE CONNECTION CHART 1.1

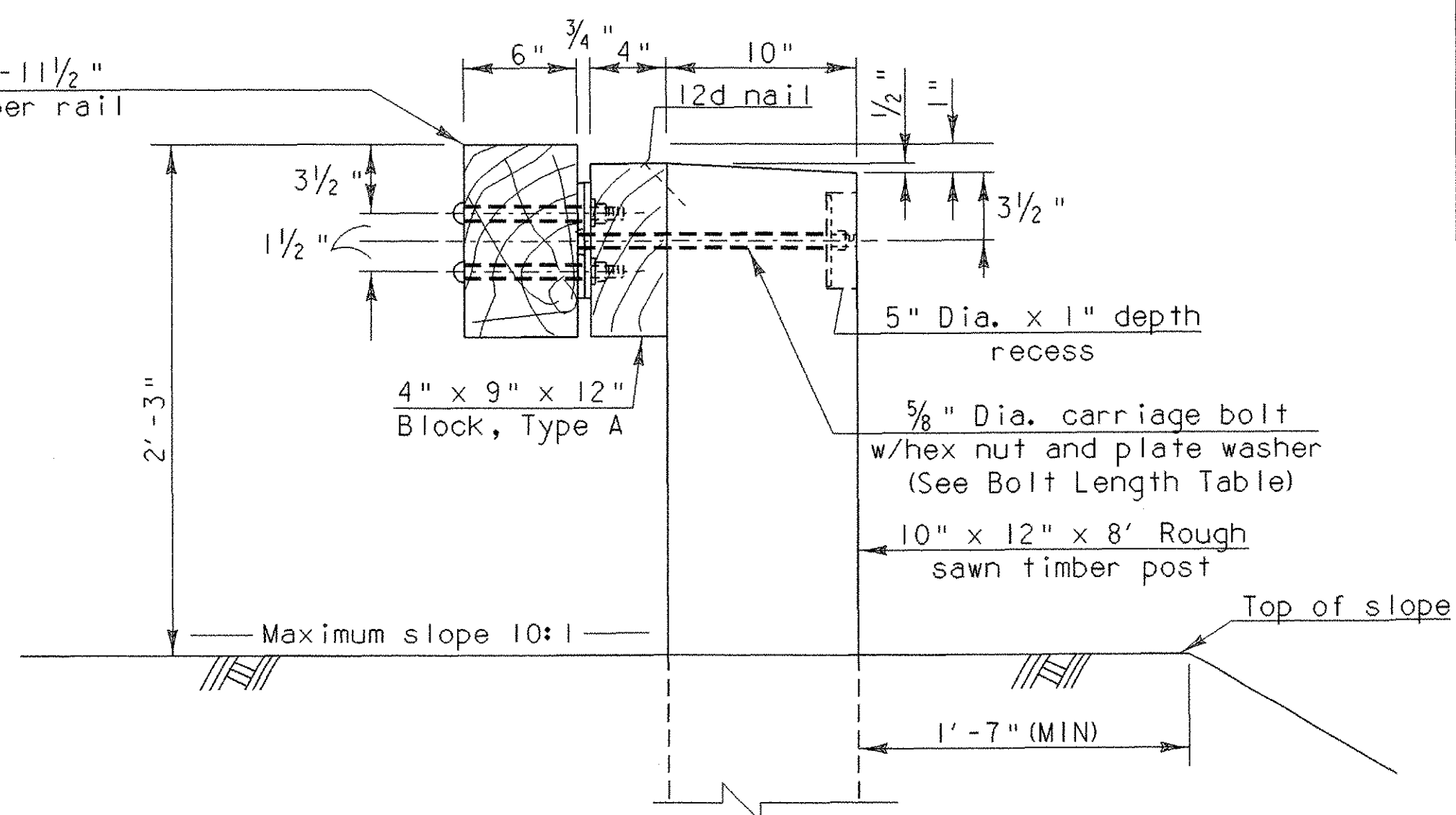
**CUTS ON TIMBER RAIL ENDS  
AT ANGLED CONNECTIONS**

CONNECTION LOCATION / TYPE	$\theta$	$1/2 \theta$	"X" DISTANCE
<b>A</b>	8.7°	4.35°	1/2"
<b>B</b>	11.0°	5.5°	5/8"
<b>C</b>	9.0°	4.50°	1/2"
<b>D</b>	9.0°	4.5°	1/2"
<b>E</b>	8.0°	4.0°	3/8"
<b>F</b>	10.6°	5.3°	5/8"
<b>G</b>	5.0°	2.5°	1/4"

**CONNECTION CHART 1.1**



**POST CONNECTION ELEVATION**

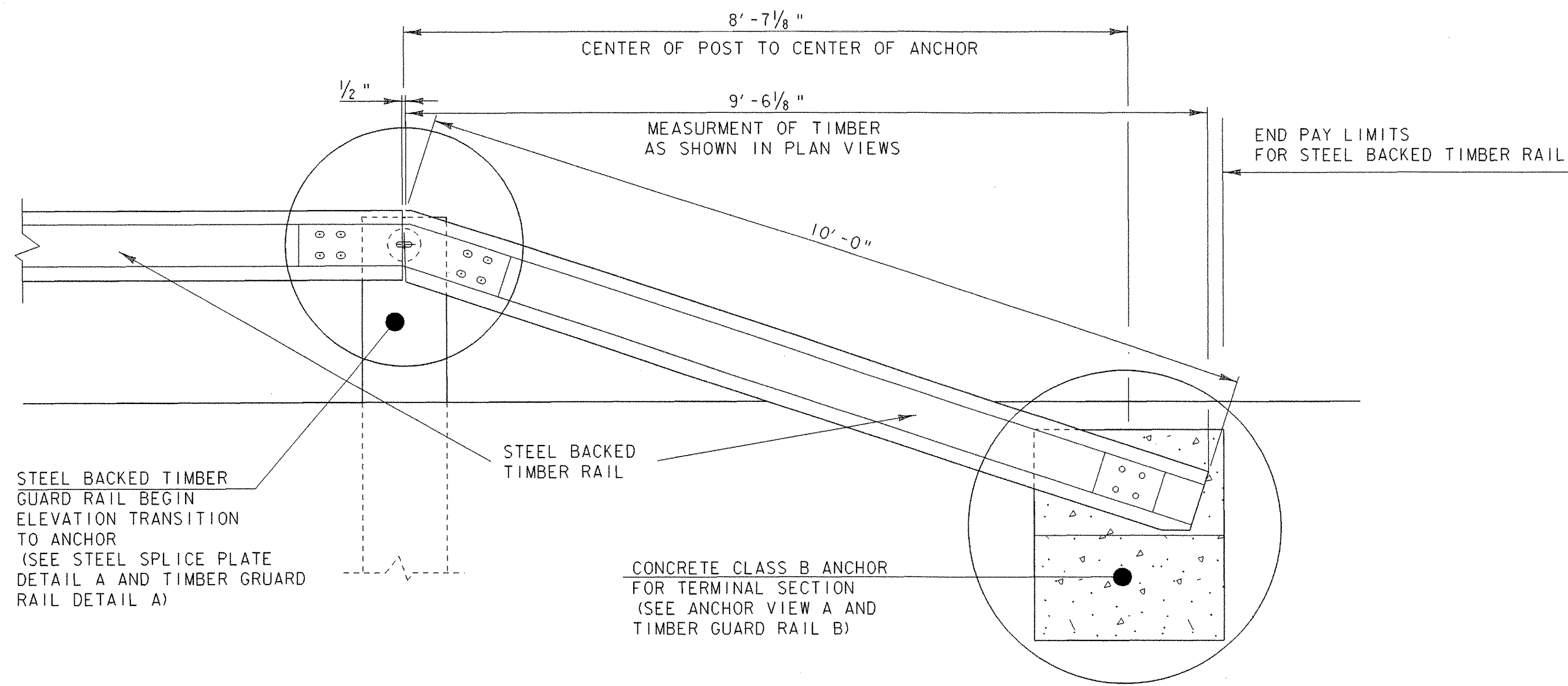


**SECTION A-A**

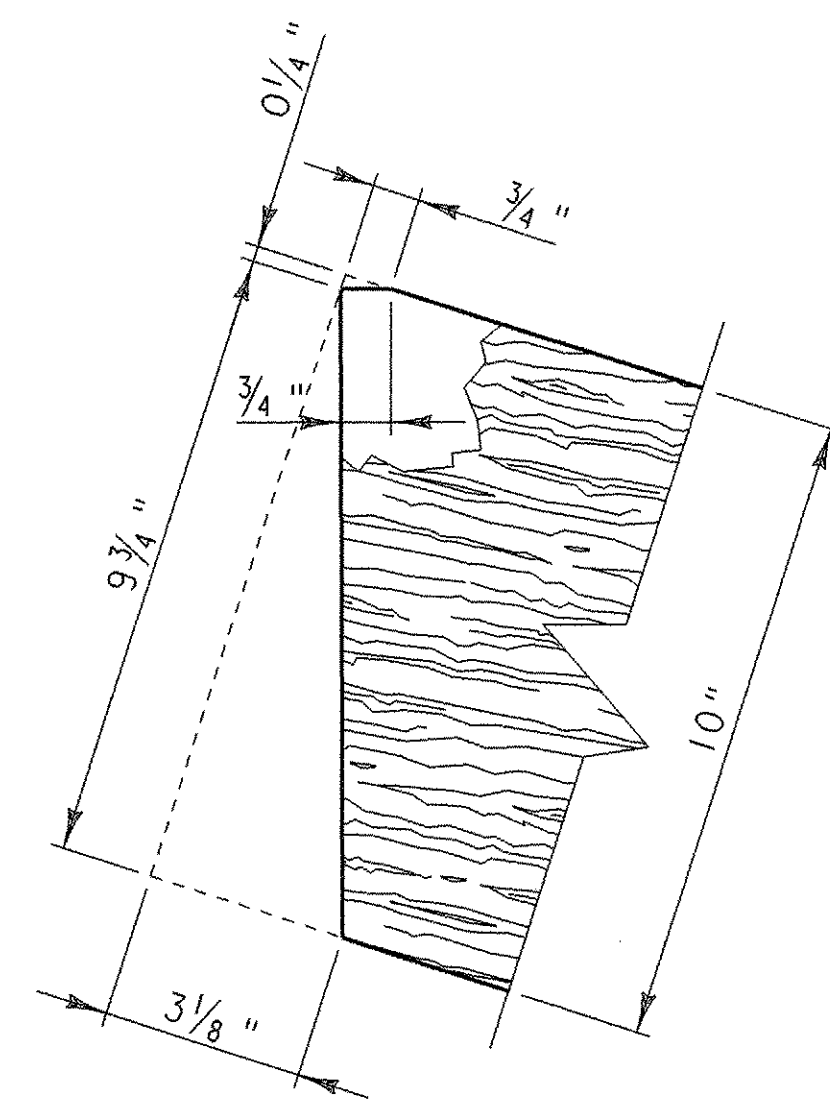
ALL DETAILS ON THIS SHEET ARE NTS

**STEEL BACKED TIMBER RAIL  
ANGLED SPLICE PLATE DETAILS**

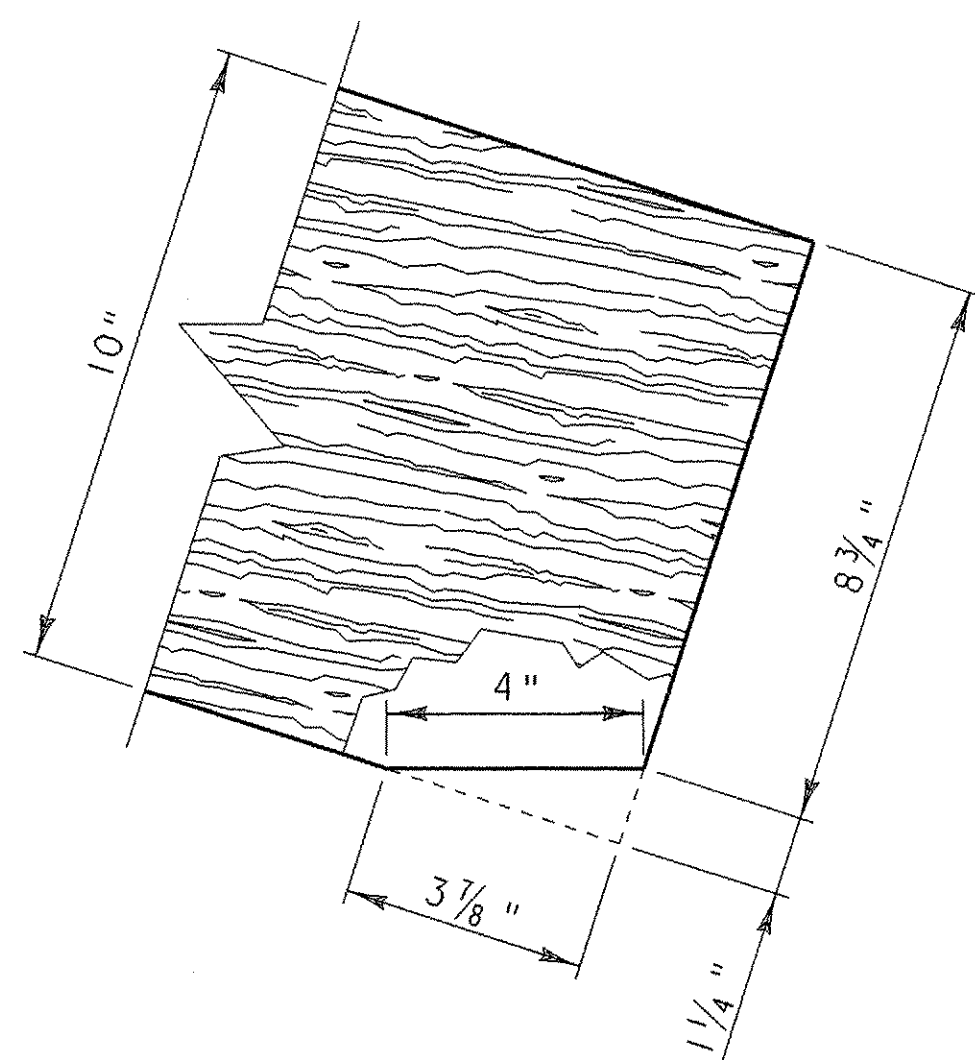
PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G. ROKES
FILE NAME: Structures\s03j036abt.dgn	CHECKED BY: S. SCRIBNER
PROJECT LEADER: M. EVANS-MONGEON	SHEET 43 OF 60
DESIGNED BY: M. EVANS-MONGEON	
IPARM: s03j036tr1.1	



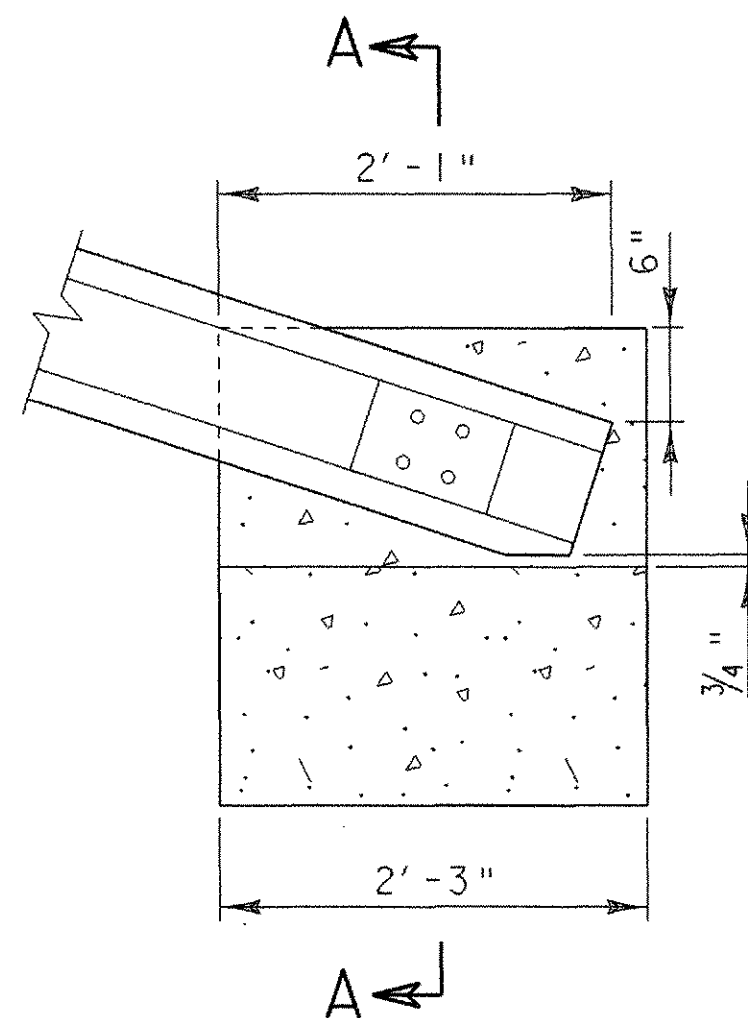
ELEVATION



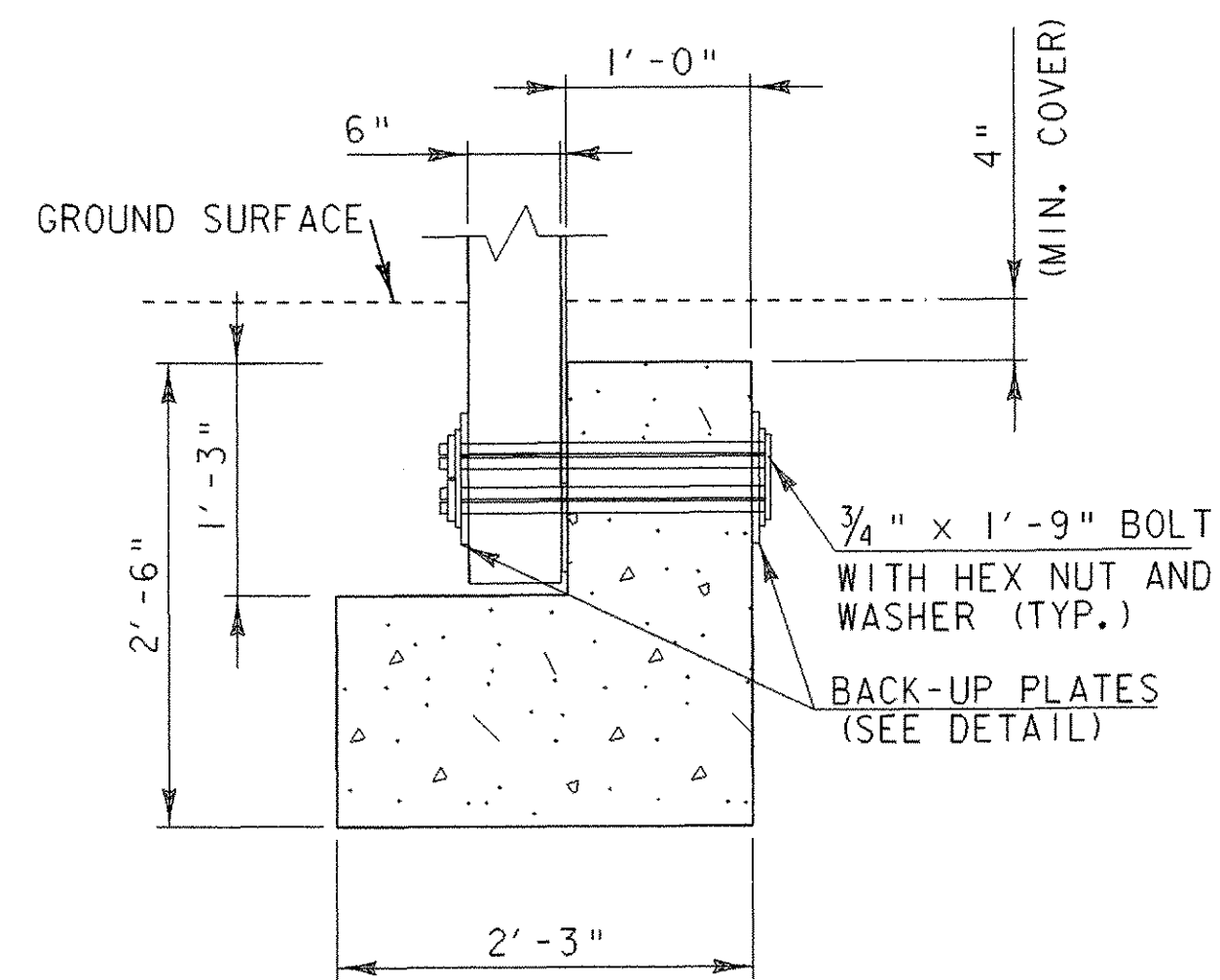
TIMBER GUARD RAIL DETAIL A



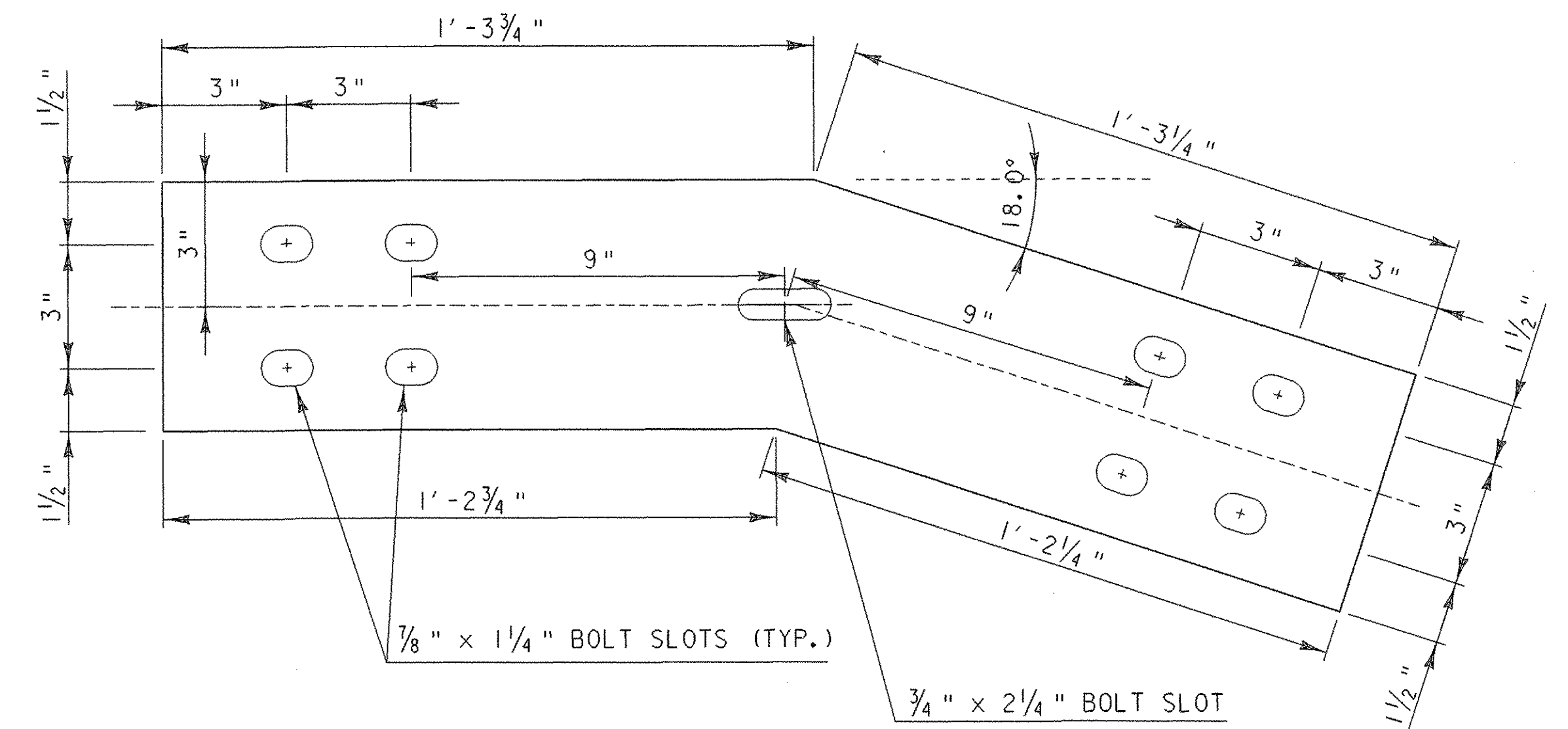
TIMBER GUARD RAIL DETAIL B



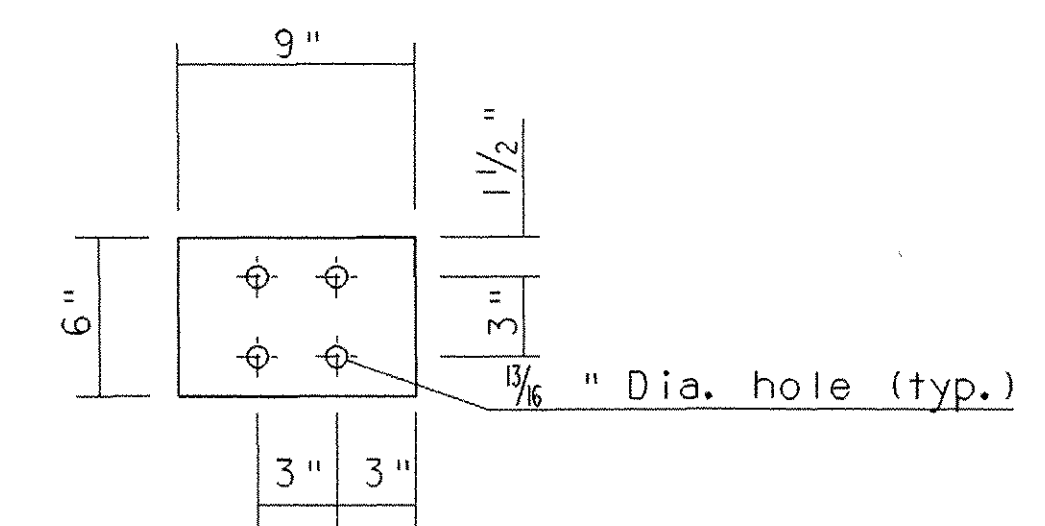
ANCHOR VIEW A



ANCHOR SECTION A-A



STEEL SPLICE PLATE DETAIL A



BACK-UP PLATE DETAIL  
6" x 1/2" x 9"

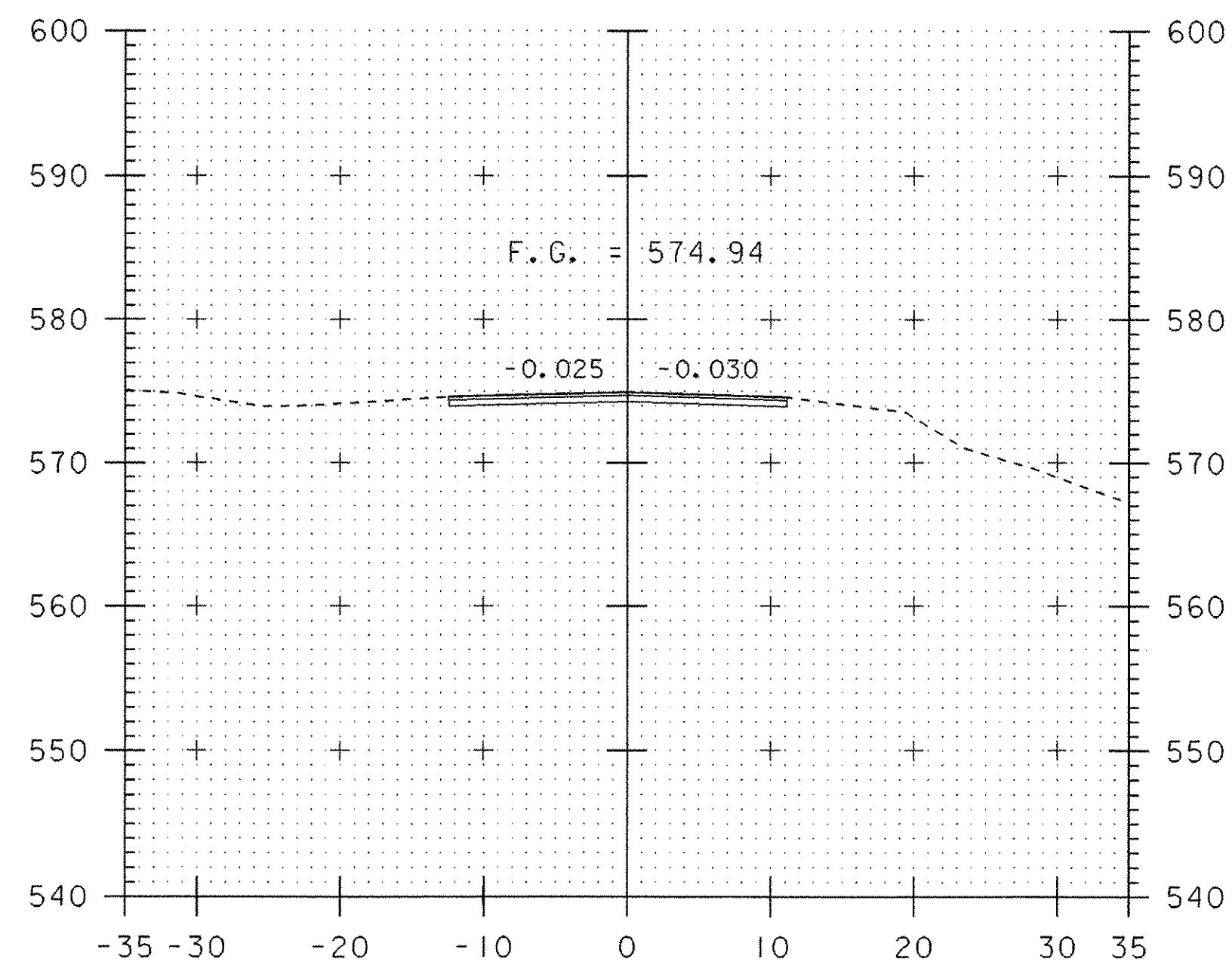
Notes:

1. See sheet 39, Steel-Backed Timber Guardrail Details For 10' Post Spacing, for timber, structural steel, and hardware details.
2. On the Type A, blocked-out guardrail, include the blocks in the terminal section, except on the concrete anchor. For the Type B, non-blocked-out guardrail, no blocks are included.
3. Begin the cut flares at the nearest post to a transition point between fill and cut as directed by the Resident Engineer.
4. Extend the flare into the cut until a minimum 1-foot cover is obtained over the guardrail end.

