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VAOT STANDARDS

B-71	07-08-05
C-1	01-03-00
C-2A	01-03-00
C-3A	09-01-04
D-3	06-01-94
D-4	06-01-94
D-6	06-01-94
D-8	01-03-00
D-9	06-01-94
D-15	06-01-94
E-100	01-02-04
E-101	05-30-03
E-102	06-30-03
E-102A	05-01-04
E-106	03-01-04
E-107	06-30-03
E-107A	08-08-95
E-108	08-18-95
E-110	08-08-95
E-121	08-08-95
E-140	08-30-96
E-142	09-20-95
E-170	11-04-99
E-171A	08-09-95
E-171B	08-09-95
E-171C	08-09-95
E-172	08-09-95
E-175	11-17-93
G-1	01-03-00
G-1d	01-03-00
G-18	06-01-94
J-3	08-07-95

STATE OF VERMONT AGENCY OF TRANSPORTATION

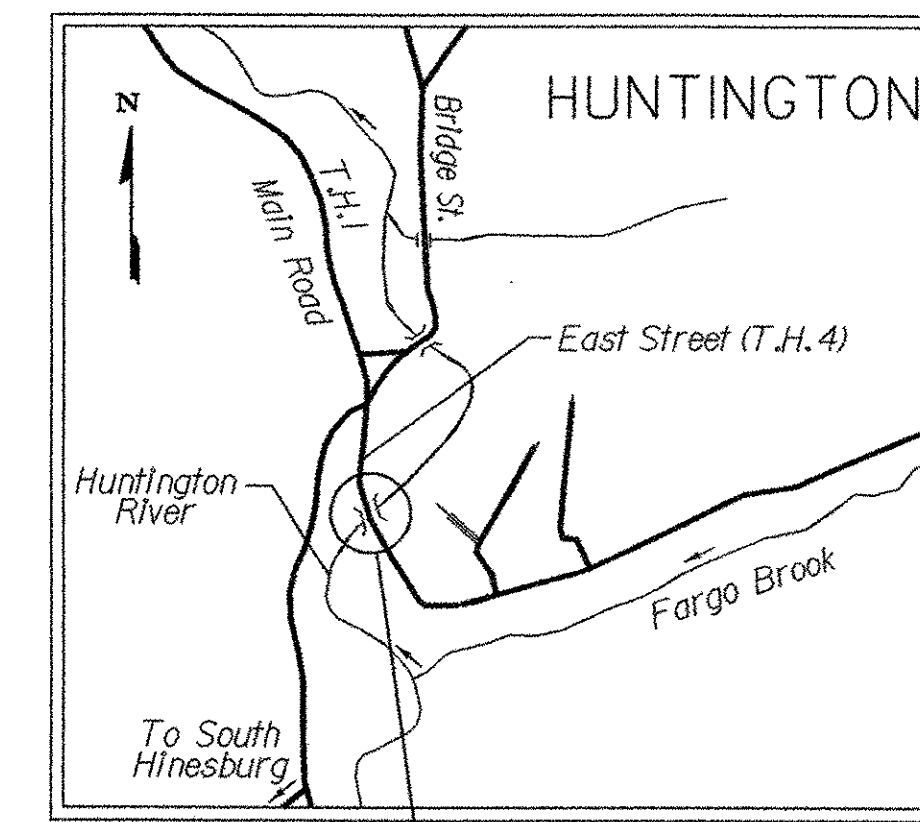


PROPOSED IMPROVEMENT BRIDGE PROJECT TOWN OF HUNTINGTON COUNTY OF CHITTENDEN

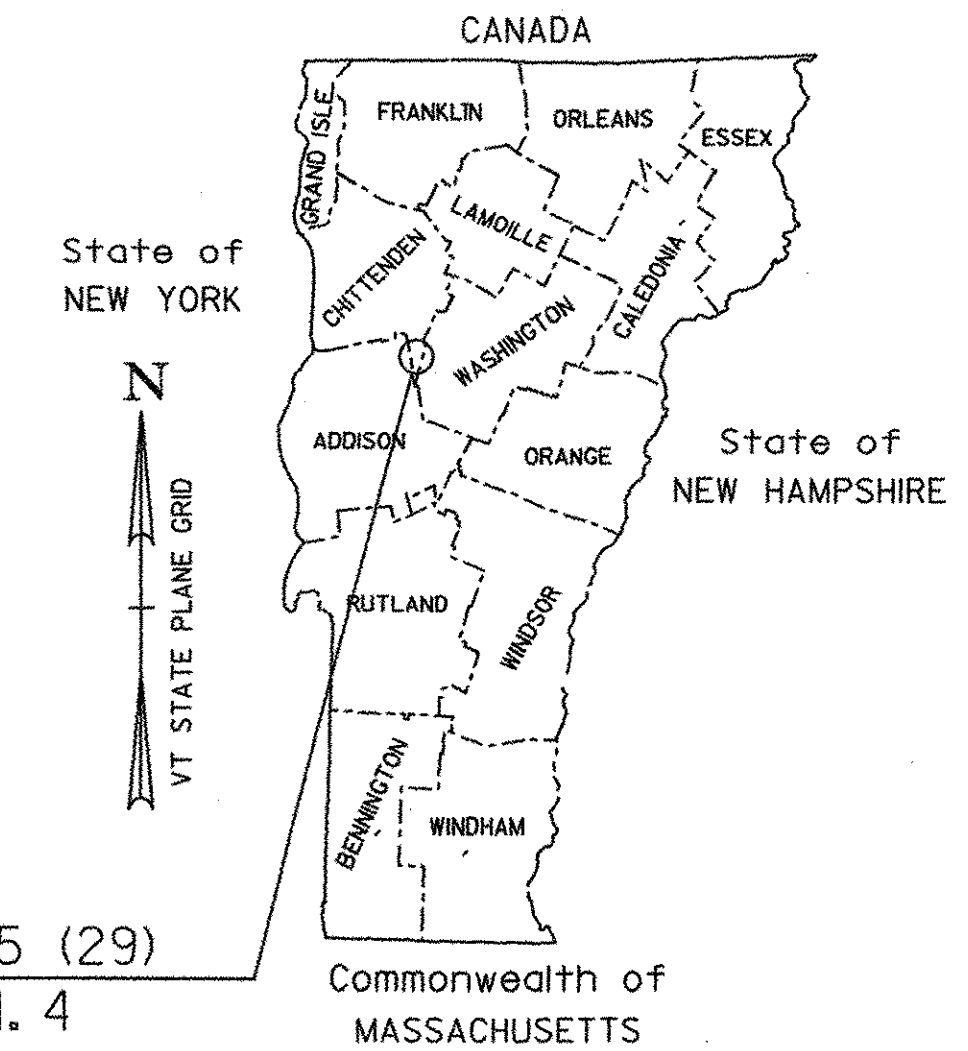
BRIDGE NO. 42 ON EAST STREET (T.H. 4) CLASS 3 (LOCAL ROAD)

PROJECT LOCATION: BEGINNING AT APPROXIMATELY 0.25 MILES SOUTH OF THE INTERSECTION OF MAIN ROAD (T.H. 1) AND EAST STREET (T.H. 4) IN HUNTINGTON, VT AND PROCEEDING SOUTHERLY APPROXIMATELY 0.10 MILES ALONG EAST STREET (T.H. 4).

PROJECT DESCRIPTION: REPLACEMENT OF EXISTING BRIDGE WITH NEW STRUCTURE, INCLUDING RELATED CHANNEL AND ROADWAY APPROACH WORK.
 LENGTH OF SUPERSTRUCTURE: 188.01 FEET (0.03 MILES)
 LENGTH OF ROADWAY: 361.99 FEET (0.07 MILES)
 LENGTH OF PROJECT: 550.0 FEET (0.10 MILES)



HUNTINGTON BRO 1445 (29)
BRIDGE 42 ON T.H. 4
LOCATION MAP
N.T.S.

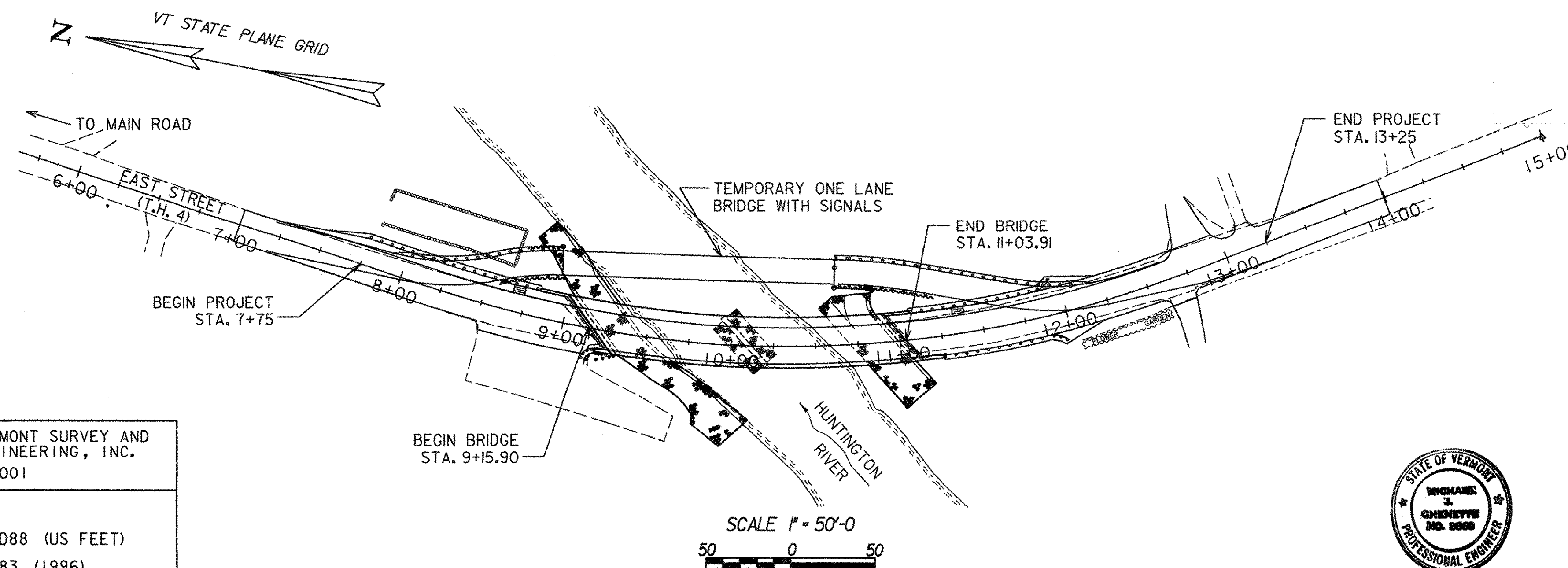


RECORD PLANS	
CONTRACTOR:	PARENT CONSTRUCTION INC. - HINESBURG, VT
RESIDENT ENGINEER:	DALE NORTON
CONSTRUCTION BEGAN:	JUNE 26, 2006
CONSTRUCTION COMPLETE:	NOVEMBER 30, 2007
RECORD PLANS BY:	DALE NORTON & BEN LOGAN
I HEREBY CERTIFY THAT ALL THE CONSTRUCTION REQUIRED BY THIS SET OF DRAWINGS HAS BEEN ACCOMPLISHED AS INDICATED HEREIN.	
BY:	<i>Dale R. Norton</i> RESIDENT ENGINEER
DATE:	19 June 2009
NOTE: Any further information concerning final quantities, amounts or other details relative to this project may be found at Central Files in the electronic archives.	

CONVENTIONAL SYMBOLS

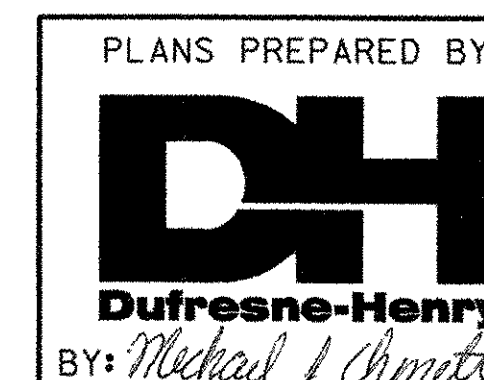
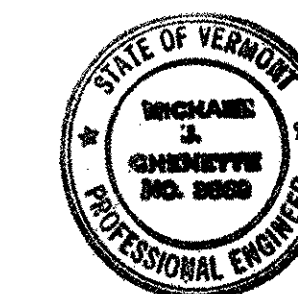
COUNTY LINE	
TOWN LINE	
LIMITS OF ACCESS	
POINT OF ACCESS	
FENCE LINE	
STONE WALL	
TRAVELED WAY	
GUARD RAIL	
RAILROAD	
SURVEY LINE	
CULVERT	
POWER POLE	
TELEPHONE POLE	
TREES	
CONTROL OF ACCESS	
PROPERTY LINE	
R.O.W. TAKING LINE	
SLOPE RIGHTS	
TOP OF CUT	
TOE OF SLOPE	

SURVEYED BY: VERMONT SURVEY AND ENGINEERING, INC.
 SURVEYED DATE: 7/2001
 DATUM
 VERTICAL: NAVD88 (US FEET)
 HORIZONTAL: NAD83 (1996) SPC VT (US FEET)

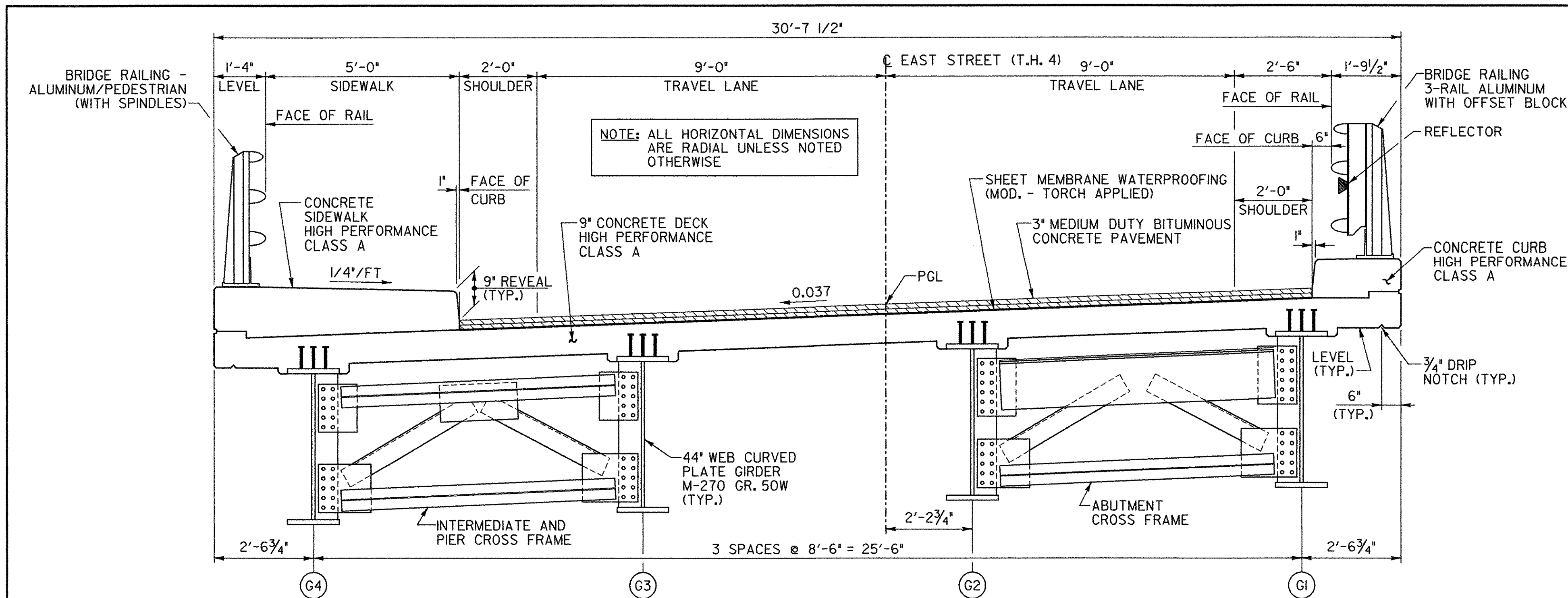


SCALE 1" = 50'-0"
50 0 50

THESE PLANS ARE SUBJECT TO SUCH ENGINEERING CHANGES AS MAY BE REQUIRED BY THE FEDERAL HIGHWAY ADMINISTRATION OR THE DIRECTOR OF PROGRAM DEVELOPMENT.
 CONSTRUCTION IS TO BE CARRIED ON IN ACCORDANCE WITH THESE PLANS AND THE STANDARD SPECIFICATIONS FOR CONSTRUCTION DATED 2001, AS APPROVED BY THE FEDERAL HIGHWAY ADMINISTRATION ON JANUARY 4, 2001 FOR USE ON THIS PROJECT, INCLUDING ALL SUBSEQUENT REVISIONS AND SUCH REVISED SPECIFICATIONS AND SPECIAL PROVISIONS AS ARE INCORPORATED IN THESE PLANS.

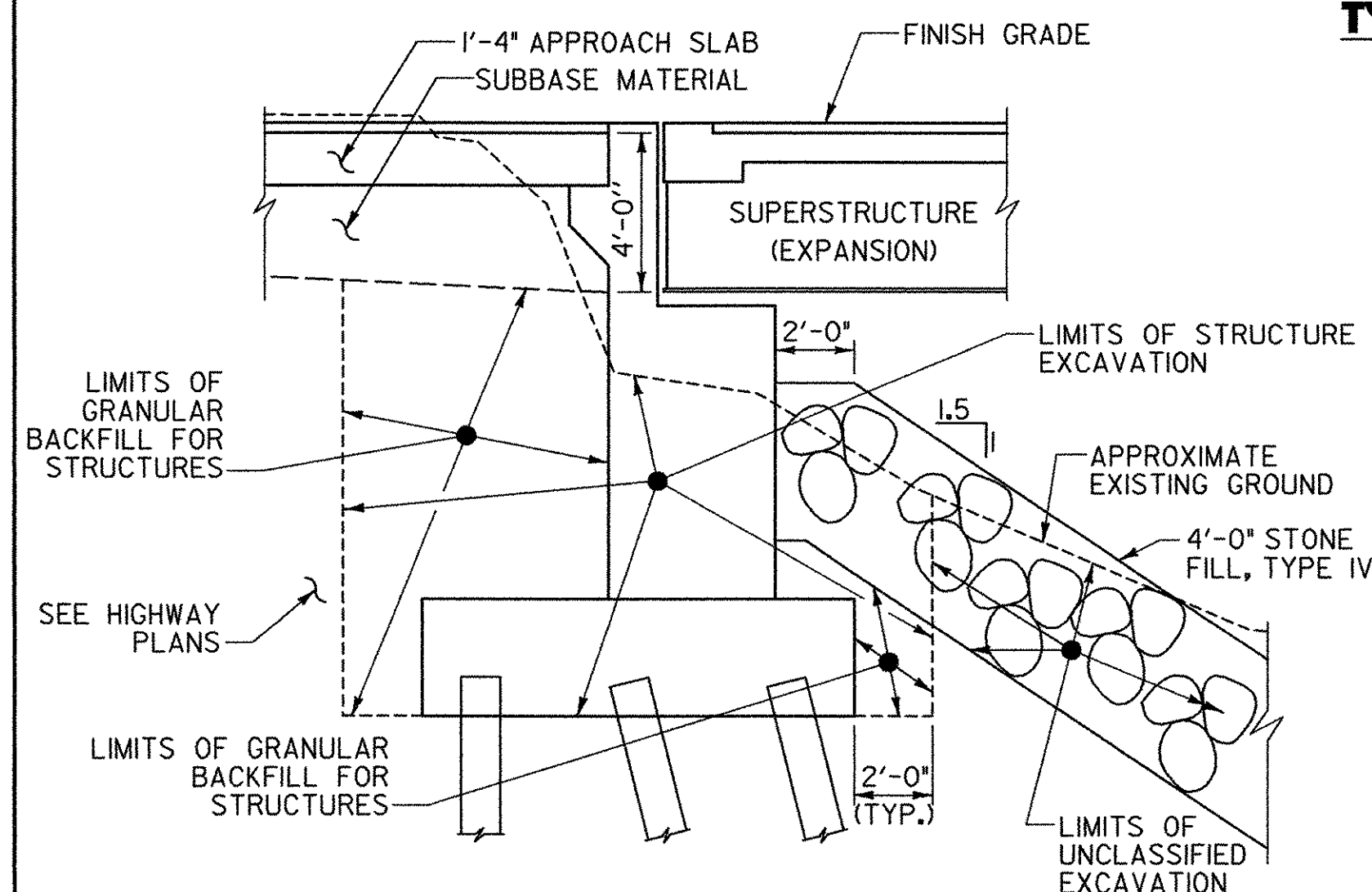


DIRECTOR OF PROGRAM DEVELOPMENT	
APPROVED:	<i>Richard J. ...</i> DATE 4-1-06
PROJECT MANAGER : P. THURBER	
PROJECT NAME : HUNTINGTON	
PROJECT NUMBER : BRO 1445 (29)	
SHEET 1 OF 63 SHEETS	



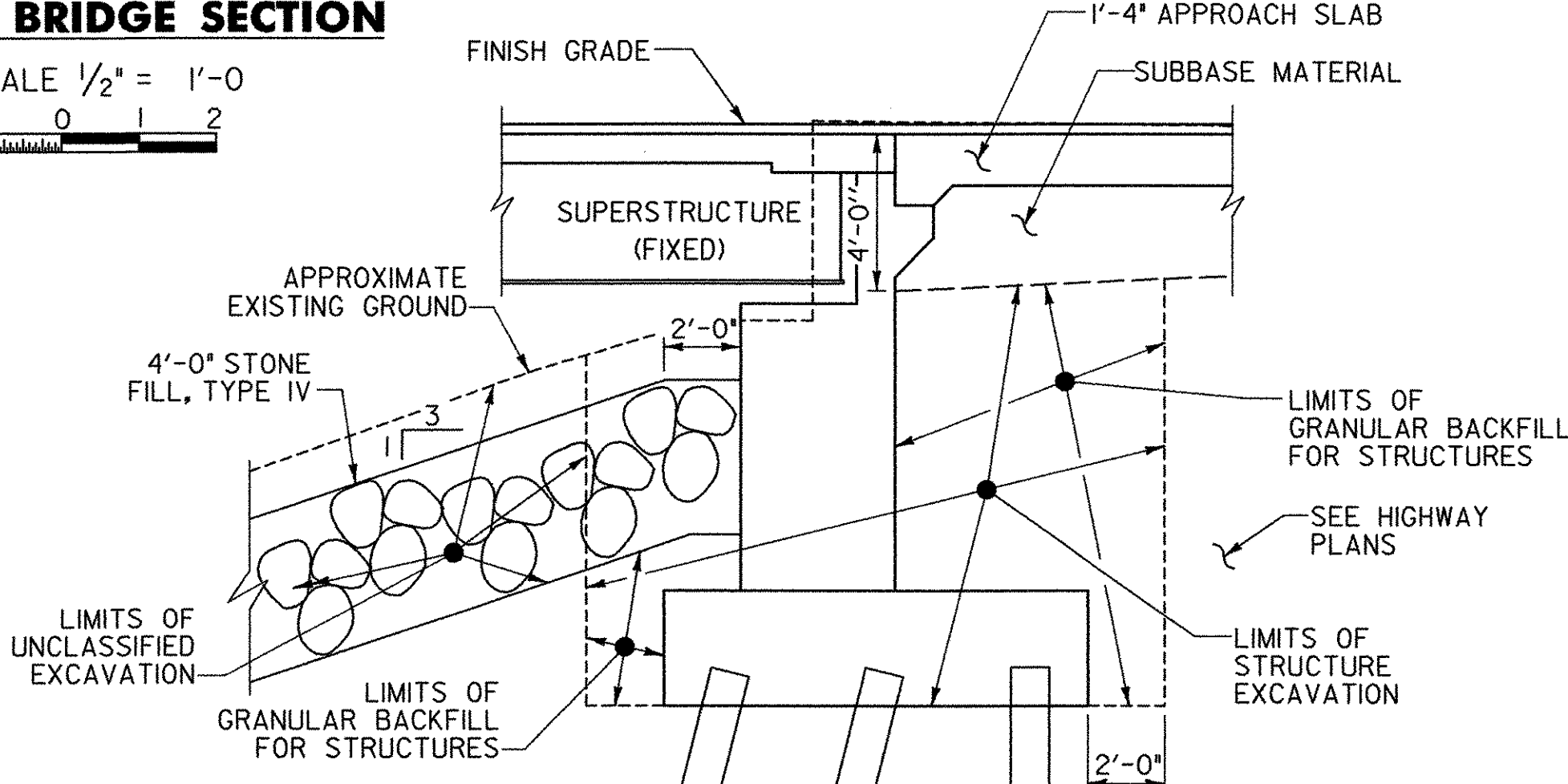
TYPICAL BRIDGE SECTION

SCALE 1/2" = 1'-0"



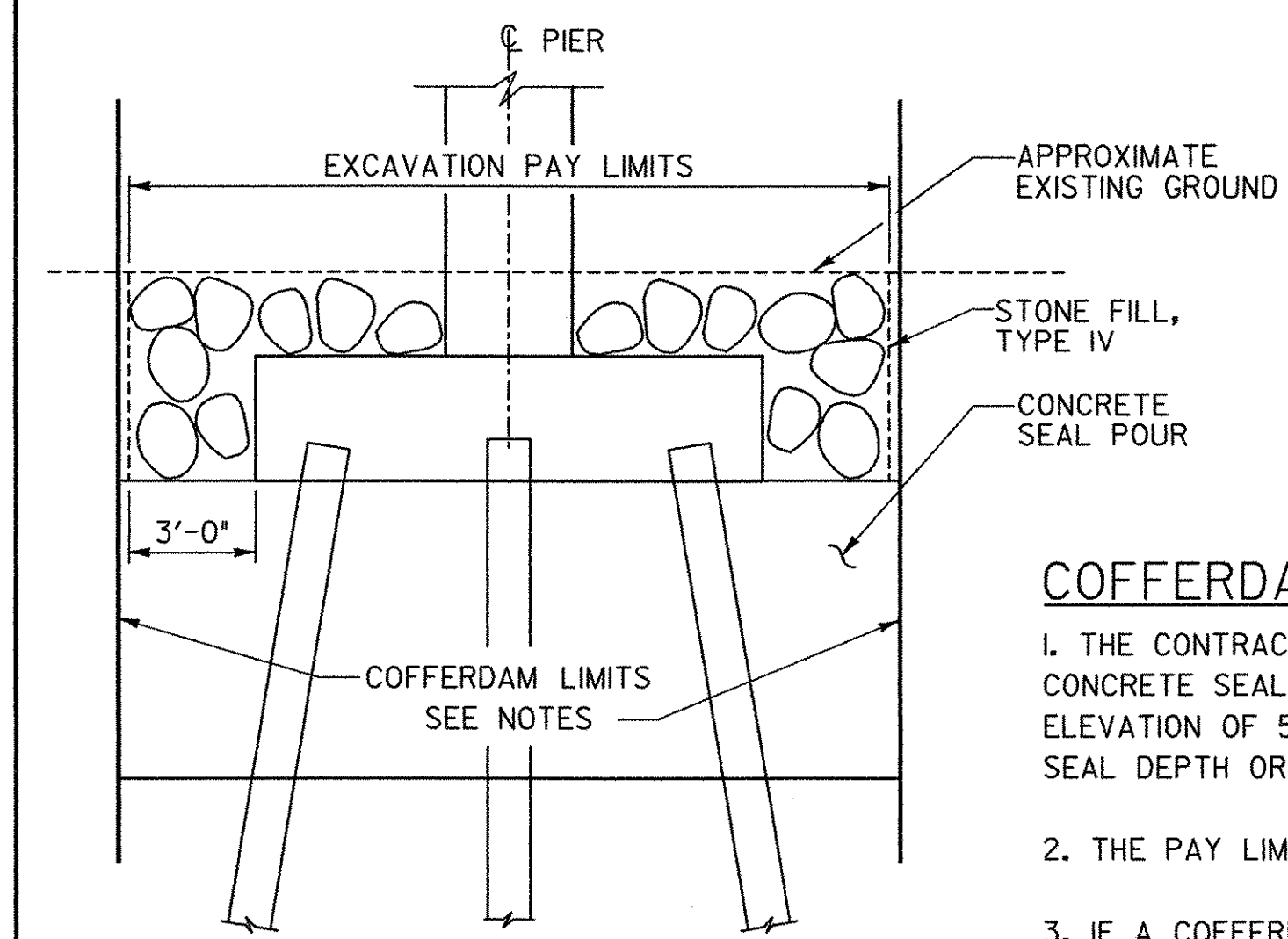
TYPICAL ABUTMENT 1 SECTION

NOT TO SCALE



TYPICAL ABUTMENT 2 SECTION

NOT TO SCALE

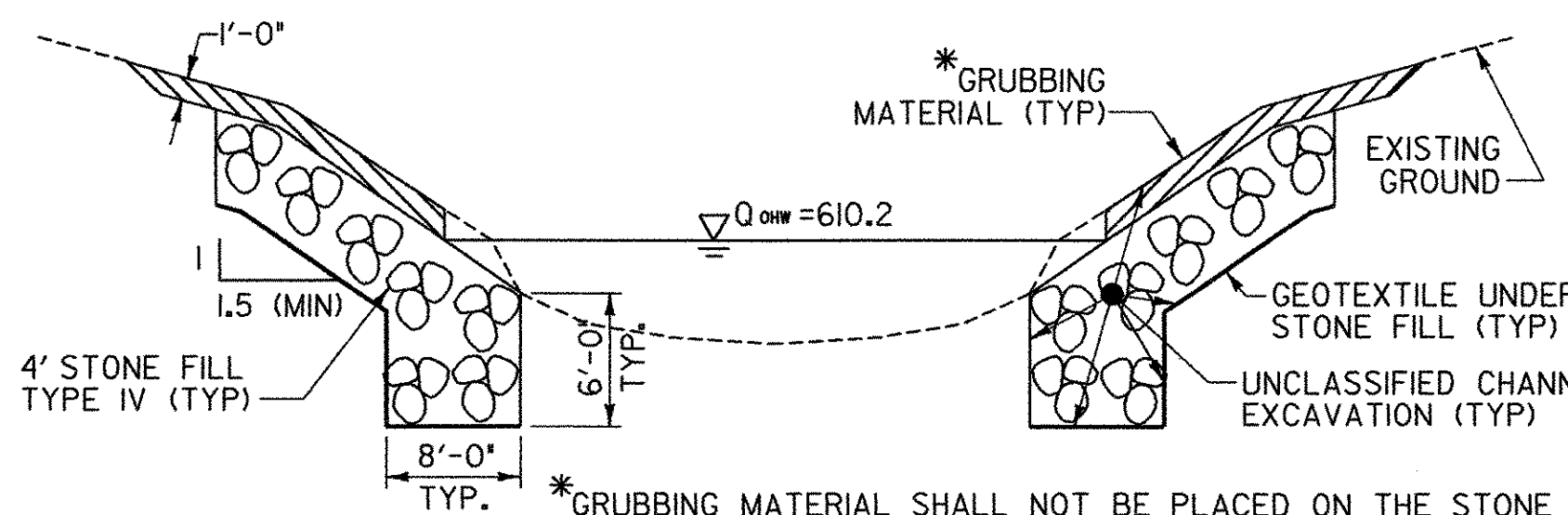


TYPICAL PIER SECTION

NOT TO SCALE

COFFERDAM NOTES:

- THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF THE COFFERDAM AND CONCRETE SEAL. THE PREDICTED SCOUR DEPTH IS BASED ON A BOTTOM OF SEAL ELEVATION OF 590.50. THE ENGINEER SHALL BE NOTIFIED OF ANY CHANGE IN THE FOUNDATION SEAL DEPTH OR SIZE. THESE CHANGES MAY IMPACT THE SIZE AND SPACING OF THE PILES.
- THE PAY LIMITS OF "COFFERDAM EXCAVATION, EARTH" AND COFFERDAM EXCAVATION, ROCK" SHALL BE 3'-0" OUTSIDE THE PERIMETER OF THE FOOTING.
- IF A COFFERDAM IS CONSTRUCTED WHICH IS LARGER THAN THE COFFERDAM EXCAVATION PAY LIMITS, PAYMENT FOR ALL UNCLASSIFIED CHANNEL EXCAVATION, INCLUDING THAT PORTION WHICH IS INSIDE THE COFFERDAM BUT OUTSIDE THE COFFERDAM EXCAVATION PAY LIMITS, WILL BE MADE AT THE CONTRACT UNIT PRICE FOR UNCLASSIFIED CHANNEL EXCAVATION.



TYPICAL CHANNEL SECTION

NOT TO SCALE

FINAL HYDRAULICS REPORT

HYDROLOGIC DATA

DRAINAGE AREA= 44.91 square miles
 CHARACTER OF TERRAIN: Predominantly mountainous woodland
 CHARACTER & TYPE OF STREAM: Moderate slopes, natural channel, irregular side slopes
 NATURE OF STREAMBED: Sand, gravel, cobbles

Q2.33= 1500 cfs Q50= 5700 cfs
 Q10= 3600 cfs Q100= 6400 cfs
 Q25= 4800 cfs Q500= 8600 cfs

DATE OF FLOOD OF RECORD: Unknown
 WATER SURFACE ELEV.: Unknown ESTIMATED DISCHARGE: Unknown
 NATURAL STREAM VELOCITY @ Q25 = 6.65 fps
 ICE CONDITIONS: Moderate DEBRIS: Moderate
 DOES THE STREAM REACH MAXIMUM HIGHWATER ELEVATION RAPIDLY? Yes
 IS ORDINARY RISE RAPID? No
 IS STAGE AFFECTED BY UPSTREAM OR DOWNSTREAM CONDITIONS? No
 IF YES, DESCRIBE.

WATERSHED STORAGE 4.0% HEADWATERS UNIFORM THROUGHOUT WATERSHED X
 IMMEDIATELY ABOVE SITE

EXISTING STRUCTURE

STRUCTURE TYPE: Three-span Steel Girder Bridge with Reinforced Concrete Deck YEAR BUILT: 1939
 CLEAR SPAN (NORMAL TO STREAM): 103 feet
 VERTICAL CLEARANCE ABOVE STREAMBED: 16.1 feet
 WATERWAY OF FULL OPENING: 1600 square feet
 DISPOSITION OF STRUCTURE: Remove
 TYPE OF MATERIAL UNDER SUBSTRUCTURE: Unknown

WATER SURFACE ELEV. @ Q2.33= 612.64 ft VELOCITY= 4.8 fps
 Q10= 616.02 ft " 6.8 fps
 Q25= 617.51 ft " 7.5 fps
 Q50= 618.50 ft " 8.0 fps
 Q100= 619.23 ft " 8.3 fps

LONG TERM STREAM BED CHANGES: Long Term Profile Degradation

IS THE ROADWAY OVERTOPPED BELOW THE Q100? No FREQUENCY: N/A
 RELIEF ELEVATION: N/A DISCHARGE OVER ROAD @ Q100: N/A

UPSTREAM STRUCTURE: TOWN: Huntington DISTANCE: 1.0 miles
 HIGHWAY NO.: Town Highway No. 20 STRUCTURE NO.: Brq. No. 39
 STRUCTURE TYPE: Single Span Steel Girder with Concrete Deck
 CLEAR SPAN: 86' +/- CLEAR HEIGHT: 14' +/-
 YEAR BUILT: 1980 FULL WATERWAY: UNKNOWN

DOWNSTREAM STRUCTURE: TOWN: Huntington DISTANCE: 0.45 miles
 HIGHWAY NO.: Town Highway No. 3 STRUCTURE NO.: Brq. No. 11
 STRUCTURE TYPE: Single Span Pony Truss
 CLEAR SPAN: 110' +/- CLEAR HEIGHT: 13' +/-
 YEAR BUILT: 2000 FULL WATERWAY: Unknown

DESIGN CRITERIA:

- DESIGN LIVE LOAD AASHTO HS-25
- DESIGN SPAN 90.75' + 90.75' = 181.5'
- ALLOWABLE LOAD FOR SPREAD FOOTINGS ON SOIL N/A ON LEDGE N/A
- ALLOWABLE LOAD FOR PILING 145 KIPS (ABUTMENTS), 195 KIPS (PIER) TYPE STEEL H-PILES ESTIMATED LENGTH 46' (ABUT. 1), 47' (ABUT. 2), 53' (PIER)
- STRUCTURAL STEEL AASHTO GRADE AASHTO M270 GRADE 50W
- REINFORCING STEEL GRADE 60
 CONCRETE, HIGH PERFORMANCE CLASS A f_c : 4000 PSI f_c : 1600 psi
 CONCRETE, HIGH PERFORMANCE CLASS B f_c : 3500 PSI f_c : 1400 psi

TRAFFIC MAINTENANCE:

- IS TRAFFIC TO BE MAINTAINED? YES IF YES, ON EXISTING STRUCTURE NO OR ON TEMPORARY BRIDGE YES
- TEMPORARY BRIDGE REQUIREMENTS: ONE OR TWO WAY ONE WAY TRAFFIC CONTROL SIGNALS REQUIRED YES
 MINIMUM CLEAR SPAN (NORMAL TO STREAM) 92 feet MINIMUM CLEAR HEIGHT SEE TEMPORARY BRIDGE SKETCH
 MINIMUM WATERWAY AREA 700 square feet
 ARE SIDEWALKS REQUIRED? No IF SO, ON WHAT SIDE?
 STRUCTURE TYPE

LOADING LEVELS (LOAD FACTOR)	LOAD FACTOR LOAD RATING (TONS)						
	H	HS	3S2	6 AXLE	3A. STR.	4A. STR.	5A. SEMI
INVENTORY A=2.17	48	55					
POSTED A=1.55	67	77	93		71	72	82
OPERATING A=1.30		91	111	133	84	85	

STRENGTH $RF = \frac{\sum M_N - 1.3 M_{DL}}{A \times M_{LL+I}}$

YEAR	TRAFFIC DATA				
	ADT	DHV	% D	% T	ADTT
2005	890	150	64	4	40
2025	1200	180	64	5	70

18 kip ESAL for flexible pavement from 2005 to 2025 = 155,000
 18 kip ESAL for flexible pavement from 2005 to 2045 = 376,000
 Design speed: 35

PROPOSED STRUCTURE

STRUCTURE TYPE: Two Span Curved Steel Girder w/Reinforced Conc. Deck
 CLEAR SPAN (NORMAL TO STREAM): 125 ft
 VERTICAL CLEARANCE ABOVE STREAMBED: 14.3 ft
 WATERWAY OF FULL OPENING: 1300 sq. ft.