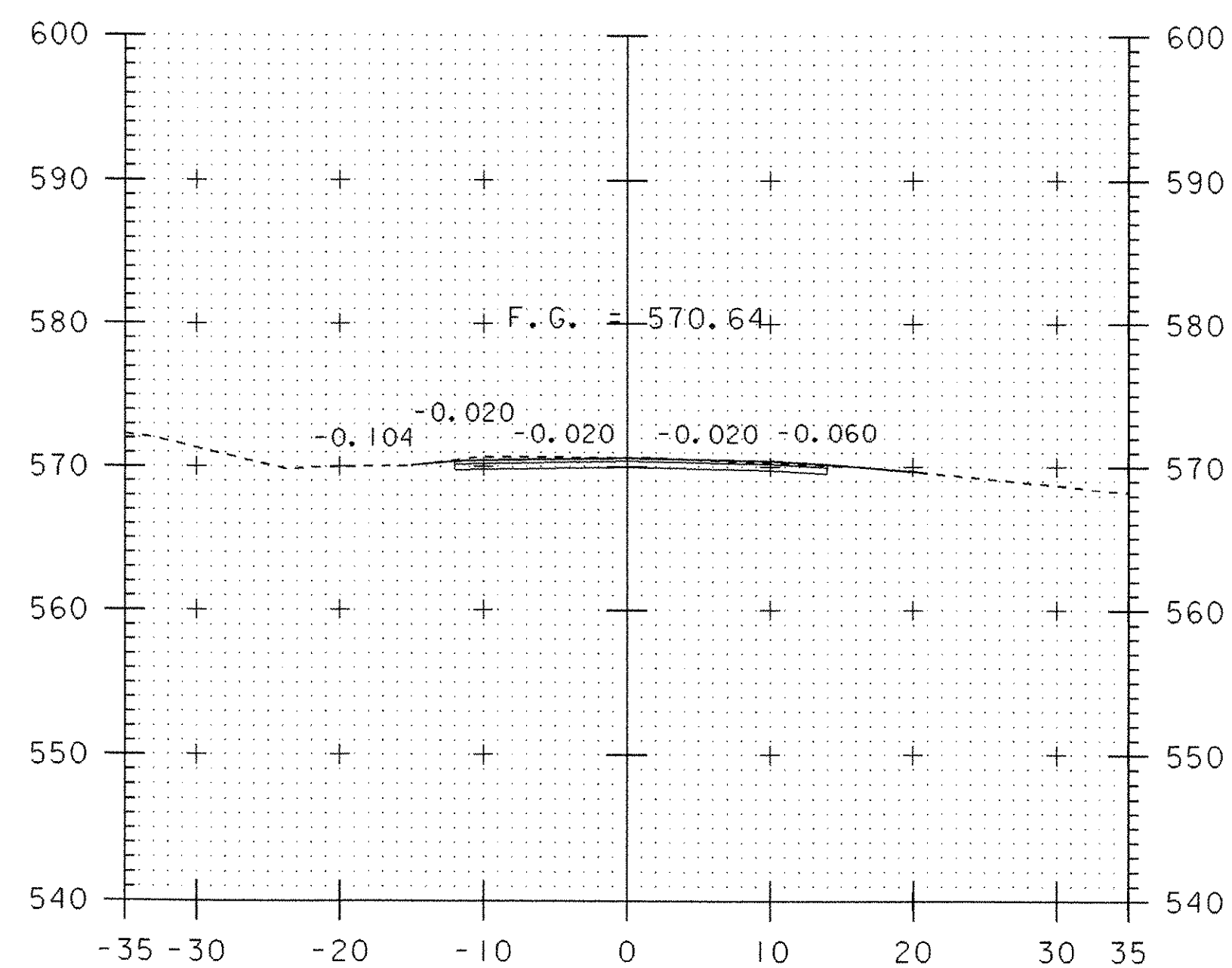
STEEL BACKED TIMBER RAIL ANCHOR DETAILS

PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures\s03j036abt.dgn
PROJECT LEADER:	M. EVANS-MONGEON
DESIGNED BY:	M. EVANS-MONGEON
IPARM:	s03j036tr2.l
PLOT DATE:	27-SEP-2006
DRAWN BY:	G. ROKES
CHECKED BY:	S. SCRIBNER
SHEET	44 OF 60

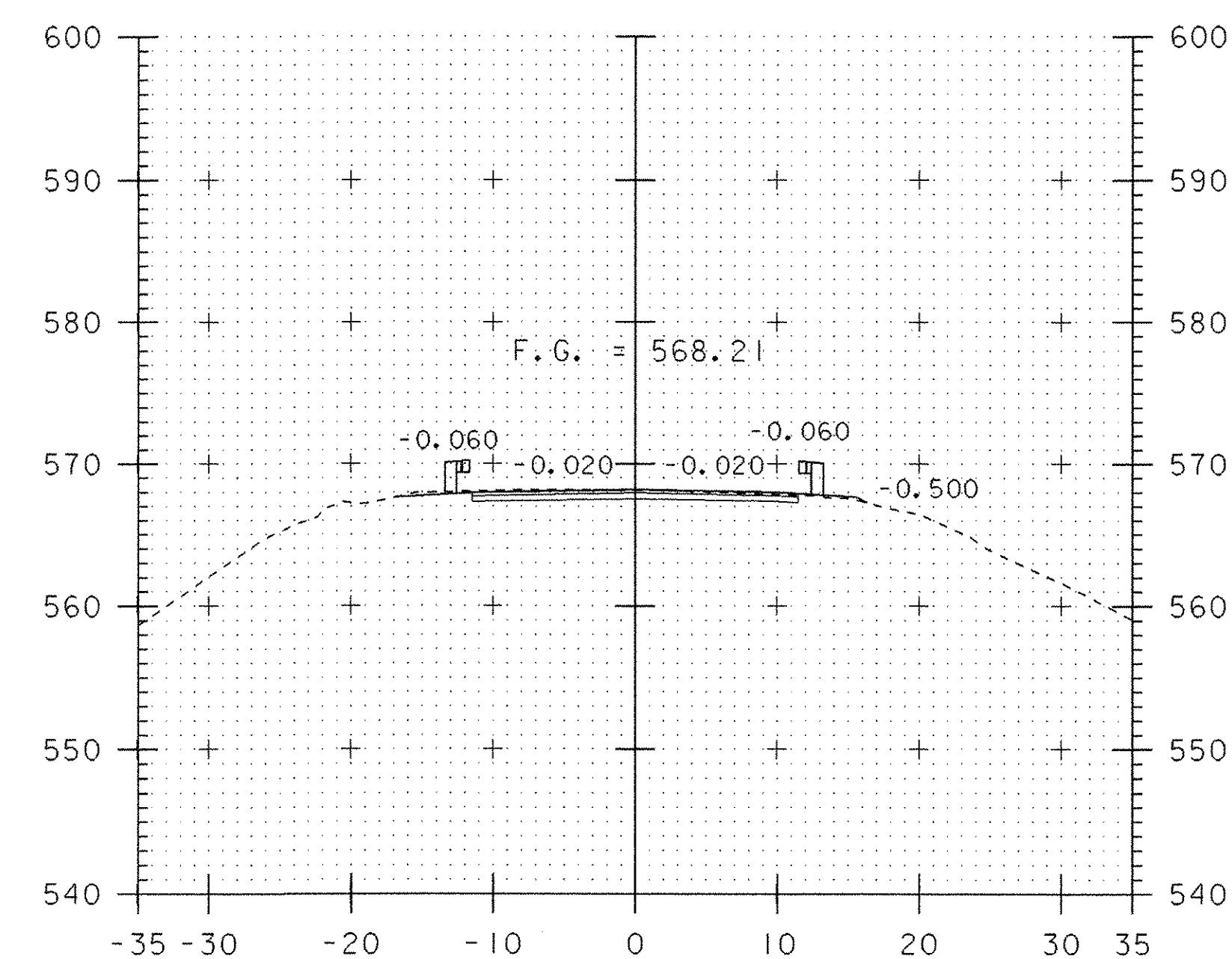
ALL DETAILS ON THIS SHEET ARE NTS



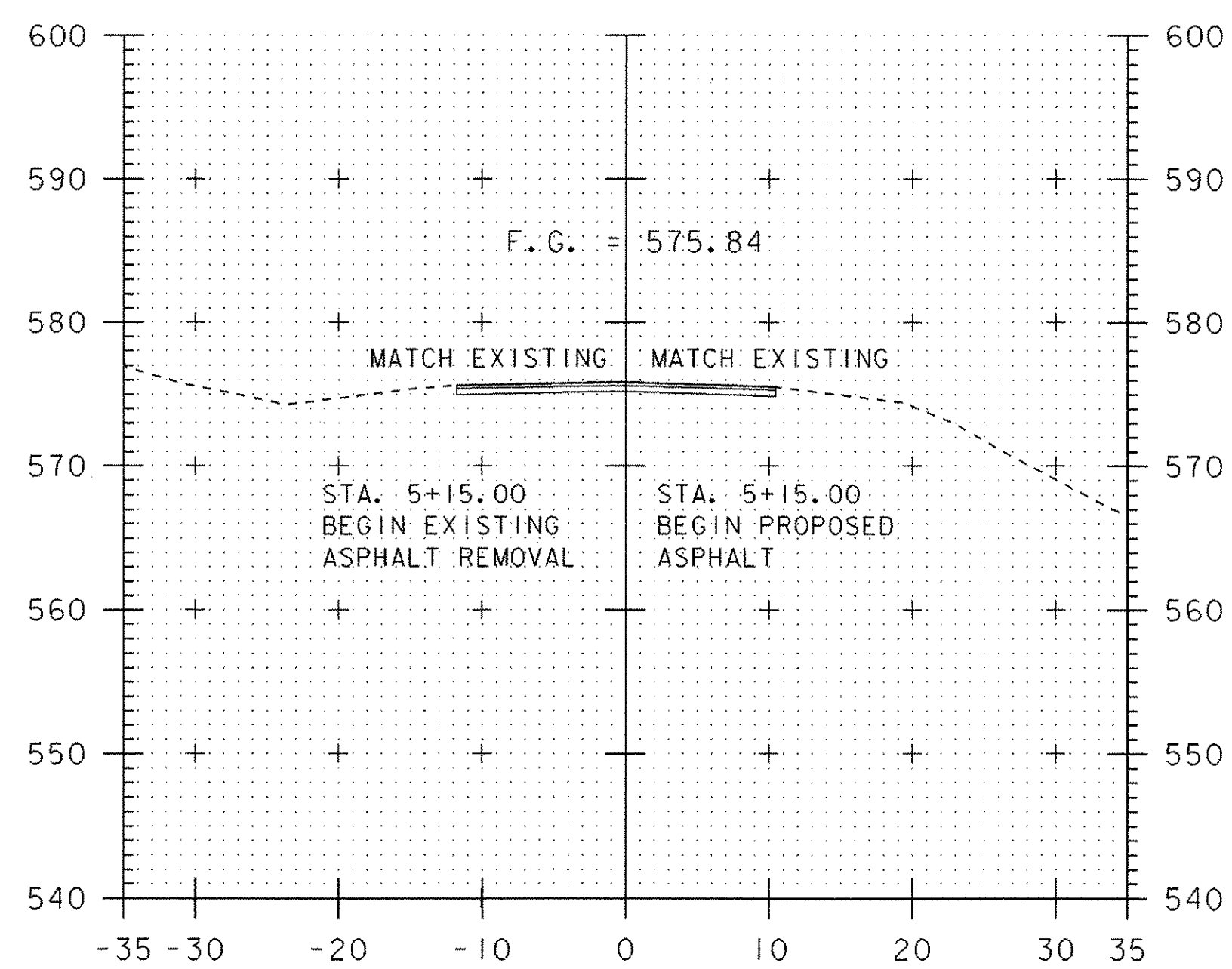
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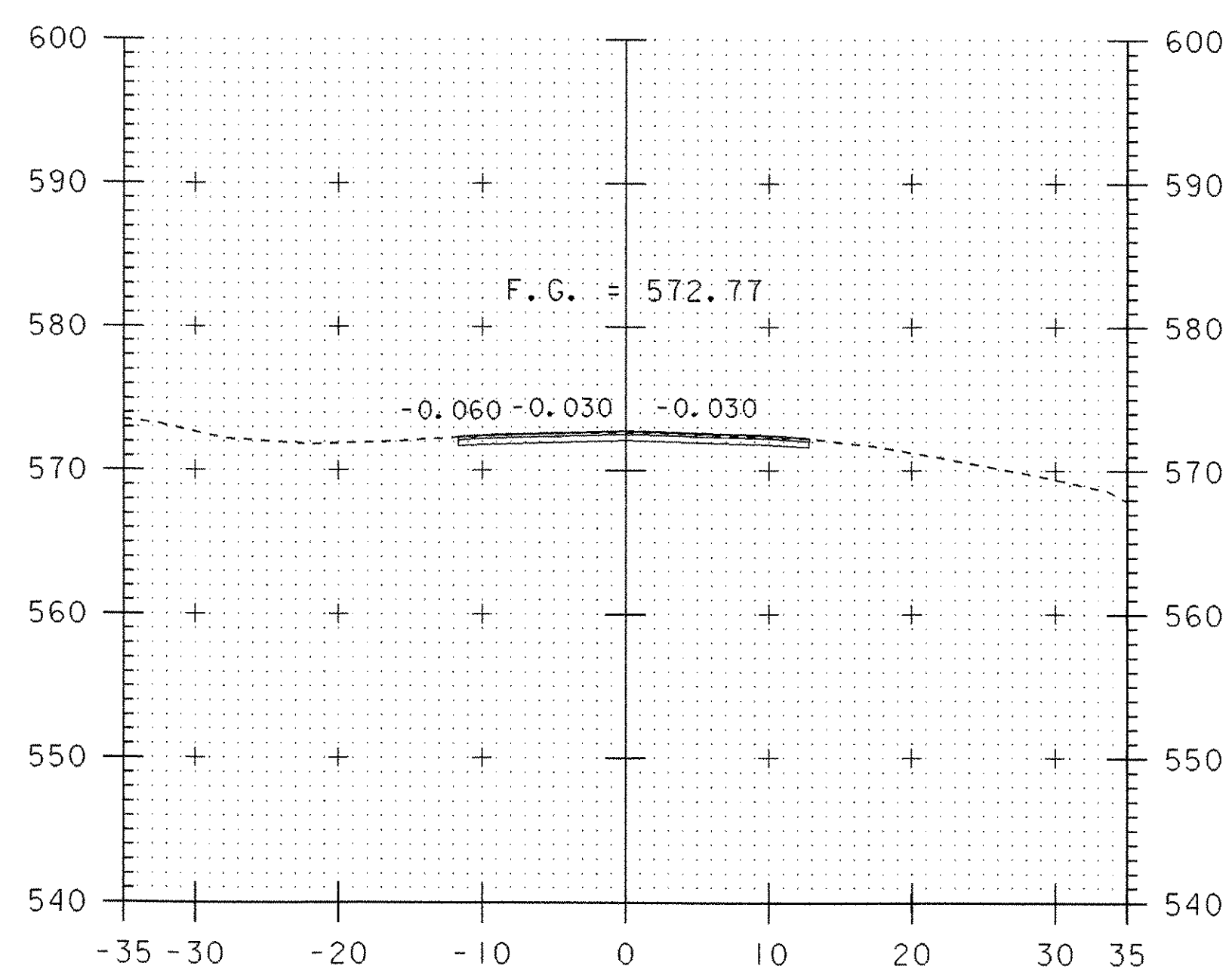
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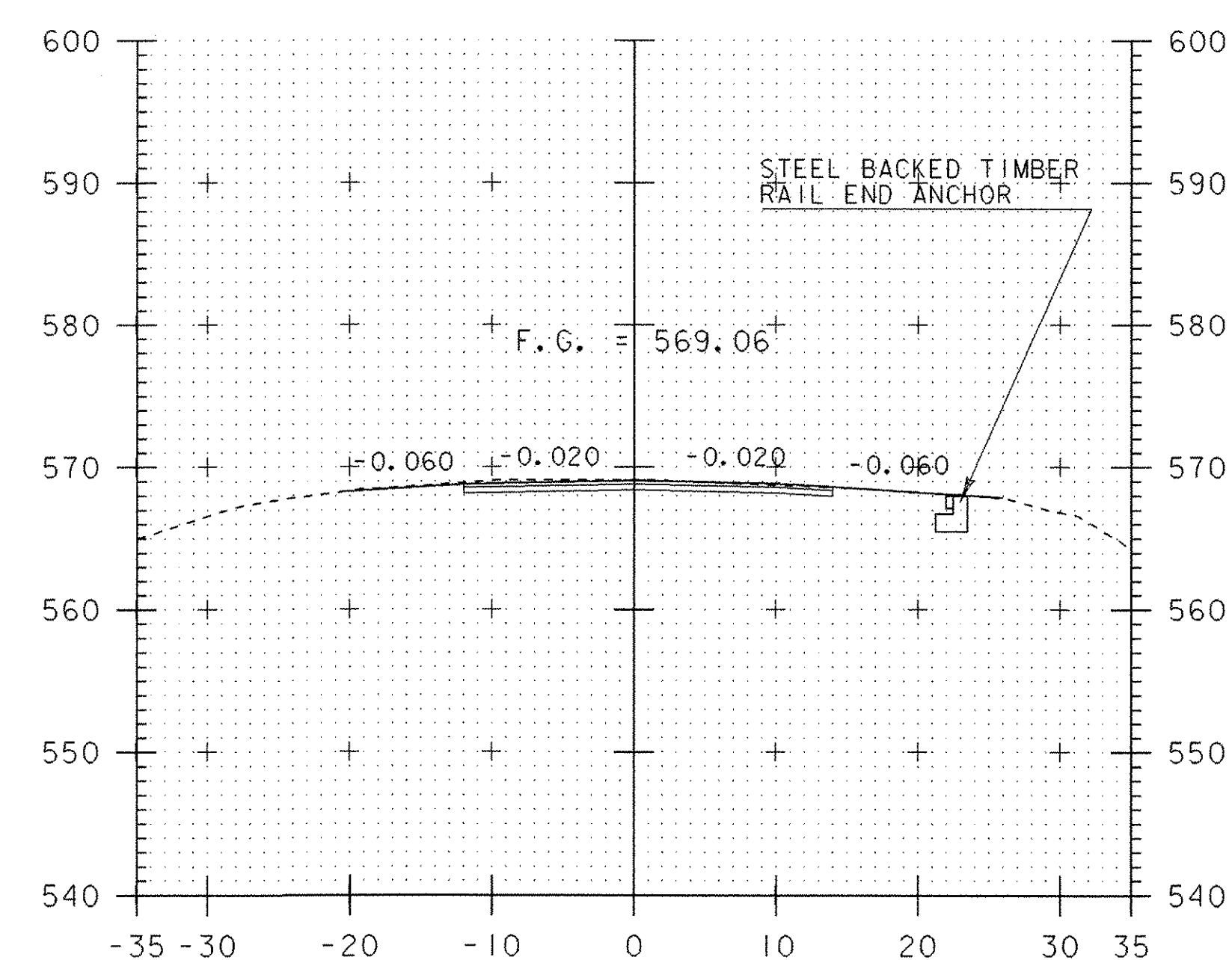
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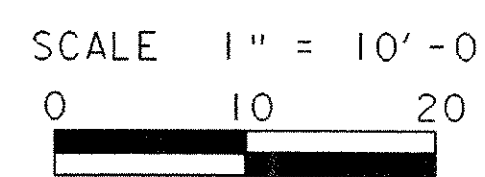
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5+50

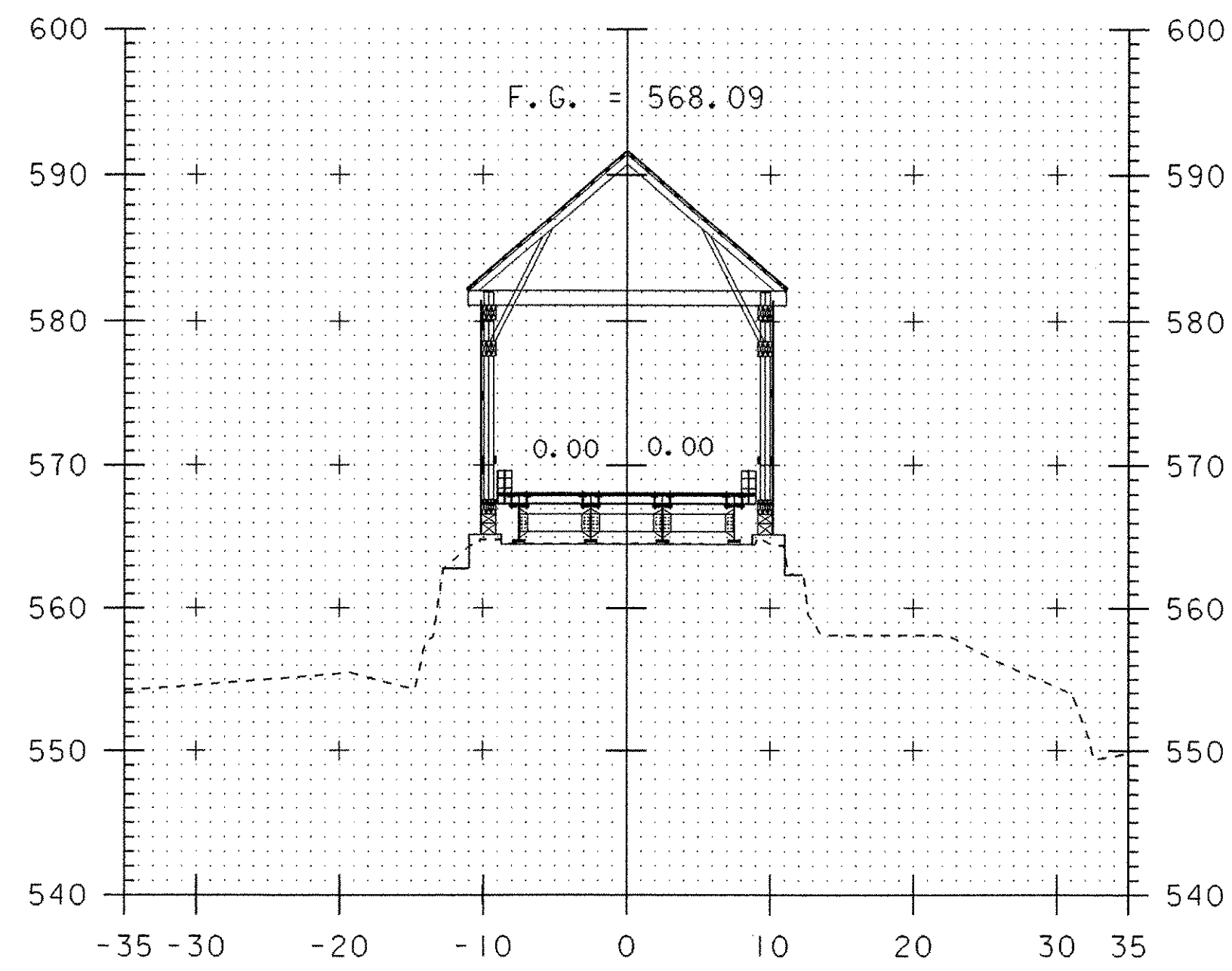


6+00

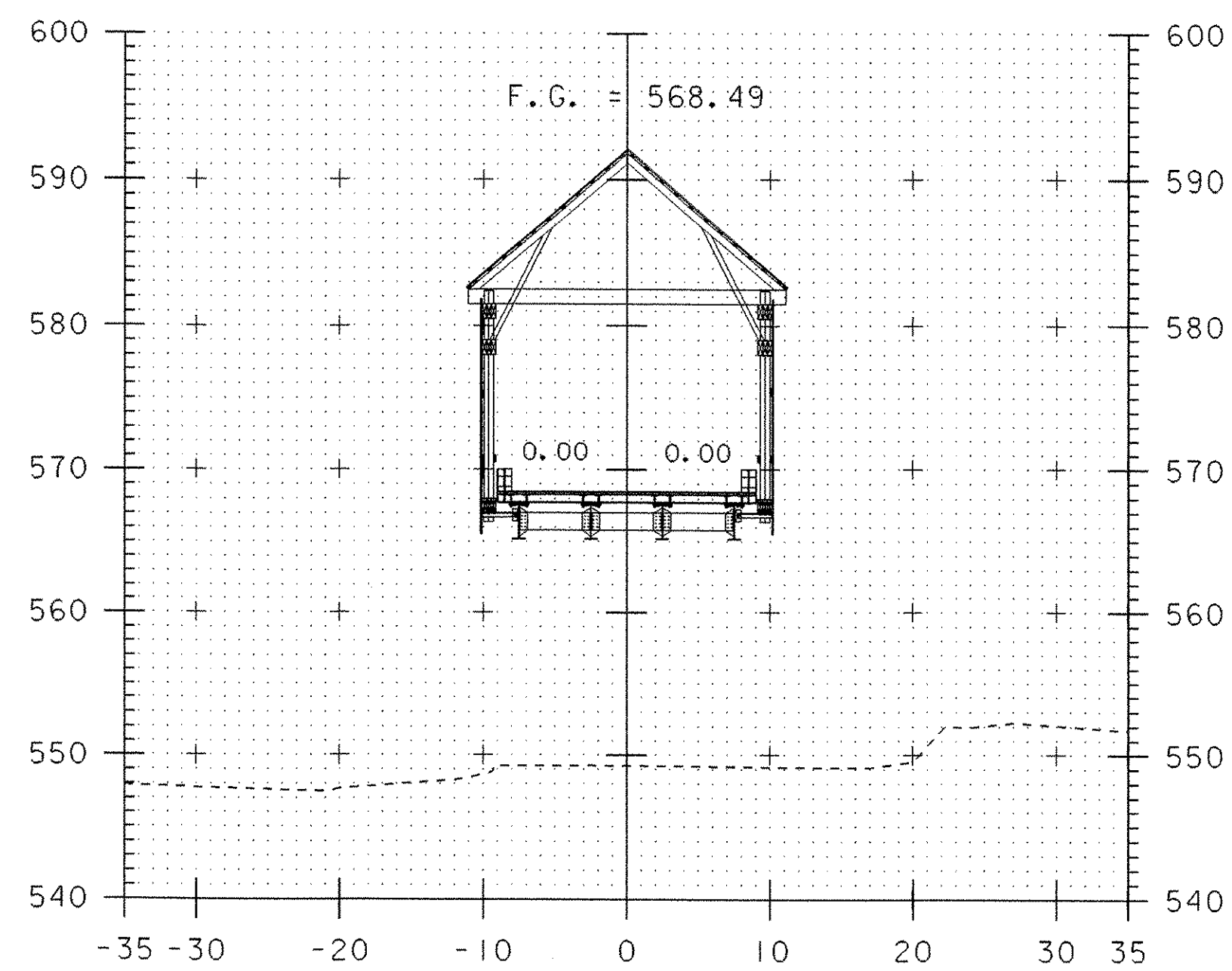


**TH 6 MAIN LINE CROSS SECTIONS (1)**

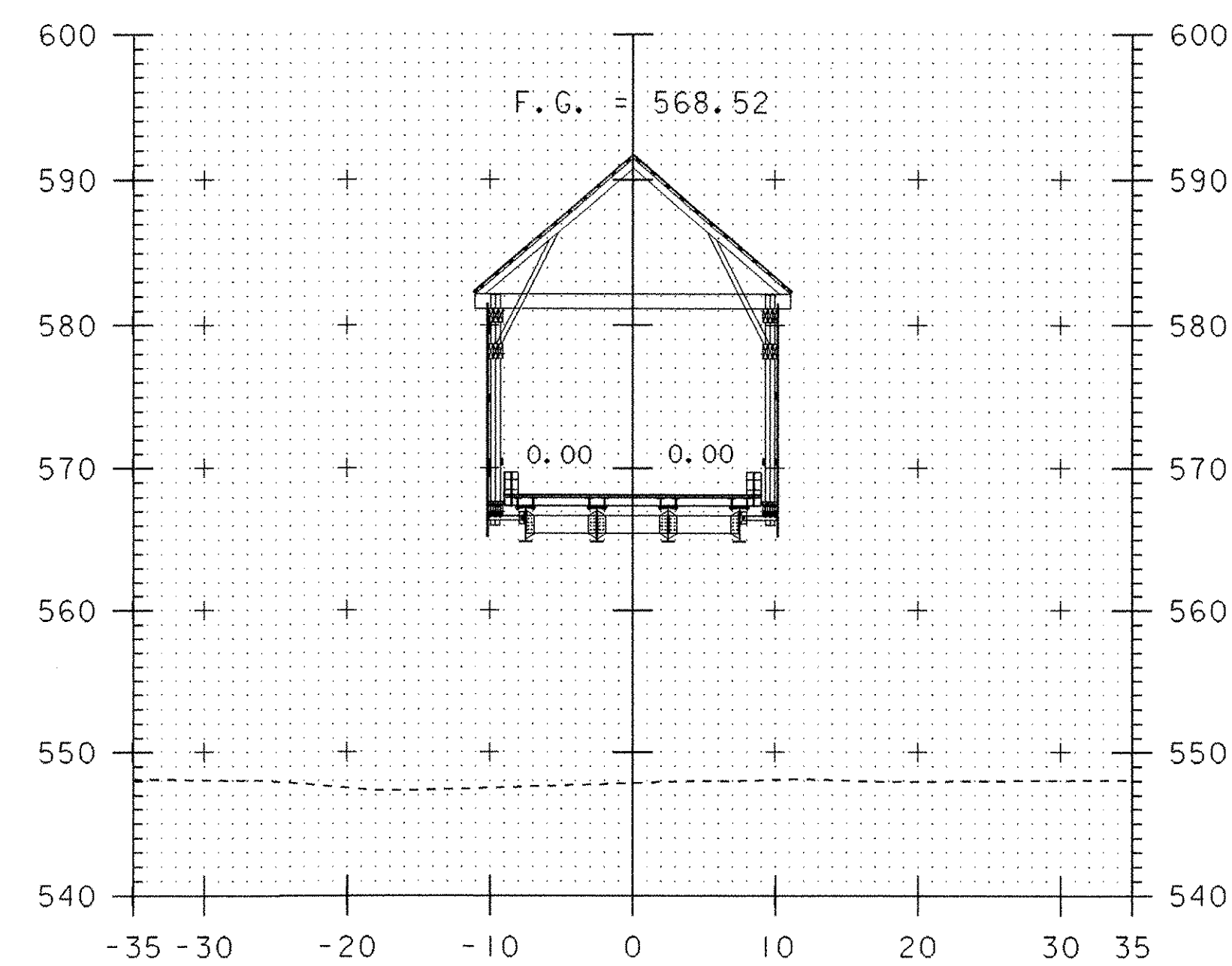
PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036wk.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 45 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036r.xl	



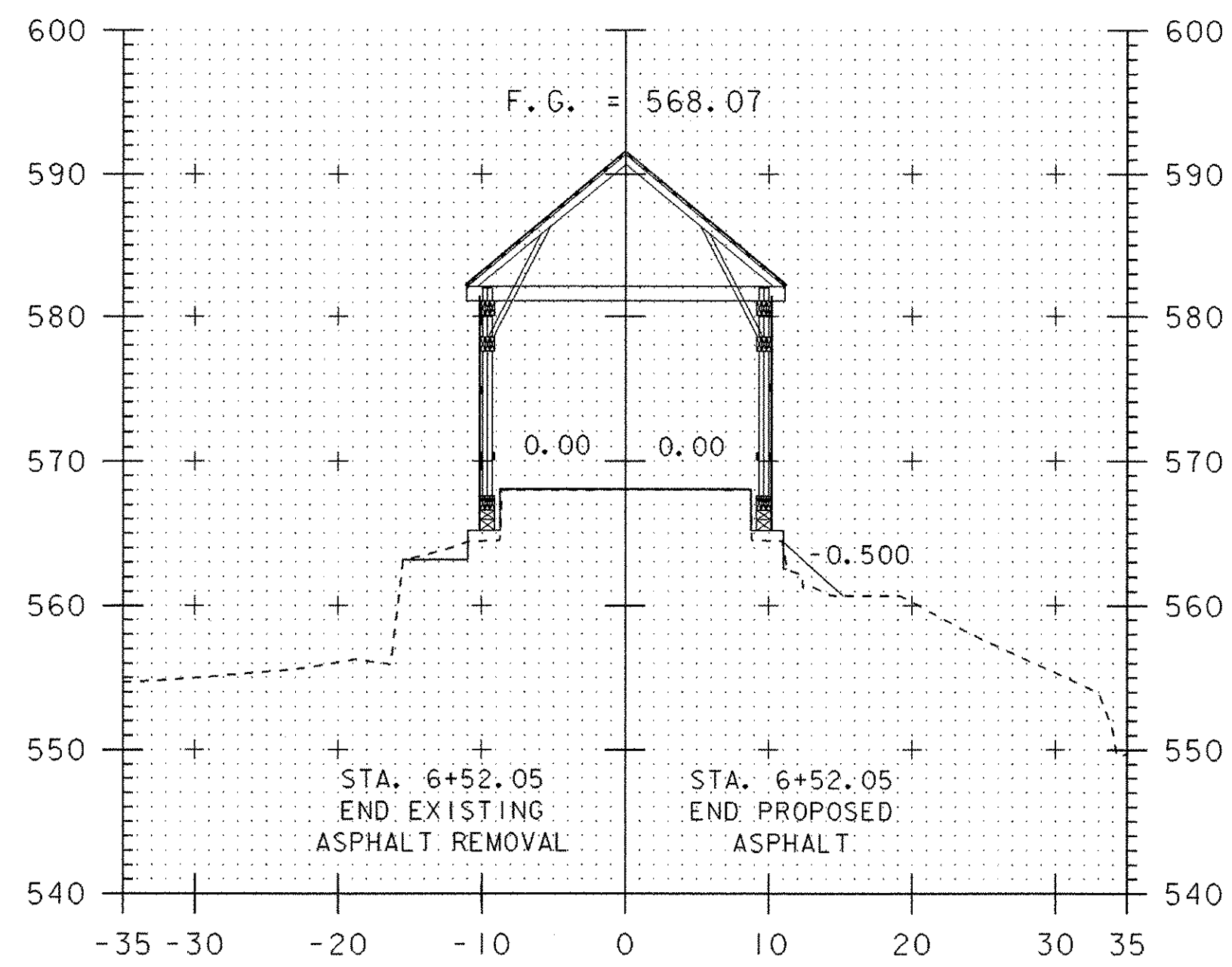
6+53.7  
 ☉ ABUTMENT #1



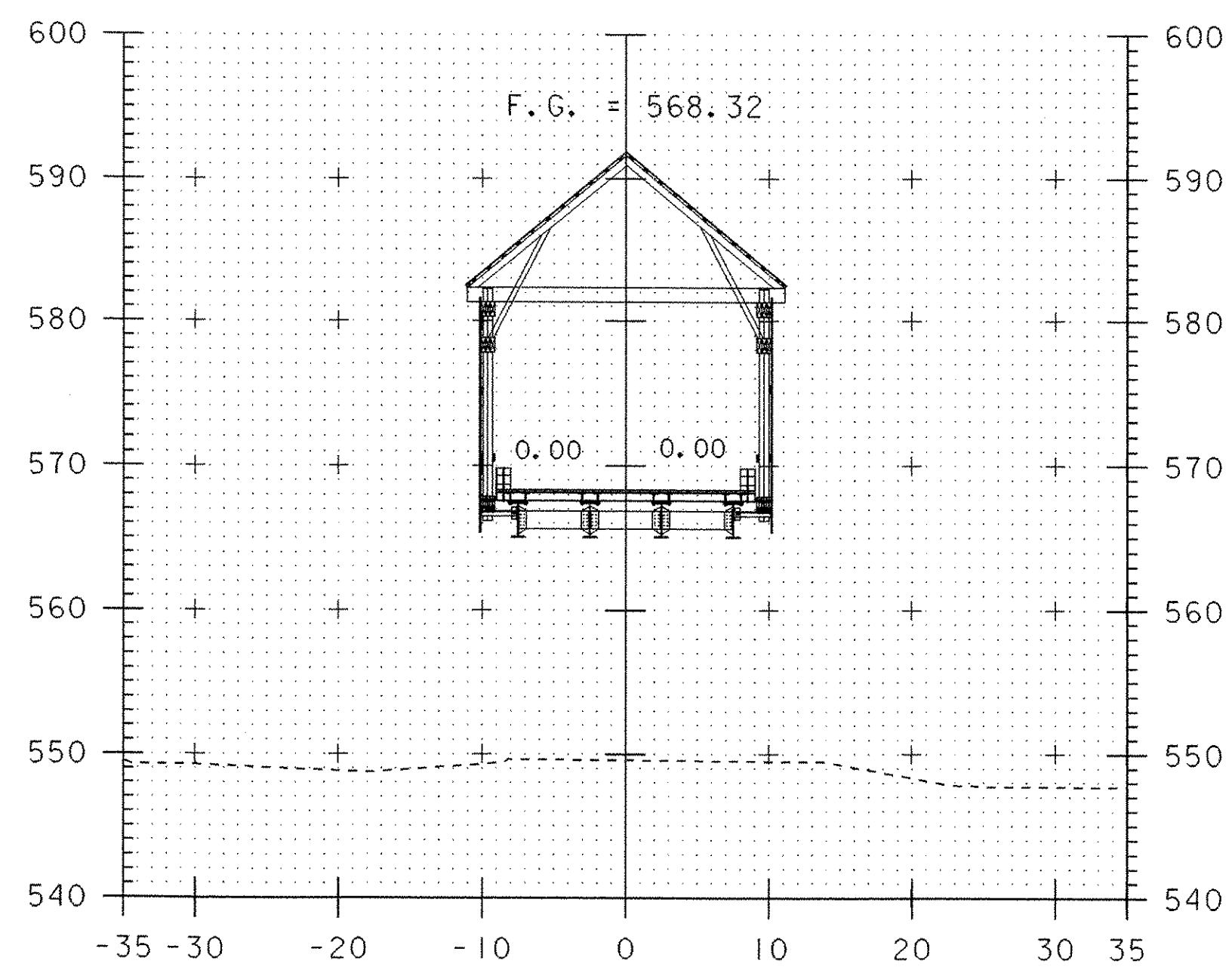
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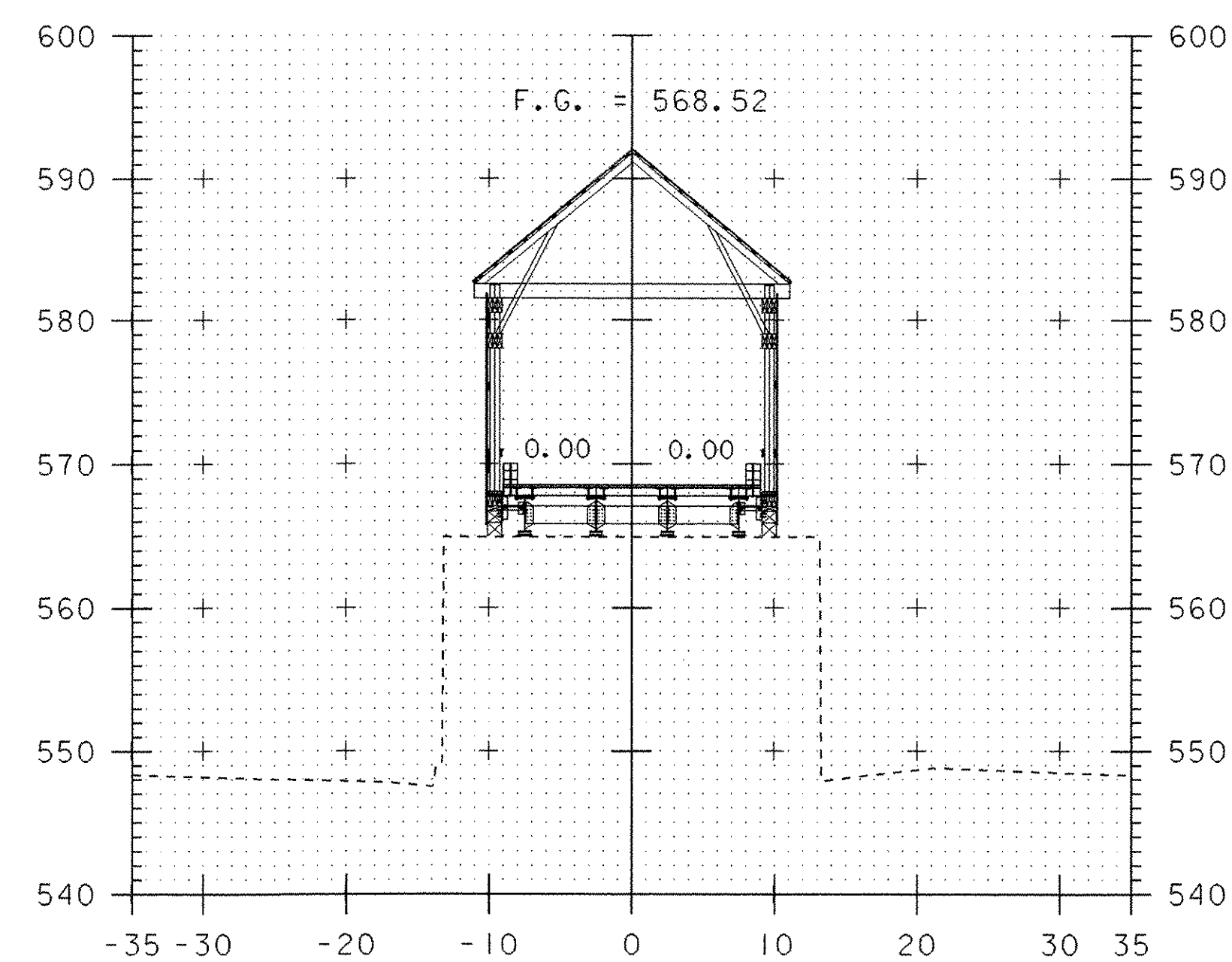
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6+50



6+75

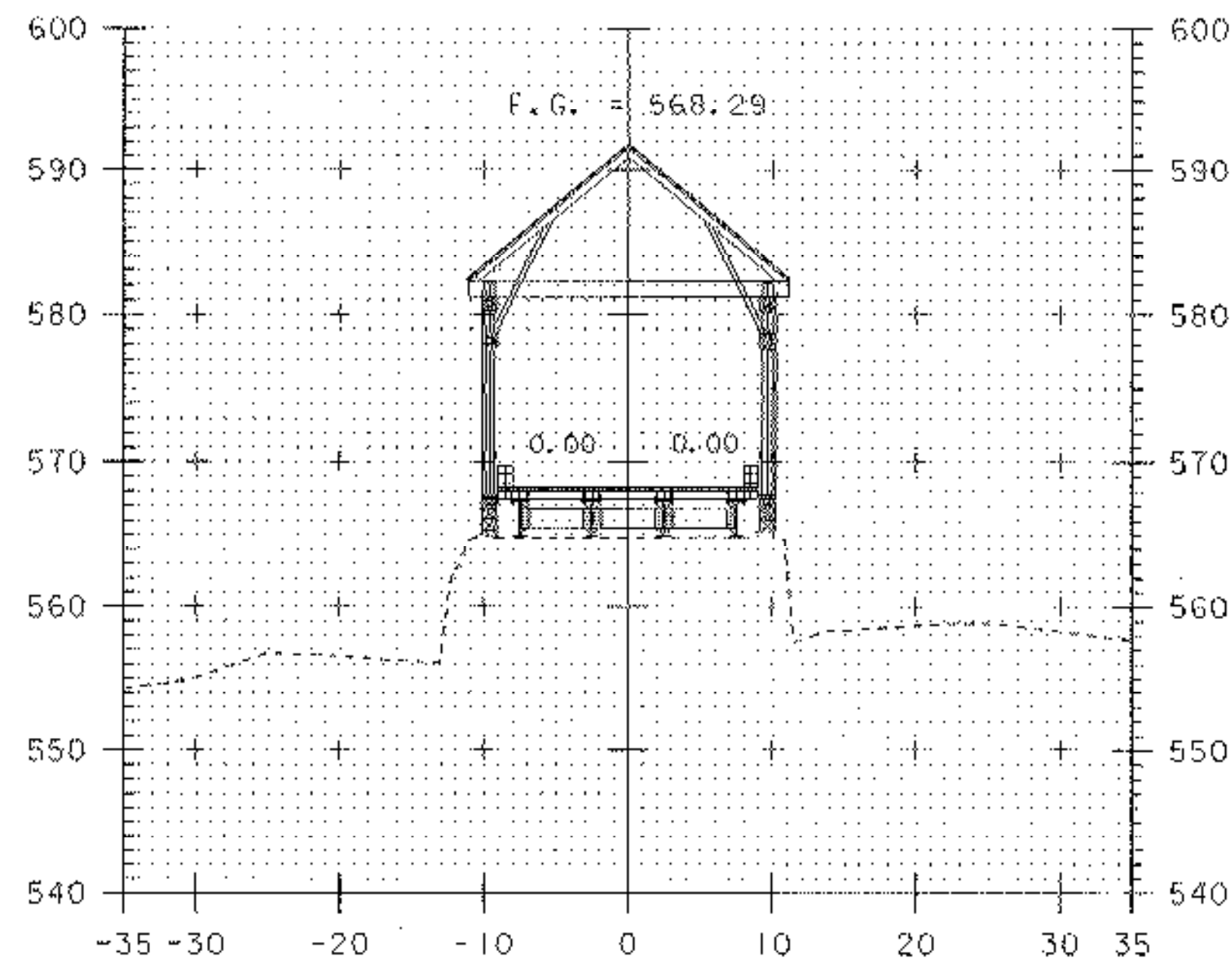


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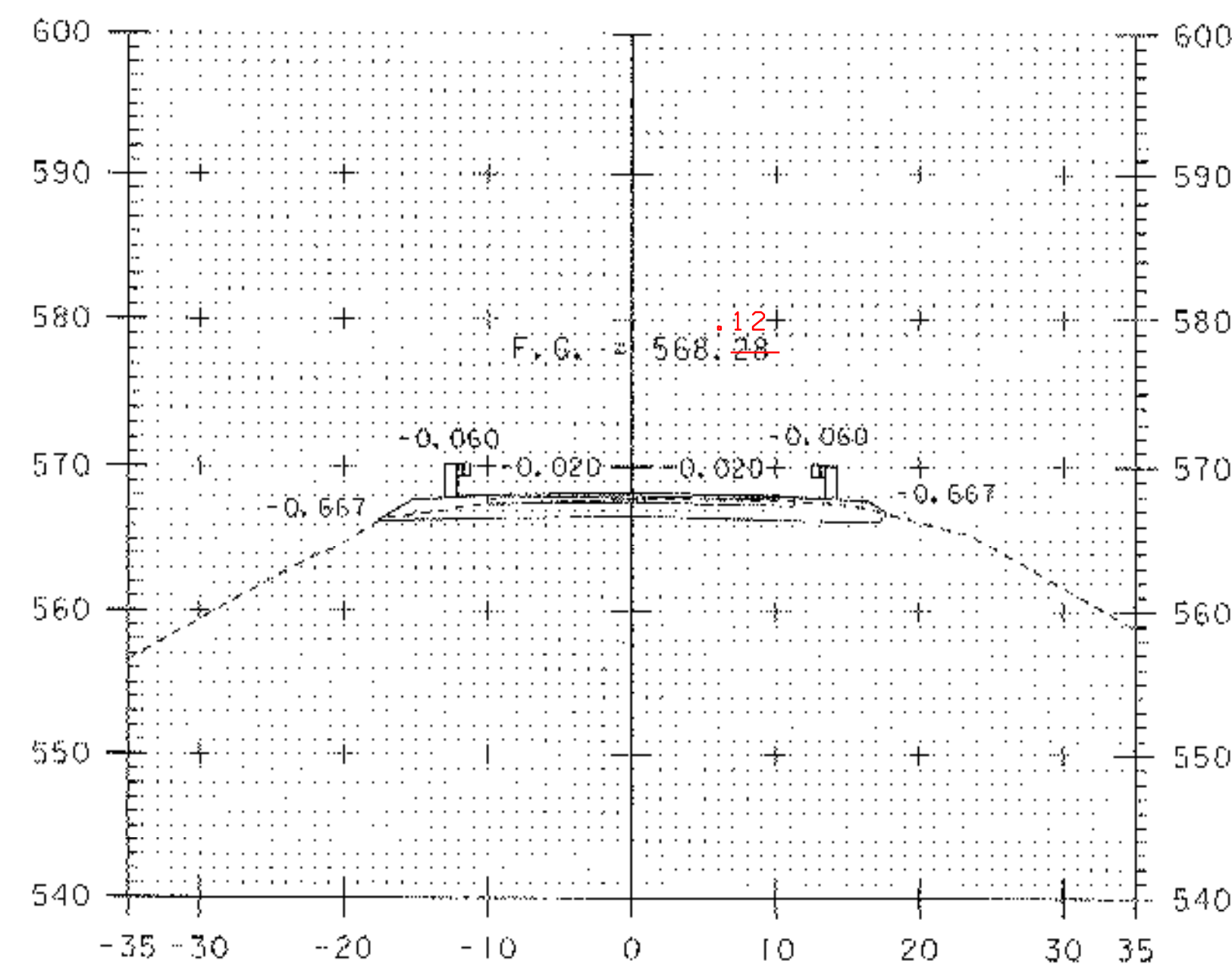
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**TH 6 MAIN LINE CROSS SECTIONS (2)**

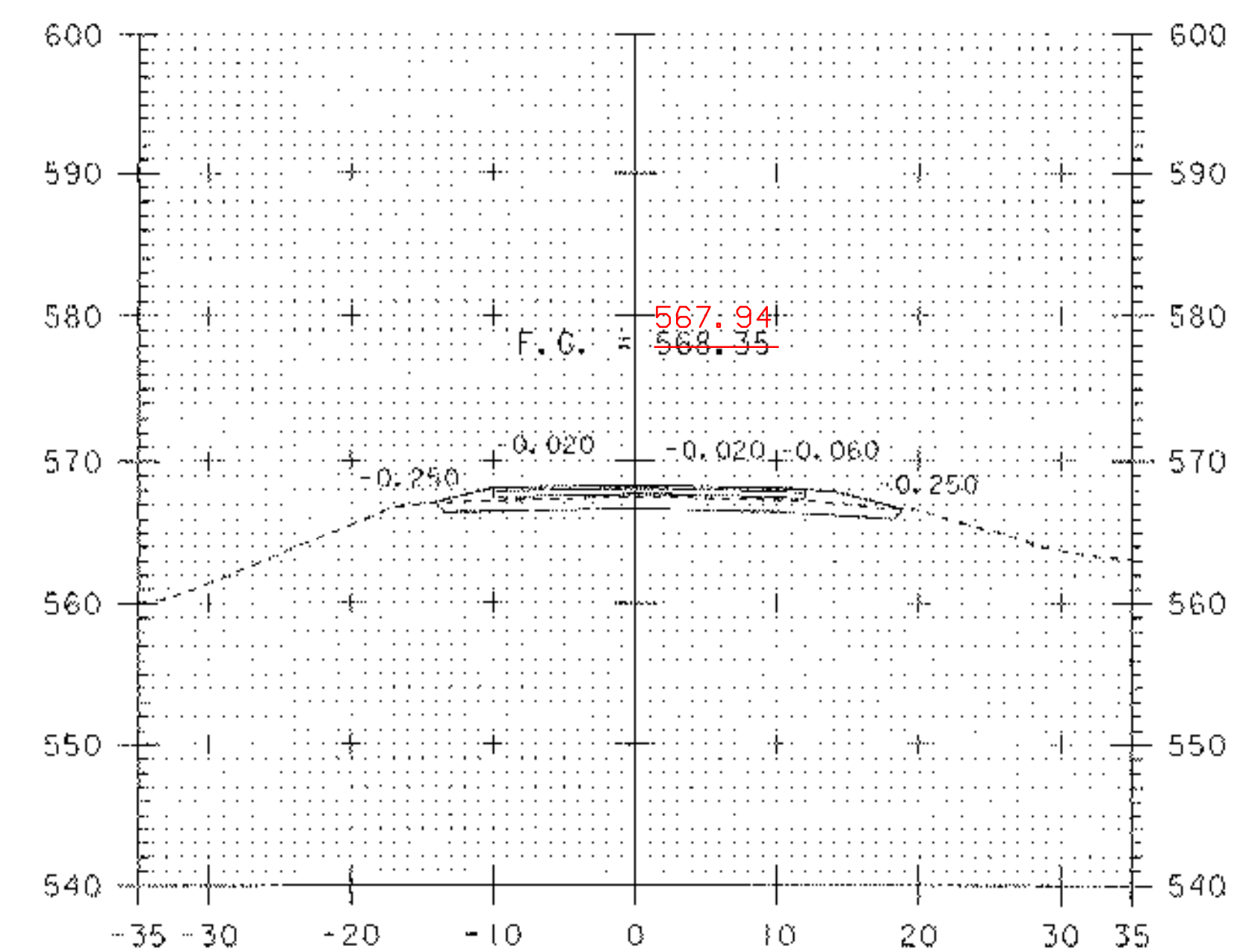
PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036wrk.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 46 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036r2.i	



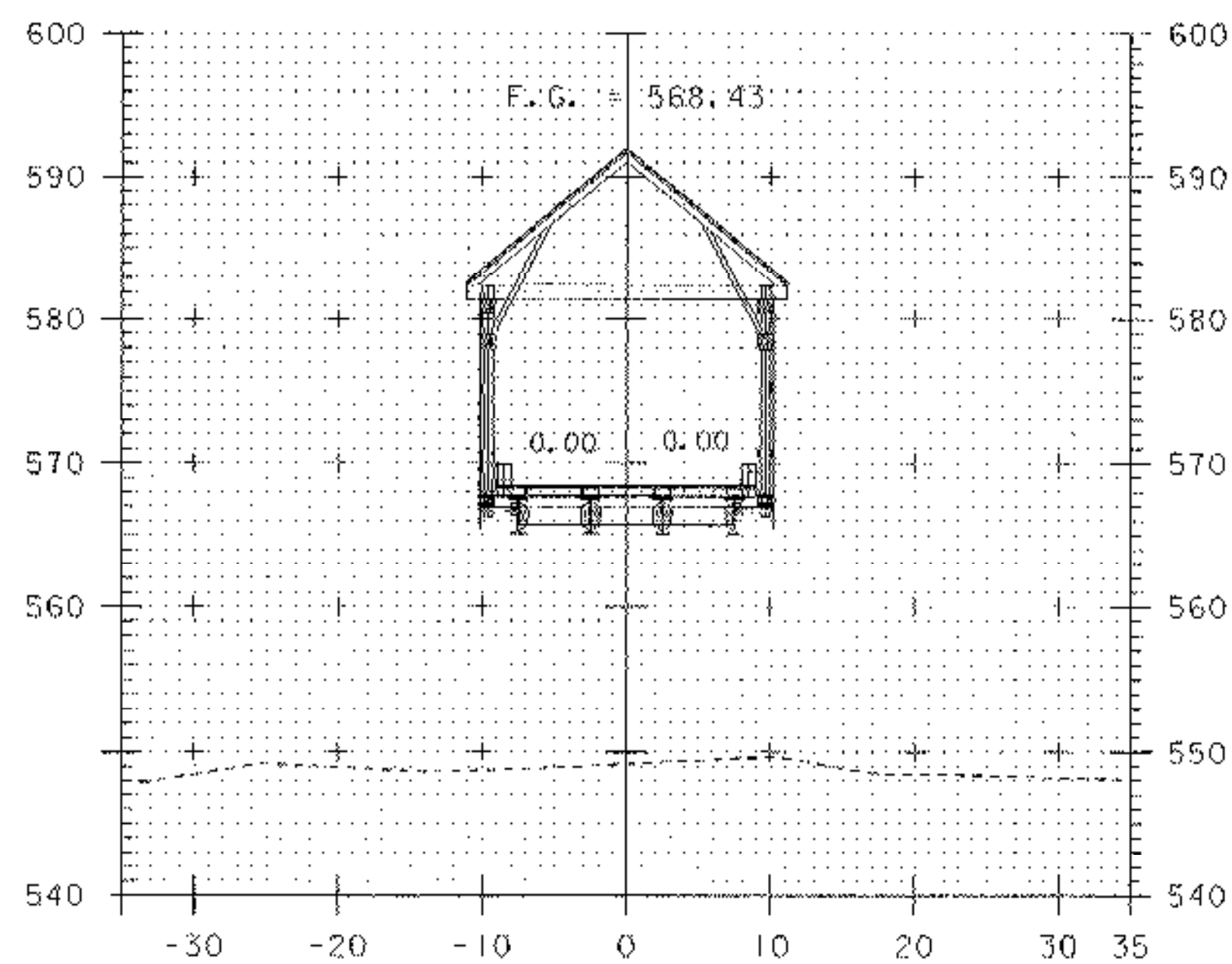
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 C ABUTMENT #2



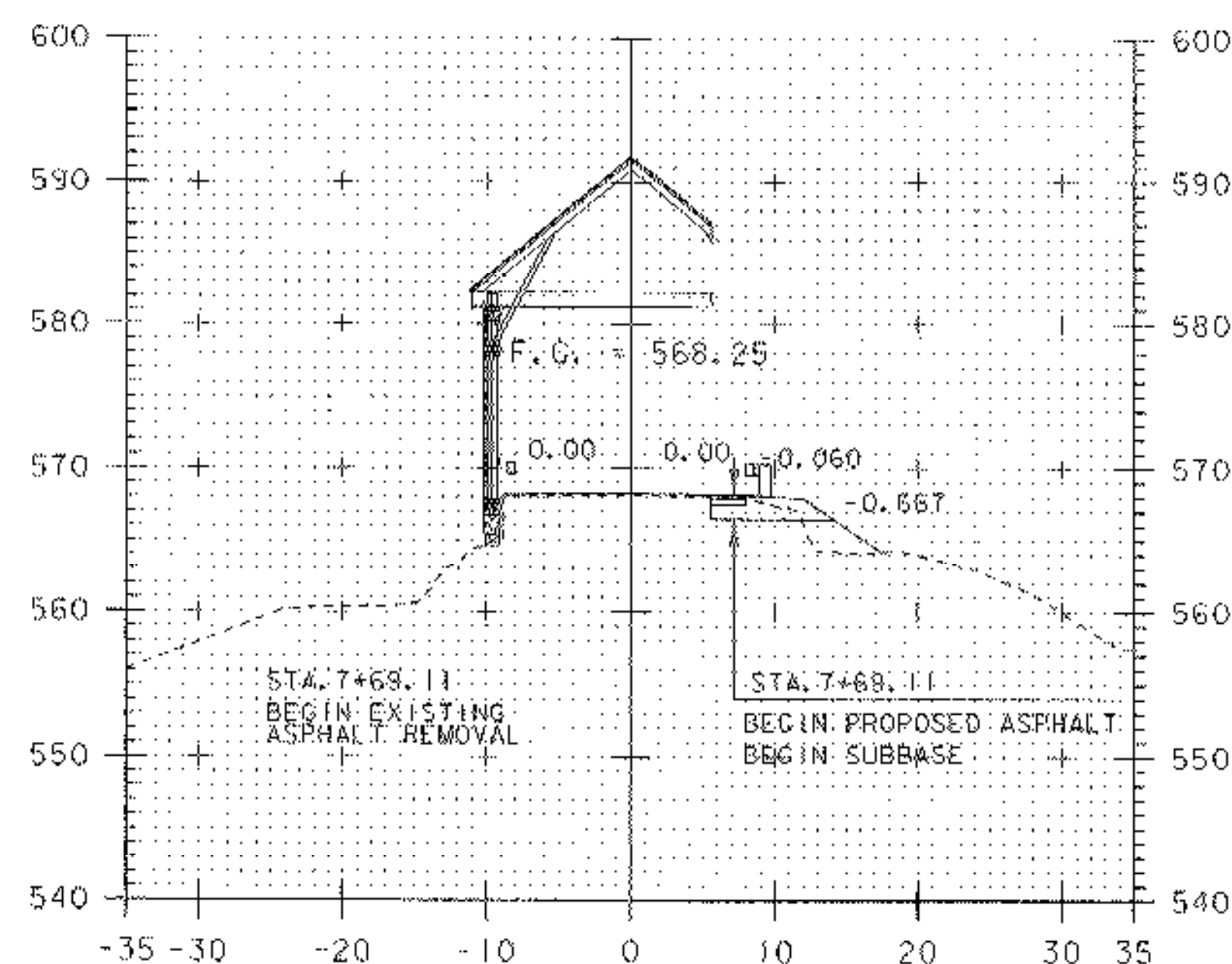
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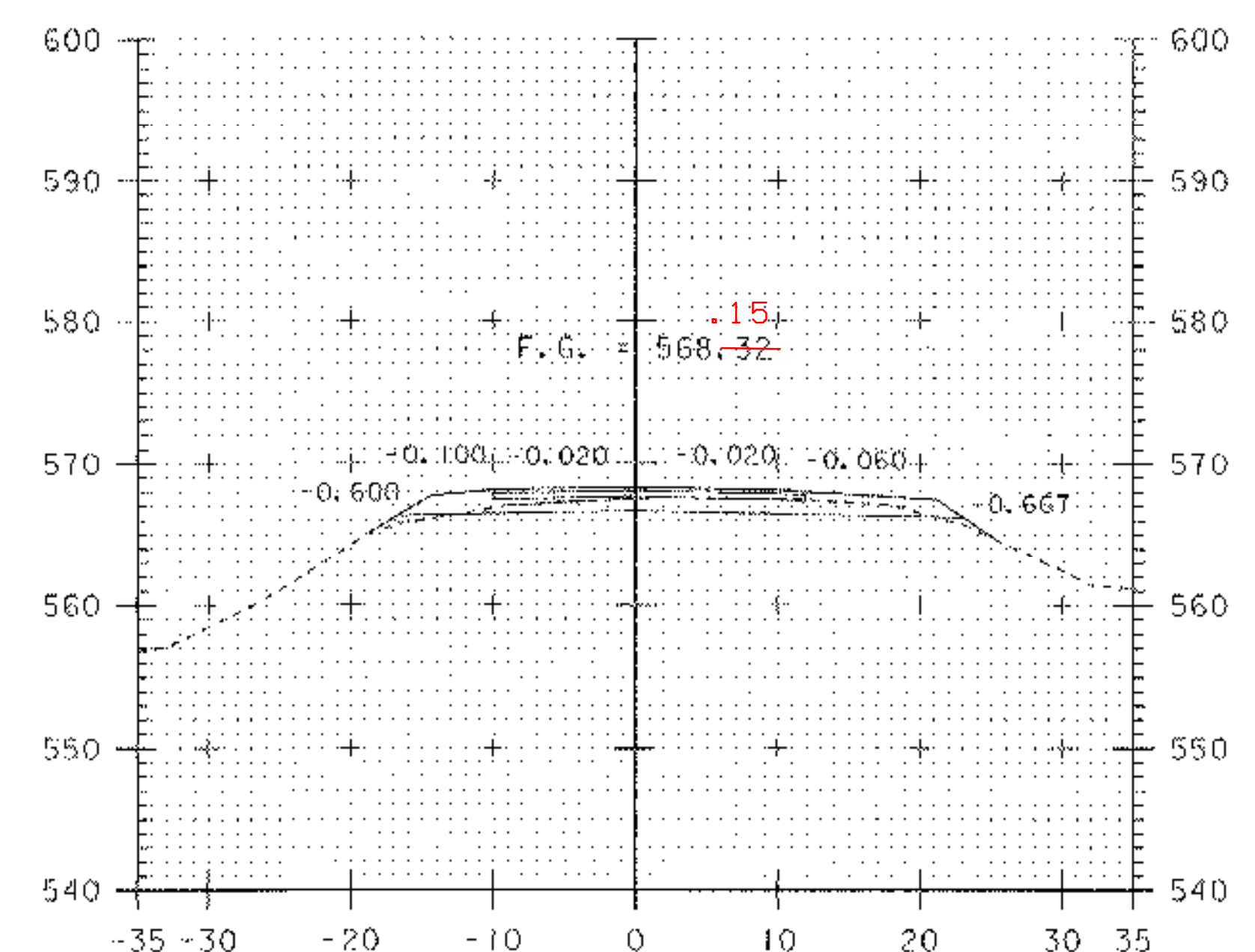
8+50



7+50



7+75

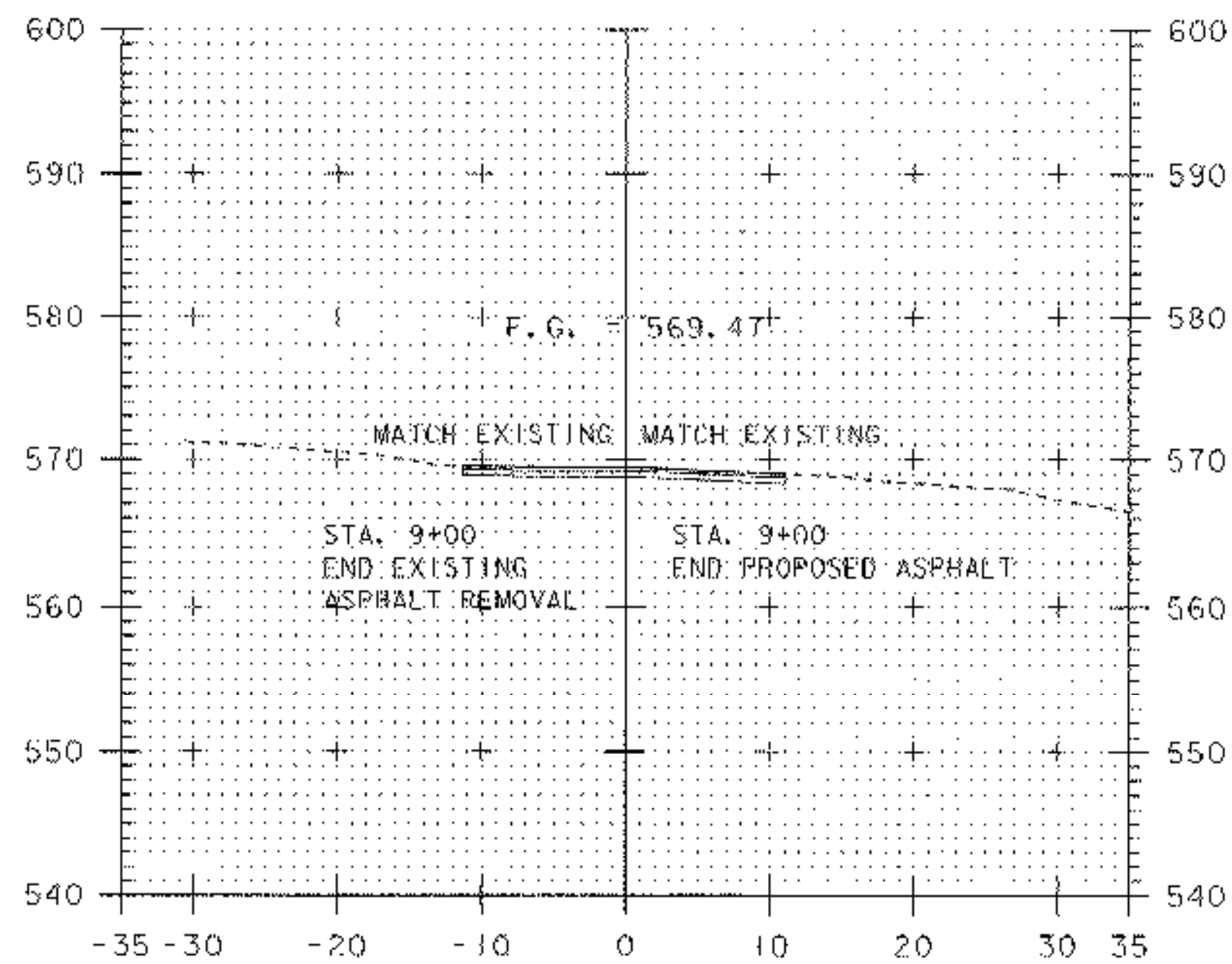


8+25

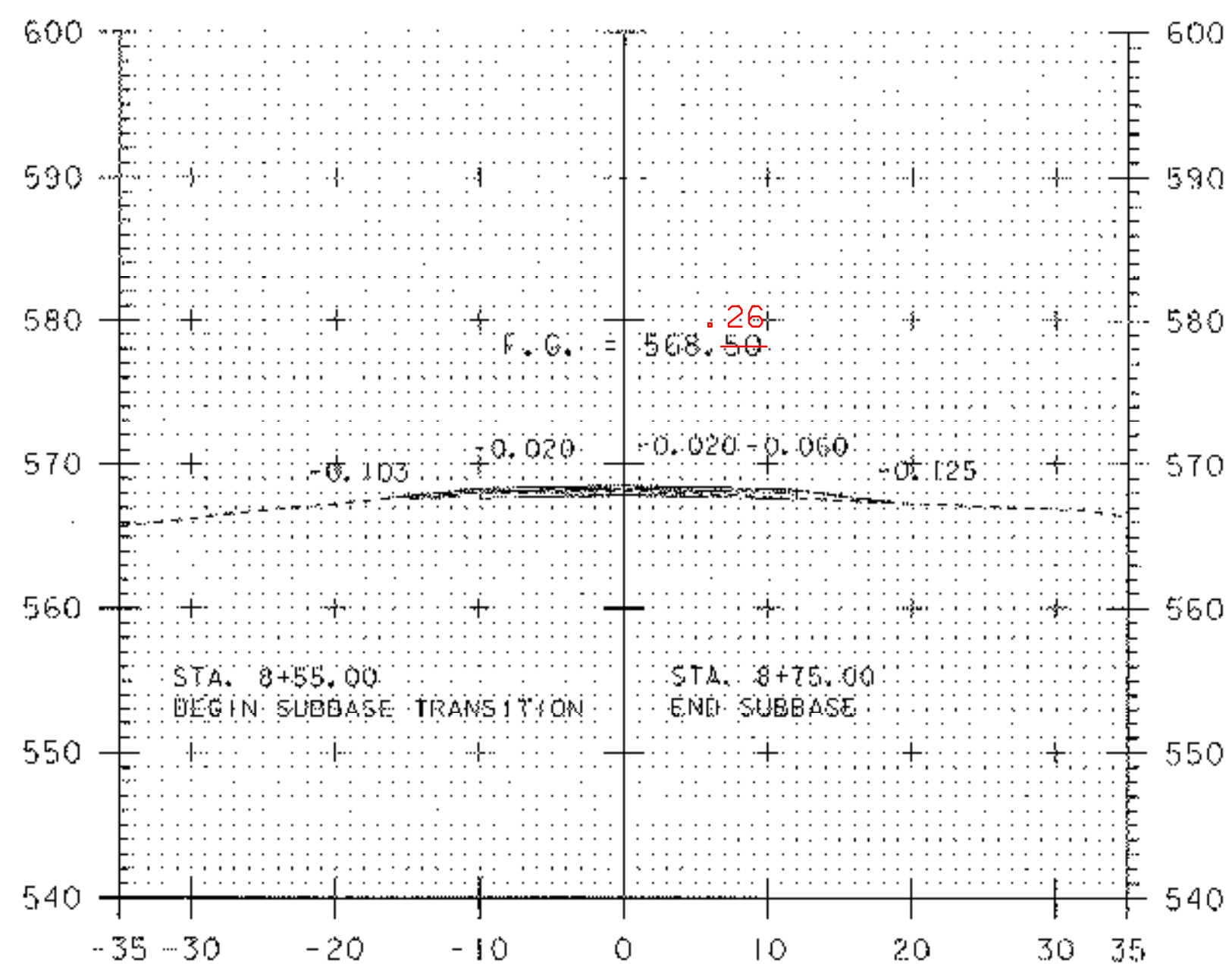
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**TH 6 MAIN LINE CROSS SECTIONS (3)**