WATER SURFACE ELEV. @ Q2.33= 612.71 ft VELOCITY= 4.6 fps
 Q10= 616.12 ft " 6.1 fps
 Q25= 617.64 ft " 6.5 fps
 Q50= 618.66 ft " 6.8 fps
 Q100= 619.43 ft " 6.9 fps

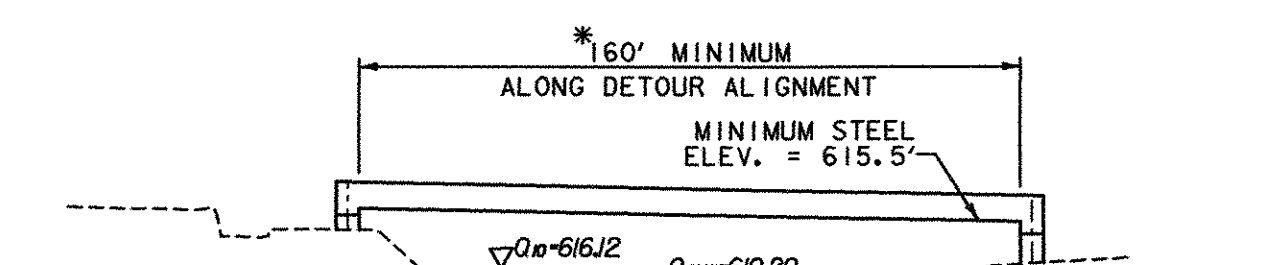
IS THE ROADWAY OVERTOPPED BELOW THE Q100? No FREQUENCY: N/A
 RELIEF ELEVATION: N/A DISCHARGE OVER ROAD @ Q100: N/A

AVERAGE LOW ELEVATION OF SUPERSTRUCTURE: 620.7 feet
 VERTICAL CLEARANCE: @ FEMA Q25 = 3.1 feet

SCOUR: Predicted Q500 Pier Scour Prism extends down to EL. 578' (this includes 2 feet of predicted long term profile degradation and 27 feet of local pier scour.)
 REQUIRED CHANNEL PROTECTION: Type IV, Stone Fill

PERMIT INFORMATION

AVERAGE DAILY FLOW: 165 cfs
 ORDINARY LOW WATER: 45 cfs DEPTH: 1 foot
 ORDINARY HIGH WATER: 620 cfs DEPTH: 3.8 feet (EL. 610.2')



* - CONTRACTOR SHALL HAVE THE OPTION OF CONSTRUCTING A SINGLE SPAN, TWO SPAN, OR THREE SPAN TEMPORARY BRIDGE. ANY PIERS SHALL BE LOCATED OUTSIDE OF THE LIMITS OF ORDINARY HIGH WATER.

DESIGN OF PIERS SHALL INCLUDE ICE LOADINGS AS FOLLOWS:
 ICE THICKNESS = 2'
 ICE PRESSURE = 250 PSI

TEMPORARY BRIDGE REQUIREMENTS

NTS

LOADING LEVELS (LOAD FACTOR)	LOAD FACTOR LOAD RATING (TONS)						
	H	HS	3S2	6 AXLE	3A. STR.	4A. STR.	5A. SEMI
INVENTORY A=2.17	48	55					
POSTED A=1.55	67	77	93		71	72	82
OPERATING A=1.30		91	111	133	84	85	

STRENGTH $RF = \frac{\sum M_N - 1.3 M_{DL}}{A \times M_{LL+I}}$

YEAR	TRAFFIC DATA				
	ADT	DHV	% D	% T	ADTT
2005	890	150	64	4	40
2025	1200	180	64	5	70

18 kip ESAL for flexible pavement from 2005 to 2025 = 155,000
 18 kip ESAL for flexible pavement from 2005 to 2045 = 376,000
 Design speed: 35

STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of HUNTINGTON Bridge No. 42
 Highway No. T.H. 4 Log Sta. Surv. Sta.

EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

PRELIMINARY INFORMATION SHEET

Designed By T. KNIGHT Drawn By D. HARRINGTON
 Checked By Date Bridge Design Supervisor
 M. CHENETTE 06/04 M. CHENETTE Date 09/05

PROJECT HUNTINGTON PROJECT NO. BRO 1445 (29)

DH Dgn.: ...Cadd\Trans\z01j302pi.dgn Plot Date: 1/12/2006
 Bridge Sheet No. Sheet 2 of 63

TYPICAL SECTIONS

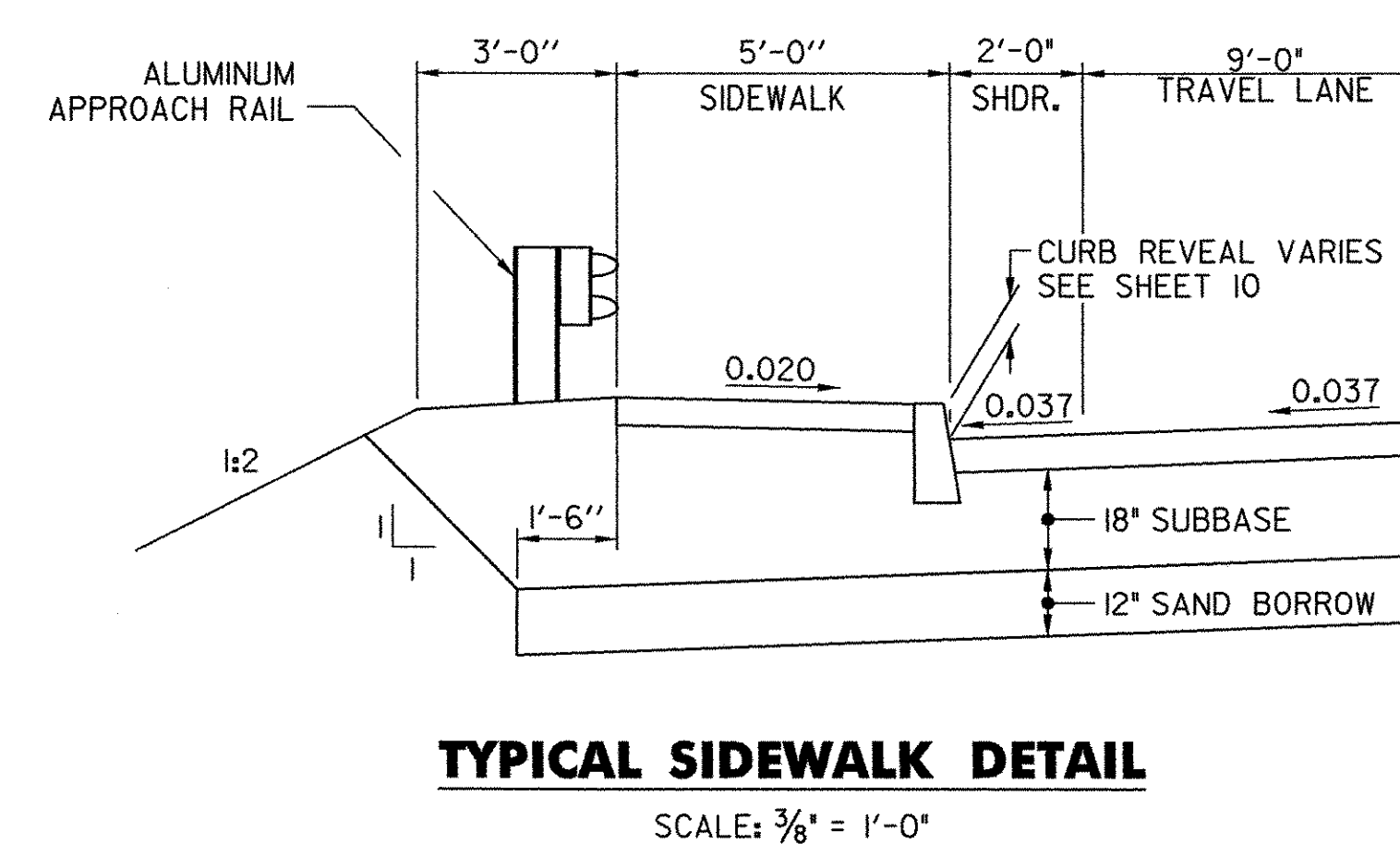
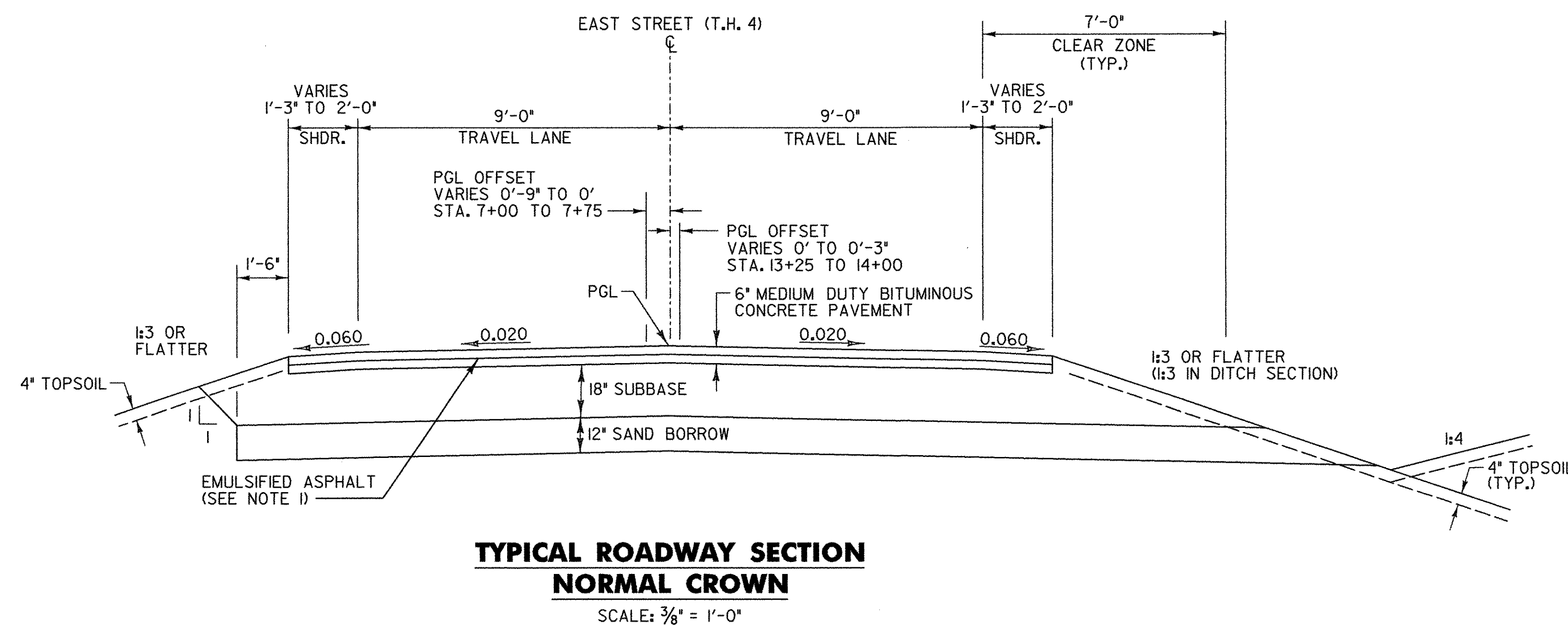
3" MEDIUM DUTY BITUMINOUS CONCRETE PAVEMENT (2 LIFTS - TYPE III)
 3" MEDIUM DUTY BITUMINOUS CONCRETE PAVEMENT (1 LIFT - TYPE I)
 18" SUBBASE OF GRAVEL
 12" SAND BORROW

MATERIAL ITEM THICKNESS / TOLERANCE

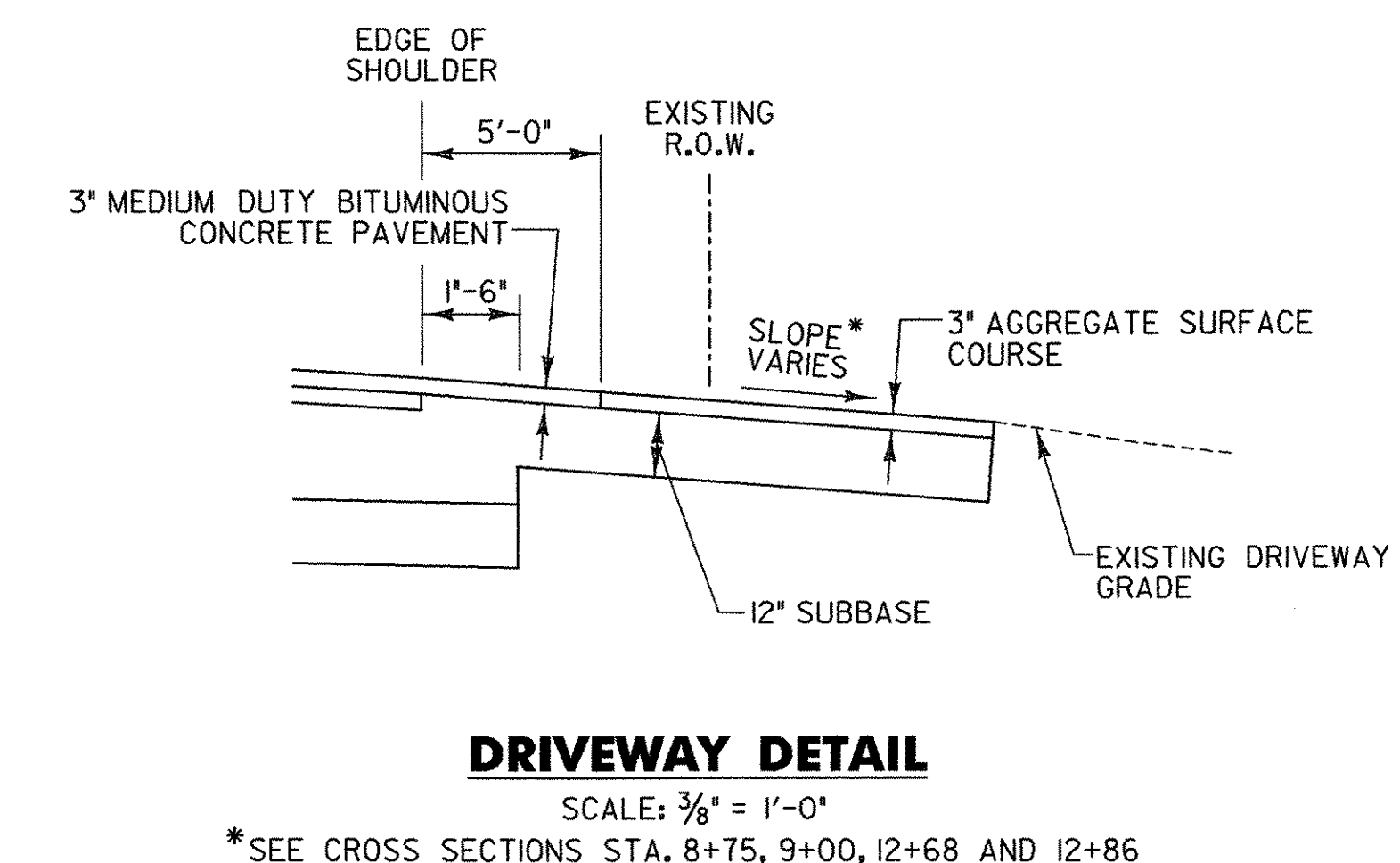
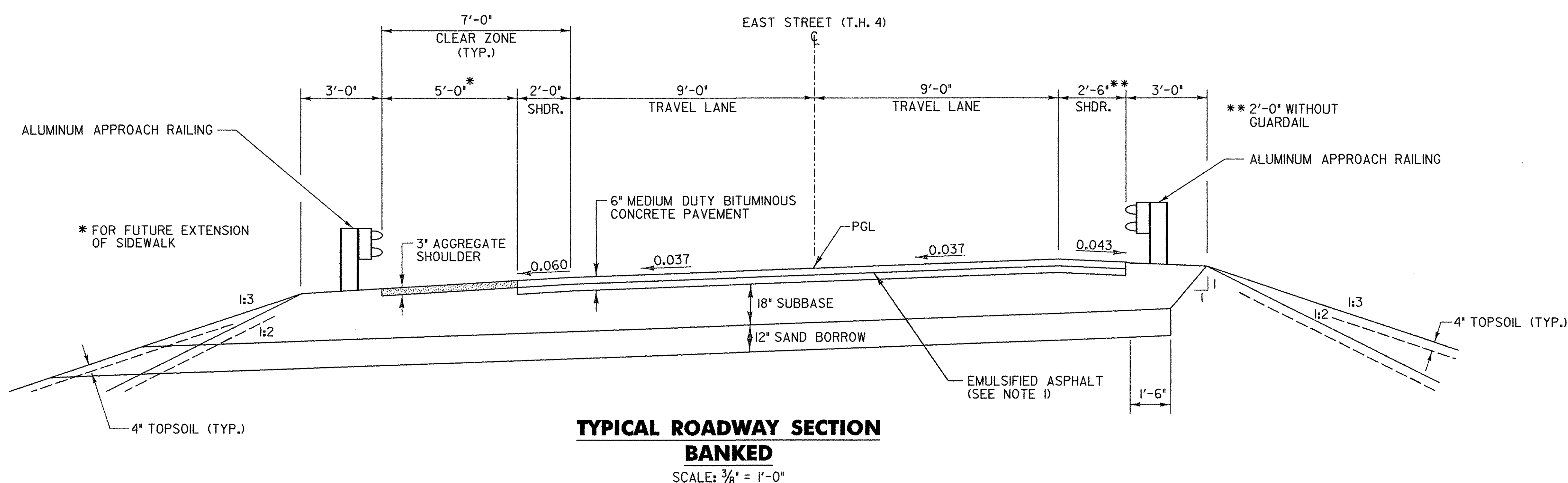
MEDIUM DUTY BITUMINOUS CONCRETE PAVEMENT	+/- 1/4" (TOTAL DEPTH)
SUBBASE	+/- 1"
SAND BORROW	+/- 1"

NOTES:

1. EMULSIFIED ASPHALT IS TO BE APPLIED AT THE RATE OF 0.015 GAL./SY BETWEEN SUCCESSIVE COURSES OF PAVEMENT OR AS DIRECTED BY THE ENGINEER.