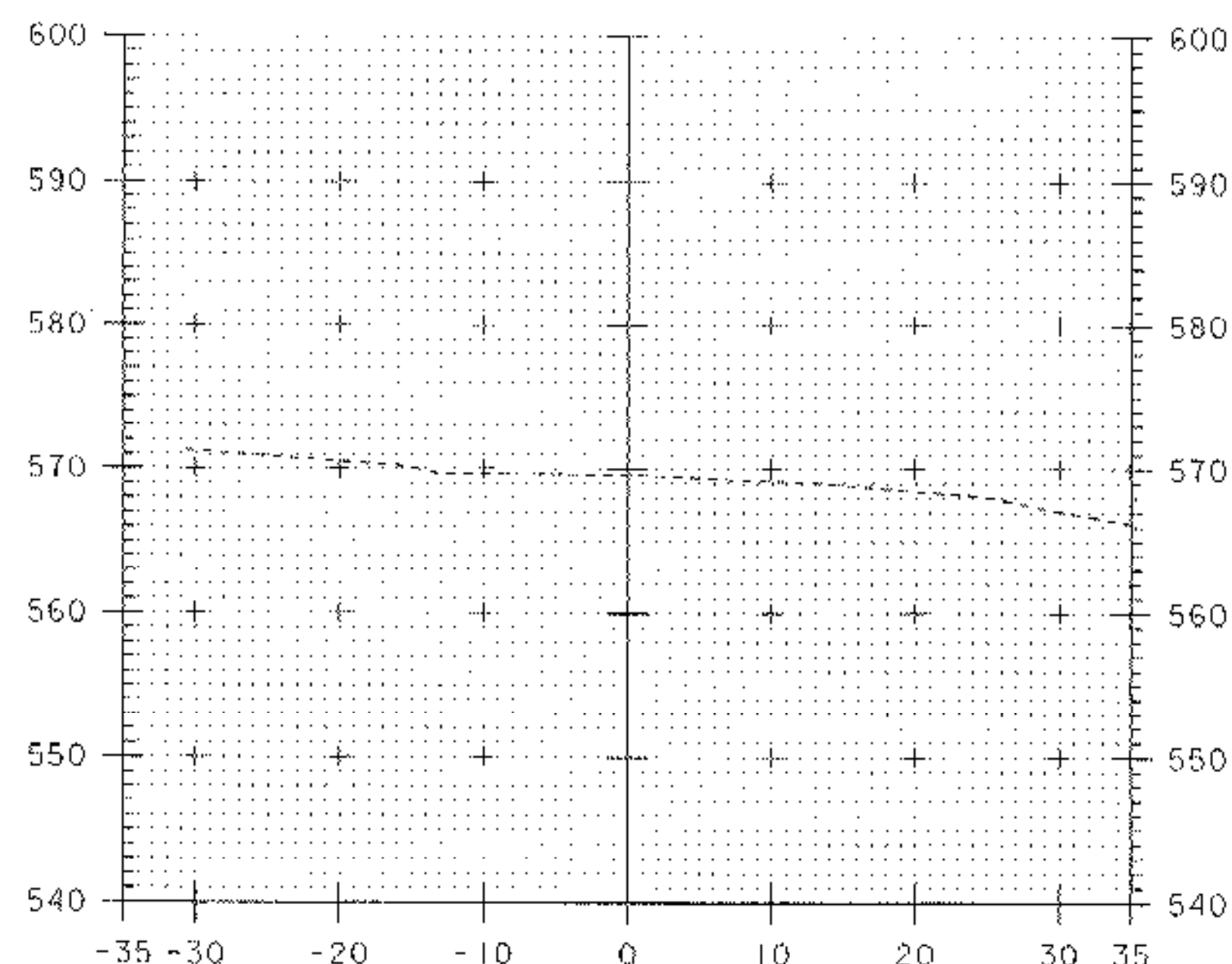
PROJECT NAME:	THETFORD
PROJECT NUMBER:	BHO 1444 (43)
FILE NAME:	Structures\s03j036wrk.dgn
PROJECT LEADER:	M.EVANS-MONGEON
DESIGNED BY:	G.ROKES
IPARM:	s03j036r.x3
PLOT DATE:	27-SEP-2006
DRAWN BY:	G.ROKES
CHECKED BY:	S.SCRIBNER
SHEET	47 OF 60



9+00



8+75

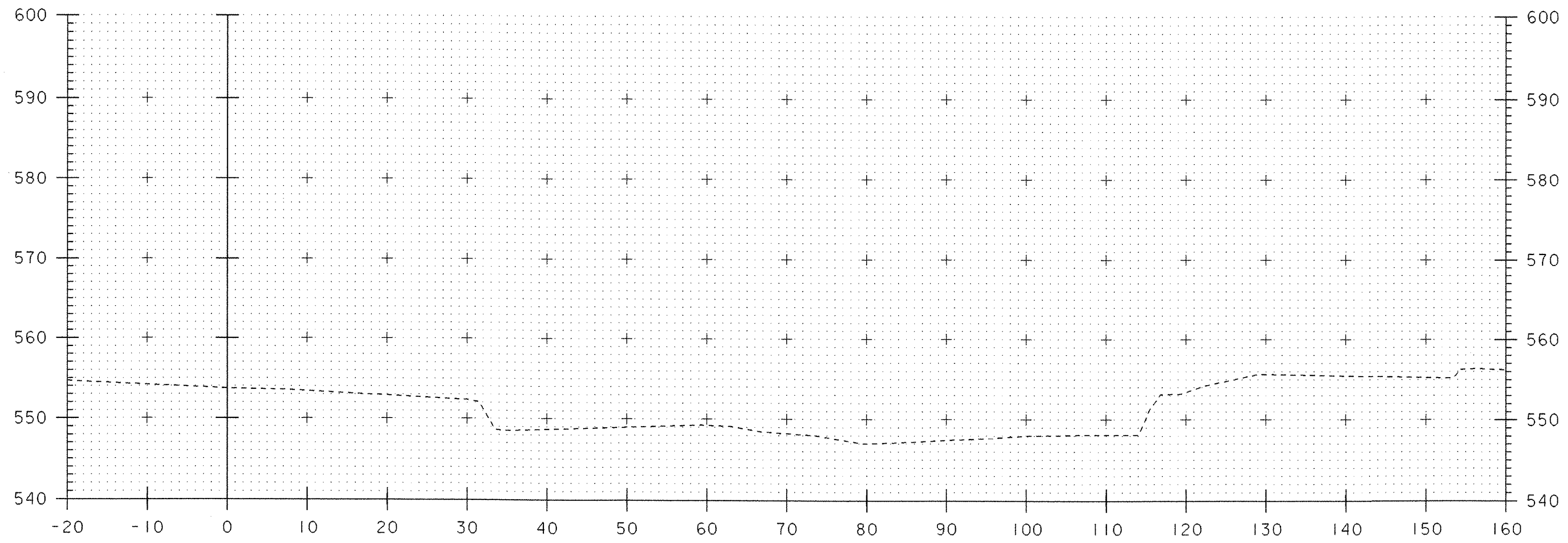


9+02

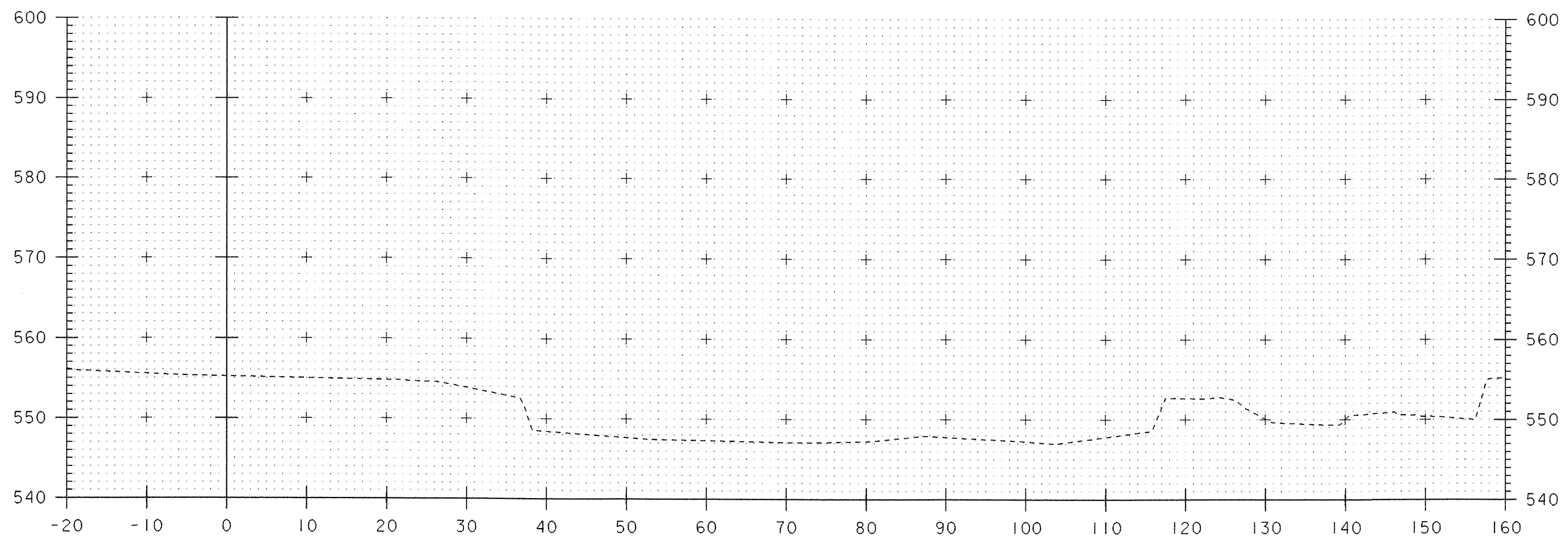
SCALE 1" = 10'-0"  
0 10 20

**TH 6 MAIN LINE CROSS SECTIONS (4)**

PROJECT NAME: THETFORD	PLOT DATE: 21-SEP-2006
PROJECT NUMBER: BRO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036wrk.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 48 OF 60
DESIGNED BY: G.ROKES	
IPARM: s0j036r x4.1	



10+20

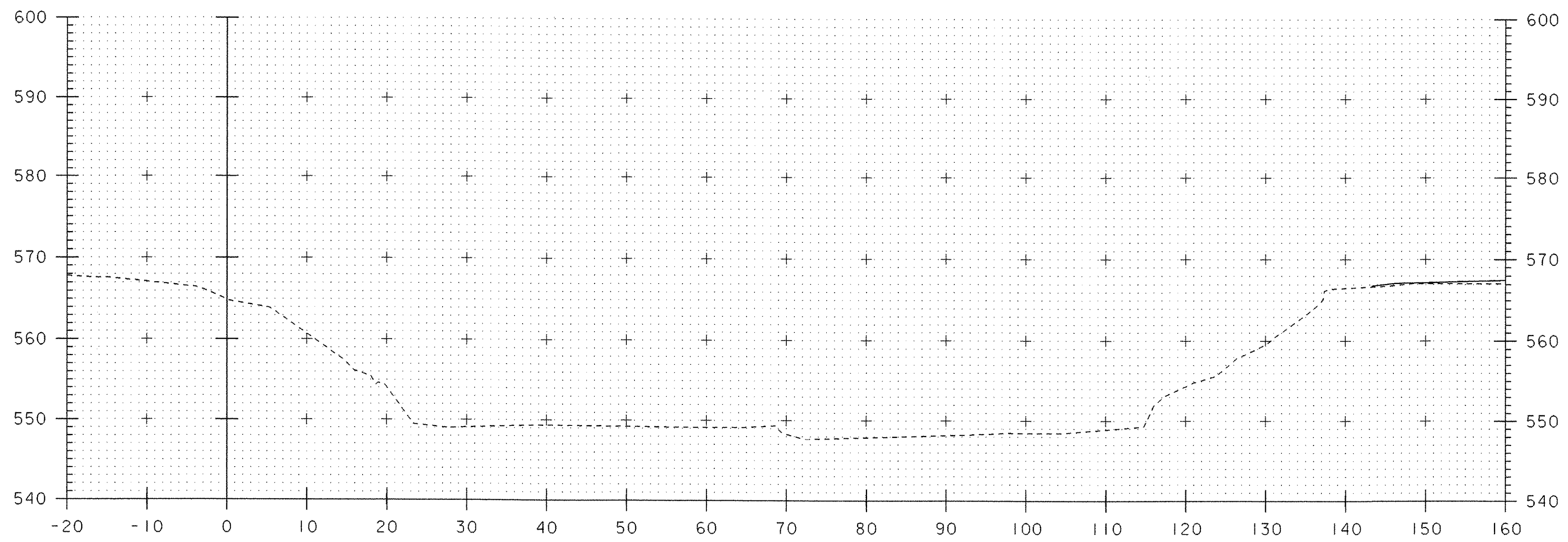


10+00

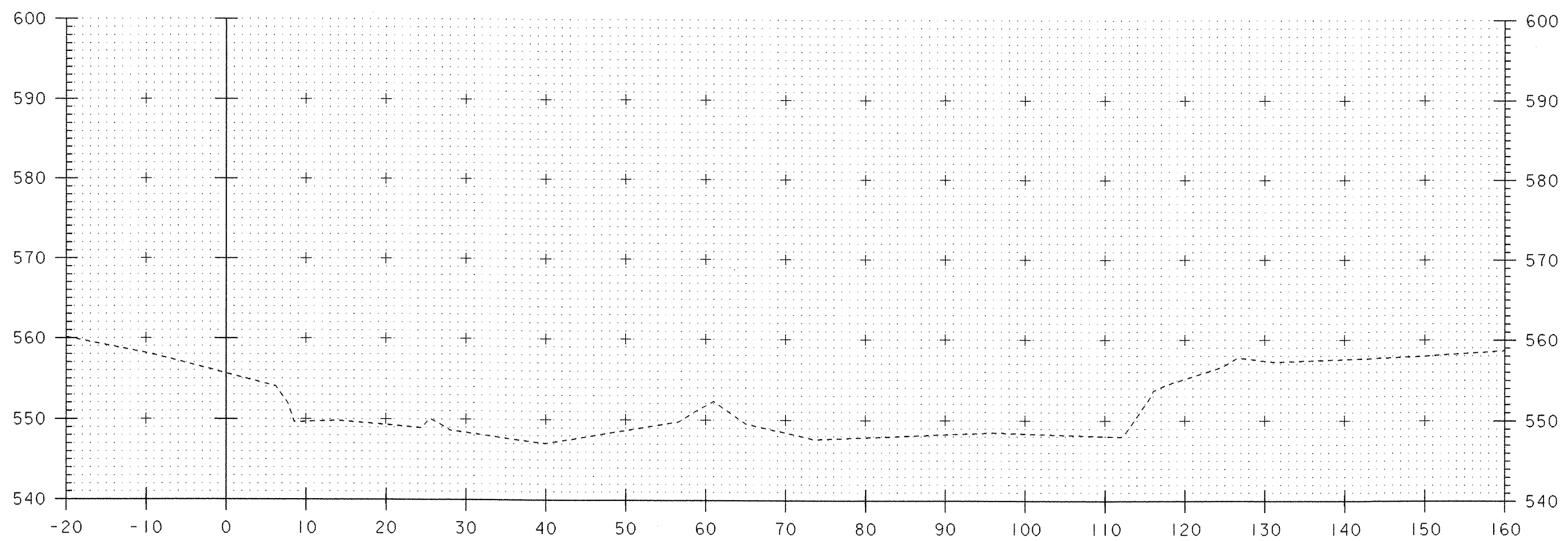
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**OMPOMANOOSUC CHANNEL SECTIONS (1)**

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036wk.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 49 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036cxl.1	



10+60

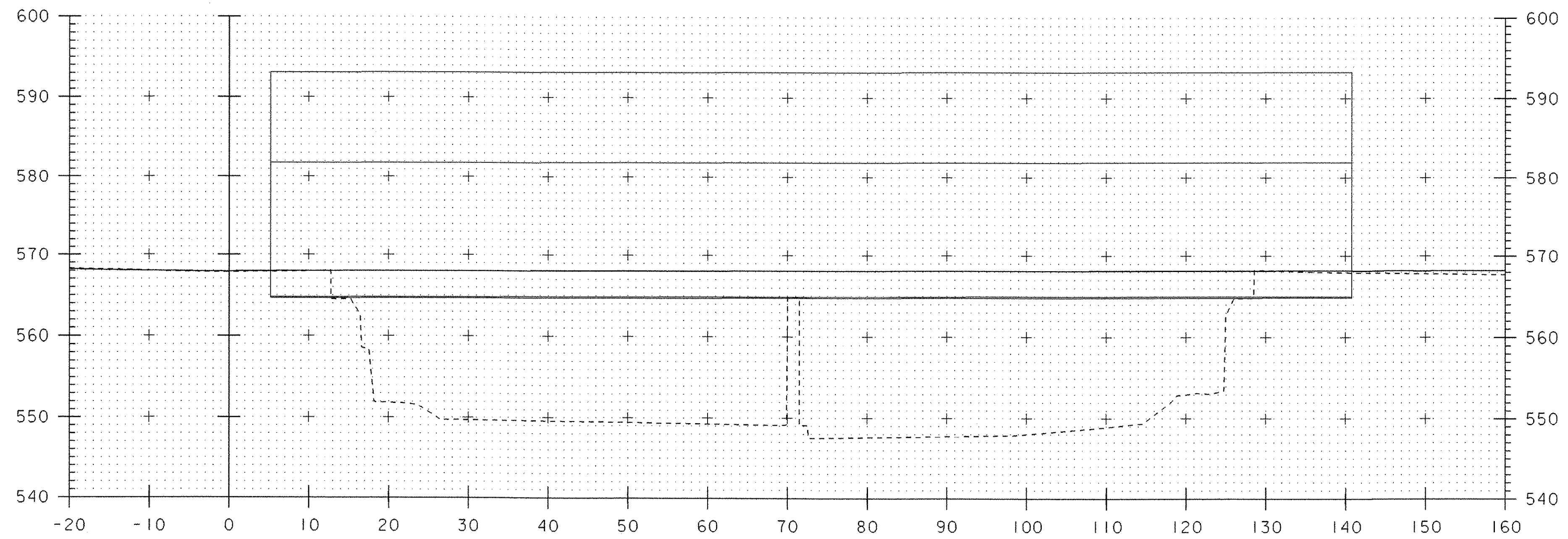


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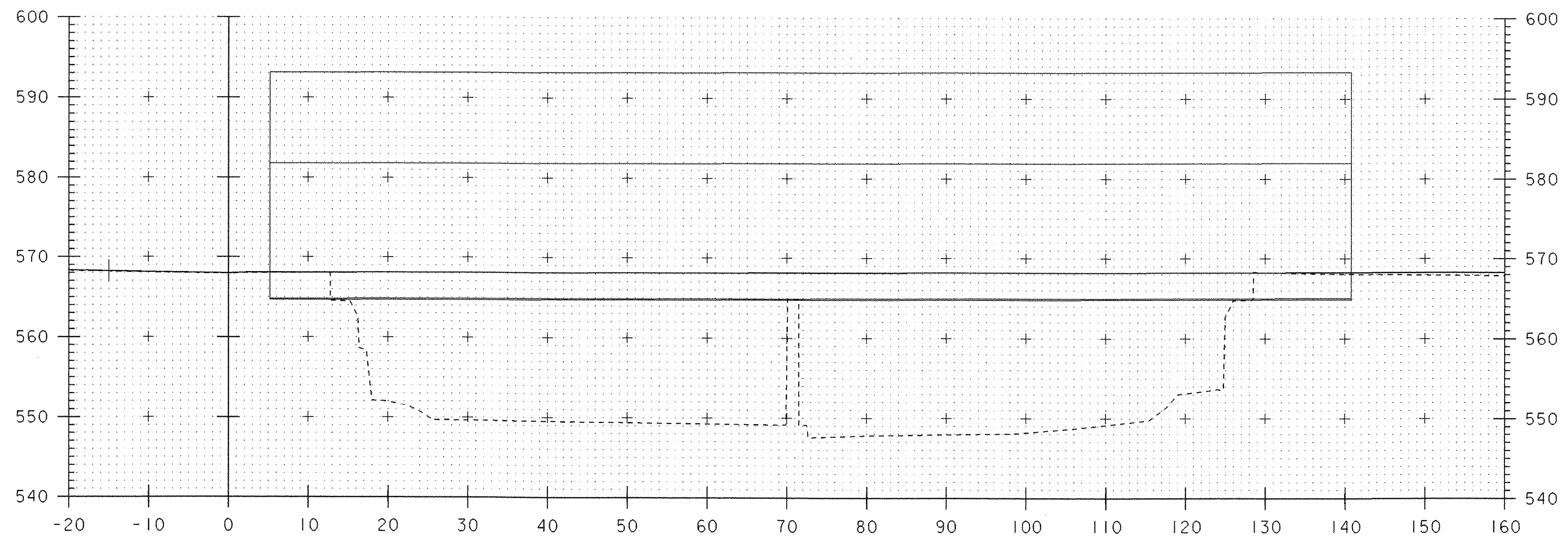
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PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036wk.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 50 OF 60
DESIGNED BY: G.ROKES	
IPAM: s03j036cx2.1	



10+80

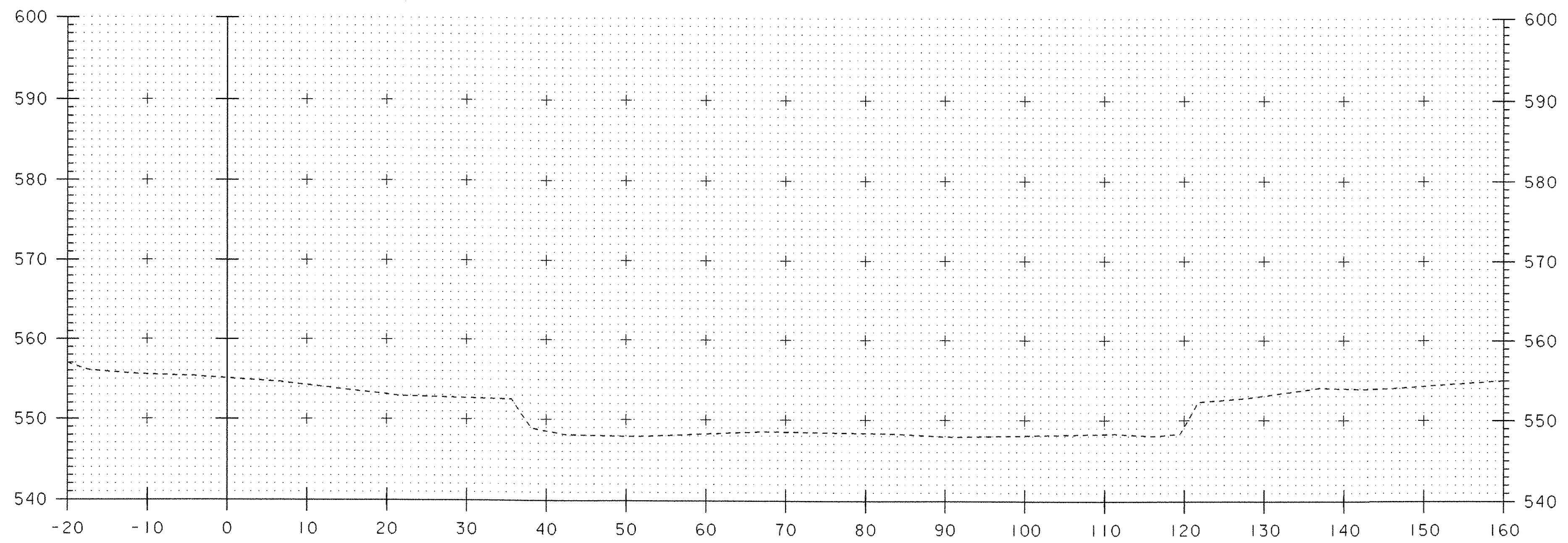


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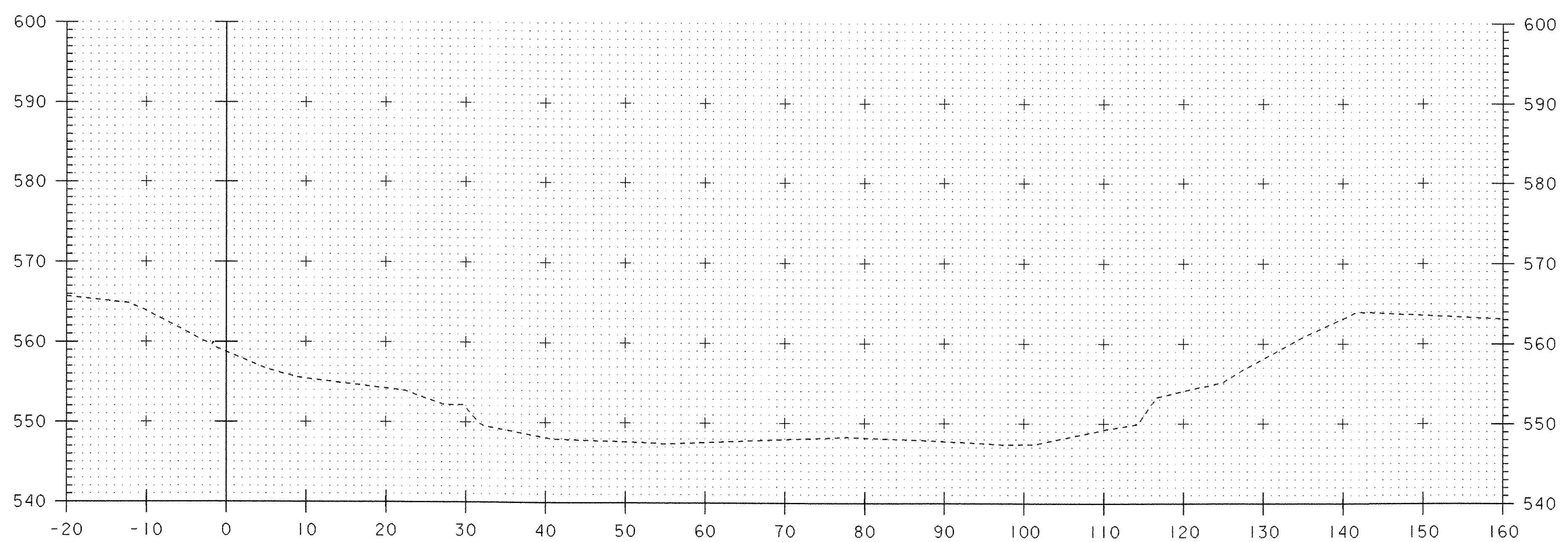
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**OMPOMPANOOSUC CHANNEL SECTIONS (3)**

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036wk.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 51 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036cx3.l	



11+20

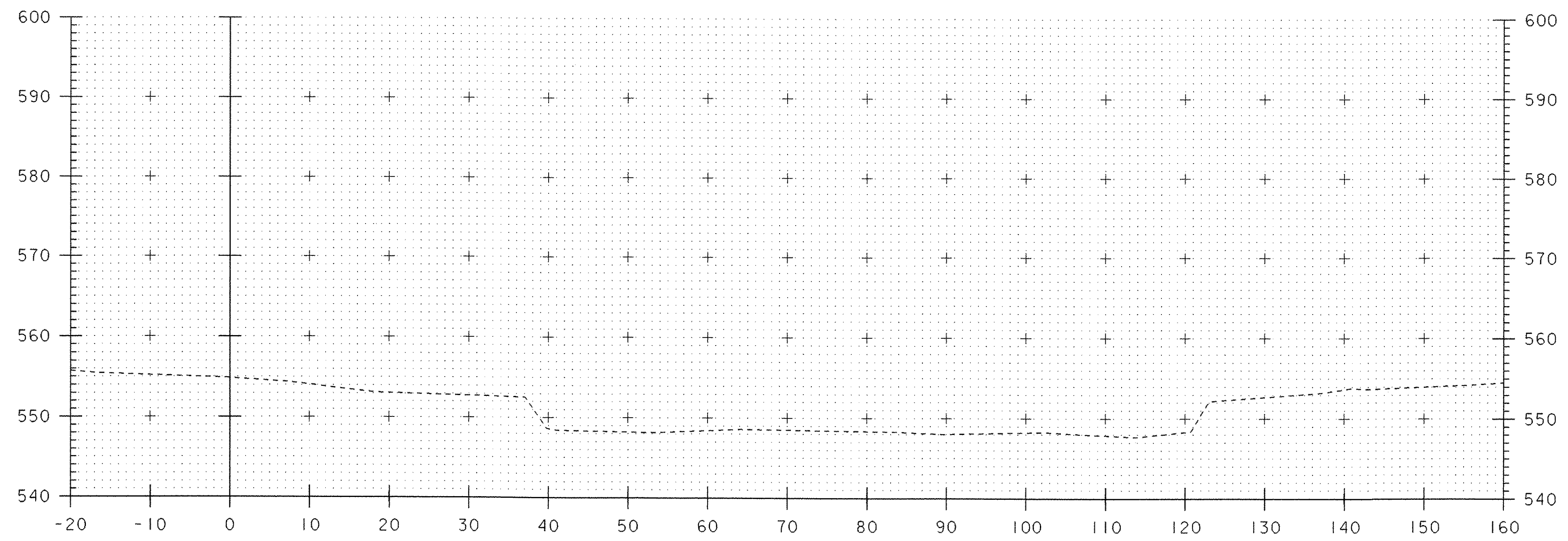


11+00

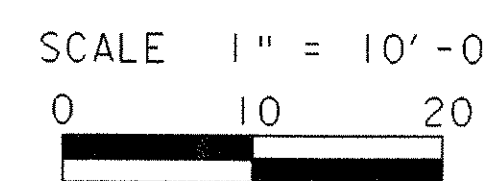
SCALE 1" = 10'-0"  
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**OMPOMPANOOSUC CHANNEL SECTIONS (4)**

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036wk.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 52 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036cx4.1	



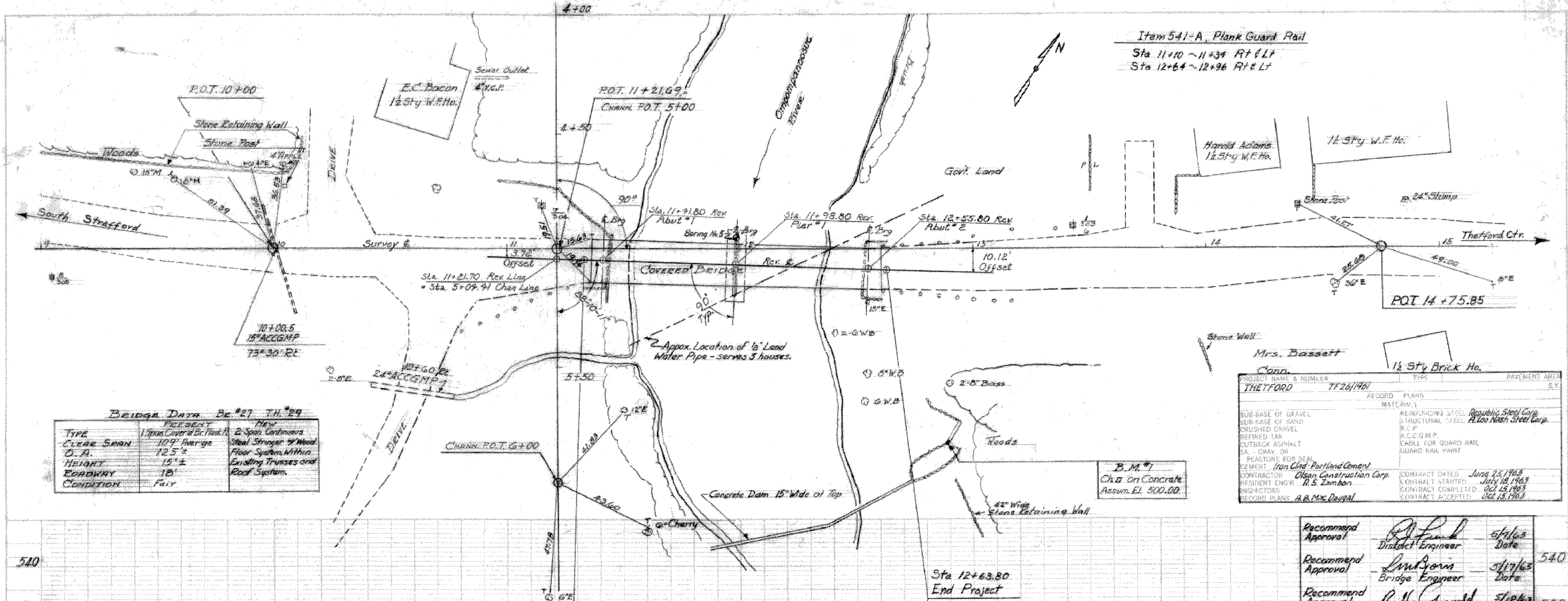
11+25



**OMPOMPANOOSUC CHANNEL SECTIONS (5)**

PROJECT NAME: THETFORD	PLOT DATE: 27-SEP-2006
PROJECT NUMBER: BHO 1444 (43)	DRAWN BY: G.ROKES
FILE NAME: Structures\s03j036wk.dgn	CHECKED BY: S.SCRIBNER
PROJECT LEADER: M.EVANS-MONGEON	SHEET 53 OF 60
DESIGNED BY: G.ROKES	
IPARM: s03j036cx5.1	

Item 541-A, Plank Guard Rail  
 Sta 11+10 ~ 11+35 Rt & Lt  
 Sta 12+64 ~ 12+96 Rt & Lt



BRIDGE DATA Bc #27 TH #29

TYPE	PRESENT	NEW
CLEAR SPAN	1 Span Cover & Br. Plank	2 Span Continuous
D.A.	10' Average	Steel Stringer w/ Wood Floor System Within Existing Trusses and Roof System.
HEIGHT	15' ±	
ROADWAY	18'	
CONDITION	Fair	

PROJECT NAME & NUMBER		TYPE	PAYMENT AREA
THETFORD TF 26/1961			
RECORD PLANS		MATERIALS	
SUB-BASE OF GRAVEL	REINFORCING STEEL	Republic Steel Corp.	
SUB-BASE OF SAND	STRUCTURAL STEEL	A. Lee Nash Steel Corp.	
CRUSHED GRAVEL	R.C.P.		
REFINED LAM	R.C.C.G.M.P.		
CUTBACK ASPHALT	CABLE FOR GUARD RAIL		
GA. GRAY OR PEASTONE FOR STR.	GUARD RAIL PAINT		
CEMENT - Van Cld. Portland Cement			
CONTRACTOR - Olson Construction Corp.	CONTRACT DATED	June 25, 1963	
RESIDENT ENGR. - R.S. Zambon	CONTRACT STARTED	July 18, 1963	
INSPECTORS	CONTRACT COMPLETED	Oct 15, 1963	
RECORD PLANS	A.B.M.C. Dauph		

B.M. #1  
 Ch. on Concrete  
 Assum. El. 500.00

Recommend Approval	<i>A. J. Paul</i> District Engineer	5/1/63 Date	
Recommend Approval	<i>Andrew J. ...</i> Bridge Engineer	5/17/63 Date	540
Recommend Approval	<i>R. N. ...</i> Asst. Chief Engineer	5/18/63 Date	530
Approved By	<i>A. J. Paul</i> Chief Engineer	5/17/63 Date	520

LIST OF SHEETS

Sht. No.	Description
1	Plan & Profile
2	Typ. Section & Abut. D.
3	Framing Plan & Steel
4	Pier Details
5	Reinforcing Steel S.
6	Std. Sht. SCB-D8
7	Std. Sht. E-2
8	Std. Sht. G-4
9 & 10	Channel Sections

THETFORD  
 TF 26/1961  
 Repairs to Covered Bridge (14' x 0 span)  
 OLSON CONSTRUCTION CORP.  
 Bridge No. 1 over Chippewa River on  
 Town Rd. No. 29 leading from Theford Center to Rt. 132  
 Town of 03

Boring No. 5  
 to 11+99, 5'lt. Surx. E.

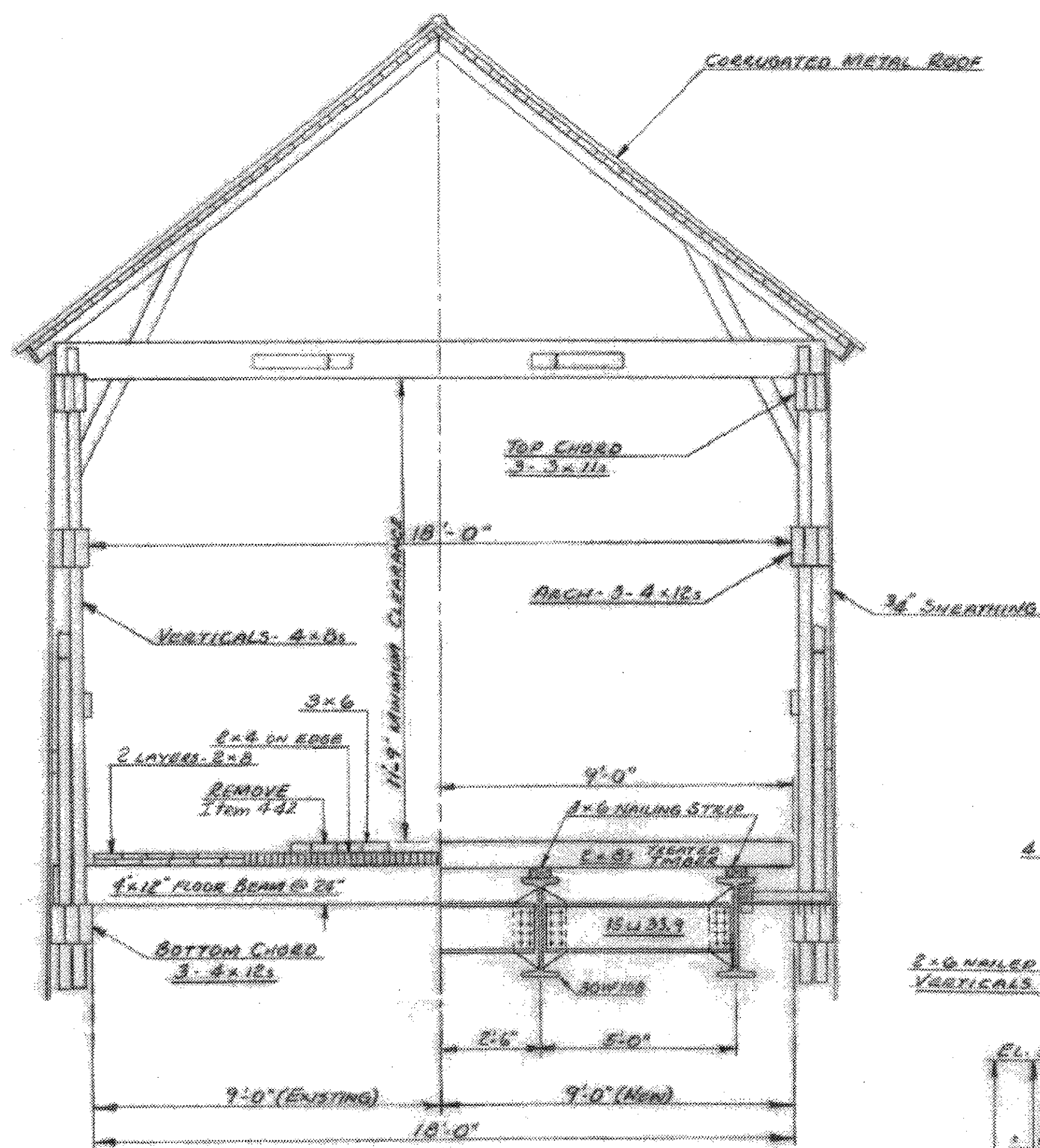
1/4"	Ice
1/4"	Coarse Sand
1/4"	Water Ledge
	E1488.6

Project Name & Number  
 THETFORD TF 26/1961  
 Attachments: Olson Construction Corp.  
 Record Plans: R.S. Zambon  
 Contract Completed: Oct 15, 1963  
 14 Rt. Assumed Elev. 500.00

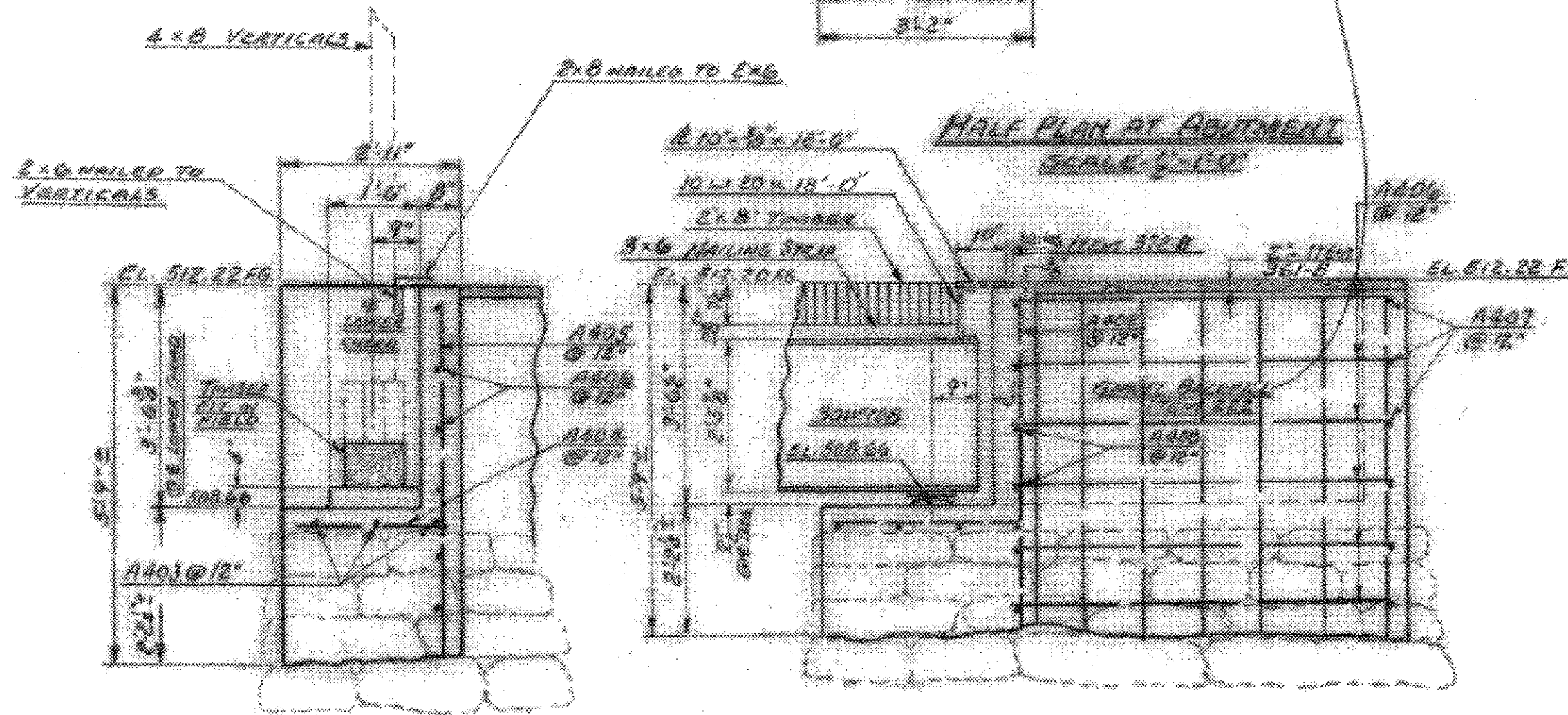
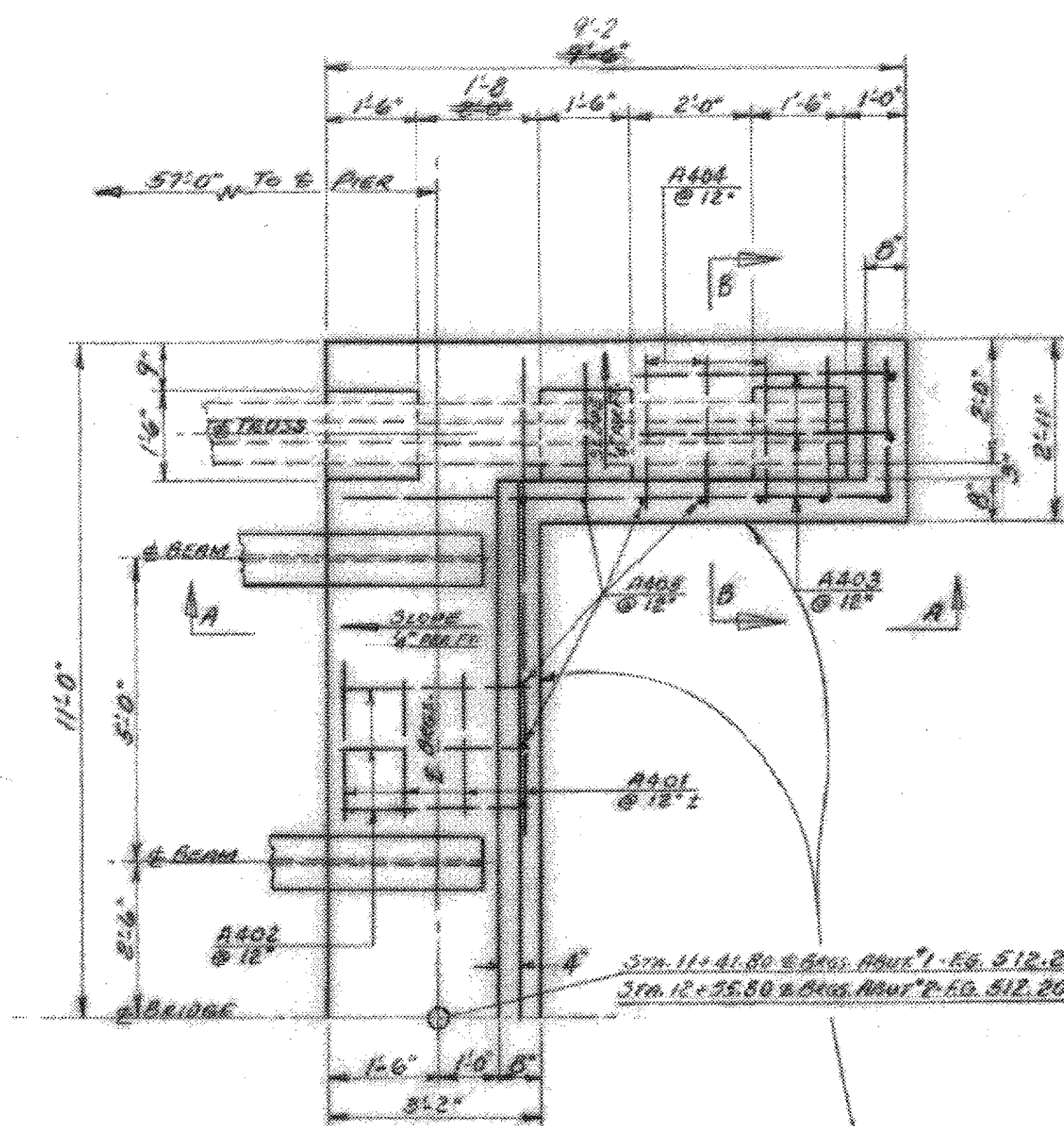
NOTE: ANY FURTHER INFORMATION CONCERNING FINAL QUANTITIES, AMOUNTS OR OTHER DETAILS RELATIVE TO THIS PROJECT MAY BE FOUND IN EITHER THE FIELD BOOKS OR THE ESTIMATE FILE.

THETFORD  
 TF-26-1961

Sheet 1 of 10

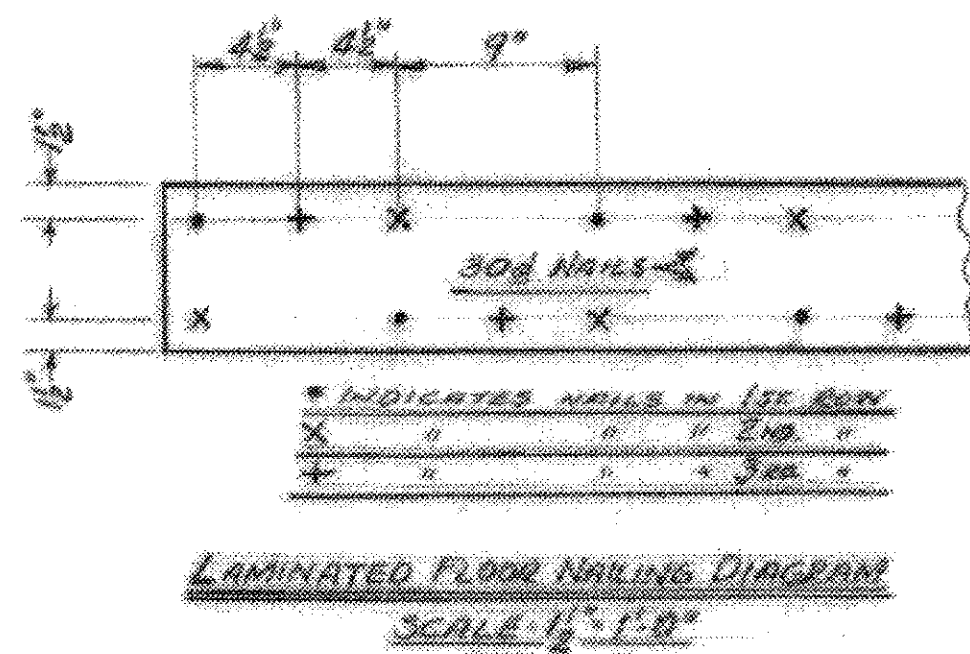


TYPICAL SECTION  
SCALE: 3/4"=1'-0"



SECTION B-B  
SCALE: 1/2"=1'-0"

SECTION A-A  
SCALE: 1/2"=1'-0"



LAMINATED FLOOR NAILING DIAGRAM  
SCALE: 1/2"=1'-0"

**- GENERAL NOTES -**

1. ALL MATERIALS AND CONSTRUCTION SHALL CONFORM TO THE STATE OF VERMONT, DEPARTMENT OF HIGHWAYS, STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, DATED JAN. 1956 AND THE AASHTO STANDARD SPECS. DATED 1961.
2. ANY STRUCTURAL TIMBER USED IN THE REPAIR OF THE EXISTING SUPERSTRUCTURE (CHORDS, ARCHES, VERTICALS, DIAGONALS, OR SHEATHING) SHALL BE OF A GRADE COMPATIBLE TO THE ADJACENT MEMBERS.
3. IF THE CAMBER OF THE SLOTTED JOISTS VARIES, THE THICKNESS OF THE 3x6 NAILING STRIPS SHALL VARY SO AS TO INSURE AN EVEN FINISH FOR THE 2x8 LAMINATED FLOORING.
4. EACH 2x8 STRUCTURAL TIMBER OF THE LAMINATED FLOORING SHALL BE 78d-NAILED AT EACH CROSSING OF A 3x6 NAILING STRIP.
5. THE COST OF NAILS SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEM 512, STRUCTURAL TIMBER.
6. THE COST OF ANY HARDWARE USED FOR TRUSS REPAIR SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEM 404-A, STRUCTURAL STEEL.
7. THE THICKNESS OF STRUCTURAL TIMBER BLOCKING, UNDER THE BOTTOM CHORDS AT THE ABUTMENTS, MAY VARY, AND SHALL BE FITTED IN THE FIELD.
8. ALL EXPOSED EDGES OF CONCRETE WORK TO BE CHAMFERED ONE INCH BY ONE INCH. CONCRETE IN ABUTMENTS TO BE ITEM 404-B, CONCRETE CLASS B (MOD).
9. THE TEMPORARY BRIDGE PROVIDED FOR BY ITEM 441 SHALL BE CONSTRUCTED FOR FOOT TRAFFIC ONLY.
10. THE GRADING OF APPROACHES AND THE USE OF ITEM 361-B, BITUMINOUS CONCRETE PAVEMENT, SHALL BE DIRECTED BY THE RESIDENT ENGINEER.
11. 2x8 STRUCTURAL TIMBERS FOR THE LAMINATED BRIDGE DECK SHALL CONFORM TO THOSE REQUIREMENTS. ALL JOINTS SHOULD BE STAGGERED AS DIRECTED BY THE RESIDENT ENGINEER. MINIMUM LENGTH OF A 2x8 STRUCTURAL TIMBER SHALL BE 6 FEET. EVERY OTHER 2x8 STRUCTURAL TIMBER SHALL BE FULL LENGTH.

**- QUANTITY SUMMARY -**

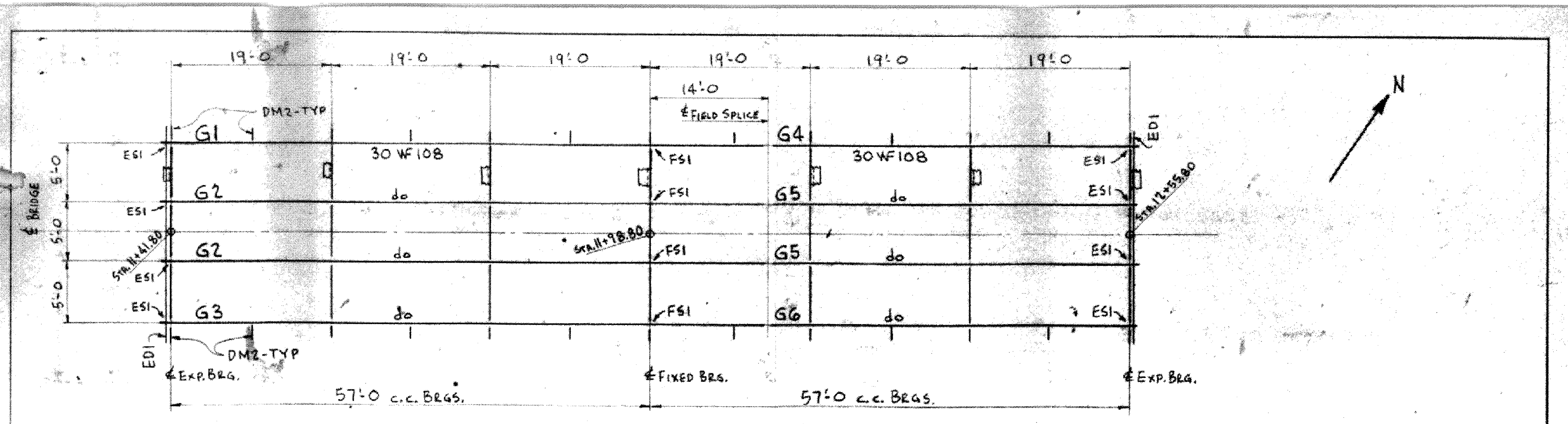
ITEM NO.	ITEM	UNIT	NET	OVERLAP	TOTAL	FINAL
106-A	CHAN EXCAV. OF EARTH	C.Y.				
106-B	CHAN EXCAV. OF ROCK	C.Y.				
106-C	UNCLASS. CHAN EXCAV.	C.Y.				
107	STRUCT. EXCAV.	C.Y.				
401-B	CONC. CLASS B (MOD) (EST)	C.Y.			110	104
402	REINF. STEEL	LBS.			6755	6752
407	ASPHALTIC. ASB. COATING	S.Y.				
502-B	TREATED TIMBER PILING	L.F.				
511-A	PLANK GUARD RAIL WITH FREQ. TREAT.	L.F.			1121	112
507	SHORING SUPERSTRUCTURE (MOD)	L.S.			11	1
512	STRUCTURAL TIMBER (MOD)	M.B.F.			221	22
222	GRAVEL BACKFILL (EST)	C.Y.			40	39
361-B	BIT. CONCRETE PAVEMENT (MOD) (EST)	TON			81	8
372-B	JOINT SEALER OLD APPLIED	L.F.			351	35
404-A	STRUCTURAL STEEL	L.B.			62,700	61,570
441-A	Temp One-way Bridge (Mod) (Est)	L.S.			11	1
442	Removal of Present Superst. (MOD)	L.S.			1	1

STATE OF VERMONT  
DEPARTMENT OF HIGHWAYS

TOWN OF **THETFORD**  
ROAD No. **29** BRIDGE No. **27**

**ABUTMENT DETAILS**  
& TYPICAL SECTION

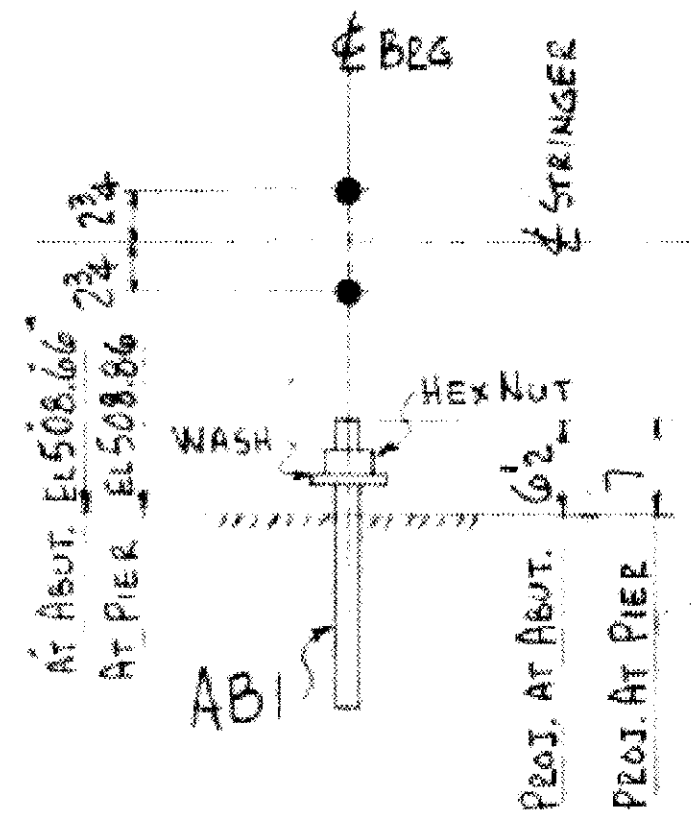
SCALE AS NOTED  
SURVEYED BY **FALL & MERCHANT**  
DRAWN BY **CRG/MS** CHECKED BY **R.P.G.**  
PROJECT No. **TF-26-1961**  
SHEET **2** OF **10**



**FRAMING PLAN**

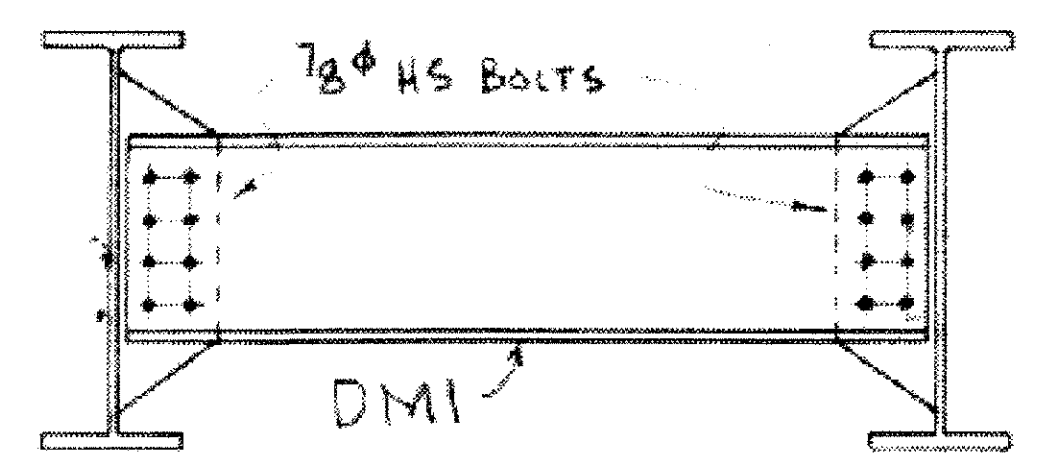
ALL DIAPHRAGMS ARE 15E33.9 - MARK-DMI

- FIELD CONN - 8 #8 BOLTS FOR STRINGER SPICE & DIAPHRAGM CONN.  
 3 # HS BOLTS FOR END DAM CONN.  
 • FIELD WELD LATERAL BRACE AS SHOWN.