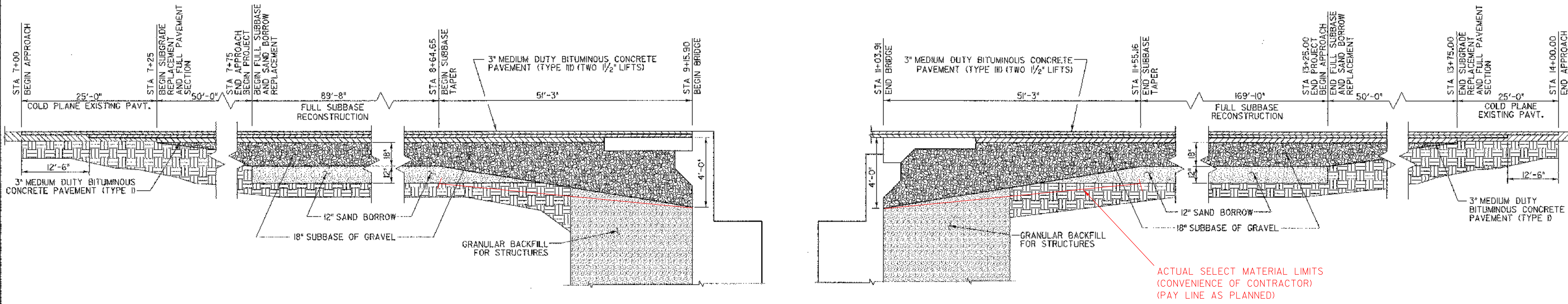


CURVE NO. 1 SUPERELEVATION TRANSITION INFORMATION	
BEGIN TANGENT RUNOUT (RT SIDE ONLY)	STA. 7+39.00
END TANGENT RUNOUT/ BEGIN SUPERELEVATION RUNOFF (RT SIDE ONLY)	STA. 7+78.00
REVERSE NORMAL CROWN (CROSS SLOPE = 2.0%)	STA. 8+17.00
END SUPERELEVATION RUNOFF/ BEGIN FULL SUPERELEVATION (CROSS-SLOPE = 3.7%)	STA. 8+50.00
END FULL SUPERELEVATION/ BEGIN SUPERELEVATION RUNOFF	STA. 12+49.00
REVERSE NORMAL CROWN	STA. 12+82.00
END SUPERELEVATION RUNOFF/ BEGIN TANGENT RUNOUT	STA. 13+21.00
END TANGENT RUNOUT	STA. 13+60.00



PROJECT NAME: HUNTINGTON
 PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...Cadd\Trans\z01j302frml.dgn PLOT DATE: 1/12/2006
 PROJECT LEADER: M. CHENETTE DRAWN BY: J. OAKMAN
 DESIGNED BY: D. ALTERI CHECKED BY: D. ALTERI
ROADWAY TYPICAL SECTIONS SHEET 3 OF 63



APPROACH SECTION
NOT TO SCALE

PROJECT NAME:	HUNTINGTON
PROJECT NUMBER:	BRO 1445 (29)
FILE NAME:	...Cadd\Trans\2011302frml.dgn
PROJECT LEADER:	M. CHENETTE
DESIGNED BY:	D. ALTERI
DRAWN BY:	J. DAKMAN
CHECKED BY:	M. CHENETTE
ROADWAY APPROACH SECTION	
PLOT DATE: 1/12/2006 SHEET 4 OF 63	



QUANTITY SHEET

SUMMARY OF ESTIMATED QUANTITIES														TOTALS			DESCRIPTIONS			DETAILED SUMMARY OF QUANTITIES		
FULL C.E. ITEMS	TRAINING	ROADWAY	EROSION CONTROL	CHANNEL	ABUTMENT No. 1	PIER	ABUTMENT No. 2	APPROACH H SLAB No. 1	APPROACH H SLAB No. 2	DECK	BRIDGE QUANTITY	ROUND	GRAND TOTAL	FINAL	UNIT	ITEMS	ITEM NUMBER	QUANTITIES	UNIT	ITEMS		
		1											1		LS	CLEARING AND GRUBBING (INCL. INDV. TREES & STUMPS)	201.10					
		930											930		CY	COMMON EXCAVATION	203.15					
				1150									1150		CY	UNCLASSIFIED CHANNEL EXCAVATION	203.27					
		325											325		CY	SAND BORROW	203.31					
					340		380						700		CY	STRUCTURE EXCAVATION	204.25					
					115		95						210		CY	GRANULAR BACKFILL FOR STRUCTURES	204.30					
							520						520		CY	COFFERDAM EXCAVATION, EARTH	208.30					
							20						20		CY	COFFERDAM EXCAVATION, ROCK	208.35					
							1						1		LS	COFFERDAM (PIER)	208.40					
		110											110		SY	COLD PLANING-BIT PAVEMENT	210.10					
		820											820		CY	SUBBASE OF GRAVEL	301.15					
		28											28		CY	AGGREGATE SURFACE COURSE	401.10					
		4								1			1		CWT	EMULSIFIED ASPHALT	404.85					
		361						10	10	76			96		TON	MEDIUM DUTY BIT. CONC. PAVEMENT (PG 58-34)	406.27					
					3					240			243		CY	CONCRETE, HIGH PERFORMANCE CLASS A	501.33					
					130	134	109	26	28				427		CY	CONCRETE, HIGH PERFORMANCE CLASS B	501.34					
													1		LS	FURNISHING EQUIPMENT FOR DRIVING PILING	504.10					
					874	954	752						2580		LF	STEEL PILING (HP 12 X 74)	505.16					
		1800											1800		SF	TEMPORARY STEEL SHEET PILING	505.36					
					1	1	1						3		EACH	DYNAMIC PILE LOADING TEST	505.45					
										194152			194152		LB	STRUCTURAL STEEL (PLATE GIRDER)	506.55					
					11404	20530	10284						42218		LB	REINFORCING STEEL	507.15					
					592		182	3241	3217	60610			67841		LB	EPOXY COATED REINFORCING STEEL	507.17					
										1			1		LS	SHEAR CONNECTORS (2610 - 7/8"X7")	508.15					
										1			1		LS	STRUCTURAL PAINTING, SHOP APPLIED (4.4 TONS)	513.25					
										1			1		LS	SURFACE PREPARATION, SHOP (4.4 TONS)	513.40					
					4	3	8			17			32		GAL	WATER REPELLENT (MOD. - SILANE)	514.10					
							27						27		LF	BRIDGE EXPANSION JOINT (ASPHALTIC PLUG)	516.10					
					44								44		LF	BRIDGE EXPANSION JOINT (VERMONT)	516.10					
										460			460		SY	SHEET MEMBRANE WATERPROOFING (MOD. - TORCH APPLIED)	519.20					
										209			209		LF	BRIDGE RAILING - 3 RAIL ALUM	525.22					
										219			219		LF	BRIDGE RAILING-ALUMINUM / PEDESTRIAN	525.23					
		1											1		LS	ONE-WAY TEMPORARY BRIDGE (3000 SF - EST) (MOD.)	528.10					
										420			420		SY	REMOVAL OF BRIDGE PAVEMENT	529.10					
										1			1		EACH	PARTIAL REMOVAL OF STRUCTURE (4400 SF - EST.)	529.20					
					4								4		EACH	BEARING DEVICE ASSEMBLY (POT) (ABUTMENT NO. 1)	531.10					
						4							4		EACH	BEARING DEVICE ASSEMBLY (POT) (PIER)	531.10					
							4						4		EACH	BEARING DEVICE ASSEMBLY (FABRIC) (ABUTMENT NO. 2)	531.10					
		220											220		LF	15" CPEP(SL)	601.261					
		2											2		EACH	PRECAST REINFORCED CONCRETE CATCH BASIN WITH CAST IRON GRATE	604.20					
		5											5		CY	STONE FILL, TYPE I	613.10					
			30										30		CY	STONE FILL, TYPE I (MOD. - CONSTRUCTION ENTRANCE)	613.10					
			2										2		CY	STONE FILL, TYPE I (MOD. - INLET PROTECTION)	613.10					
				1200									1200		CY	STONE FILL, TYPE IV	613.13					
		84											84		LF	CAST-IN-PLACE CONCRETE CURB, TYPE B (MOD.)	616.28					

PROJECT NAME: HUNTINGTON
 PROJECT NUMBER: BRO 1445 (29)
 FILE NAME: Quantity Sheet
 PROJECT LEADER: M. CHENETTE
 DESIGNED BY: D. ALTERI
 PLOT DATE: 4/20/2004
 DRAWN BY: J. OAKMAN
 CHECKED BY: D. ALTERI
 QUANTITY SHEET #1 SHEET 5 OF 83



STATE OF VERMONT
AGENCY OF TRANSPORTATION

QUANTITY SHEET

SUMMARY OF ESTIMATED QUANTITIES													TOTALS			DESCRIPTIONS			DETAILED SUMMARY OF QUANTITIES		
FULL C.E. ITEMS	TRAINING	ROADWAY	EROSION CONTROL	CHANNEL	ABUTMENT No. 1	PIER	ABUTMENT No. 2	APPROACH SLAB No. 1	APPROACH SLAB No. 2	DECK	BRIDGE QUANTITY	ROUND	GRAND TOTAL	FINAL	UNIT	ITEMS	ITEM NUMBER	QUANTITIES	UNIT	ITEMS	
		3											3		EACH	RELOCATE MAILBOX, SINGLE SUPPORT	617.10				
		42											42		SY	PORTLAND CEMENT CONCRETE SIDEWALK, 5 INCH (MOD.)	618.10				
		2											2		SY	DETECTABLE WARNING SURFACE	618.30				
		600											600		LF	SNOW FENCE (MOD. - PDF)	620.70				
		2											2		EACH	ENERGY ABSORPTION ATTENUATOR	621.56				
		282											282		LF	ALUMINUM APPROACH RAILING	621.74				
		175											175		LF	REMOVL AND DISP OF GUARD RAIL	621.80				
		60											60		LF	TEMPORARY TRAFFIC BARRIER	621.90				
1													1		LS	FIELD OFFICE - ENGINEERS	631.10				
1													1		LS	TESTING EQUIPMENT - CONCRETE	631.16				
1													1		LS	TESTING EQUIPMENT - BITUMINOUS	631.17				
1													1		LU	FIELD OFFICE - TELEPHONE (N.A.B.I.)	631.25				
		520											520		HR	EMPLOYEE TRAINEESHIP	634.10				
		1											1		LS	MOBILIZATION/DEMOBILIZATION	635.11				
		1											1		LS	TRAFFIC CONTROL	641.10				
		1400											1400		LF	DURABLE 4" YELLOW LINE (THERMOPLASTIC)	646.41				
		22											22		LF	TEMPORARY 24" STOP BAR (PAINT)	646.66				
													900		SY	GEOTEXTILE UNDER STONE FILL	649.31				
													330		SY	GEOTEXTILE FOR SILT FENCE	649.51				
													370		SY	GEOTEXTILE FOR FILTER CURTAIN	649.61				
													40		LB	SEED	651.15				
													40		LB	SEED-WINTER RYE	651.17				
													40		LB	FERTILIZER	651.18				
													300		TON	AGRICULTURAL LIMESTONE	651.20				
													2		TON	HAY MULCH	651.25				
													2		TON	HAY MULCH	651.25				
													280		CY	TOPSOIL	651.35				
													550		SY	GRUBBING MATERIAL	651.40				
													1		LS	EROSION PREVENTION AND SEDIMENT CONTROL PLAN	652.10				
													160		HR	MONITORING EROSION PREVENTION AND SEDIMENT CONTROL PLAN	652.20				
													1		LU	MAINTENANCE OF EROSION PREVENTION AND SEDIEMTN CONTROL PLAN (N.A.B.I.)	652.30				
													620		SY	EROSION MATTING	654.10				
		8											8		EACH	REMOVING SIGNS	675.50				
		1											1		EACH	TEMPORARY TRAFFIC SIGNAL SYSTEM	678.40				
		2											2		EACH	TEMPORARY DETECTOR	678.42				

PROJECT NAME: HUNTINGTON
 PROJECT NUMBER: BRO 1445 (29)
 FILE NAME: Quantity Sheet PLOT DATE: 4/20/2004
 PROJECT LEADER: M. CHENETTE DRAWN BY: J. OAKMAN
 DESIGNED BY: D. ALTERI CHECKED BY: D. ALTERI
 QUANTITY SHEET #2 SHEET 6 OF 63



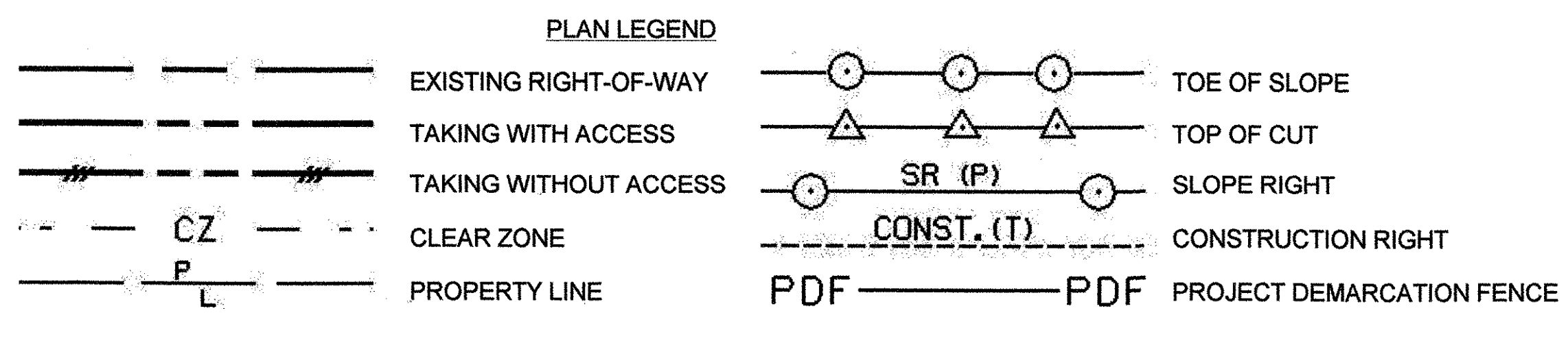
RIGHT - OF - WAY DETAIL SHEET

TABLE OF PROPERTY ACQUISITION

PARCEL NO.	PROPERTY OWNER	SHEET NO.	BEGINNING STATION	ENDING STATION	TAKE	REMAINDER	RIGHT			RECORDING DATA				REMARKS
					AREA±	AREA±	TYPE	(T)/(P)	AREA ±	TITLE	DATE	TOWN / CITY	BOOK	
1	BEANE, VANCE G. & VIOLA S.	7	7+14.8 RT.				INSTALL	(T)						GUY ANCHOR
2	MELVILLE, DIANNA	7	7+07.3 LT. 7+16.6 LT. 7+47.9 LT. 7+94.6 LT.	7+96.1 LT. 7+92.5 LT. 9+78.7 LT.			INSTALL SLOPE DETOUR	(T) (T) (T)	863 SF 164 SF 0.09 A			HUNTINGTON		MONITOR WELL EROSION CONTROL 3,766 SF±; INCL. EROSION CONTROL & TEMP. UTILITY RELOCATION EROSION CONTROL 1,692 SF±
			8+27.6 LT. 8+30.8 LT. 8+73.9 LT. 8+77.9 LT. 8+78.2 LT.	8+76.2 LT. 9+78.7 LT. 9+21.5 LT. 8+95.7 LT. 9+03.2 LT.			INSTALL UTILITY CHANNEL SLOPE INSTALL	(T) (P) (P) (P) (T)	1216 SF 0.04 A 821 SF 55 SF 115 SF					EROSION CONTROL
3	LACAILLADE, LEIGH	7	8+33.0 RT. 9+03.9 RT. 9+51.4 RT. 9+18.5 RT.	9+70.0 RT. 10+11.2 RT. 10+56.1 RT.			ACCESS CHANNEL CONST.	(T) (P) (T)	1,050 SF 976 SF 2,650 SF			HUNTINGTON		EXCEPT & RESERVE SEPTIC SYSTEM INCL. EROSION CONTROL
4	BRACE, LISA M. & DELISLE, LESTER J.	7	9+32.7 LT.	12+78 LT.			DETOUR	(T)	0.29 A			HUNTINGTON		12,590 SF±; INCL. EROSION CONTROL, TEMP. UTILITY RELOCATION, & PDF 2,999 SF±
			9+54.5 LT. 10+59.0 LT. 10+81.0 LT. 10+84.3 LT. 11+30.0 LT. 12+84 LT.	11+61.6 LT. 10+85.2 LT. 10+95.6 LT. 12+15.0 LT.			UTILITY CHANNEL CUL., DIT. & DR. SLOPE INSTALL DRIVE	(P) (P) (P) (P) (T) (T)	0.07 A 55 SF 546 SF					GUY ANCHOR 12' GRAVEL
5	BLODGETT, GARY A. & RUTH A.	7	10+30.1 RT.	11+56.0 RT.			CONST.	(T)	0.05 A			HUNTINGTON		2,203 SF±; INCL. EROSION CONTROL & PDF
			10+88.2 RT. 11+15.1 RT. 11+15.8 RT. 11+70.7 RT. 12+44.6 RT. 12+68 RT. 12+70.1 RT. 12+83.9 RT.	11+18.4 RT. 12+60.5 RT. 11+36.0 RT. 12+34.1 RT.			CHANNEL INSTALL SLOPE SLOPE DRIVE INSTALL SLOPE	(P) (T) (P) (T) (T) (T) (T)	309 SF 0.03 A 58 SF 221 SF					PDF, 1296 SF±; INCL. EROSION CONTROL INCLUDES EROSION CONTROL INCLUDES EROSION CONTROL MONITOR WELL 10' GRAVEL PDF, INCLUDES EROSION CONTROL
6	WISNIOWSKI, MARK & JENNIFER A.	7	12+87.6 RT. 12+87.6 RT. 12+89.6 RT.	13+09.8 RT. 14+01.6 RT. 12+93.6 RT.			SLOPE INSTALL INSTALL	(T) (T) (T)	34 SF 0.02 A					PDF, 1120 SF±; INCL. EROSION CONTROL GUY ANCHOR
7	GREEN MOUNTAIN POWER CORPORATION													UTILITY
8	CHAMPLAIN VALLEY TELECOM													UTILITY
9	ADELPHIA CABLE COMMUNICATIONS													UTILITY

TABLE OF REVISIONS

REVISION NO.	SHEET NO.	DESCRIPTION	DATE
1	6,7	PARCEL NO. 3 LACAILLADE, CHANGE ENDING STA. OF ACCESS (T) FROM 9+99.9 TO 9+70.0 ; 1,050 SF±. CHANGE BEGINNING STA. OF CONST. (T) FROM 9+83.0 TO 9+18.5 ; 2,650 SF±. PER C.O. 9444. MADE BY: MR APPROVED BY: RD	10/06/06



- EC - EROSION CONTROL
- (P) - PERMANENT
- (T) - TEMPORARY
- DR. - DRAINAGE RIGHT
- DIT. - DITCHING RIGHT
- CH. - CHANNEL RIGHT
- DRIVE - DRIVE RIGHT
- CUL. - CULVERT RIGHT
- C&T - CLEARING & TRIMMING RIGHT
- SR - SLOPE RIGHT
- UE - UTILITY EASEMENT

APPROVED: ROGER P. DUMAS DATE: 07-18-05
CHIEF, PLANS & TITLES

PLOT DATE 10/06/05

PROJECT NAME:	HUNTINGTON	PLOT DATE:	4/25/05
PROJECT NUMBER:	BRO 1445 (29)	DRAWN BY:	MR
FILE NAME:	r01j302.xls	CHECKED BY:	GF
PROJECT LEADER:	M. CHENETTE	R.O.W. SHEET	6 OF 7
DESIGNED BY:	D. ALTERI		

OCT 06 2005



2

MELVILLE, DIANNA

BEGIN APPROACH
TH 4 - BR 42
STA. 7+00.00

EXISTING FOUNDATION SHALL BE COVERED
AND PROTECTED FROM DAMAGE DURING
CONSTRUCTION, SEE DETAIL ON SHEET 11

4

**BRACE, LISA M.
& DELISLE, LESTER J.**

END APPROACH
TH 4 - BR 42
STA. 14+00.00

EXISTING R.O.W.
3 RODS
TO HUNTINGTON
6+00
7+00
EXISTING R.O.W.

BEGIN R.O.W. PROJECT
BRO 1445(29)
STA. 7+14.8 29.57' RT.

**BEANE, VANCE G.
& VIOLA S. & BEANE,
KEITH P. LIFE ESTATE**

1

5'-0" WIDE AGGREGATE SHOULDER
* STA. 7+25 LT. TO 8+65 LT.
STA. 11+42 LT. TO 12+72 LT.

* - TAPER AGGREGATE SHOULDER FROM
STA. 7+25 LT. (MATCH EXISTING)
TO STA. 7+75 LT. (5'-0")

PORTLAND CEMENT CONCRETE SIDEWALK, 6 INCH
STA. 8+67 LT. TO 9+04 LT.
STA. 10+93 LT. TO 11+40 LT.

* CAST-IN-PLACE CONCRETE CURB, TYPE B
STA. 8+67 LT. TO 9+04 LT.
STA. 10+93 LT. TO 11+40 LT.

* - CURB REVEAL TRANSITIONS FROM 7" AT
STA. 8+74 LT. TO 9" AT STA. 8+84 LT.,
AND FROM 9" AT STA. 8+08 LT. TO 7"
AT STA. 11+18 LT.

CONSTRUCT SIDEWALK RAMP, TYPE J
(SEE VAOT STD. C-3A)
STA. 8+74 LT.
STA. 11+33 LT.

TEMPORARY ACCESS EASEMENT 15'-0"
FROM EDGE OF GRAVEL PARKING LOT
(T.H. STA. 9+03.92 AND 5'-0" FROM EDGE
OF STONE FILL (CHANNEL STA. 50+82.00))

LACAILLADÉ, LEIGH

3

TAPER EDGE OF BILUMINOUS CONCRETE FROM:
STA. 7+25 LT. & RT. (MATCH EXISTING) TO STA. 7+75 LT. & RT. (11'-0")
STA. 12+00 RT. (11'-6") TO STA. 12+25 RT. (11'-0")
STA. 13+25 LT. & RT. (11'-0") TO STA. 13+75 LT. & RT. (MATCH EXISTING)

BRIDGE RAILING - ALUMINUM/PEDESTRIAN
STA. 8+86 LT. TO 10+97 LT.

BRIDGE RAILING - 3 BAR. ALUMINUM
STA. 9+23 RT. TO 11+23 RT.