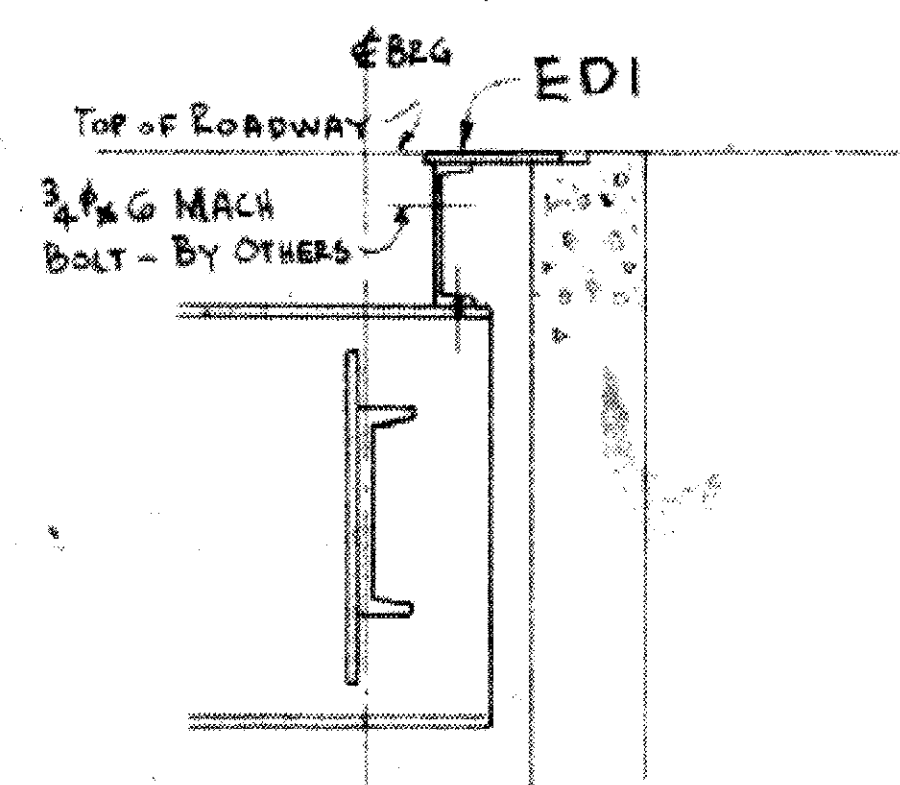


**TYPICAL BEARING**

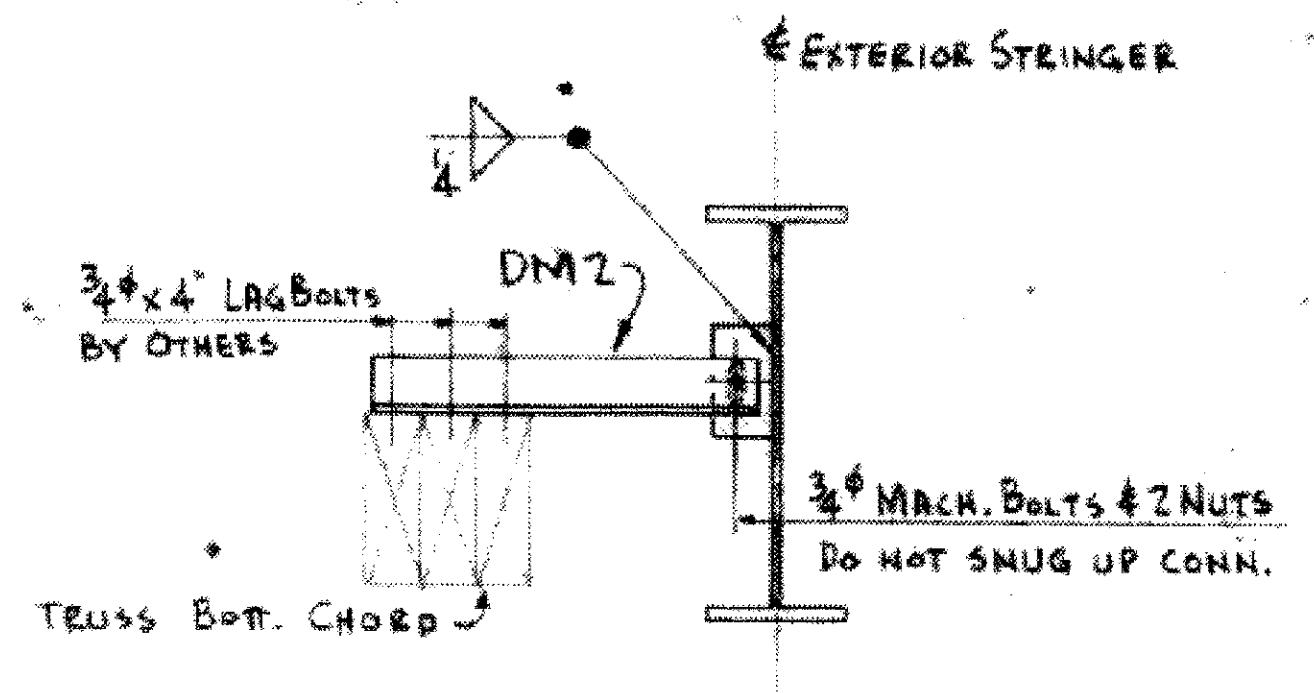
SEE PLAN FOR SHOE MARKS



**TYPICAL DIAPHRAGM DETAIL**



**END DAM DETAIL**

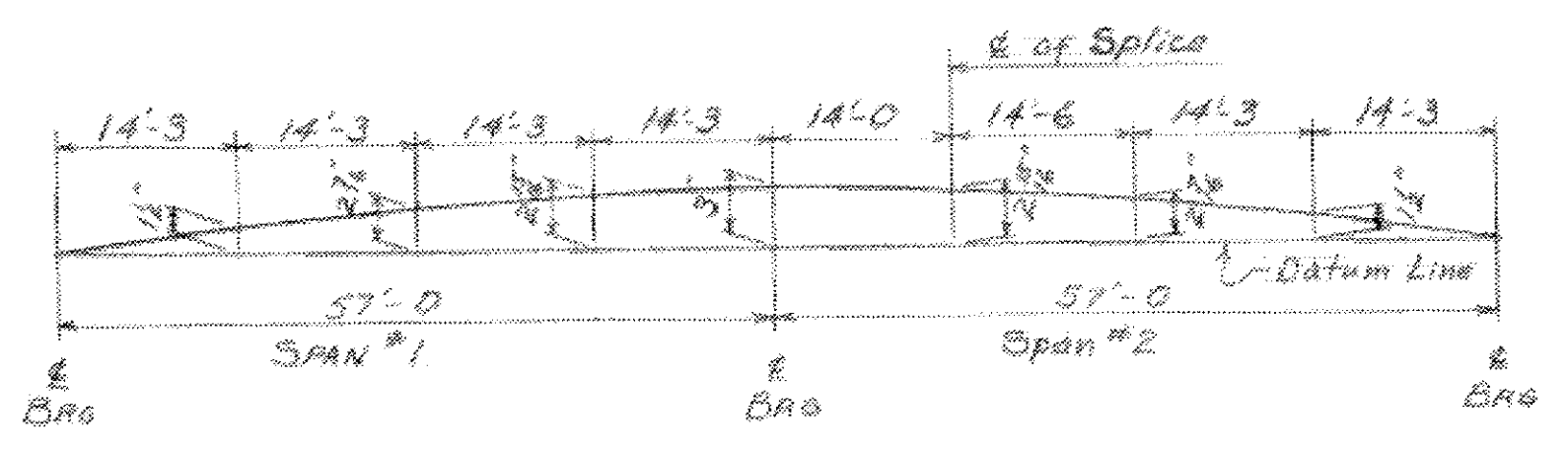
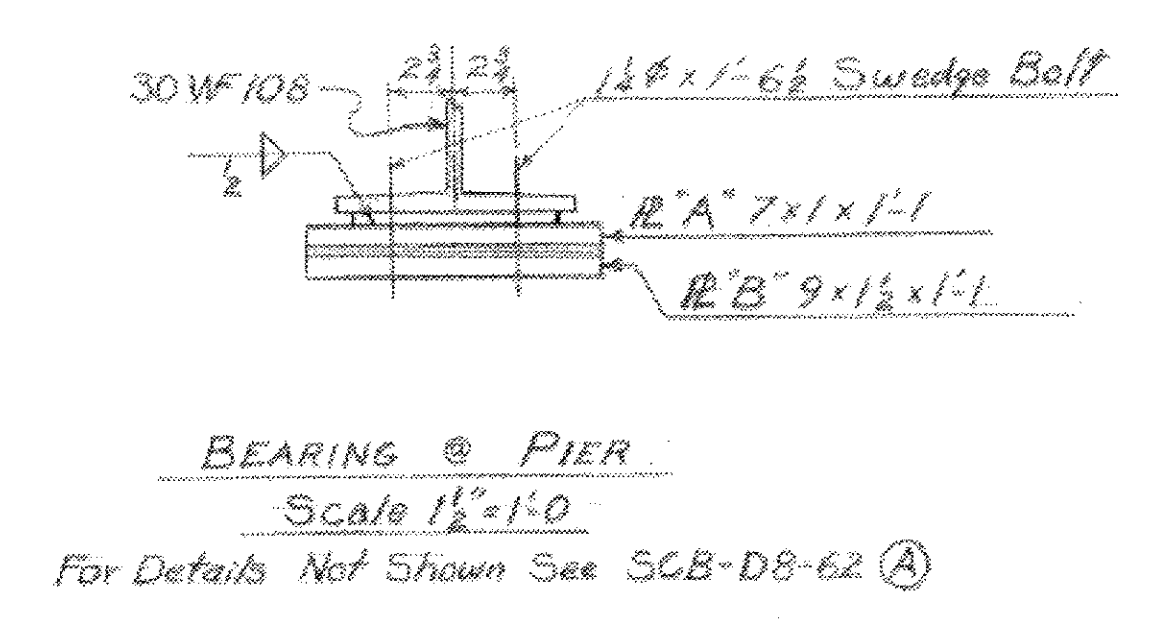
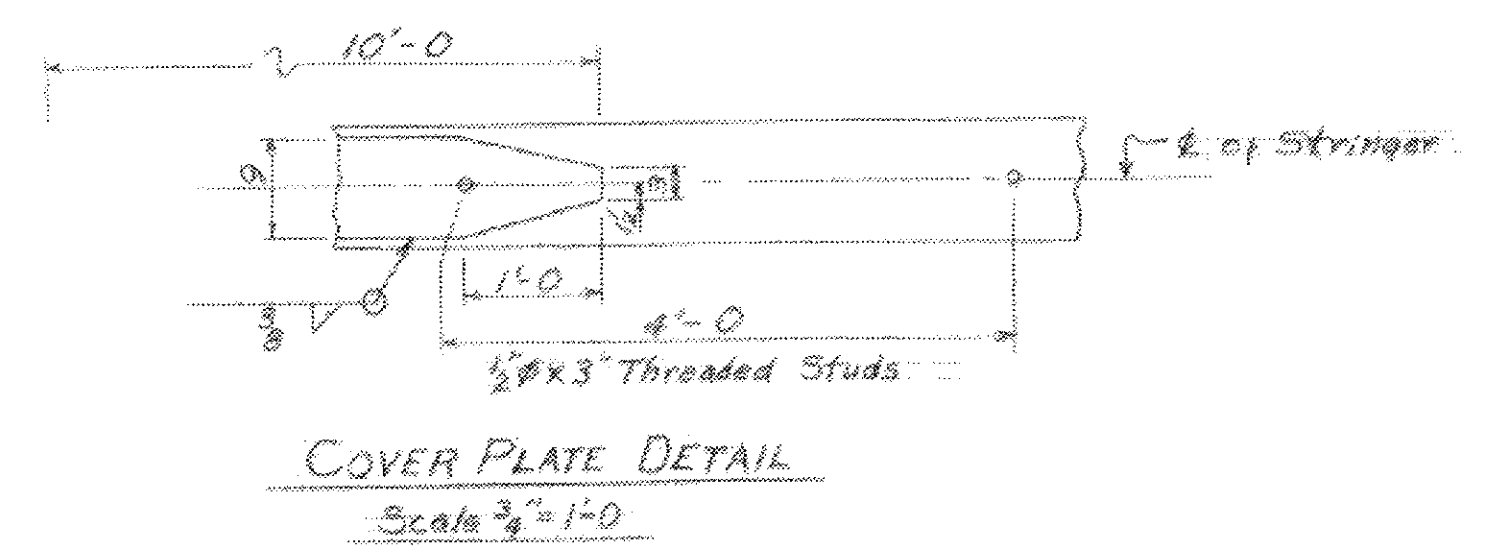
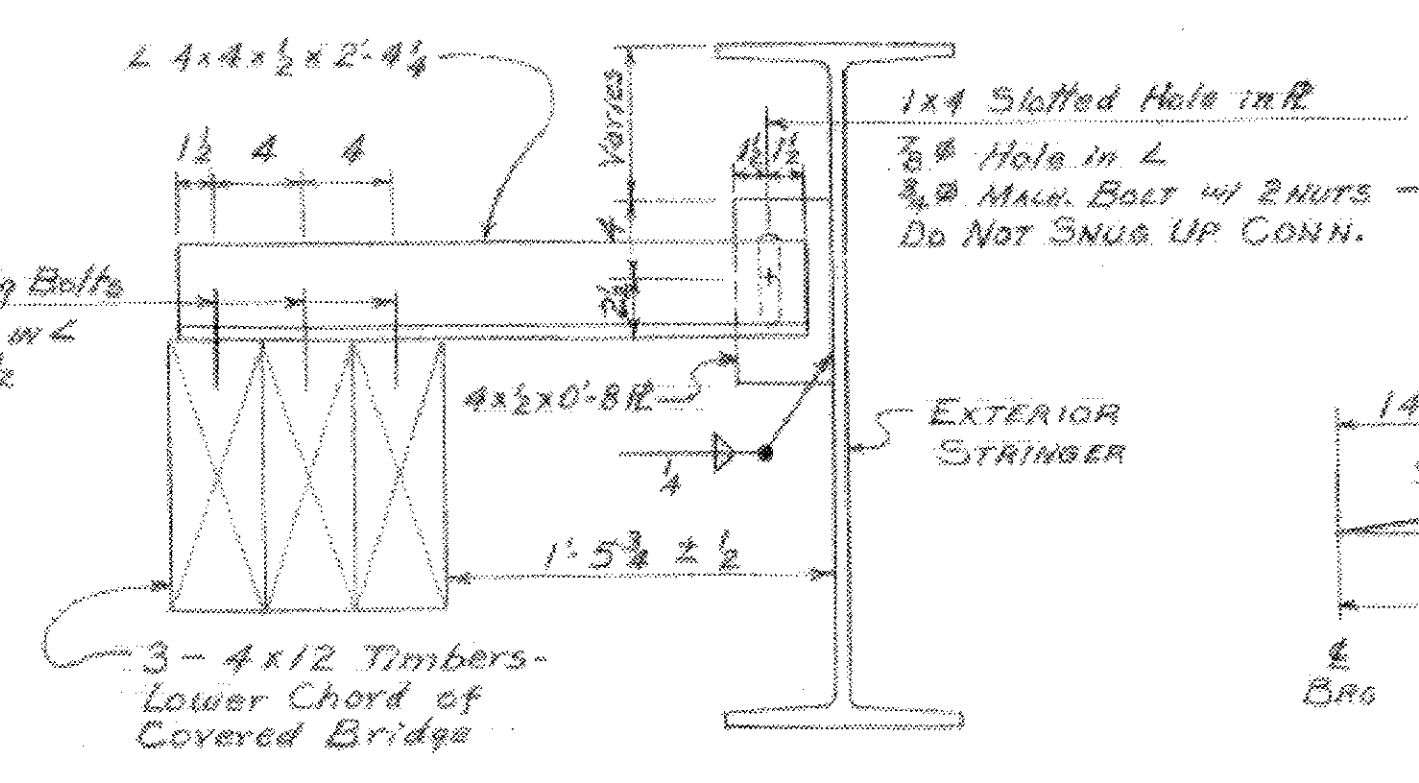
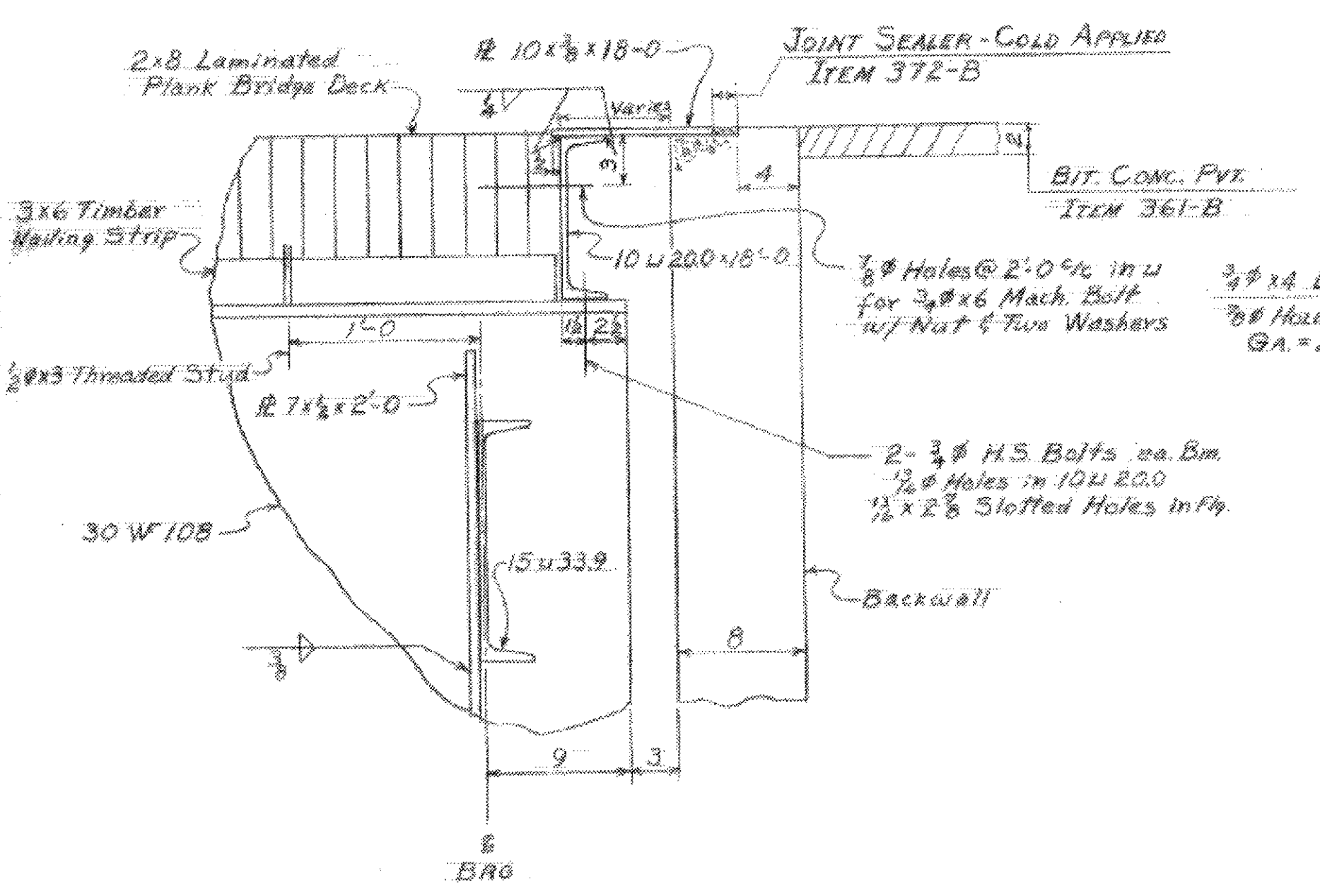
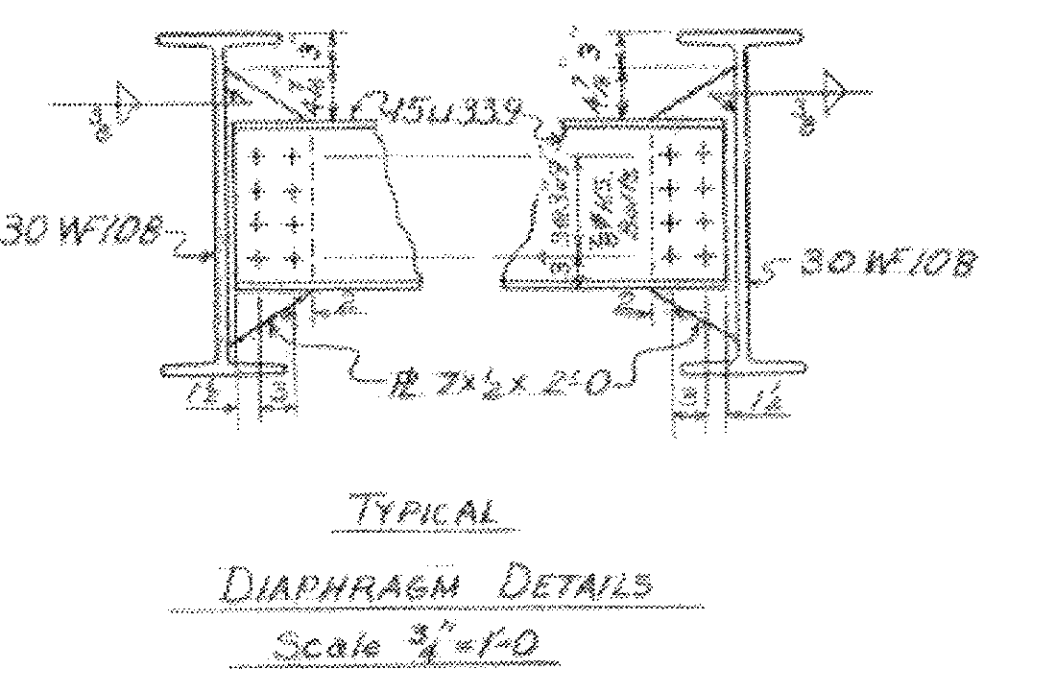
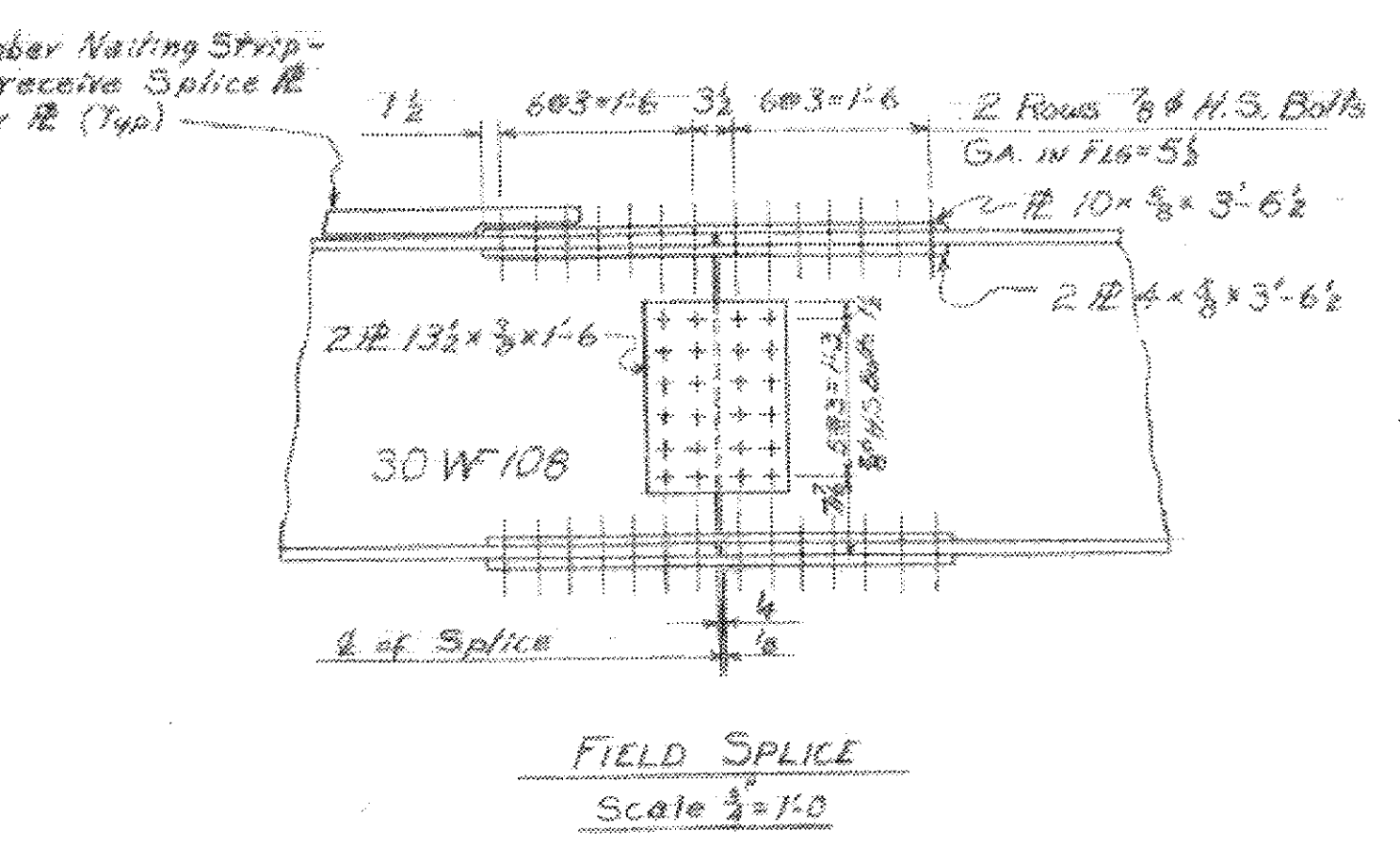
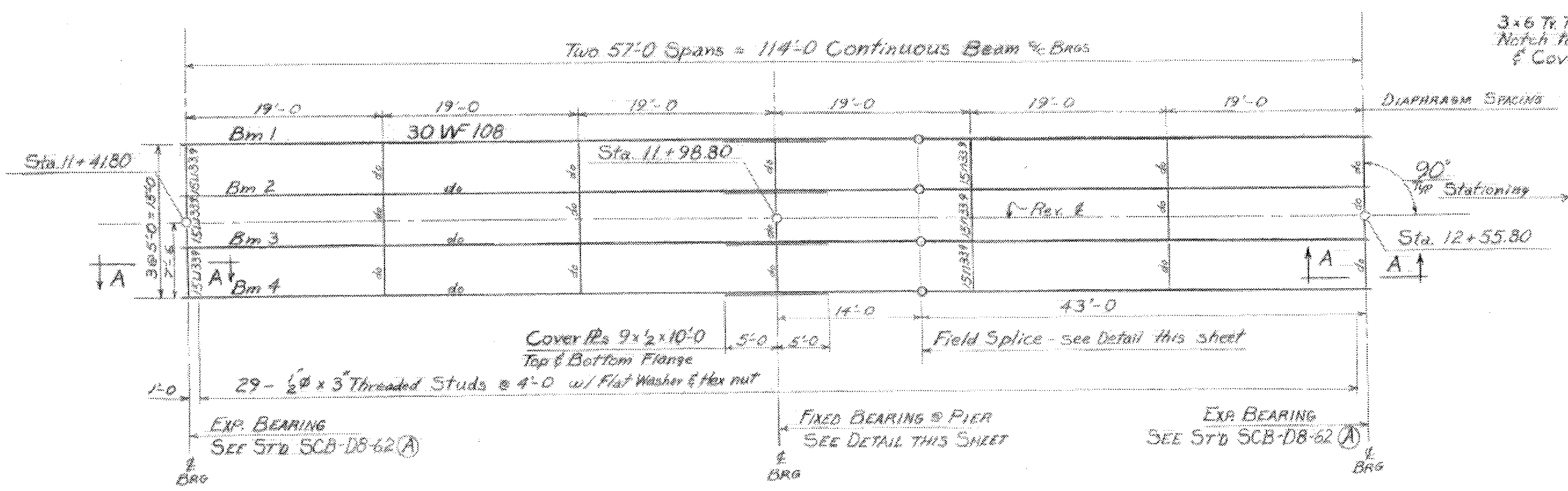


**LATERAL BRACE DETAIL**

UNLESS OTHERWISE NOTED

- RIVETS.....
- BOLTS.....
- MOLS.....
- WELDS.....
- PAINT.....

<b>A. LEO NASH STEEL CORP.</b>		ALNASCO	
Fabricating - Warehousing		PHONE: MILWAUKEE 2-8227 CENTRAL BERKSHIRE BLVD. PITTSFIELD, MASS.	
OWNER STATE OF VERMONT			
STRUCTURE COVERED BRIDGE OVER OMPOMPANOOSUC RIVER			
LOCATION TOWN OF THETFORD, VERMONT			
ARCHITECT VT. DEPT. OF HIGHWAYS		CONTRACTOR OLSON CONST.	
OWN BY LB	CHK'D BY	DATE 8/2	DWG NO 371-E1



BRACE MID-WAY BETWEEN AND AT EACH DIAPHRAGM LOCATION (26 Req'd)

- NOTES**
- All structural steel shall conform to ASTM Designation A36-62T. Shop drawings of the steel details shall be submitted to the State of Vermont, Department of Highways for approval.
  - Unless otherwise noted all bolts shall be High-Strength conforming to ASTM Designation A325.
  - Final Field Coat of paint shall be Dark Gray. Spraying of Shop Coat will be permitted.
  - Distances are measured horizontally or vertically.
  - Stringers are designed for H20-44 loading with 7 1/2" concrete deck. Laminated wood floor is designed for H15-44 loading.

ITEM NO.	ITEM	UNIT	NET	OVERRUN	TOTAL	FINAL
106-A	CHAN. EXCAV. OF EARTH	C Y				
106-B	CHAN. EXCAV. OF ROCK	C Y				
106-C	UNCLASS. CHAN. EXCAV.	C Y				
107	STRUCT. EXCAV.	C Y				
401-B	CONC. CLASS B (MOD.1)	C Y				
402	REINF. STEEL	LBS.				
407	ASPHALTIC ASB. COATING	S F				
502-B	TREATED TIMBER PILING	L F				
503	SPLICERS FOR STEEL PILING	EA.				
504	STEEL PILING	L F				
502-A	UNTREATED TIMBER PILING	L F				
404-A	STRUCTURAL STEEL	LBS.			62,700	61,576

STATE OF VERMONT  
DEPARTMENT OF HIGHWAYS

TOWN OF **THETFORD**

ROAD NO. **27** BRIDGE NO. **29**

FRAMING PLAN & STEEL DETAILS

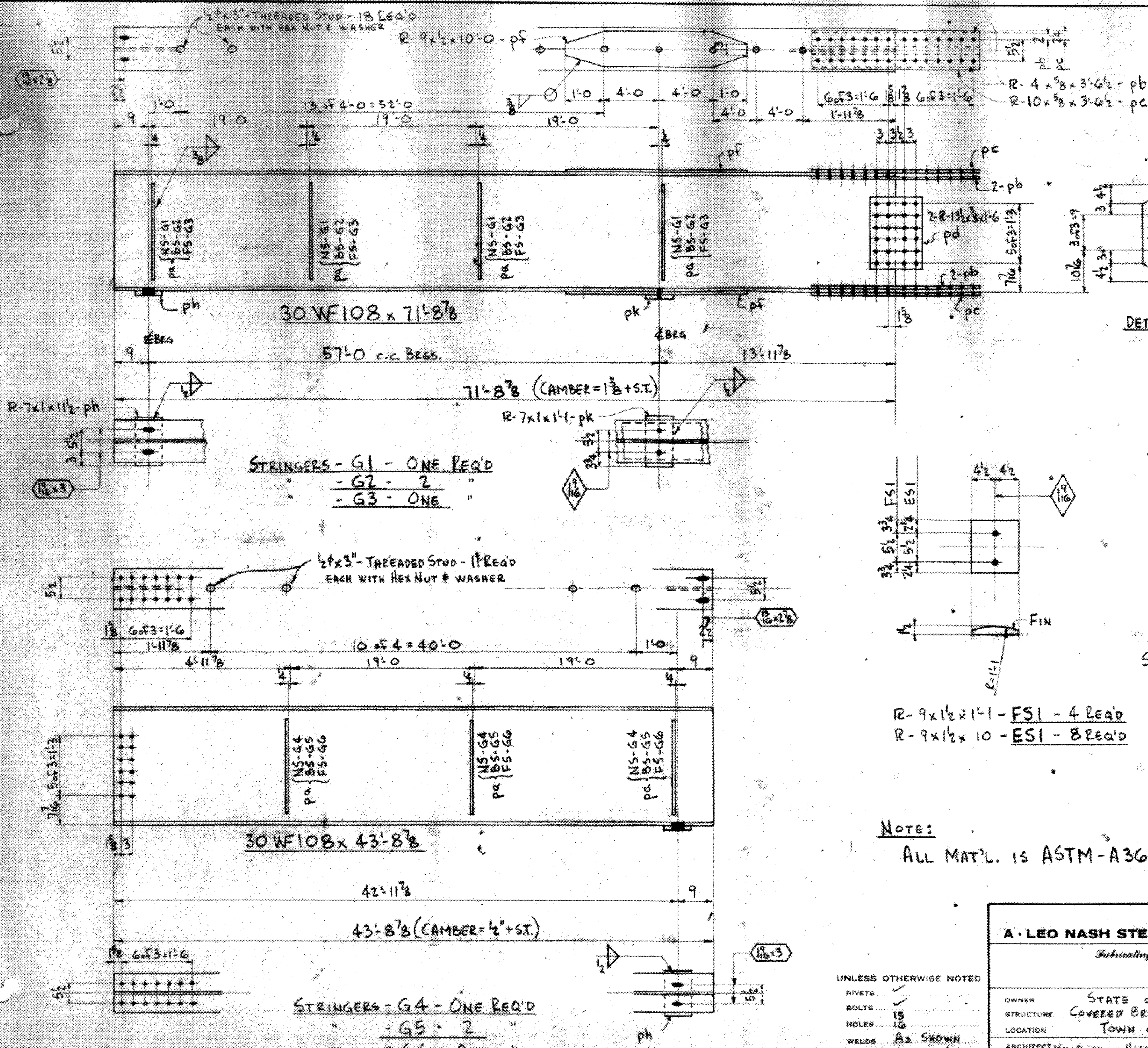
SCALE **AS NOTED**

SURVEYED BY **FALL**

DRAWN BY **RIM-25-4** CHECKED BY **RND**

PROJECT NO. **TF 26-1961**

SHEET **3** OF **10**



STRINGERS - G1 - ONE REQ'D  
 - G2 - 2  
 - G3 - ONE

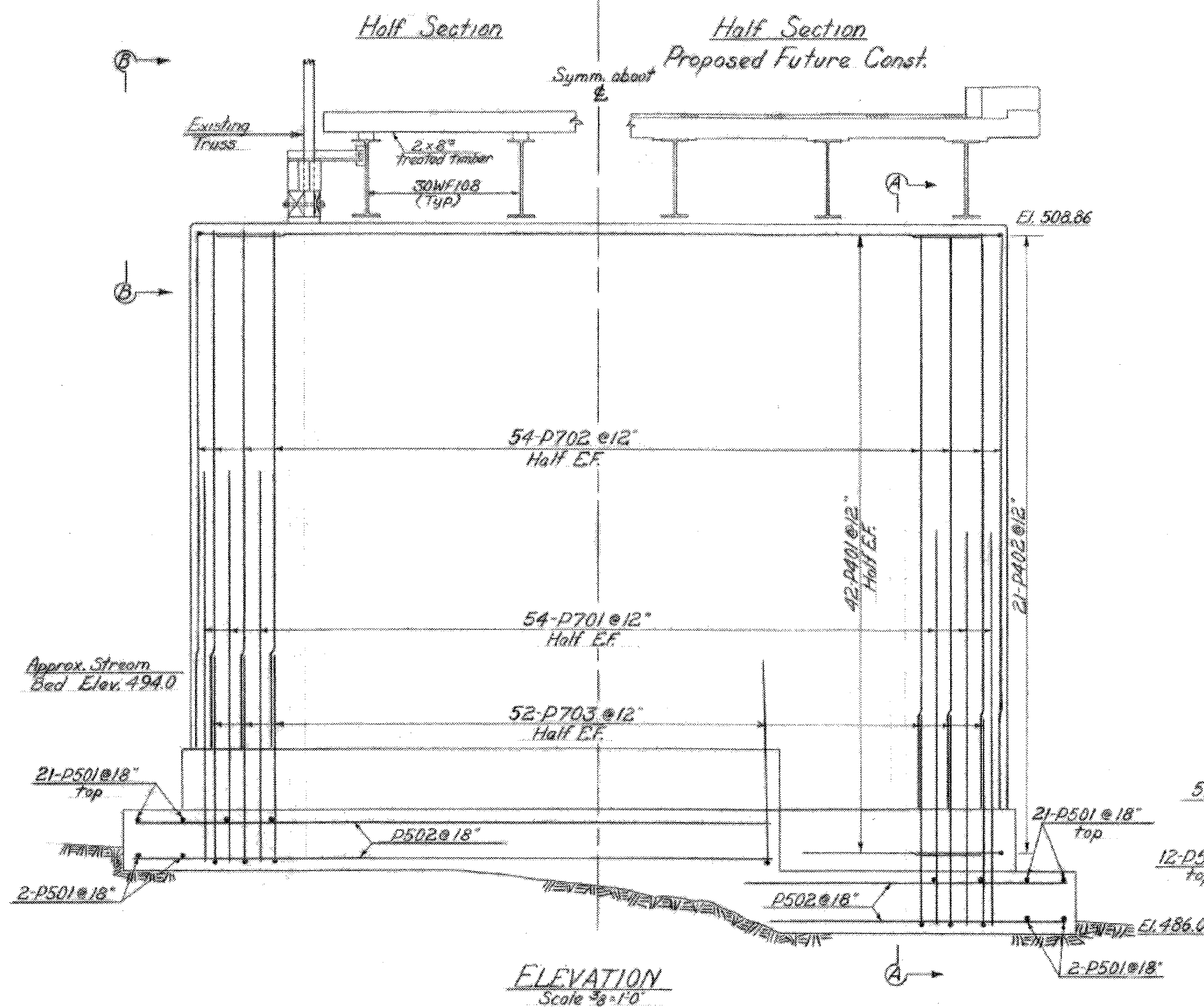
STRINGERS - G4 - ONE REQ'D  
 - G5 - 2  
 - G6 - ONE

NOTE:  
 ALL MAT'L. IS ASTM-A36.

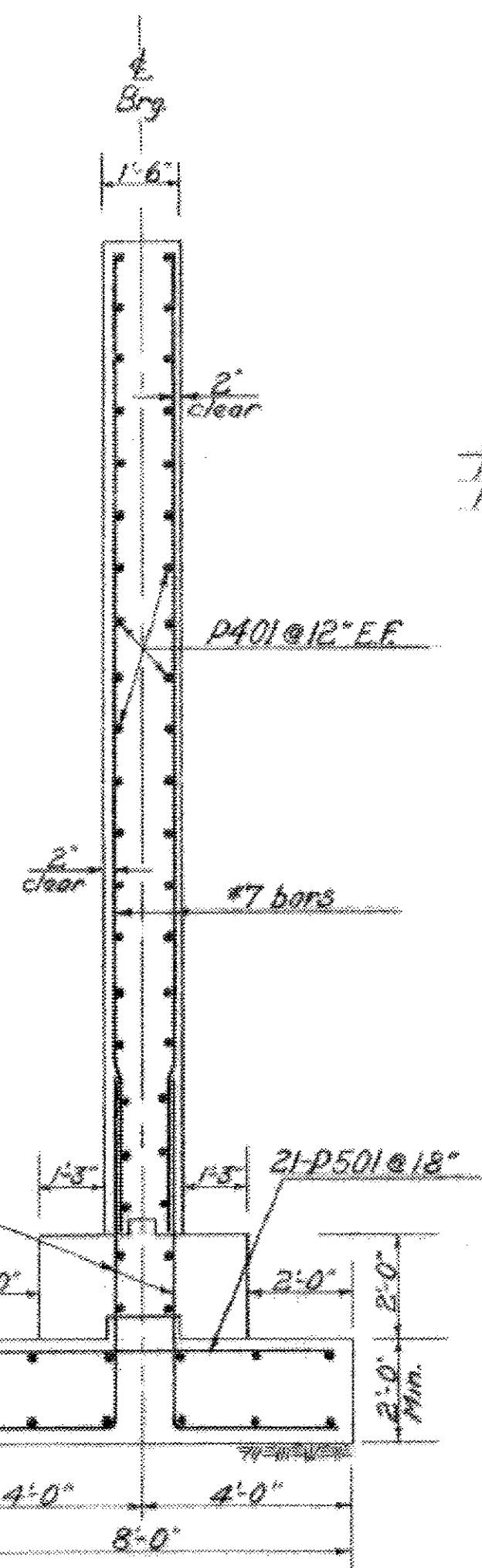
UNLESS OTHERWISE NOTED  
 RIVETS ✓  
 BOLTS 15  
 HOLES 16  
 WELDS AS SHOWN  
 PAINT VERMONT SPEC.  
 RED LEAD

A. LEO NASH STEEL CORP.		ALNASCO	
Fabricating, Warehousing		PHONE: HILLCREST & BOST. CENTRAL VERMONT BLVD. PITTSFIELD, MASS.	
OWNER STATE OF VERMONT			
STRUCTURE COVERED BRIDGE OVER OMPOMPANOOSUC RIVER			
LOCATION TOWN OF THETFORD, VERMONT			
ARCHITECT VT. DEPT. OF HIGHWAYS		CONTRACTOR OLSON CONST.	
DWR BY JB	CHKD BY	DATE 8/1	10-63-371 DWG NO. 371-1

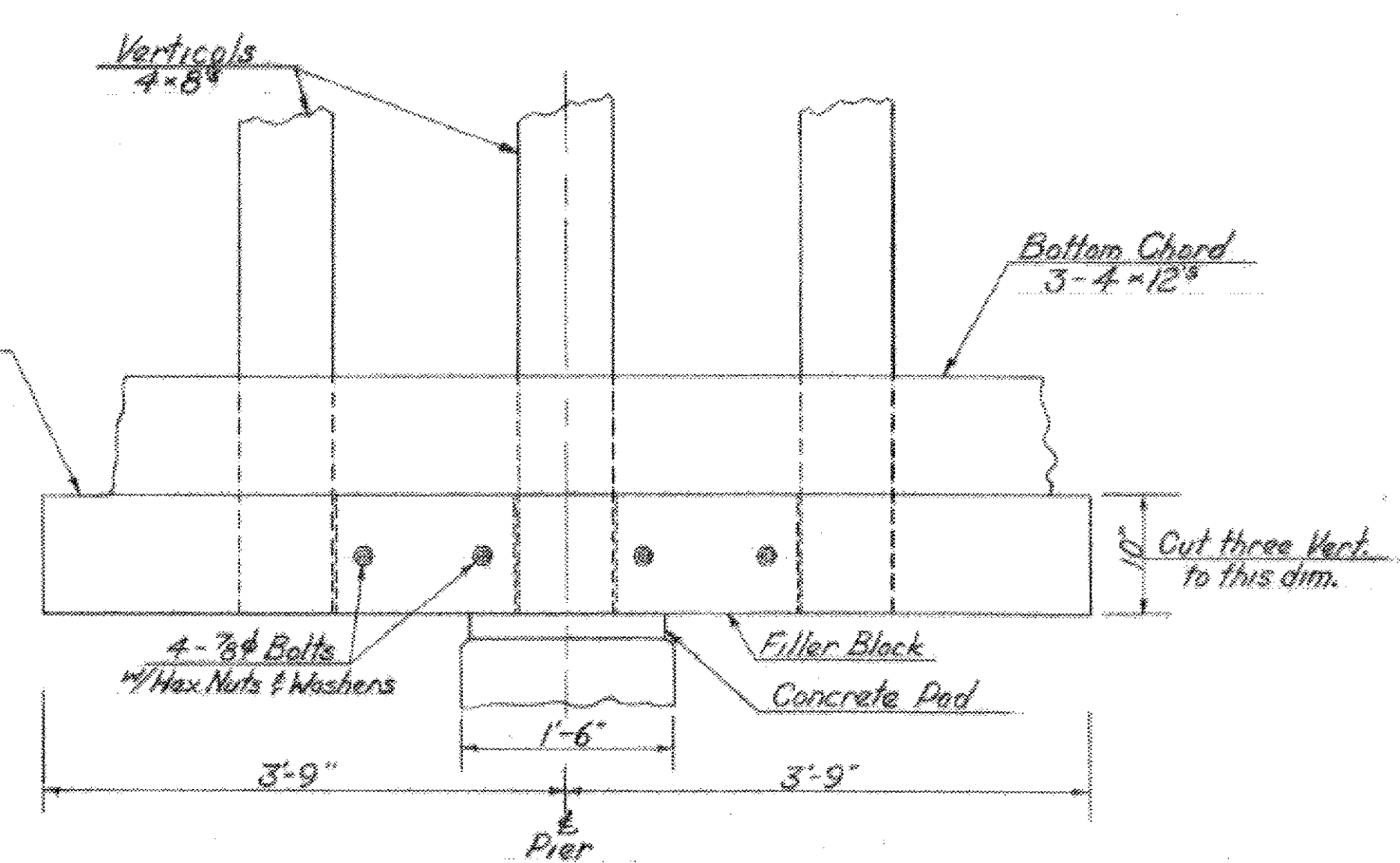
REQUIRED						ASTM-A36				
SHOP BILL						MILL ORDER				
LN	ASSEMBLY MARK	NO.	SHAPE & SECTION	LENGTH	REMARKS	WEIGHT	NO.	SHAPE & SECTION	LENGTH	ITEM
1										
2			ONE STRINGER	G1						
3			2 STRINGERS	G2						
4			ONE STRINGER	G3						
5			4	30 WF108	71 8 7/8					CAMBER 1/8
6	ph	8	R	9 x 1/2	10 0					
7	pc	8	R	10 x 5/8	3 6 1/2					
8	pb	16	R	4 x 5/8	3 6 1/2					
9	pd	8	R	13 1/2 x 3/8	1 6					
10	ph	4	R	7 x 1	11 1/2					
11	pk	4	R	7 x 1	1 1					
12	pa	24	R	7 x 2	2 0					4-G1, G3 18-G2
13			72	STUD - 1/2 x 3"						
14			72	HEX NUTS - 1/4" TAP						
15			72	WASH - 1/6" HOLE						
16			40	SHIPPING BOLTS - 7/8"						
17										
18										
19										
20										
21										
22										
23										
24			ONE STRINGER	G4						
25			2 STRINGERS	G5						
26			ONE STRINGER	G6						
27			4	30 WF108	43 8 7/8					CAMBER 1/8
28	ph	4	R							
29	pa	18	R							13-44, G6 16-G5
30			44	STUD - 1/2 x 3"						
31			44	HEX NUTS - 1/4" TAP						
32			44	WASH - 1/6" HOLE						
33										
34										
35										
36										
37	FS1	4	R	9 x 1/2	1 1					MACH
38										
39	ESI	8	R	9 x 1/2	10					MACH
40										
41										
42										
43			24	SWEDGE BOLTS - ABI						
44			24	1/4" 1/4"	1 6 1/2					
45			24	WASH - 3/8 x 3/8 - 1/16" HOLE						
46			24	HEX NUTS - 1/4" TAP						
47										
48										
49										
50										
51										
52										
53										
54										
55										
56										
57										
58										
59										
60										
61										
62										



ELEVATION  
Scale 3/8"=1'-0"

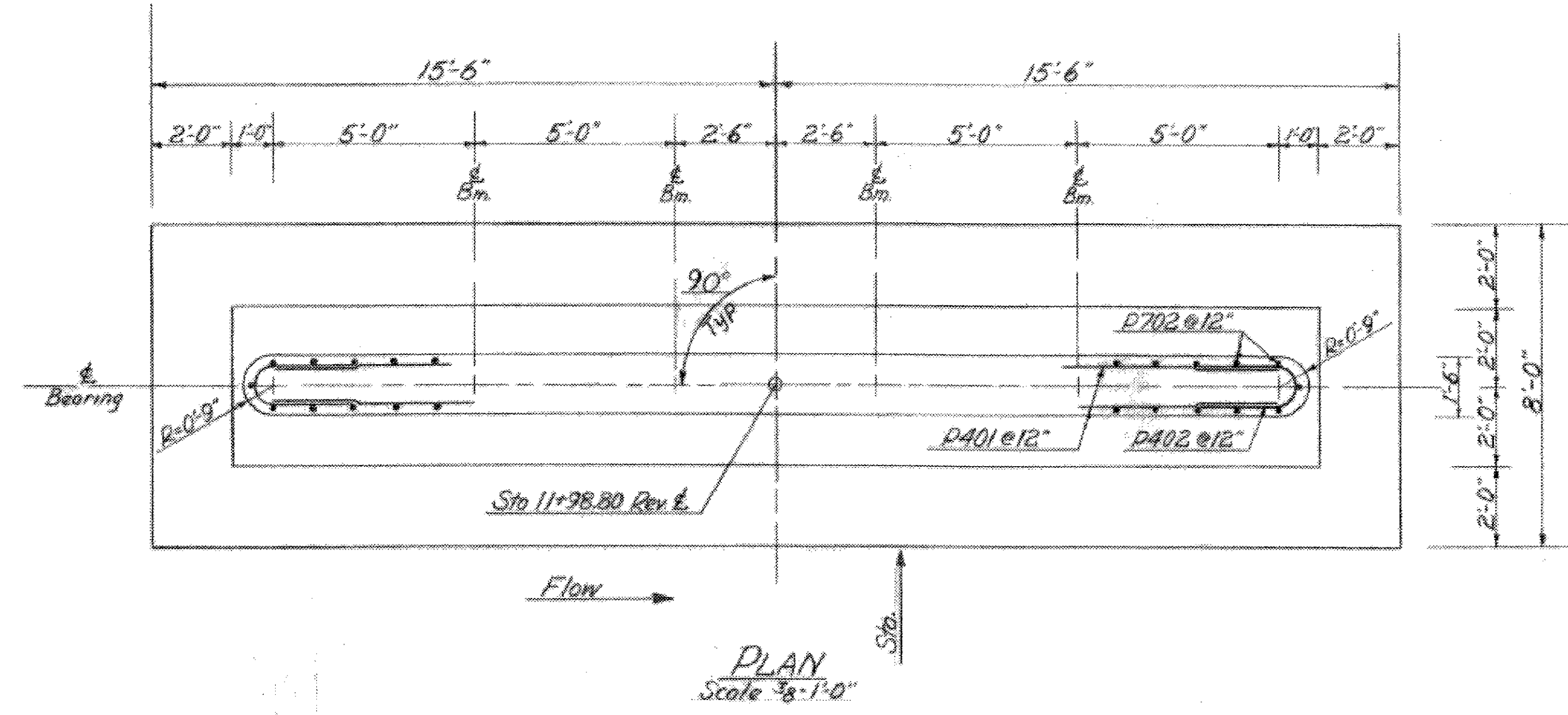


SECTION AA  
Scale 3/8"=1'-0"



SECTION B-B  
Scale 1/4"=1'-0"

Note: All wood blocking shall be of Treated Timber.



PLAN  
Scale 3/8"=1'-0"

Note: All reinforcing steel to have clear cover of 3" unless otherwise noted.

ITEM NO.	ITEM	UNIT	NET	OVERRUN	TOTAL	FINAL
105-A	CHAN EXCAV OF EARTH	C.Y.				
106-B	CHAN EXCAV OF ROCK	C.Y.				
106-C	UNCLAS. CHAN EXCAV	C.Y.				
107	STRUCT EXCAV	C.Y.			90	80
401-B	CONC CLASS B (MOD.)	C.Y.			54	60
402	REINF. STEEL	LEBS			5930	5928
407	ASPHALTIC-ARB. COATING	S.Y.				
502-B	TREATED TIMBER PILING	L.F.				
503	SPLICES FOR STEEL PILING	EA				
504	STEEL PILING	L.F.				
502-A	UNTREATED TIMBER PILING	L.F.				

STATE OF VERMONT  
DEPARTMENT OF HIGHWAYS

TOWN OF THETFORD  
ROAD No. TH-29 BRIDGE No. 27  
PIER

SCALE as noted  
SURVEYED BY \_\_\_\_\_  
DRAWN BY RND CHECKED BY RPG  
PROJECT NO. TF-26-1961  
SHEET 4 OF 10





State of Vermont  
PDD/Structures Design Section  
National Life Building - Drawer 33  
Montpelier, VT 05633-5001  
www.aol.state.vt.us

[phone] 802-898-2621  
[fax] 802-898-2556  
[toll] 800-253-0191

Agency of Transportation

D. S. Brown  
300 E. Cherry Street  
North Baltimore, OH 45872

DATE: April 23, 2007

Project Name: Thetford Project #: BRO 1444 (43)

Structure Identification: Bridge # 27 on Town Highway 26, Savres Covered Bridge

The following Bearing details [Item #531.10, Bearing Device Assembly (Fabric)] for the above project (Vendor's Job #19866) transmitted with your letter dated March 30, 2007, have been reviewed and are being returned herewith.

Sheets: 1 of 1  
Is approved.

The welding and bonding procedures are also approved.

You must provide written notice to this office as to the date fabrication represented by these drawings will begin. That notice must be received at least seven days prior to that date, as per Specification 506.03. Any material fabricated prior to the notification date is subject to rejection without further cause.

Sincerely,

Martha Evans-Mongeon  
Project Manager

Attachments

cc:  Resident Engineer, Danny Breer  
 Shop Inspector, Jeff Clark  
 Contractor w/prints, Alpine Construction  
 Subcontractor - letter only \_\_\_\_\_  
 Construction Division - letter only  
 Materials & Research Section (C&IA Unit) - letter only  
 Files (Structures & Central)





**DSBROWN Production Joint Welding Procedure Specification (D1.5-02)**

Procedure No: A-GTF-01 Date Issued: 1-9-04 Revision No: 0 Rev. Date: \_\_\_\_\_  
 Contractor (Fabricator) D.S. Brown Company Prepared by: James R. Connor, Quality Assurance Manager  
 1. Non-Fracture Critical  Fracture Critical  WPS Expiration Date: \_\_\_\_\_  
 2. Qualified in accordance with: AWS D1.5-2002, AWS D1.6-99  
 Referenced PQR No(s): PQR-GTAW-01-03  
 Referenced FWST No(s): PQR-GTAW-01(03)  
 3. Material specification(s) ASTM A709 Gr. 36, 50, 50W, 304SS, 316SS For DOT Approval  
 4. Material Thickness (es) Unlimited  
 5. Welding process GTAW  
 6. Manual , machine , or semiautomatic   
 7. Position(s) of welding 1F, 2F  
 8. Filler metal specification AWS A5.9  
 9. Filler metal class and brand name ER309L (Murrex)  
 10. Flux class & brand N/A, Type N/A  
 11. Shielding gas 100% Argon Flow rate 20 CFH  
 12. Single pass  Or multiple pass   
 13. Single arc  Or multiple arc   
 14. Welding Current DCEN  
 15. Polarity Straight  
 16. Welding progression stringers  
 17. Root treatment Clean to bright sound metal or per AWS D1.5 (3.2.1 & 3.11)  
 18. Postheat treatment N/A  
 19. Calculated Heat Input (KJ/in) Min 10.9 KJ Max 20.4 KJ  
 20. Electrode extension (electrical stickout) N/A

TRANS RECEIVED  
 APR 09 2007  
 APPROVED  
 DATE 4/18/07

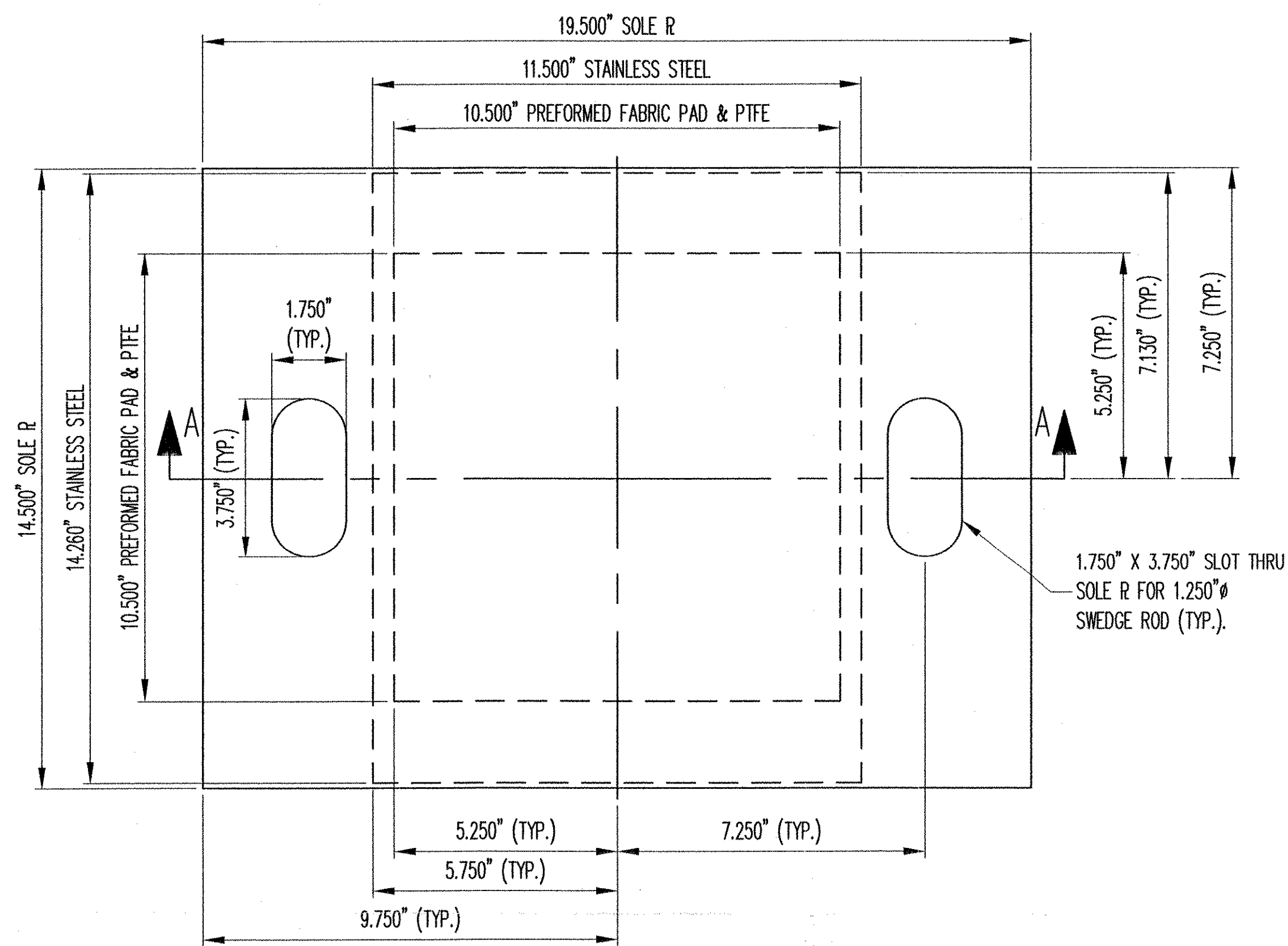
Weld size (in)	Pass	Electrode Size (in)	Welding Process Variables		Travel Speed (IPM)	Joint Detail (Flare Bevel)
			AMPS/WFS*	VOLTS		
20 ga.	1	1/8"	170-200	15-17	10-14	<p>T<sub>1</sub> = Varies                      S = Fillet Weld Size                      (Fillet weld must not exceed thickness of stainless steel)</p>
16 ga.	1	1/8"	170-200	15-17	10-14	
14 ga.	1	1/8"	170-200	15-17	10-14	
12 ga.	1	1/8"	170-200	15-17	10-14	
11 ga.	1	1/8"	170-200	15-17	10-14	
10 ga.	1	1/8"	170-200	15-17	10-14	
8 ga.	1	1/8"	170-200	15-17	10-14	
3/16"	1	1/8"	170-200	15-17	10-14	

\* Wire feed speed may be used along with amperage (include chart)

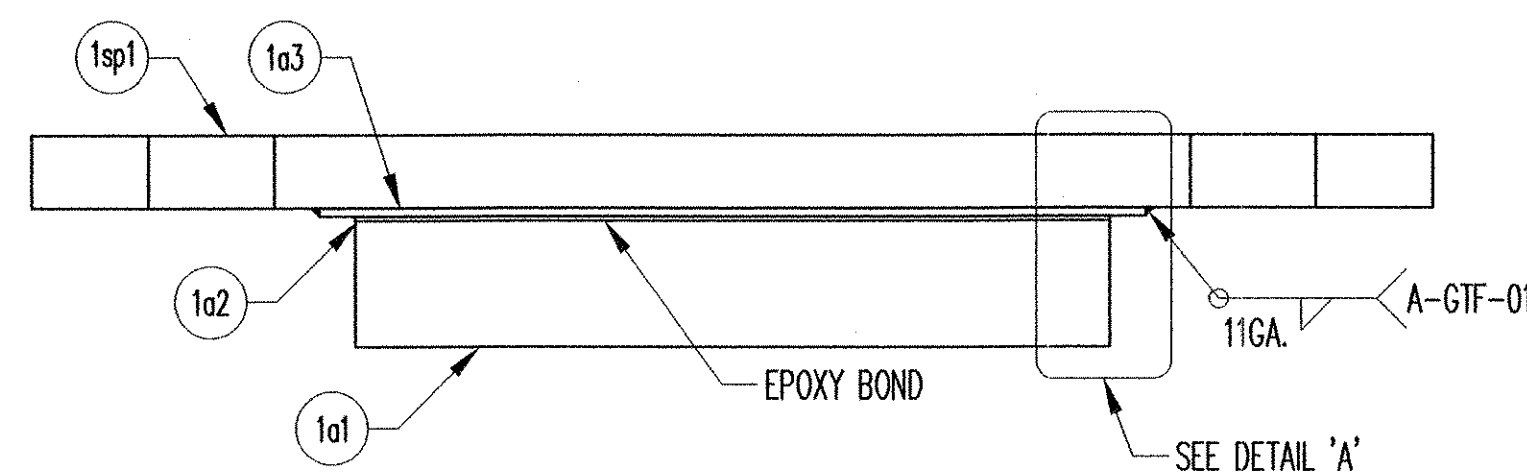
Base Metal Thickness range	Preheat and Interpass Temperature Chart	
	Minimum Preheat (°F)	Max Preheat & Interpass (°F)
≤ 3/4"	50°F	450°F
>3/4" to ≤1.5"	70°F	450°F
>1.5" to ≤2.5"	150°F	450°F
>2.5"	225°F	450°F

Prepared By: James R. Connor DSB QA Manager  
 Project: \_\_\_\_\_  
 DSB Job: 19866-1106

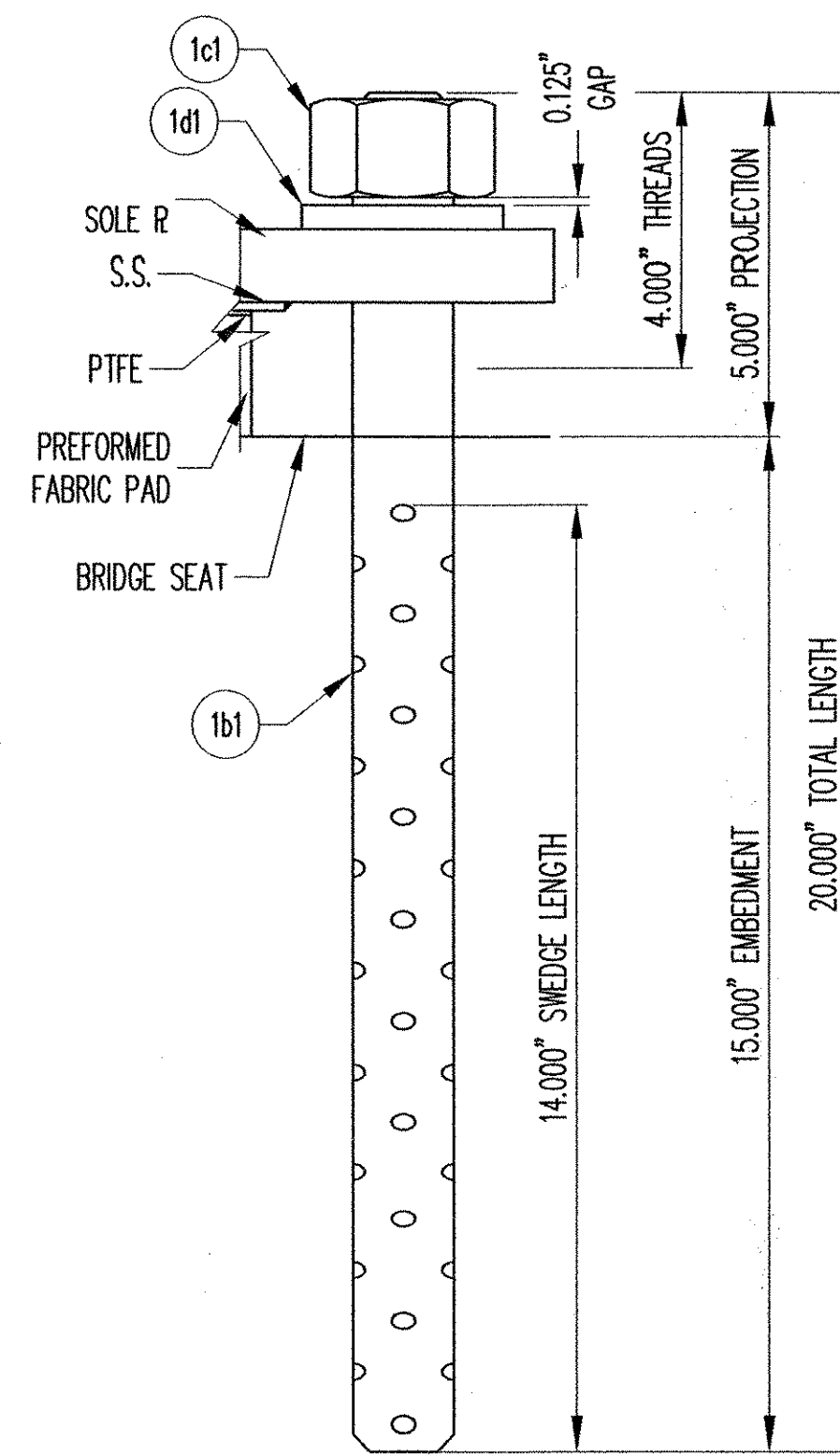
Note: When this procedure is used for A709Gr50W materials, it shall be limited to 5/16" single pass or material be coated.



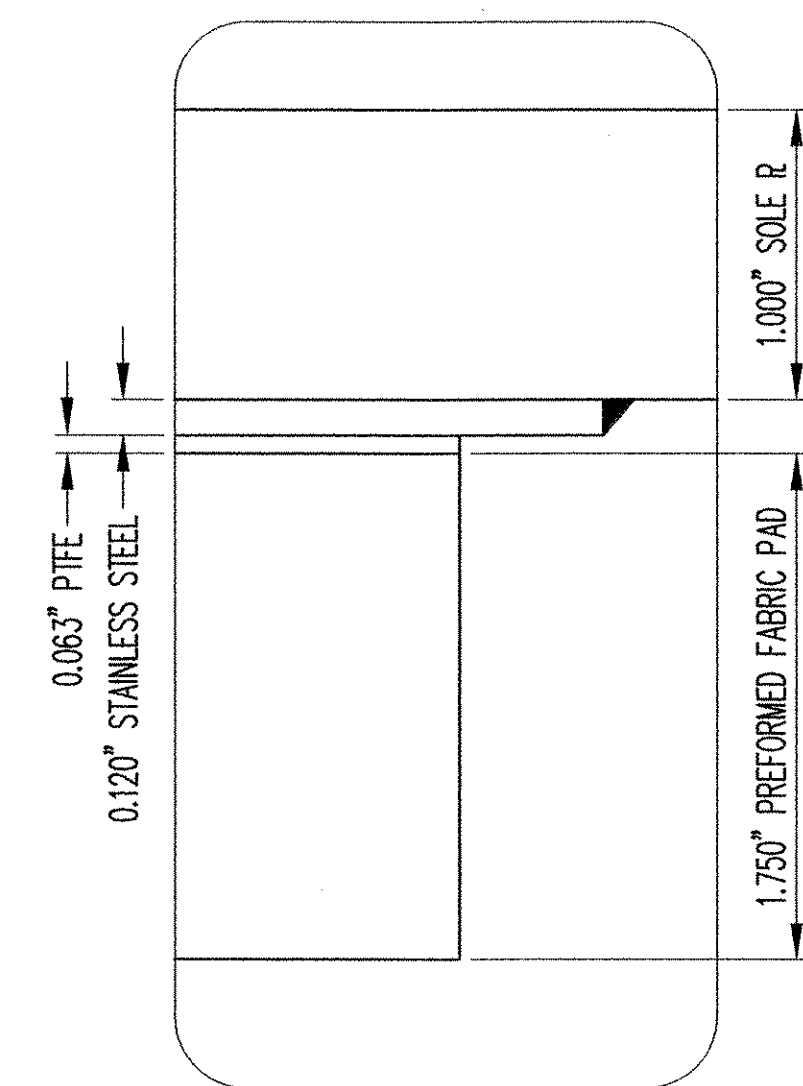
PLAN VIEW  
SLIDING PREFORMED FABRIC BEARING  
(4) REQ'D @ ABUT. 1



SECTION 'A-A'



SWEDGE ROD DETAIL



DETAIL 'A'

LOAD TABLE	
LOAD DATA	
DEAD LOAD REACTION (KIPS)	10 -kips
LIVE LOAD REACTION (KIPS)	16 -kips
TOTAL LOAD REACTION (KIPS)	26 -kips

PLEASE PROVIDE

MK	QTY.	DESCRIPTION	MATERIAL	LENGTH	REMARKS	PART NO.
1A	4	EXPANSION FABRIC BEARING				
1a1	4	1.750" X 10.500"	AASHTO 18.4.9.1	10.500"		
1a2	4	0.063" X 10.500"	PTFE	10.500"	PURE VIRGIN UNFILLED	
1a3	4	11 GA. X 11.500"	A240 1304, #8 & 2B	11.500"	PLAIN	
1sp1	4	1.000" X 19.500"	A709 GR 50 OR 50W	14.500"	A123-HDG	
1B	8	SWEDGE ROD				
1b1	8	1.250" X 20.000" SWEDGED ROD	A449		4" THREADS, 14" SWEDGE, A153-HDG	
1C	8	HEAVY HEX NUT				
1c1	8	1.250" HEX NUT	A563-DH OR A194-2H		A153-HDG	
1D	8	PLATE WASHER				
1d1	8	0.375" X 3.000"	A36	3.000"	1.313" DIA. HOLE @ CENTER, A123-HDG	

NOTE: ALL MATERIAL SPECIFICATION DESCRIPTIONS ARE ASTM, UNLESS OTHERWISE NOTED.