ALUMINUM APPROACH RAIL (MOD. II)
STA. 7+75 LT. TO 8+83 LT.
STA. 9+15 RT. TO 9+23 RT.
STA. 8+07 LT. TO 11+95 LT.
STA. 11+23 RT. TO 11+98 RT.

CONSTRUCT DRIVE
STA. 8+83 RT.
STA. 12+68 RT.
STA. 12+84 LT.
STA. 13+16 LT.

SCALE 1"=30'



**WISNIOWSKI,
MARK &
JENNIFER A.**

6

END PROJECT
BEGIN APPROACH
TH 4 - BR 42
STA. 13+25.00

END R.O.W. PROJECT
BRO 1445(29)
STA. 14+01.6 33.66' RT.

**BLODGETT,
GARY A. & RUTH A.**

5

CURVE 1 DATA
PI STA. 10+58.88
R = 650.000 FT.
L = 446.726 FT.
DELTA = 39°22'40"
T = 232.591 FT.
E = 40.361 FT.

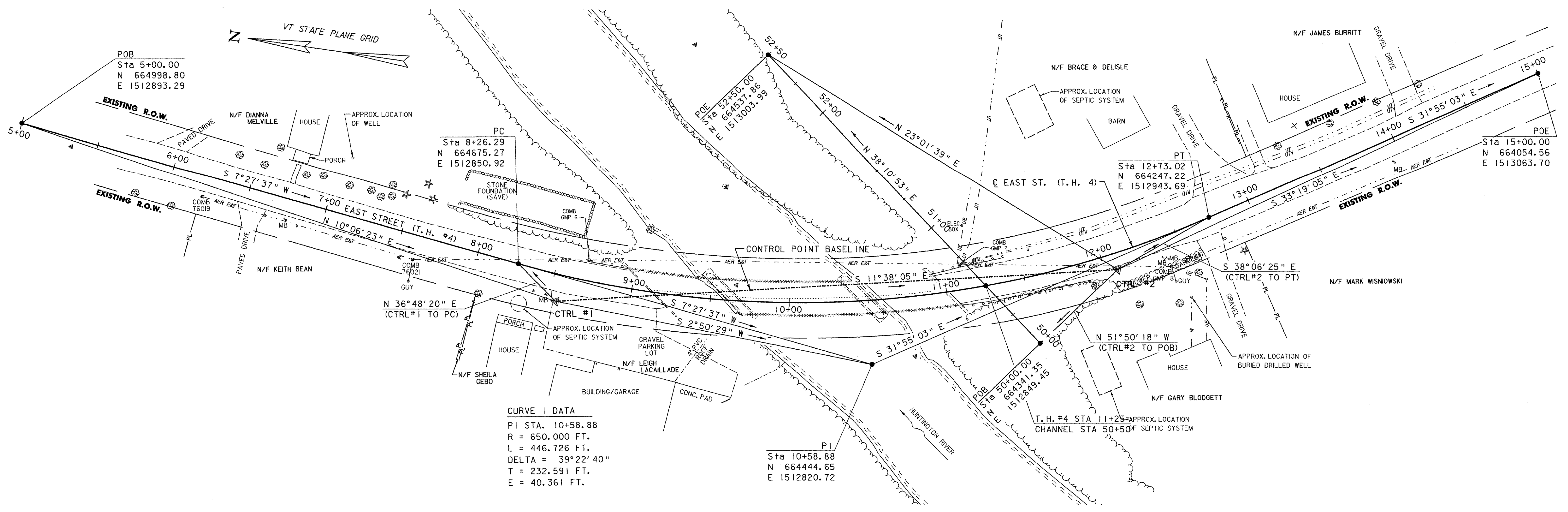
1 STA. 10+86 - 11+26 LT.
NEW 4'x4'x4' DEEP D1 w/ TYPE D GRATE
NEW 15" x 38" CPEP (SL)
INVERT AT INLET = 622.40 - 40'
INVERT AT OUTLET = 620.00
NEW 2'-6"W x 5'L x 1'D
STONE FILL, TYPE 1 OUTLET PAD
RIM = 625.91

2 STA. 11+10 - 12+90 RT.
NEW 4'x4'x3.5' DEEP D1 w/ TYPE D GRATE
NEW 15" x 18" CPEP (SL)
INVERT AT INLET = 618.18 - 178'
INVERT AT OUTLET = 611.27
NEW 15" CPEP'S AT OUTLET
REMOVE EXISTING 12" CPEP
RIM = 621.93

LINES SHOWN ON THIS PLAN AS EXISTING
PROPERTY LINES P/L ARE BELIEVED TO
BE ACCURATE BUT SHOULD NOT BE RELIED
UPON FOR PURPOSES UNRELATED TO THE
TOWN OF HUNTINGTON'S ACQUISITION OF LAND
AND RIGHTS FOR THIS PROJECT.

OCT 06 2005

PROJECT NAME:	HUNTINGTON	PLOT DATE:	06-OCT-2005
PROJECT NUMBER:	BRO 1445 (29)	DRAWN BY:	J. OAKMAN
FILE NAME:	%FILEABBREV%	CHECKED BY:	D. ALTERI
PROJECT LEADER:	M. CHENETTE	R.O.W. SHEET	7 OF 7

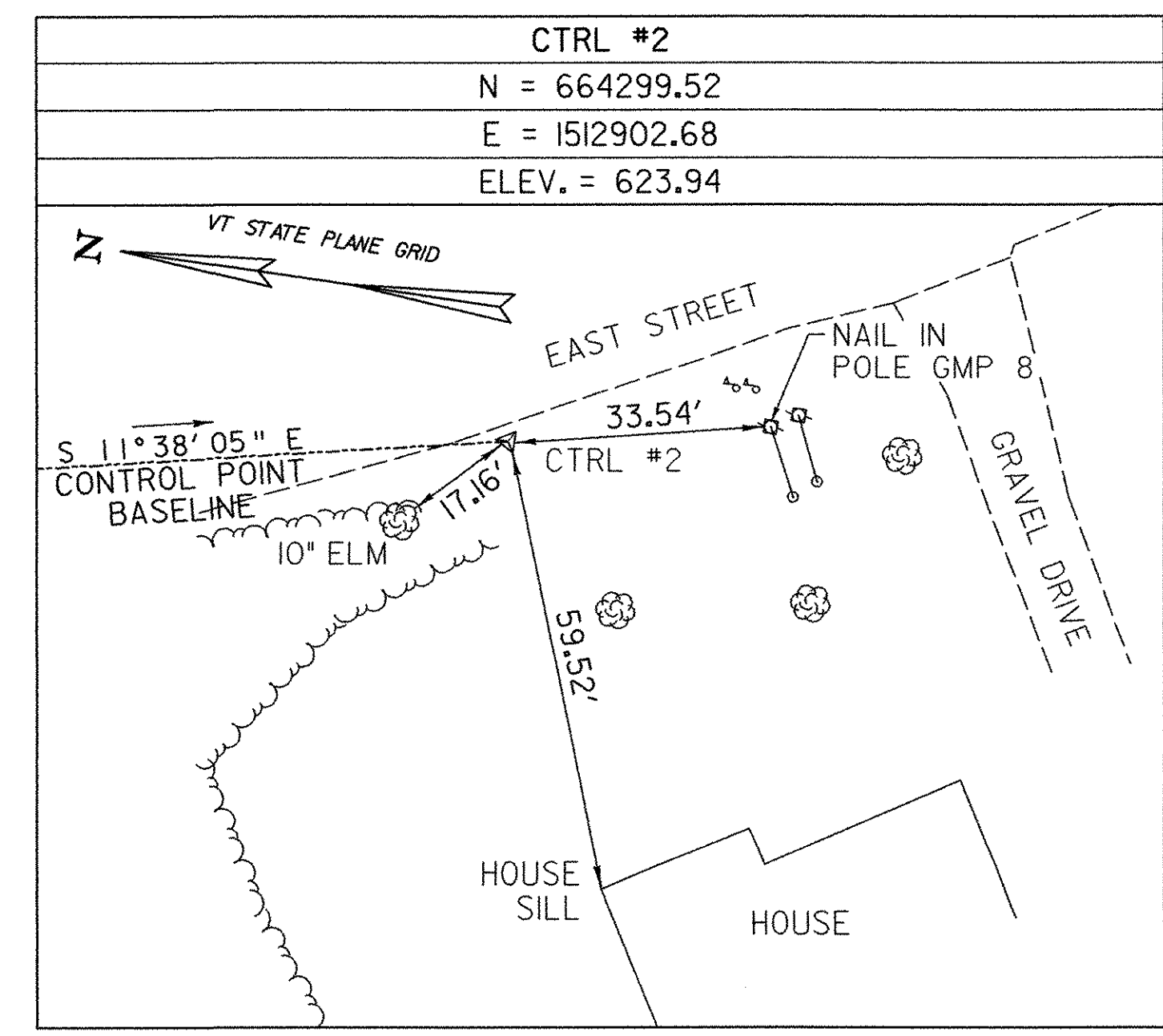
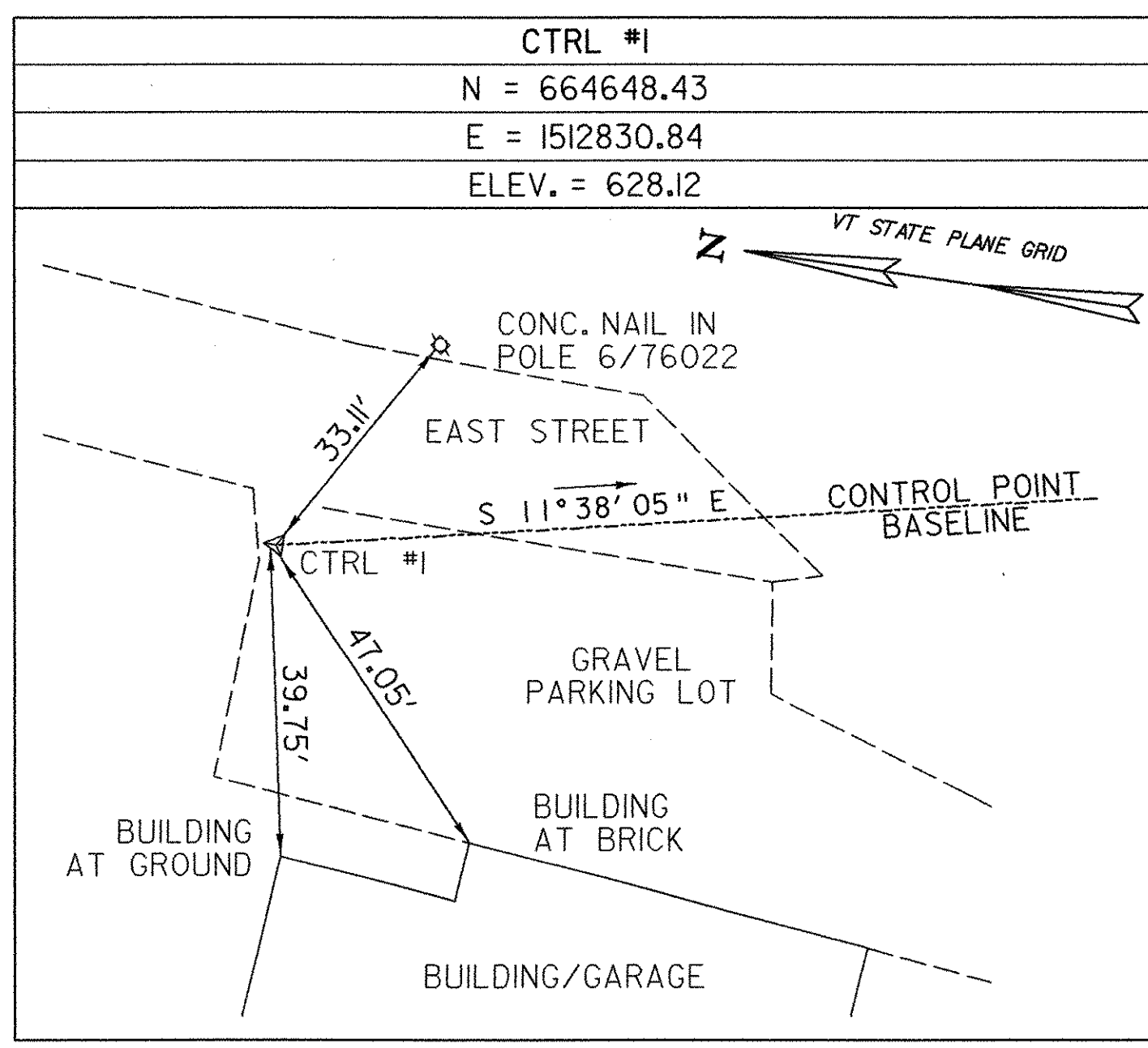


CURVE 1 DATA
 PI STA. 10+58.88
 R = 650.000 FT.
 L = 446.726 FT.
 DELTA = 39°22'40"
 T = 232.591 FT.
 E = 40.361 FT.

P.I.
 Sta 10+58.88
 N 664444.65
 E 1512820.72

PLAN

SCALE 1" = 30'-0"
 30 0 30



CENTERLINE EAST STREET (T.H. 4) ALIGNMENT				
BS	OC	FS POINT	ANGLE RIGHT	DISTANCE
CTRL #2	CTRL #1	POB	201°44'-28"	355.90'
CTRL #2	CTRL #1	PC	228°26'-25"	33.53'
CTRL #2	CTRL #1	PI	14°28'-34"	204.03'
CTRL #1	CTRL #2	PT	153°-31'-39"	66.46'
CTRL #1	CTRL #2	POE	158°-19'-00"	293.14'

BS = BACKSITE STATION
 OC = OCCUPIED STATION
 FS = FORESITE STATION

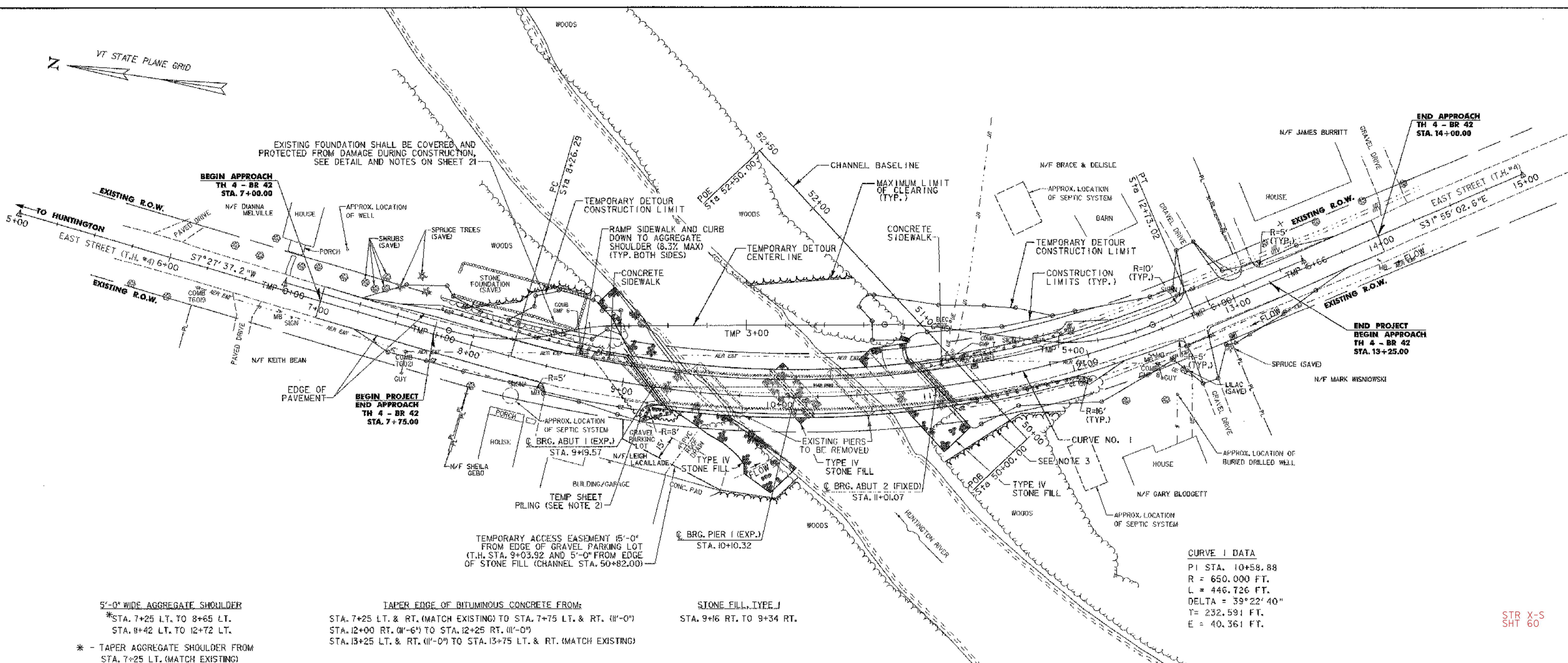
CHANNEL ALIGNMENT				
BS	OC	FS POINT	ANGLE RIGHT	DISTANCE
CTRL #1	CTRL #2	POB	319°-47'-47"	67.70'
CTRL #1	CTRL #2	POE	34°-39'-44"	258.98'

BS = BACKSITE STATION
 OC = OCCUPIED STATION
 FS = FORESITE STATION

PROJECT NAME: HUNTINGTON
 PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...Cadd\Trans\2013021e.dgn PLOT DATE: 1/12/2006
 PROJECT LEADER: M. CHENETTE DRAWN BY: D. HARRINGTON
 DESIGNED BY: T. KNIGHT CHECKED BY: T. KNIGHT
TIE SHEET SHEET 9 OF 63





EXISTING FOUNDATION SHALL BE COVERED AND PROTECTED FROM DAMAGE DURING CONSTRUCTION, SEE DETAIL AND NOTES ON SHEET 21

BEGIN APPROACH
TH 4 - BR 42
STA. 7+00.00

BEGIN PROJECT
END APPROACH
TH 4 - BR 42
STA. 7+75.00

END APPROACH
TH 4 - BR 42
STA. 14+00.00

END PROJECT
BEGIN APPROACH
TH 4 - BR 42
STA. 13+25.00

CURVE 1 DATA
 P1 STA. 10+58.88
 R = 650.000 FT.
 L = 446.726 FT.
 DELTA = 39°22'40"
 T = 232.591 FT.
 E = 40.361 FT.

STR X-S
SHT 60

5'-0" WIDE AGGREGATE SHOULDER
 *STA. 7+25 LT. TO 8+65 LT.
 STA. 8+42 LT. TO 12+72 LT.

* - TAPER AGGREGATE SHOULDER FROM STA. 7+25 LT. (MATCH EXISTING) TO STA. 7+75 LT. (15'-0")

TAPER EDGE OF BITUMINOUS CONCRETE FROM:
 STA. 7+25 LT. & RT. (MATCH EXISTING) TO STA. 7+75 LT. & RT. (11'-0")
 STA. 12+00 RT. (11'-6") TO STA. 12+25 RT. (11'-0")
 STA. 13+25 LT. & RT. (11'-0") TO STA. 13+75 LT. & RT. (MATCH EXISTING)

STONE FILL, TYPE J
 STA. 9+16 RT. TO 9+34 RT.

PORTLAND CEMENT CONCRETE SIDEWALK, 5 INCH
 STA. 8+67 LT. TO 9+04 LT.
 STA. 10+93 LT. TO 11+40 LT.

BRIDGE RAILING - ALUMINUM/PEDESTRIAN
 STA. 8+86 LT. TO 10+97 LT.

BRIDGE RAILING - 3 RAIL ALUMINUM
 STA. 9+23 RT. TO 11+23 RT.

***CAST-IN-PLACE CONCRETE CURB, TYPE B**
 STA. 8+67 LT. TO 9+04 LT.
 STA. 10+93 LT. TO 11+40 LT.

* - CURB REVEAL TRANSITIONS FROM 7" AT STA. 8+74 LT. TO 9" AT STA. 8+84 LT. AND FROM 9" AT STA. 11+08 LT. TO 7" AT STA. 11+18 LT.

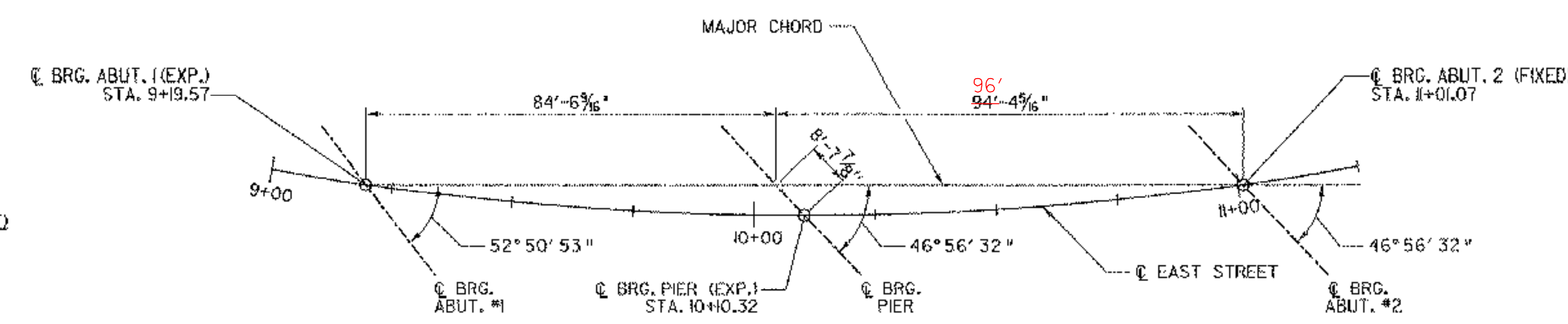
ALUMINUM APPROACH RAIL (MOD. I)
 STA. 7+75 LT. TO 8+83 LT.
 STA. 9+15 RT. TO 9+23 RT.
 STA. 11+07 LT. TO 11+95 LT.
 STA. 11+23 RT. TO 11+98 RT.

RELOCATE MAIL BOX (SINGLE SUPPORT)
 STA. 8+49 RT.
 STA. 12+38 RT.
 STA. 12+40 RT.

CONSTRUCT SIDEWALK RAMP, TYPE I
 (SEE VAOT STD. C-3A)
 STA. 8+74 LT.
 STA. 11+33 LT.

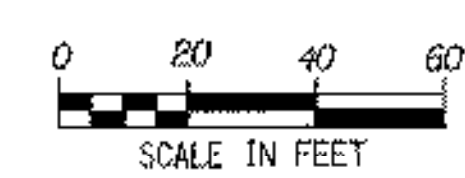
CONSTRUCT DRIVE
 STA. 8+83 RT.
 STA. 12+68 RT.
 STA. 12+84 LT.
 STA. 13+16 LT.

DURABLE 4" YELLOW LINE
 STA. 7+00 TO 14+00, DOUBLE CENTERLINE



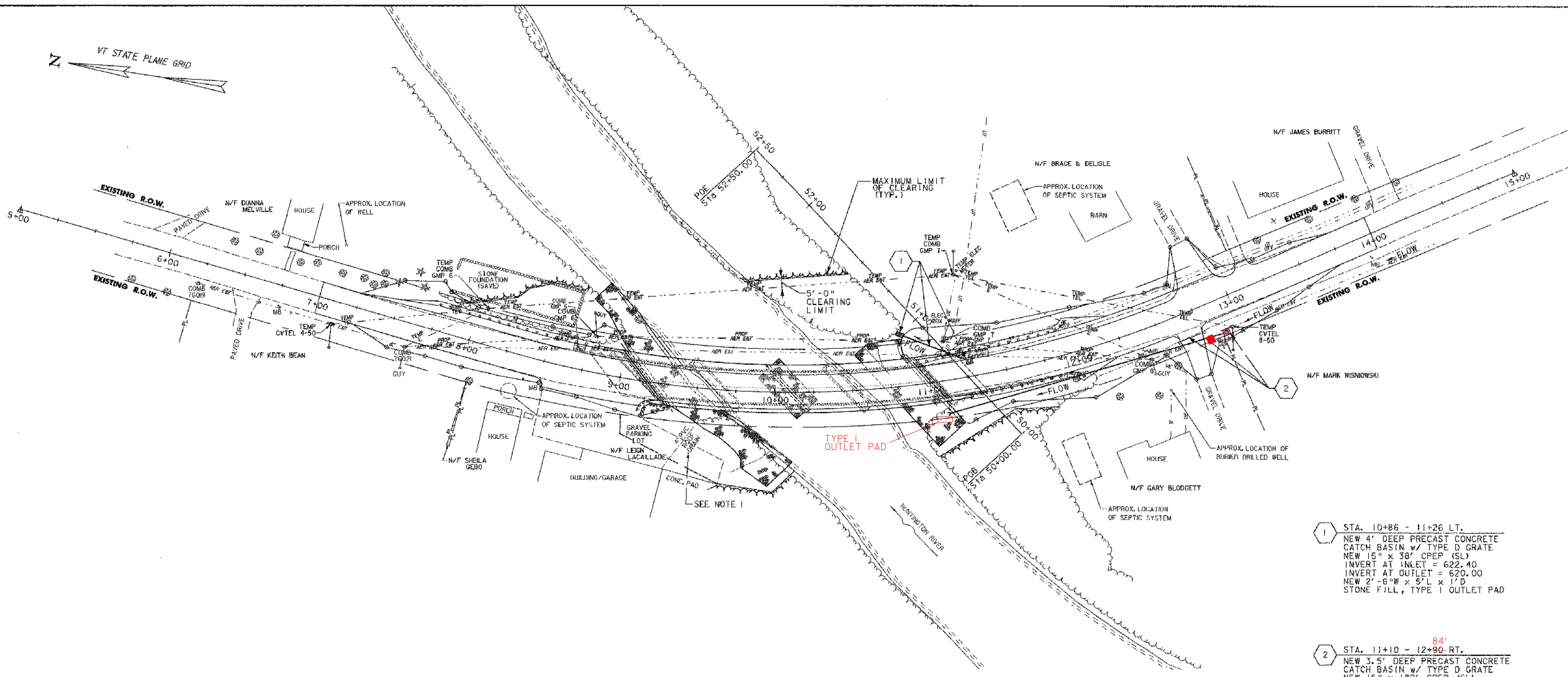
STRUCTURE LAYOUT
 SCALE: 1"=20'

- NOTE:**
- FOR UTILITIES AND DRAINAGE INFORMATION SEE NEXT SHEET.
 - TEMPORARY STEEL SHEET PILING, STATION 9+00 RT TO 9+40 RT WILL BE PAID FOR UNDER ITEM 505.36. ANY SHEET PILING OR TEMPORARY RETAINING STRUCTURES ADJACENT TO THE TEMPORARY BRIDGE (SEE SHEET 21) WILL BE PAID FOR UNDER ITEM 528.10, ONE-WAY TEMPORARY BRIDGE (MOD.).
 - HIGHER TREE LIMBS OVERHANGING THE CLEARING LIMIT IN THIS AREA SHALL NOT BE TRIMMED.



PROJECT NAME:	HUNTINGTON	FILE NAME:	...Trans\PlotFiles\08Layout.ptf	PLOT DATE:	1/30/2006
PROJECT NUMBER:	BRO 1445 (29)	PROJECT LEADER:	M. CHENETTE	DRAWN BY:	J. OAKMAN
		DESIGNED BY:	D. ALTERI	CHECKED BY:	D. ALTERI
		LAYOUT SHEET			SHEET 10 OF 63





1 STA. 10+86 - 11+26 LT.
 NEW 4' DEEP PRECAST CONCRETE CATCH BASIN w/ TYPE D GRATE
 NEW 15" x 38" CPEP (SL)
 INVERT AT INLET = 622.40
 INVERT AT OUTLET = 620.00
 NEW 2'-6"W x 5'L x 1'D STONE FILL, TYPE I OUTLET PAD

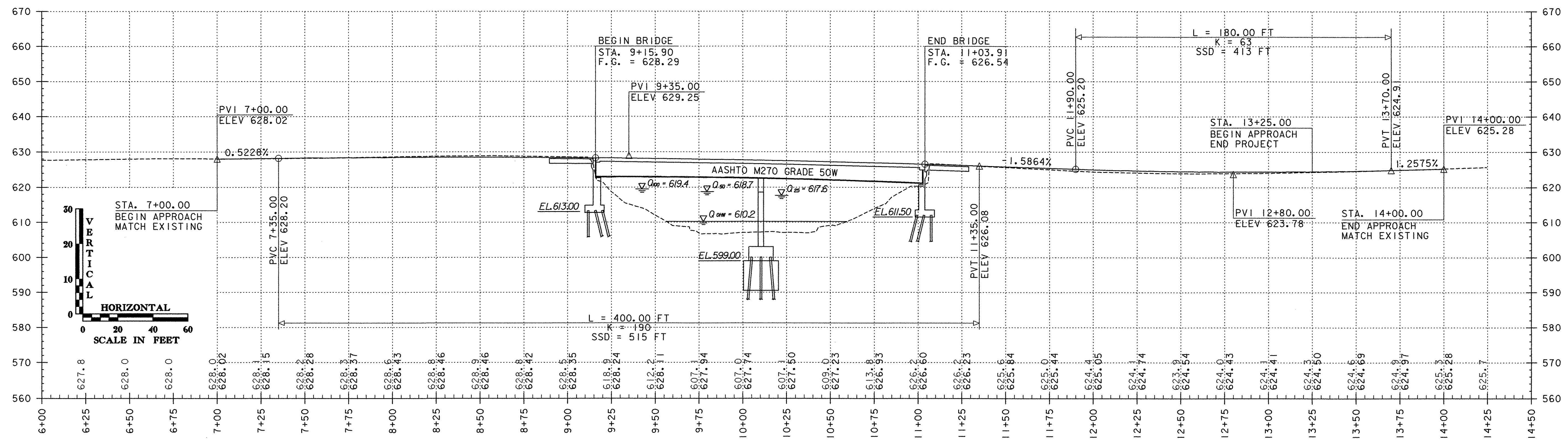
2 STA. 11+10 - 12+90 RT. ^{84'}
 NEW 3.5' DEEP PRECAST CONCRETE CATCH BASIN w/ TYPE D GRATE
 NEW 15" x 48" CPEP (SL)
 INVERT AT INLET = 618.18
 INVERT AT OUTLET = 617.27 ^{178'} 617.27
 NEW 15" CPEPES AT OUTLET
 REMOVE EXISTING 12" CPEP

NOTE:

1. THE CONTRACTOR SHALL TAKE PROPER PRECAUTION TO AVOID DAMAGE TO THE EXISTING ROOF DRAIN. THE CONTRACTOR SHALL REPAIR ANY DAMAGE TO THE DRAIN AT NO COST TO THE OWNER OR TO THE PROJECT.

PROJECT NAME:	HUNTINGTON
PROJECT NUMBER:	BRO 1445 (29)
FILE NAME: ..\Trans\PlotFiles\09Drn&Util.plt	PLOT DATE: 1/30/2006
PROJECT LEADER: M. CHENETTE	DRAWN BY: J. OAKMAN
DESIGNED BY: D. ALTERI	CHECKED BY: D. ALTERI
DRAINAGE AND UTILITY PLAN	
SHEET 11 OF 63	





NOTE: GRADES SHOWN TO THE NEAREST TENTH ARE EXISTING GROUND ALONG CENTERLINE. GRADES SHOWN TO THE NEAREST HUNDREDTH ARE FINISH GRADES ALONG CENTERLINE.

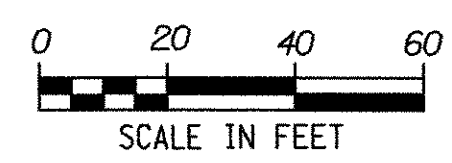
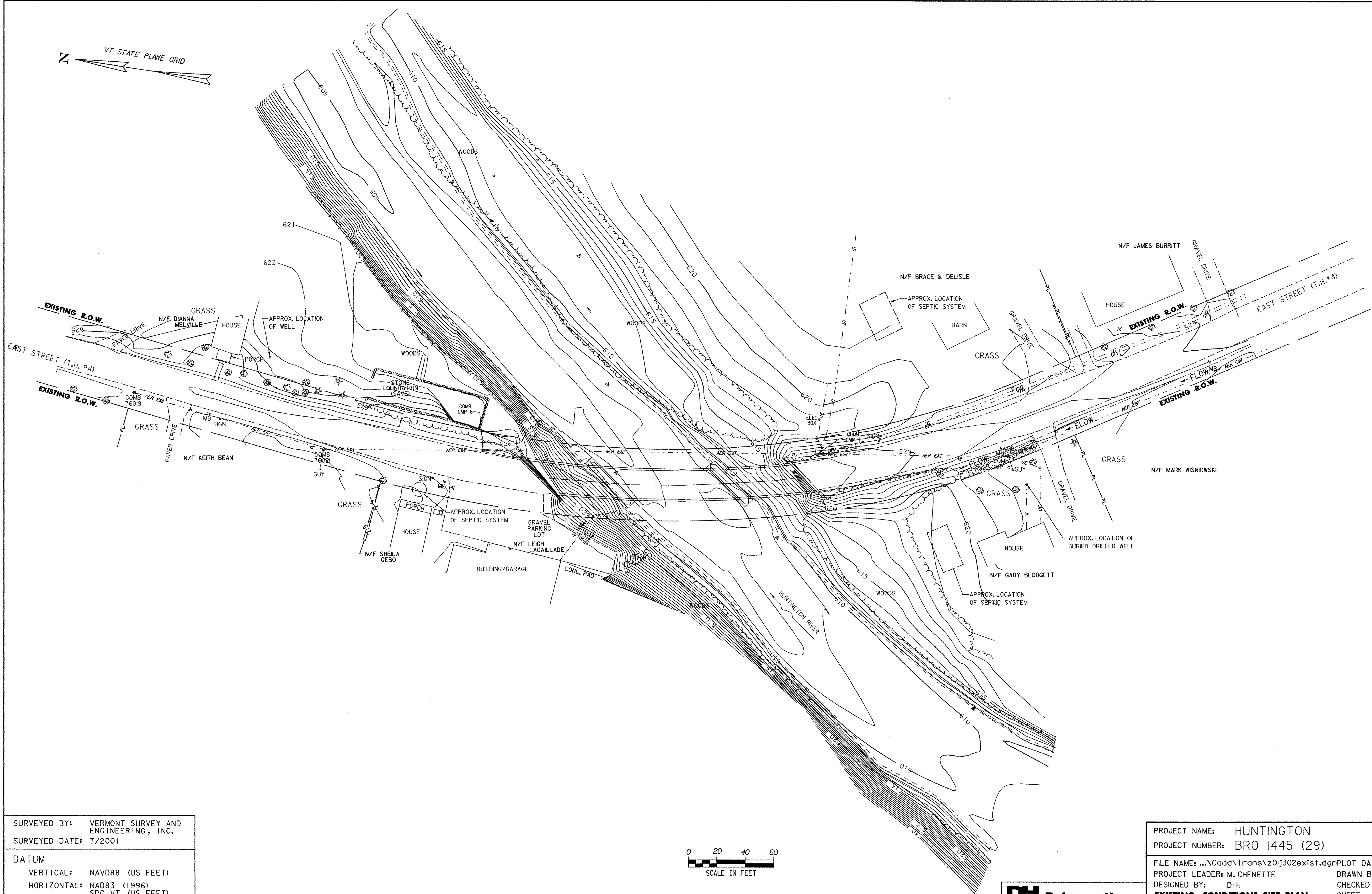
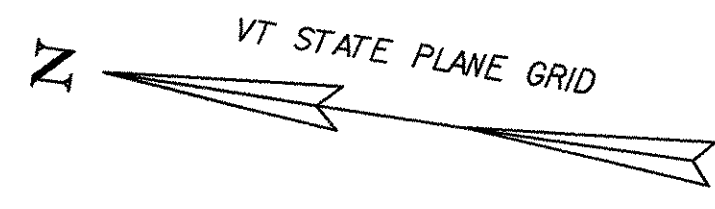
PROFILE ALONG CENTERLINE OF EAST STREET (T.H. #4)

SCALE: HORIZ.: 1" = 30'-0"
 VERT.: 1" = 15'-0"

PROJECT NAME: HUNTINGTON
 PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...Cadd\Trans\z01j302pro.dgn PLOT DATE: 1/12/2006
 PROJECT LEADER: M. CHENETTE DRAWN BY: J. OAKMAN
 DESIGNED BY: D. ALTERI CHECKED BY: D. ALTERI
PROFILE SHEET SHEET 12 OF 63





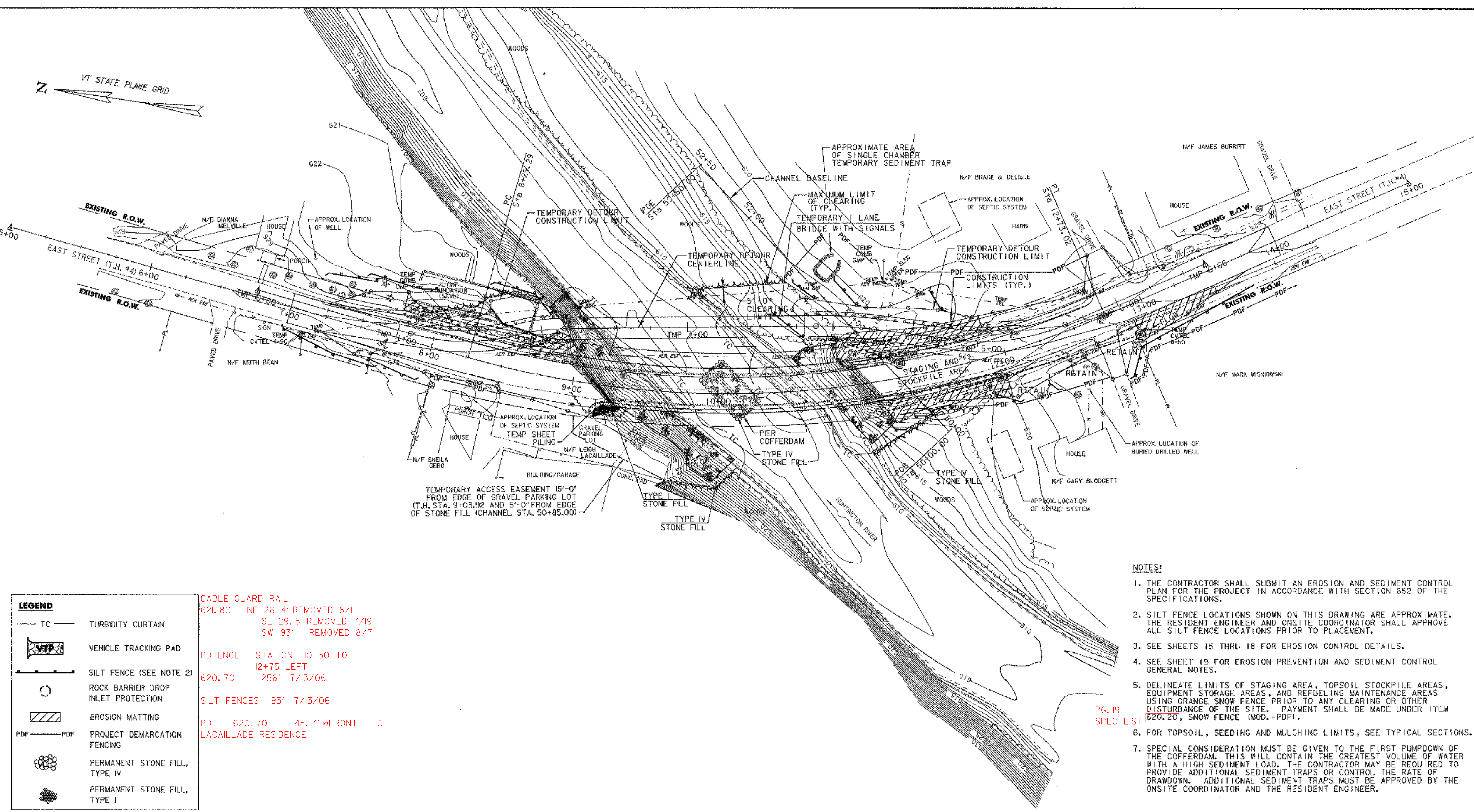
SURVEYED BY: VERMONT SURVEY AND ENGINEERING, INC.
SURVEYED DATE: 7/2001

DATUM
VERTICAL: NAVD88 (US FEET)
HORIZONTAL: NAD83 (1996) SPC VT (US FEET)