GENERAL NOTES:

- PAD AND MATERIALS SHALL CONFORM TO STATE OF VERMONT, AGENCY OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, DATED 2001 AND THE LATEST REVISIONS, INCLUDING SUPPLEMENTARY SPECIFICATIONS, CONTRACT PLANS, AND THE SPECIAL PROVISIONS. GENERAL SHOP PRACTICES, STRUCTURAL FABRICATION, WELDING AND ASSEMBLY SHALL BE GOVERNED BY ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.
- THIS SHOP DRAWING WAS PREPARED IN ACCORDANCE WITH THE CONTRACT PLANS AND SPECIFICATIONS. THE D.S. BROWN COMPANY DOES NOT ACCEPT LIABILITY FOR THE DESIGN OF THE PRODUCTS DETAILED IN THIS SHOP DRAWING.
- THE D.S. BROWN COMPANY TO SUPPLY ONLY THE PARTS SHOWN ON THIS DRAWING.
- ALL STEEL SHALL BE PRODUCED IN THE UNITED STATES OF AMERICA.
- ALL CORNERS AND EDGES OF STEEL PLATES SHALL BE GROUND TO A 0.063" RADIUS FOR GALVANIZING.
- PTFE SHALL BE PURE VIRGIN UNFILLED POLYTETRAFLUOROETHYLENE.
- STAINLESS STEEL SHALL CONFORM TO ASTM A240-TYPE 304 AND SHALL BE 11 GA. (0.120"). STAINLESS STEEL SLIDING SURFACES IN CONTACT WITH PTFE SHALL HAVE A NO. 8 MIRROR FINISH AND ALL OTHERS SHALL HAVE A 2B FINISH.
- ALL STRUCTURAL STEEL SURFACES TO BE GALVANIZED SHALL BE CLEANED IN ACCORDANCE WITH SSPC-SP10.
- ALL EXTERNAL STEEL SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123. GALVANIZATION LIFTING DEVICES MAY BE WELDED TO PARTS IF NECESSARY. WHEN THEIR USE IS COMPLETE, REMOVE AND GRIND FLUSH ALL CONNECTION LOCATIONS. REPAIR ALL GALVANIZATION IN ACCORDANCE WITH ASTM A780. GALVANIZING REPAIR PAINTS CONTAINING ALUMINUM SHALL NOT BE USED ON SURFACES TO BE IN CONTACT WITH FRESH CONCRETE.
- UNLESS OTHERWISE NOTED, DIMENSIONAL TOLERANCES & SURFACE FINISHES SHALL SATISFY THE REQUIREMENTS OF AASHTO DIVISION II, SECTION 18, TABLE 18.5.1.5-1.
- THE TEMPERATURE OF THE STEEL ADJACENT TO THE PTFE & FABRIC PAD SHALL NOT EXCEED 300°F (149°C). TEMPERATURE SHALL BE CONTROLLED BY THE USE OF TEMPERATURE INDICATING CRAYON OR OTHER DEVICES APPROVED BY THE ENGINEERS.
- DS BROWN MAY SUBSTITUTE A709 GR. 50W FOR A709 GR. 50 DUE TO AVAILABILITY AT NO ADDITIONAL COST TO THE OWNER OR CONTRACTOR.

**D.S. BROWN**

THE D.S. BROWN COMPANY  
300 E. CHERRY STREET  
NORTH BALTIMORE, OHIO 45872  
419.257.3561  
FAX: 419.257.0332  
WWW.DSBROWN.COM

REV.	DESCRIPTION	DATE	DET.	CRD.

LOCATION — TUCKER HILL RD/116 OVER OMPOMPANOSUC BRIDGE REHABILITATION PROJECT	ITEM	QUANTITY
BRIDGE — 27	19866-1106-1	4 OF 4
PROJECT — BHO 1444 (43)		
F.A.P. NO. — THETFORD		
P.O. NO. — 007-049-008		
DESIGNER — VT DOT		
CUSTOMER — ALPINE CONSTRUCTION, LLC		

SCALE: N.T.S.	DRAWN BY: DA	CHECKED BY: NRH	DATE: 3/29/07
PROJECT NUMBER: 19866	PRODUCT CODE: 1106	RELEASE: 1	SHEET: 01



State of Vermont  
PDD/Structures Design Section  
National Life Building - Drawer 33  
Montpelier, VT 05633-5001  
www.aot.state.vt.us

[phone] 802-828-2621  
[fax] 802-828-2666  
[tdd] 800-233-0191

Agency of Transportation

LCI, Laminate Concepts Inc.  
P. O. Box 369  
3310 State Rte. 352  
Big Flats, NY 14814

DATE: March 09, 2007

Project Name: Thetford Project #: BRO 1444 (43)

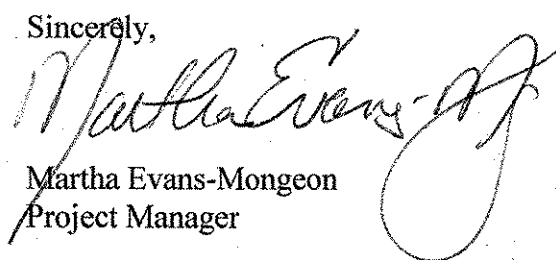
Structure Identification: Bridge # 27 on Town Highway 26, Savres Covered Bridge

The following Glu-laminated Deck details [Item #522.40, Structural Glued Laminated Timber (Deck)] for the above project (Vendor's Job #0207-115-LT) transmitted with your letter dated Feb. 26, 2007, have been reviewed and are being returned herewith.

Sheets: 1 and 2 of 2  
Are Approved.

You must provide written notice to this office as to the date fabrication represented by these drawings will begin. That notice must be received at least seven days prior to that date, as per Specification 506.03. Any material fabricated prior to the notification date is subject to rejection without further cause.

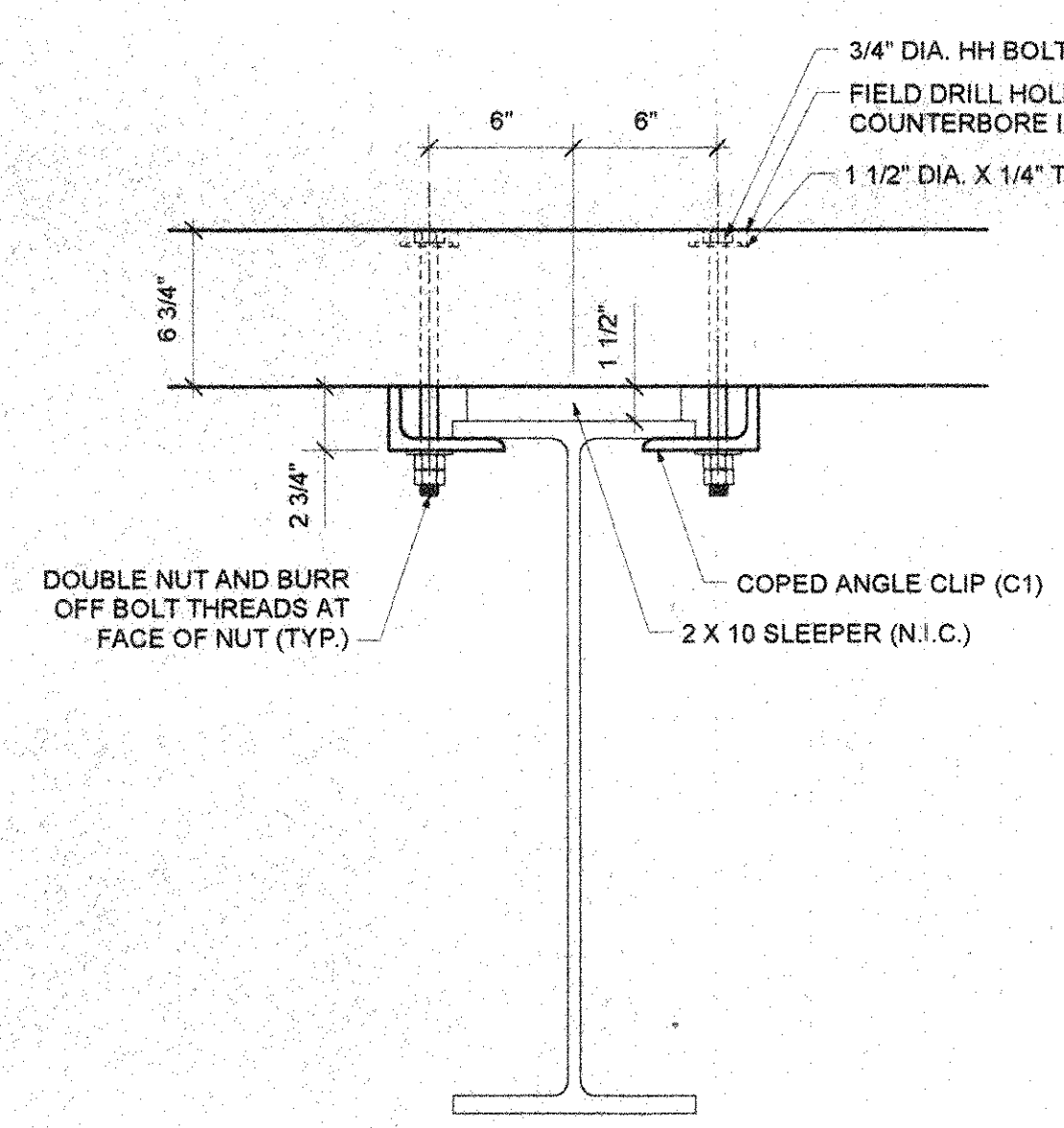
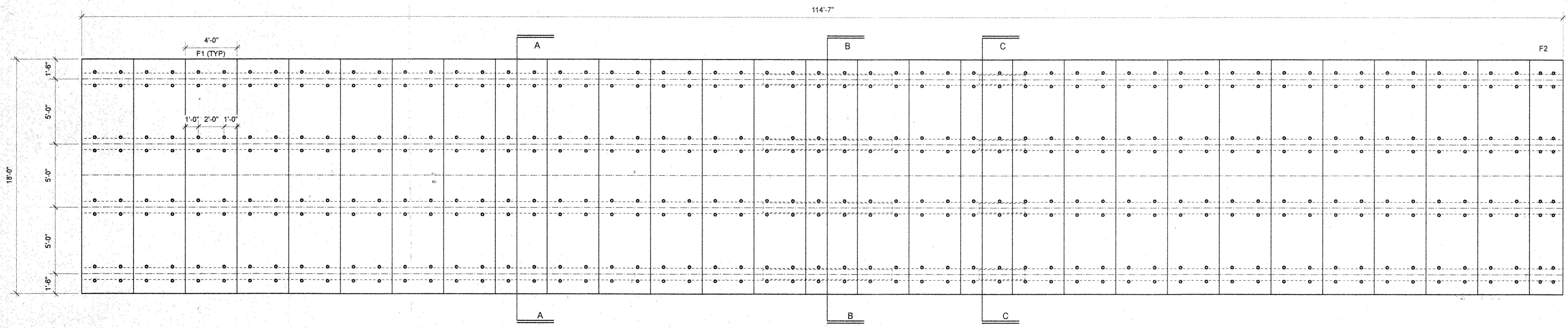
Sincerely,



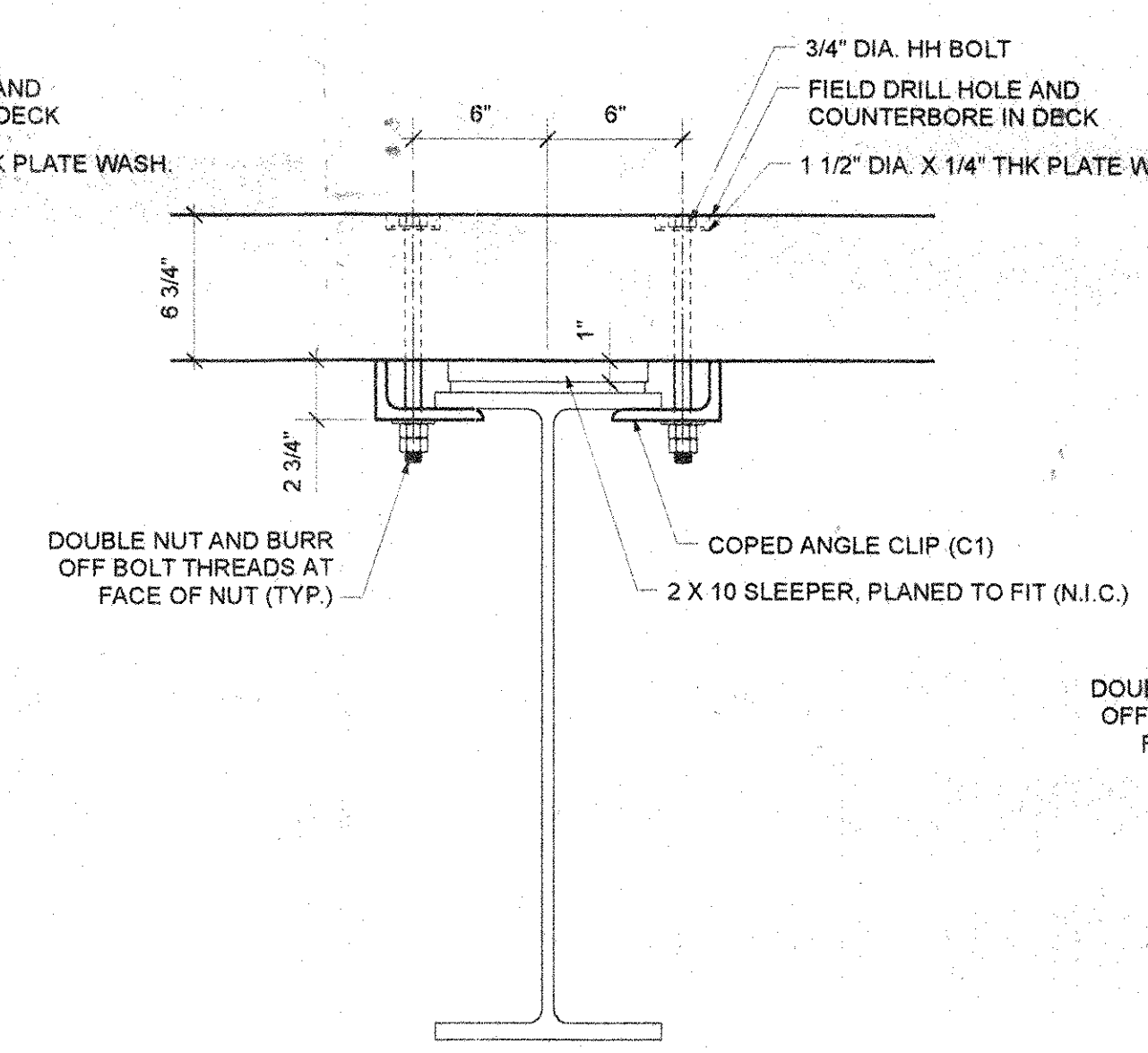
Martha Evans-Mongeon  
Project Manager

Attachments  
cc:  Resident Engineer w/prints, Danny Breer  
 Shop Inspector w/prints, Jeff Clark  
 Contractor w/prints, Alpine Construction  
 Subcontractor - letter only  
 Construction Division - letter only  
 Materials & Research Section (C&IA Unit) - letter only  
 Files (Structures & Central)

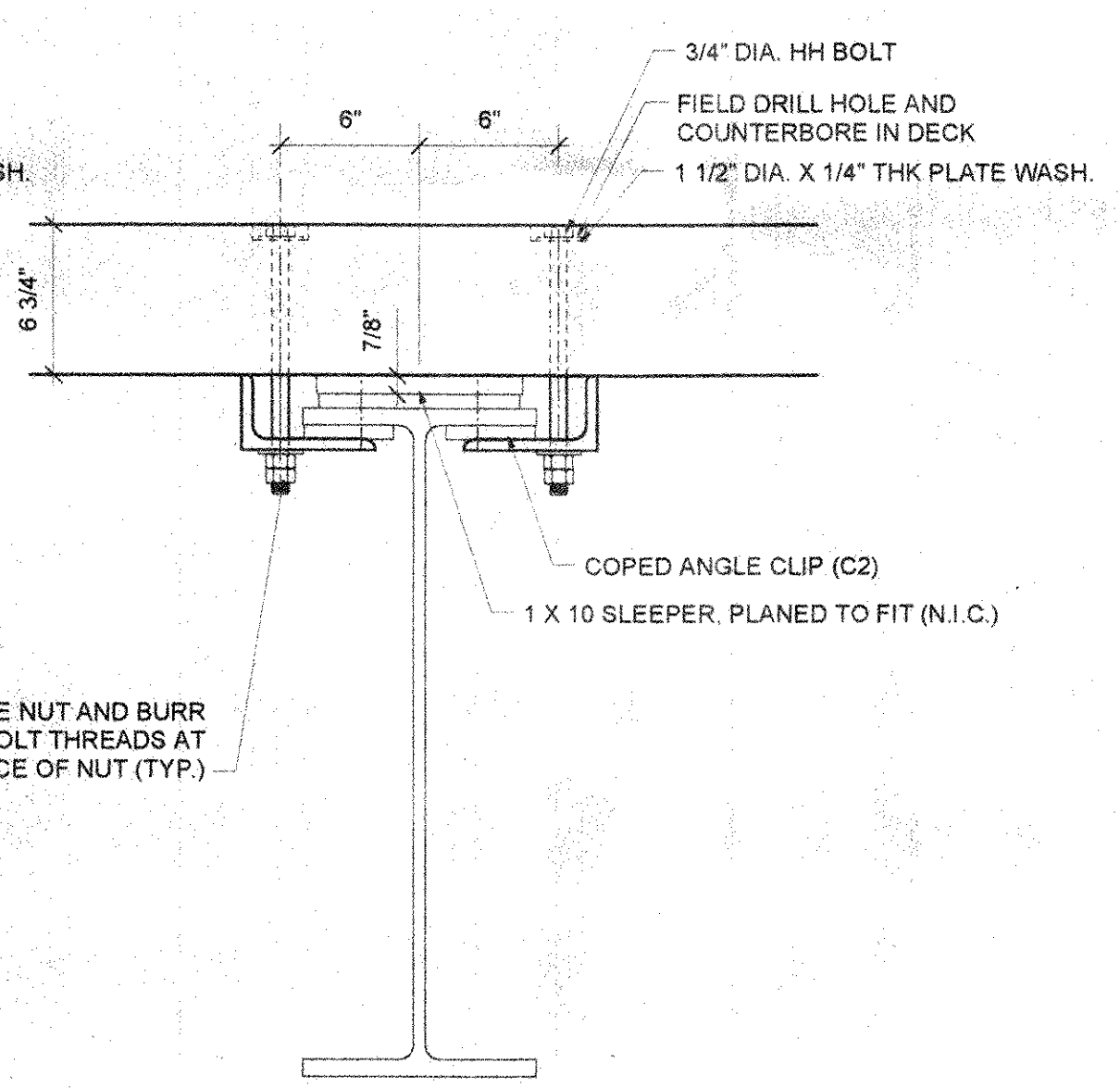




DECK TO STEEL DETAIL SECTION A-A



DECK TO STEEL DETAIL SECTION B-B



DECK TO STEEL DETAIL SECTION C-C

**SCOPE**

This standard is intended to augment, or support, design requirements that may be issued by the owner.

**2. DEFINITIONS AND ABBREVIATIONS**

**AITC** American Institute of Timber Construction; **APA/EWS** American Wood Systems members, a related Corp. of the American Plywood Association; **AWPA** American Wood Preservers Association; **NDS** National Design Specifications. **TTE** tag this end **UNO** unless noted otherwise **DAC** Dimensions are cumulative **NIC** not in contract

**3. CODES AND STANDARDS**

In addition to complying with all pertinent codes and regulations, materials shall comply with the following:  
 3.1 American Association of State Highway and Transportation Officials (AASHTO)  
 3.2 American National Standard for Wood Products-Structural Glued Laminated Timber ANSI A190. 1-1992  
 3.3 Proof-loading system for laminated finger joints to be in accordance with ANSI A190.1

**4. CERTIFICATIONS**

4.1 Certifications required by the laminator:  
 4.1.1 The AITC or APA/EWS Certificate of Conformance with ANSI A190.1-1992  
 4.2 Preservative treatment certification required (if applicable)  
 4.2.1 Certificate of treatment furnished by a certified AWPA treating facility

**5. STRUCTURAL DESIGN**

The structure shall be designed to carry the following loads:

- 5.1 Dead Load
- 5.2 Live Load H-20

**6. MATERIALS**

- 6.1 Lumber-intended for structural use with design stress shall be graded in conformance with accepted standards for allowable unit stresses (See AASHTO Section 13)
- 6.2 Grade-Architectural Appearance Grade as per AITC 110.
- 6.3 All lumber to be Incised Southern Yellow Pine, Glulam as per AITC 117-98.
- 6.4 Adhesive: all members bonded with exterior "wet-use" conforming to ASTM D 2559

**7. PRESERVATIVE TREATMENT**

7.1 All GLU-lam material to be treated with the preservative Pentachlorophenol in Type A oil conforming to AWPA Standard C-28.C-14, P-8 and P-9. Retention level shall be 0.60 PCF. Final steaming and vacuuming shall be performed.

**8. HARDWARE**

- 8.1 All bolts and nuts shall galvanized (A-153) steel ASTM A307, unless noted otherwise.
- 8.2 Timber Washers to be galvanized cast iron or malleable iron.
- 8.3 All steel plates and shapes to be galvanized (A-123) steel ASTM A36 unless noted otherwise.

**9. MISC**

- 9.1 LCI is not responsible for materials or manufacturing not shown on these drawings.
- 9.2 Contractor is responsible for verification of all sizes, quantities and dimensions shown on shop drawings
- 9.3 For wearing surface information, refer to the USDA Forest Service manual "Timber bridges -Design, Construction, Inspection and Maintenance" Chapter 11, for types and applications.

RECEIVED  
 OK'D BY *SSS* OK'D BY *MCM*  
 MAR 08 2007  
 RESUBMIT APPROVED ✓  
 BY DATE 3/9/07



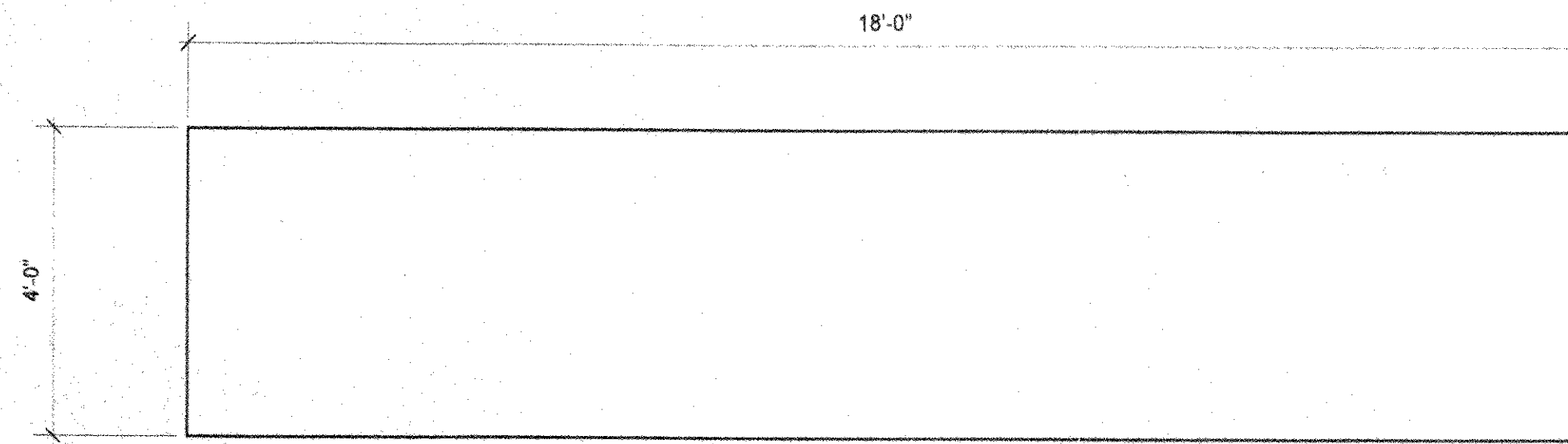
P.O. BOX 369  
 3310 STATE RTE 352  
 BIG FLATS, NEW YORK 14814  
 PHONE: 607.562.8110  
 FAX: 607.562.8105

DRAWN	DATE	REVISIONS			
		NO.	DATE	BY	REVISIONS
MSS	1/31/07				
CHKD	DATE	1	2/26/07	SS	ADD WASHERS
APPD	DATE				
SCALE					

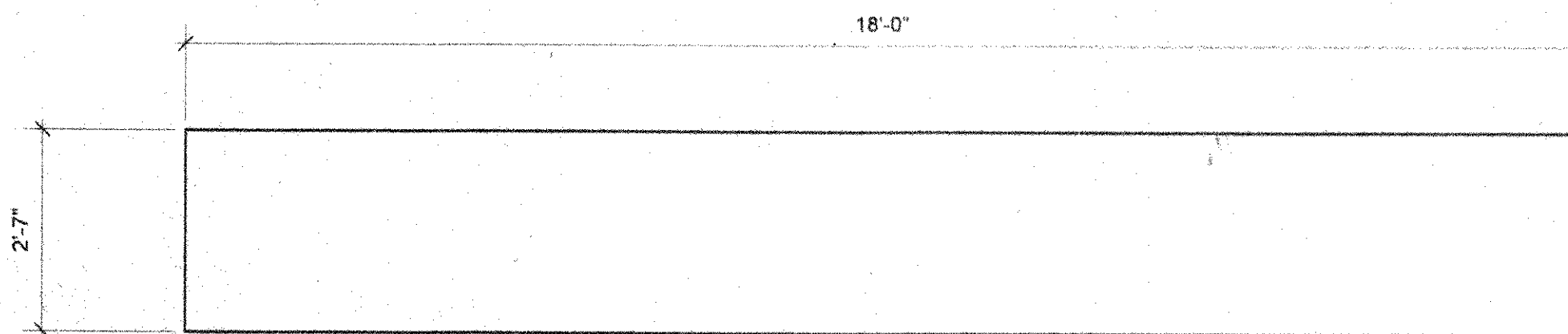
PROJECT NAME	THETFORD BHO 1444 (43) THETFORD BRIDGE NO. 27 TOWN HIGHWAY 6 TOWN OF THETFORD CENTER COUNTY OF ORANGE, VERMONT	PROJECT JOB NO.	0207-115-LT
PREPARED FOR	ALPINE CONSTRUCTION, LLC SCHUYLERVILLE, NY	SHEET NO.	1 OF 2

LAMINATED CONCEPTS INC.

0666LD



MK-F1 6 3/4" X 48" X 18'-0 28 REQ'D COMB. 48 APPROX. WT. = 1800#



MK-F2 6 3/4" X 31" X 18'-0 1 REQ'D COMB. 48 APPROX. WT. = 1200#

LAMINATED CONCEPTS INC. LIST OF LAMINATED TIMBER MATERIALS

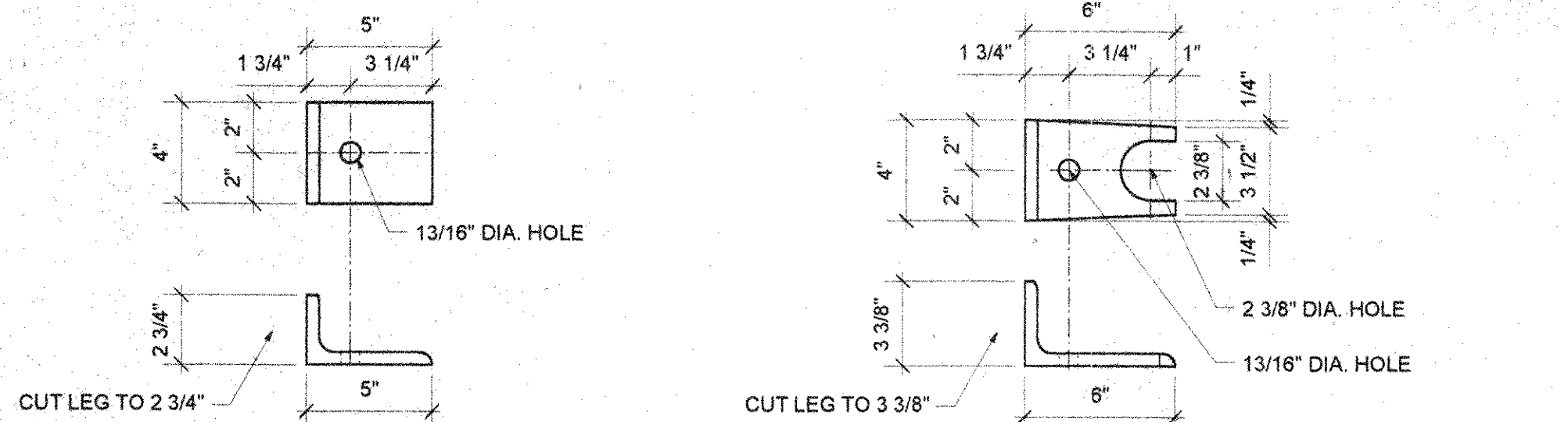
02 07 115 TD		Thetford, VT Covered Bridge							
qty	mk#	width	depth	length		grade	species	remarks	bl lot
				ft	in				
28	F1	6.75	48	18	0	COMB. 48	SYP		13608.00
1	F2	6.75	31	18	0	COMB. 48	SYP		313.87

LAMINATED CONCEPTS INC. LIST OF HARDWARE MATERIALS

02 07 115 TD		Thetford, VT Covered Bridge							
bolts		description	qty	dia.	gr.	wh.	wt.	n.	type
		Deck to steel	448	3/4"	11"	P	F	1	Hex Head Bolts
			16	3/4"	12"	P	F	1	Hex Head Bolts
			470	3/4"					2" Dia. Flat Washers
			950	3/4"					Hex Nuts

NOTE: wh = washer type at bolt head, wn = washer type at nut, n = nuts, washer type (T = Timber / F = Flat / P = Plate / D = Dock)

steel		description	qty	thk.	width	length	type
		Deck Clips C1 5" x 5"	448	1/2"	4"		Angle
		Deck Clips C2 6" x 3 1/2"	16	1/2"	4"		Angle
		Plate Washers	470	1/4"	1 1/2" dia.		Plate



DECK CLIP C1 5" X 5" X 1/2" ANGLE 4" LG. 448 REQ'D DECK CLIP C2 6" X 3 1/2" X 1/2" ANGLE 4" LG. 16 REQ'D

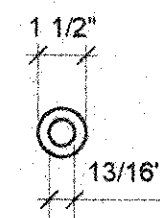
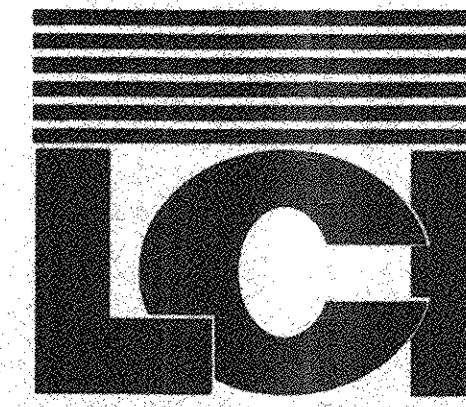


PLATE WASH 1/4" X 1 1/2" DIA. 470 REQ'D

RECEIVED  
 CK'D BY SSS CK'D BY MEM  
 MAR 08 2007  
 RESUBMIT APPROVED  
 BY DATE 3/9/07



P.O. BOX 369 3310 STATE RTE 352 BIG FLATS, NEW YORK 14814 PHONE: 607.562.8110 FAX: 607.562.8105	DRAWN MSS DATE 1/31/07	REVISIONS NO. DATE BY REVISIONS 1 2/28/07 SS ADD WASHERS	PROJECT NAME THETFORD BHO 1444 (43) THETFORD BRIDGE NO. 27 TOWN HIGHWAY 6 TOWN OF THETFORD CENTER COUNTY OF ORANGE, VERMONT	PROJECT JOB NO. 0207-115-LT
	APP'D DATE	SCALE	PREPARED FOR ALPINE CONSTRUCTION, LLC SCHUYLERVILLE, NY	SHEET NO. 2 OF 2