PROJECT NAME: HUNTINGTON
PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...Cadd\Trans\z01j302exist.dgn PLOT DATE: 1/12/2006
PROJECT LEADER: M. CHENETTE DRAWN BY: D. HARRINGTON
DESIGNED BY: D-H CHECKED BY: M. CHENETTE
EXISTING CONDITIONS SITE PLAN SHEET 13 OF 63





LEGEND	
	TURBIDITY CURTAIN
	VEHICLE TRACKING PAD
	SILT FENCE (SEE NOTE 2)
	ROCK BARRIER DROP INLET PROTECTION
	EROSION MATTING
	PROJECT DEMARCATION FENCING
	PERMANENT STONE FILL, TYPE IV
	PERMANENT STONE FILL, TYPE I

CABLE GUARD RAIL
 621.80 - NE 26.4' REMOVED 8/1
 SE 29.5' REMOVED 7/19
 SW 93' REMOVED 8/7

PDFENCE - STATION 10+50 TO 12+75 LEFT
 620.70 256' 7/13/06

SILT FENCES 93' 7/13/06
 PDF - 620.70 - 45.7' @FRONT OF LACAILLADE RESIDENCE

TEMPORARY ACCESS EASEMENT 15'-0" FROM EDGE OF GRAVEL PARKING LOT (T.H. STA. 9+03.92 AND 5'-0" FROM EDGE OF STONE FILL (CHANNEL STA. 50+85.00))

NOTES:

1. THE CONTRACTOR SHALL SUBMIT AN EROSION AND SEDIMENT CONTROL PLAN FOR THE PROJECT IN ACCORDANCE WITH SECTION 652 OF THE SPECIFICATIONS.
2. SILT FENCE LOCATIONS SHOWN ON THIS DRAWING ARE APPROXIMATE. THE RESIDENT ENGINEER AND ONSITE COORDINATOR SHALL APPROVE ALL SILT FENCE LOCATIONS PRIOR TO PLACEMENT.
3. SEE SHEETS 15 THRU 18 FOR EROSION CONTROL DETAILS.
4. SEE SHEET 19 FOR EROSION PREVENTION AND SEDIMENT CONTROL GENERAL NOTES.
5. DELINEATE LIMITS OF STAGING AREA, TOPSOIL STOCKPILE AREAS, EQUIPMENT STORAGE AREAS, AND REFUELING MAINTENANCE AREAS USING ORANGE SNOW FENCE PRIOR TO ANY CLEARING OR OTHER DISTURBANCE OF THE SITE. PAYMENT SHALL BE MADE UNDER ITEM 620.20, SNOW FENCE (MOD.-PDF).
6. FOR TOPSOIL, SEEDING AND MULCHING LIMITS, SEE TYPICAL SECTIONS.
7. SPECIAL CONSIDERATION MUST BE GIVEN TO THE FIRST PUMPDOWN OF THE COFFERDAM. THIS WILL CONTAIN THE GREATEST VOLUME OF WATER WITH A HIGH SEDIMENT LOAD. THE CONTRACTOR MAY BE REQUIRED TO PROVIDE ADDITIONAL SEDIMENT TRAPS OR CONTROL THE RATE OF DRAWDOWN. ADDITIONAL SEDIMENT TRAPS MUST BE APPROVED BY THE ONSITE COORDINATOR AND THE RESIDENT ENGINEER.

PG. 19
SPEC LIST

SURVEYED BY: VERMONT SURVEY AND ENGINEERING, INC.
 SURVEYED DATE: 7/2001

DATUM
 VERTICAL: NAVD88 (US FEET)
 HORIZONTAL: NAD83 (1996) SPC VT (US FEET)



PROJECT NAME: HUNTINGTON
 PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...\\Cadd\Trans\z01302eros.dgn PLOT DATE: 1/30/2006
 PROJECT LEADER: M. CHENETTE DRAWN BY: J. OAKMAN
 DESIGNED BY: D. ALTERI CHECKED BY: D. ALTERI
EROSION & SEDIMENT CONTROL PLAN SHEET 14 OF 63

PROJECT DESCRIPTION

THE PROJECT IS LOCATED APPROXIMATELY 0.25 MILES SOUTH OF THE INTERSECTION OF MAIN ROAD (T.H. 1) AND EAST STREET (T.H. 4) IN HUNTINGTON, VT AND PROCEEDS SOUTHERLY APPROXIMATELY 0.10 MILES ALONG EAST STREET (T.H. 4).

WORK TO BE PERFORMED FOR THIS PROJECT IS THE CONSTRUCTION OF A TEMPORARY ROADWAY AND BRIDGE TO THE EAST (DOWNSTREAM) SIDE OF THE EXISTING STRUCTURE. THE COMPLETE REMOVAL OF THE EXISTING SUPERSTRUCTURE AND SUBSTRUCTURE. THE CONSTRUCTION OF A NEW SUBSTRUCTURE (PIER AND ABUTMENTS) AND SUPERSTRUCTURE, ARMORING OF THE RIVER BANKS WITH STONE FILL AND RECONSTRUCTION OF THE APPROACHES. THE NEW BRIDGE, WHICH WILL BE CONSTRUCTED ALONG THE EXISTING ALIGNMENT, WILL BE WIDER THAN THE EXISTING STRUCTURE AND WILL HAVE A SIDEWALK ON THE EAST (DOWNSTREAM) SIDE. THE TOTAL DISTURBED AREA WILL BE LESS THAN 1.25 ACRES OVER THE ENTIRE PROJECT.

SITE INVENTORY AND ANALYSIS

THE AREA NORTH OF THE SITE IS A VILLAGE SETTING (HUNTINGTON VILLAGE). THERE ARE HOUSES WITH GRASS LAWNS AND SOME TREES FOR LANDSCAPING. ALONG THE EDGE OF THE RIVER ON THE NORTHEAST SIDE THERE IS A FORESTED AREA. ON THE NORTHWEST SIDE THERE IS A BUILDING WITH A GRAVEL PARKING LOT LOCATED AT THE TOP OF THE RIVER BANK. THE AREA SOUTH OF THE SITE IS ALSO A VILLAGE SETTING WITH NEWER HOUSES, LARGER GRASS LAWNS, AND SOME TREES FOR LANDSCAPING. ON THE SOUTH SIDE THE LAWNS EXTEND TO THE TOP OF THE RIVER BANK. ALL HOMES ALONG THE SITE HAVE SEPTIC SYSTEMS AND WELLS ON THEIR PROPERTIES. THE UNIVERSITY OF VERMONT CONSULTING ARCHAEOLOGY PROGRAM CONDUCTED A PHASE I ARCHAEOLOGICAL SITE IDENTIFICATION SURVEY IN THE PROJECT AREA AND MADE A DETERMINATION OF NO EFFECT FOR THE PURPOSED PROJECT.

AERIAL ELECTRICAL AND TELEPHONE LINES ARE LOCATED ON THE WESTERN SIDE OF THE ROADWAY ON THE NORTH SIDE OF THE BRIDGE THEN SWITCH TO THE EASTERN SIDE OF THE ROADWAY BEFORE CROSSING THE HUNTINGTON RIVER. ONCE ON THE SOUTH SIDE OF THE BRIDGE THEY SWITCH BACK TO THE WESTERN SIDE OF THE ROADWAY. UNDERGROUND ELECTRICAL AND TELEPHONE LINES RUN FROM THE POLE ON THE SOUTHEASTERN SIDE (GMP #7) OF THE BRIDGE TO THE HOME ON THE SAME SIDE. AN UNDERGROUND ELECTRICAL LINE RUNS FROM THE POLE ON THE SOUTHWESTERN SIDE (GMP #8) OF THE BRIDGE TO THE HOME ON THE SAME SIDE. THE ROADWAY IS PAVED, WITH THE LAWNS EXTENDING TO THE EDGE OF THE ROADWAY. THE ROADWAY HAS A RELATIVELY FLAT PROFILE WITH A TYPICAL CROWN. STORMWATER RUNS EAST AND WEST FROM THE ROADWAY, AND CASCADES OVER THE LAWNS. STORMWATER ON THE NORTHWEST GRAVEL PARKING LOT DRAINS TO THE SOUTHWEST AND INTO THE HUNTINGTON RIVER. ON THE NORTHEAST SIDE THE LAWNS SLOPE EASTWARD TOWARD THE HUNTINGTON RIVER. ON THE NORTHWEST SIDE THE LAWNS ARE FLAT. ON THE SOUTH SIDE BOTH LAWNS SLOPE NORTHWARD TO THE HUNTINGTON RIVER.

THE HUNTINGTON RIVER GOES DIRECTLY THROUGH THE SITE. THE RIVER HAS A GRAVEL AND STONE BOTTOM WITH STEEP FORESTED BANKS ON ITS NORTH SIDE AND SHALLOWER BANKS WITH TALL GRASS AND SHRUBS AND SMALL CLUMPS OF TREES ON ITS SOUTH SIDE. THE SOUTHERN BANK BECOMES STEEPER AT THE BRIDGE ABUTMENT. THE NORTH ABUTMENT IS LOCATED AT THE TOP OF THE BANK WITH A PIER LOCATED AT THE RIVER'S EDGE. THE SOUTHERN ABUTMENT IS LOCATED BEYOND THE RIVER'S EDGE, BUT HAS SIDE SLOPES BECAUSE THE ROADWAY IS BUILT UP FROM THE NATURAL GRADE. THERE IS ALSO A PIER LOCATED AT THE RIVER'S EDGE ON THE SOUTH SIDE. THERE IS A DITCH ALONG THE ROADWAY'S TOE OF SLOPE ON THE SOUTHWEST SIDE. THERE IS A SWALE FROM THE GRAVEL DRIVEWAY TO THE RIVER ON THE NORTHWEST SIDE. THERE ARE NO WETLANDS ON OR NEAR THE SITE.

THE NATIVE SOIL THROUGHOUT MOST OF THIS PROJECT IS AGAWAM FINE SANDY LOAM ($k_w = 0.28$) WHICH IS MODERATELY ERODIBLE. THERE IS ALSO SOME WINOOSKI VERY FINE SANDY LOAM ($k_w = 0.49$) WHICH IS HIGHLY ERODIBLE ON THE SOUTHWEST SIDE OF THE PROJECT. HOWEVER, SINCE MOST OF THE WORK WILL OCCUR IN AREAS PREVIOUSLY DISTURBED FOR THE CONSTRUCTION OF THE EXISTING BRIDGE AND ROADWAY MOST OF THE DISTURBED SOILS WILL BE SAND AND GRAVEL ($k_w < 0.23$) WHICH HAS A LOW ERODIBILITY.

TEMPORARY EROSION AND SEDIMENT CONTROL

THE FOLLOWING EROSION PREVENTION AND SEDIMENT CONTROL MEASURES SHALL BE USED AT THIS SITE:

SNOW FENCE (MOD. - PDF)
FENCING WILL BE USED TO DEMARCAT LIMITS OF CONSTRUCTION AS DIRECTED BY THE ENGINEER. FENCING SHALL BE PAID FOR UNDER ITEM 620.70, SNOW FENCE (MOD. - PDF)

SILT FENCE:
SILT FENCE IS TO BE CONSTRUCTED AS SHOWN ON THE PLANS, WITH PROPER EMBEDMENT AND ANCHORING, AS WELL AS ENDS THAT ARE "CURLED" UPHILL TO PROMOTE PONDING AND SETTLING OF SEDIMENT. SEE DETAILS ON SHEET 16.

TEMPORARY SEED AND MULCH:
ALL EXPOSED SOILS SHALL BE MULCHED WITHIN 48 HOURS IF NOT WORKED WITHIN 7 DAYS AND SEEDED AND MULCHED AND/OR MATTED WITHIN 48 HOURS IF NOT WORKED WITHIN 30 DAYS.

EROSION MATTING:
EROSION MATTING SHALL BE USED ON ANY SLOPE GREATER THAN 3H:1V, ON ALL DISTURBED DRAINAGE SWALES AND AS SHOWN ON THE PLANS. EROSION MATTING SHALL BE INSTALL IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS.

VEHICLE TRACKING PAD:
VEHICLE TRACKING PAD SHALL BE USED TO PROVIDE A STABILIZED ENTRANCE TO ALL CONSTRUCTION STAGING AREAS AND TO ASSIST WITH THE REMOVAL OF SEDIMENT CAPTURED ON THE TIRES OF CONSTRUCTION VEHICLES ENTERING AND EXITING THESE STAGING AREAS. SEE DETAILS ON SHEET 17.

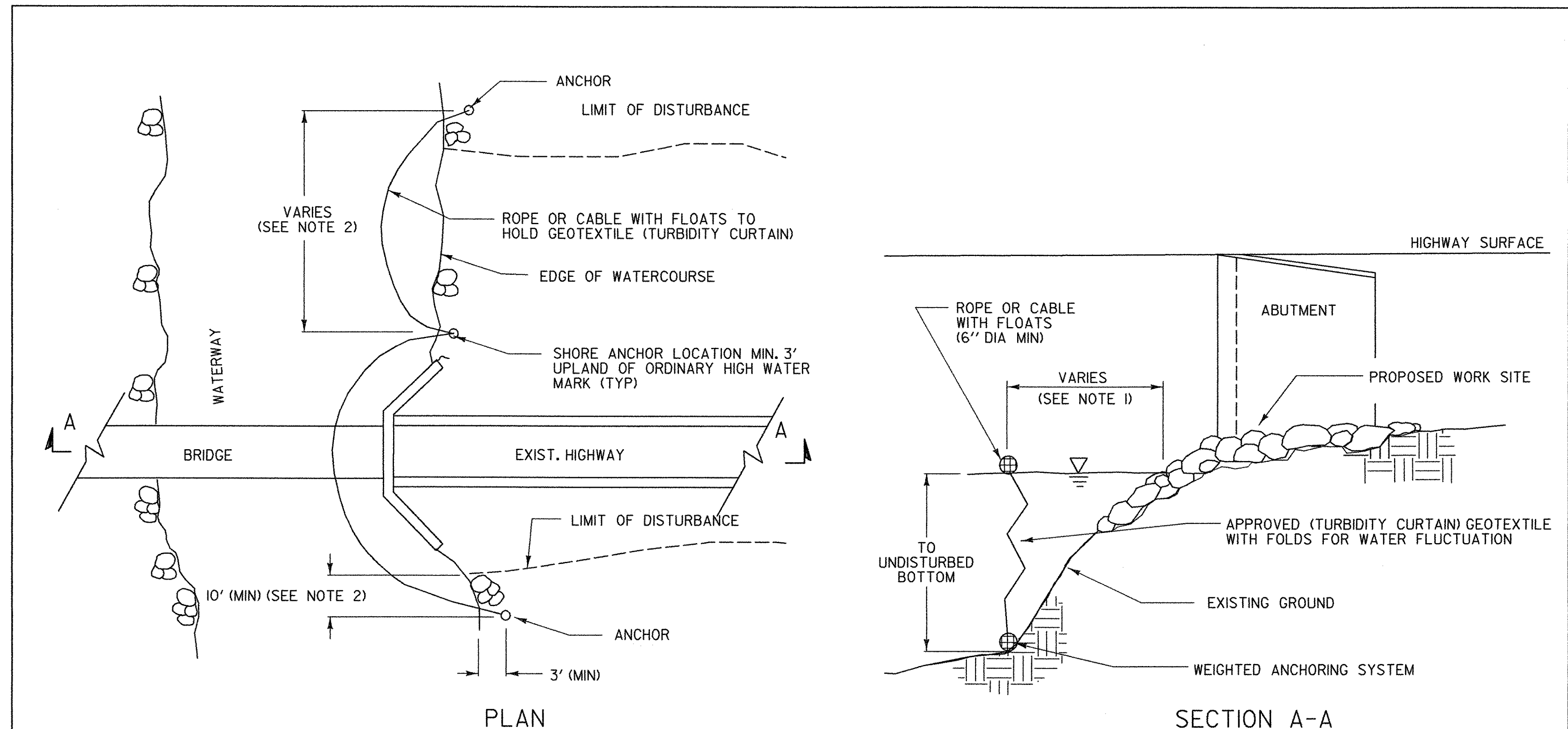
TURBIDITY CURTAIN:
TURBIDITY CURTAIN SHALL BE USED TO PREVENT SILT LADEN WATER FROM ENTERING THE RIVER DURING THE REMOVAL OF THE EXISTING PIERS AND THE ARMOURING OF THE BANKS OF THE RIVER. THE TURBIDITY CURTAIN IS TO BE CONSTRUCTED AS SHOWN ON THE PLANS. SEE DETAILS ON THIS SHEET.

ROCK BARRIER INLET PROTECTION:
ROCK BARRIER INLET PROTECTION SHALL BE USED TO FILTER RUNOFF AND CREATE PONDING TO ALLOW SETTLING OF SEDIMENT. IT IS NOT TO BE USED IF THE INLET IS ON A SLOPE, AS THIS COULD CAUSE WATER TO BYPASS THE INLET AND POTENTIALLY CREATE PROBLEMS WITH FLOODING DOWNSTREAM. THE ROCK BARRIER INLET PROTECTION SHOULD ONLY BE USED IN A SUMP SITUATION, WHERE WATER WILL POND UP, FLOW OVER OR FILTER THROUGH THE ROCK, AND ENTER THE INLET. SEE THE DETAIL ON SHEET 18.

SINGLE CHAMBER TEMPORARY SEDIMENT TRAP:
SEDIMENTATION TRAP SHALL BE USED TO SETTLE SEDIMENT LADEN WATER FROM THE PUMP DOWN OF THE PIER COFFERDAM. SEE THE DETAIL ON SHEET 17.

FINAL EROSION CONTROL MEASURES

THE FINAL EROSION CONTROL MEASURES CONSIST OF STONE FILL, TYPE IV PLACED IN FRONT OF THE NEW ABUTMENTS AND AROUND THE NEW PIER. A NEW CULVERT WITH A DI AND STONE OUTLET PROTECTION AT THE SOUTHEAST END OF THE BRIDGE TO COLLECT ALL THE STORMWATER FROM THE BRIDGE. A NEW CULVERT WITH A DI AND STONE OUTLET PROTECTION JUST SOUTH OF THE FIRST DRIVEWAY ON THE SOUTHEAST SIDE TO COLLECT STORMWATER FROM LAWN AREAS SOUTH OF THE SITE. THE RESEEDING OF ALL DISTURBED AND NEW SIDE SLOPES AND EXPOSED SOILS.



TURBIDITY CURTAIN - TEMPORARY

APPLICATION NOTES:

- THE PURPOSE OF A TURBIDITY CURTAIN IS TO SEPARATE WORK AREAS IN OR ADJACENT TO WATERS, TO PREVENT SEDIMENT FROM ENTERING THE WATERS.
- TURBIDITY CURTAIN SHALL NOT BE PLACED ACROSS A FLOWING WATERWAY. THE TURBIDITY CURTAIN SHALL BE A TYPE III (HEAVY DUTY) CURTAIN CAPABLE OF RESISTING STREAM VELOCITIES UP TO 5 FT/SEC.

GENERAL NOTES:

- THE TURBIDITY CURTAIN SHALL BE PLACED AS CLOSE TO THE WORK AS POSSIBLE WITHOUT INTERFERING WITH CONSTRUCTION OPERATIONS.
- THE TURBIDITY CURTAIN SHALL BE A MAXIMUM OF 100 FEET LONG BETWEEN ANCHORS. LAST SECTION SHALL TERMINATE A MINIMUM OF 10 FEET BEYOND THE LIMIT OF DISTURBANCE.
- THE CONTRACTOR SHALL MONITOR THE TURBIDITY CURTAIN, TAKING INTO ACCOUNT WEATHER PATTERNS AND PREVAILING WIND DIRECTIONS THAT MAY AFFECT WATER LEVELS, VELOCITY AND MOVEMENT OF THE TURBIDITY CURTAIN.
- THE TURBIDITY CURTAIN SHALL BE REMOVED BY SLOWLY PULLING TOWARD THE SHORE TO MINIMIZE ESCAPE OF SEDIMENTS INTO THE WATERWAY.
- THE WEIGHTED ANCHOR SYSTEM SHALL BE A TYPE THAT ALLOWS THE CURTAIN TO CONFORM TO THE CONTOUR ON THE BOTTOM OF THE WATERWAY.
- PAYMENT FOR INSTALLATION AND REMOVAL OF THE TURBIDITY CURTAIN SHALL BE MADE UNDER THE GEOTEXTILE FOR FILTER CURTAIN ITEM.
- PAYMENT FOR MONITORING TURBIDITY CURTAIN SHALL BE MADE UNDER THE MONITORING EROSION PREVENTION & SEDIMENT CONTROL PLAN ITEM.
- PAYMENT FOR MAINTAINING TURBIDITY CURTAIN SHALL BE MADE UNDER THE MAINTENANCE OF EROSION PREVENTION & SEDIMENT CONTROL PLAN ITEM, UNLESS MAINTENANCE IS REQUIRED DUE TO POOR INSTALLATION PRACTICES.

PROJECT NAME: HUNTINGTON
PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...Cadd\Trans\z01j302frm2.dgn PLOT DATE: 1/12/2006
PROJECT LEADER: M. CHENETTE DRAWN BY: S. BURBANK
DESIGNED BY: S. BURBANK CHECKED BY: M. CHENETTE
NARRATIVE & EROS. CONTROL DETAILS SHEET 15 OF 63

SILT FENCE

APPLICATION NOTES:

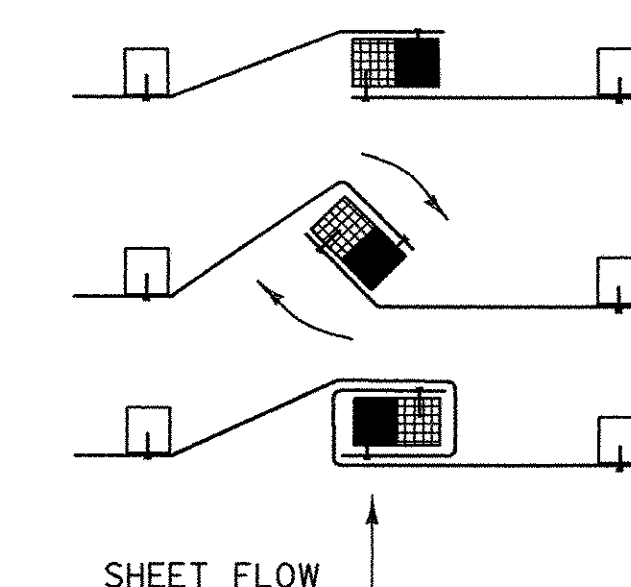
- THE PRIMARY PURPOSE OF SILT FENCE IS TO REDUCE RUNOFF VELOCITY AND TRAP SEDIMENT. VELOCITY IS REDUCED, WATER IS IMPOUNDED BEHIND THE MEASURE, AND SEDIMENT FALLS OUT OF SUSPENSION.
- SILT FENCE SHALL BE INSTALLED ON A LINE OF EQUAL ELEVATION (CONTOUR). IT MAY BE INSTALLED AT INTERMEDIATE POINTS UP SLOPES AS WELL AS AT THE BOTTOM, AS SHOWN IN THE DETAIL.
- SILT FENCE SHALL NOT BE USED ACROSS CONCENTRATED FLOW.

GENERAL NOTES:

- SILT FENCE SHALL GENERALLY BE PLACED A MINIMUM OF 5 FEET BEYOND TOE OF SLOPE, 10 FEET PREFERRED, TO PROVIDE ADEQUATE AREA FOR SEDIMENT STORAGE AND FACILITATE MAINTENANCE OF SEDIMENT CONTAINMENT AREA.
- ALL ENDS SHALL BE "J" HOOKED TO TRAP SEDIMENT.
- IN AREAS WITH TWO SLOPES, SILT FENCE SHALL BE USED TO ERECT A DAM AND TRAP SEDIMENT AT THE BASE OF THE STEEPER SLOPE.
- THE BOTTOM EDGE OF SILT FENCE SHALL BE BURIED A MINIMUM OF 6 INCHES BELOW GROUND, AND KEYED IN 4 INCHES. THE FENCE SHALL BE INSTALLED WITH THE POSTS ON THE DOWNSTREAM SIDE OF THE FABRIC.
- MAXIMUM DRAINAGE AREA TRIBUTARY TO 100 FEET OF SILT FENCE SHALL BE 0.25 ACRES.
- THE FOLLOWING ARE MAXIMUM SLOPE LENGTHS FOR THESE MEASURES:

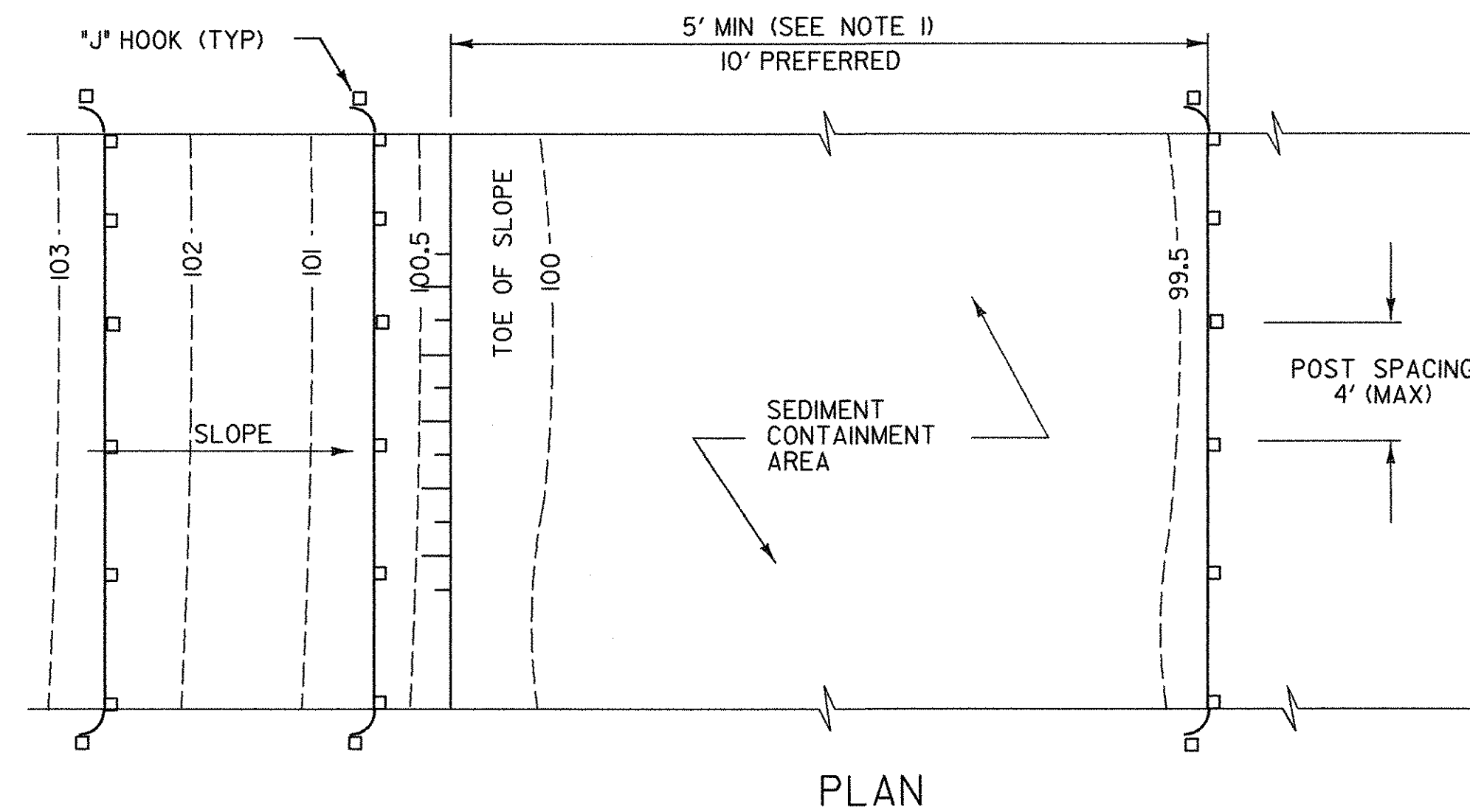
CONSTRUCTED SLOPE	SLOPE LENGTH (LS) FT	HORIZONTAL LENGTH (LH) FT
3 : 1	80	75
4 : 1	130	125
5 : 1	200	200
> 5 : 1	250	250

- MEASURES SHALL BE INSPECTED EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF A STORM EVENT GREAT ENOUGH TO CAUSE WATER TO LEAVE THE CONSTRUCTION SITE.
- MEASURES SHALL BE CLEANED AND REPAIRED AS NEEDED. SEDIMENT SHALL BE REMOVED WHEN ACCUMULATION REACHES ONE-HALF OF THE MEASURE HEIGHT. SEDIMENT SHALL BE DISPOSED OF AS UNSUITABLE MATERIAL.
- SILT FENCE SHALL BE REMOVED WHEN THE AREA HAS BEEN STABILIZED. AT TIME OF REMOVAL OF THE SILT FENCE, THE DISTURBED AREA SHALL BE REPAIRED AND STABILIZED.
- PAYMENT FOR INSTALLATION AND REMOVAL OF SILT FENCE SHALL BE MADE UNDER THE GEOTEXTILE FOR SILT FENCE ITEM.
- PAYMENT FOR MONITORING SILT FENCE SHALL BE MADE UNDER THE MONITORING EROSION PREVENTION & SEDIMENT CONTROL PLAN ITEM.
- PAYMENT FOR MAINTAINING SILT FENCE SHALL BE MADE UNDER THE MAINTENANCE OF EROSION PREVENTION & SEDIMENT CONTROL PLAN ITEM, UNLESS MAINTENANCE IS REQUIRED DUE TO POOR INSTALLATION PRACTICES.

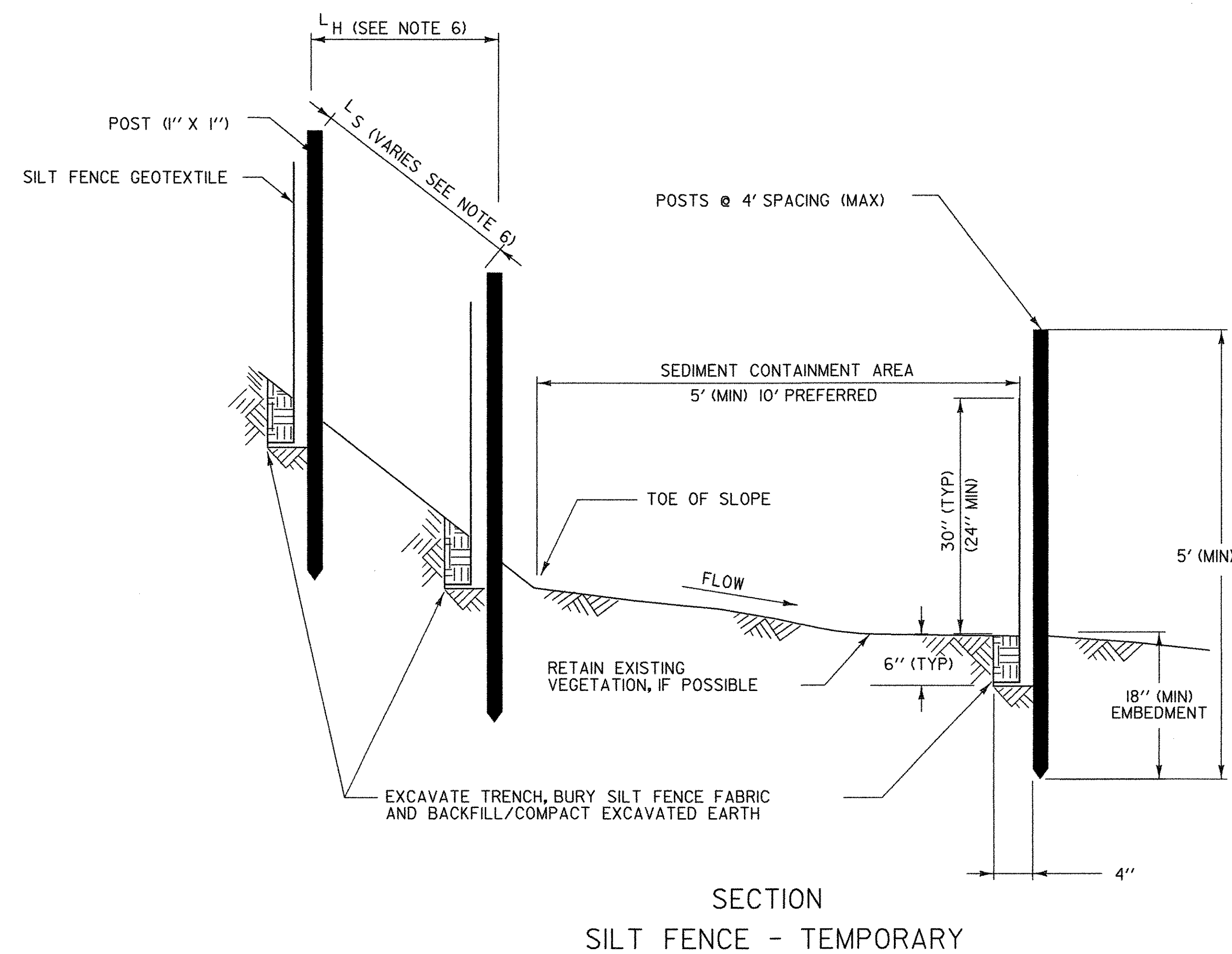


- PLACE THE END POST OF ONE FENCE INSIDE THE END POST OF THE OTHER FENCE.
- ROTATE BOTH POSTS AT LEAST 180 DEGREES IN A CLOCKWISE DIRECTION TO CREATE A TIGHT SEAL WITH THE FABRIC MATERIAL.
- DRIVE BOTH POSTS 18 INCHES INTO THE GROUND AND BURY THE FLAP IN THE TRENCH.

SPLICING DETAIL



PLAN



SECTION
SILT FENCE - TEMPORARY

PROJECT NAME: HUNTINGTON
PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...\\Cadd\Trans\z01j302frm2.dgn PLOT DATE: 1/12/2006
PROJECT LEADER: M. CHENETTE DRAWN BY: T. KNIGHT
DESIGNED BY: T. KNIGHT CHECKED BY: M. CHENETTE
EROSION CONTROL DETAILS SHEET 16 OF 63

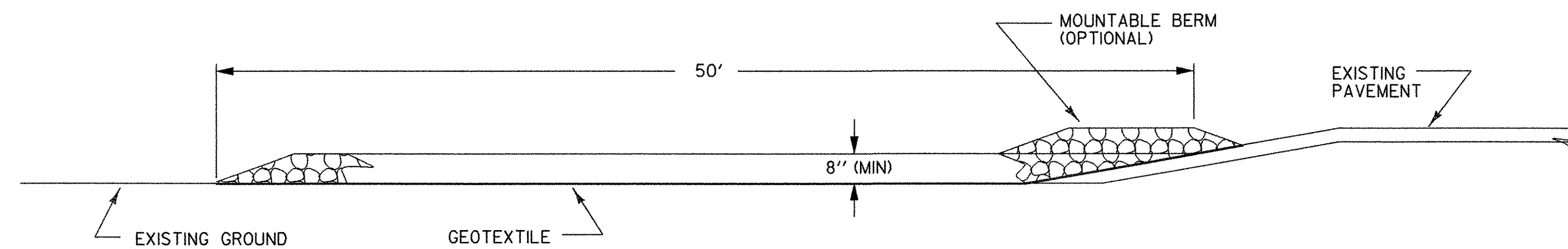
STABILIZED CONSTRUCTION ENTRANCE

APPLICATION NOTES:

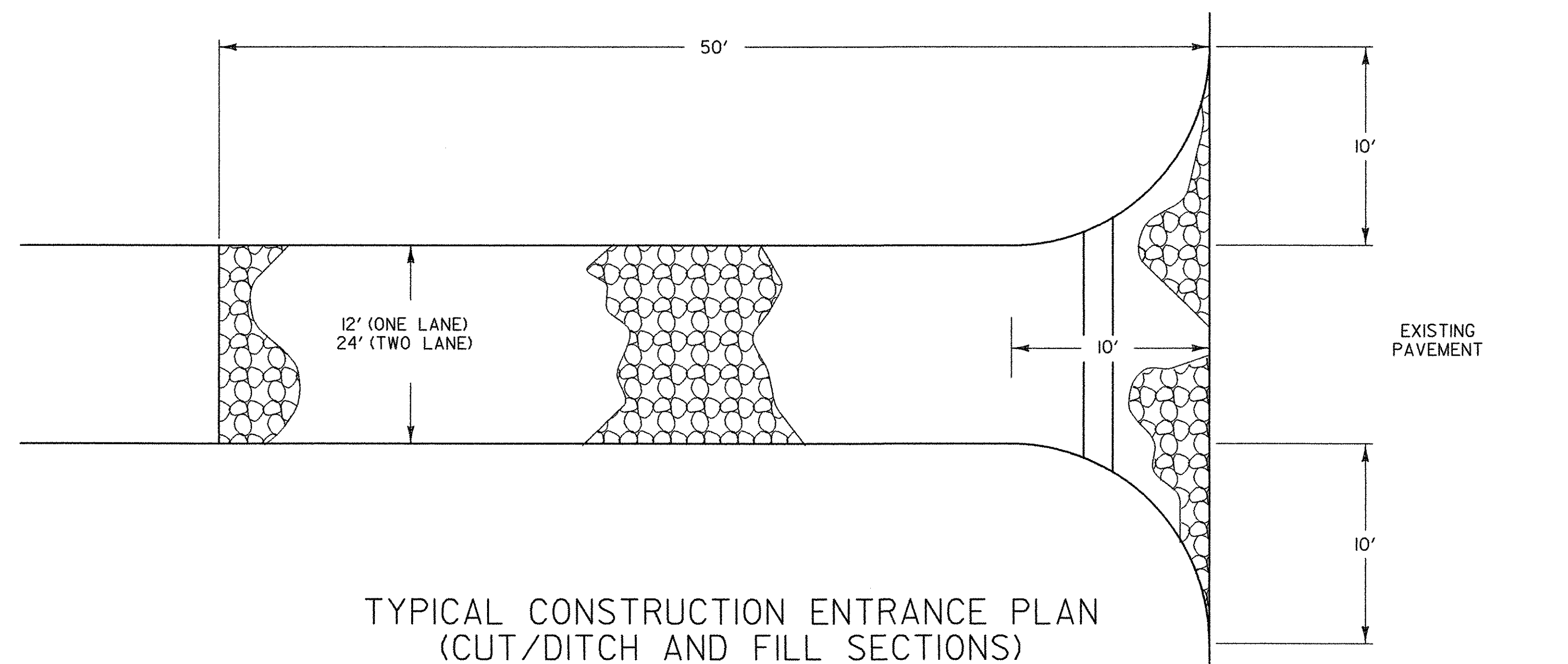
A. THE PURPOSE OF A STABILIZED CONSTRUCTION ENTRANCE IS TO REDUCE OR ELIMINATE THE TRACKING OF SEDIMENT ONTO PUBLIC RIGHTS OF WAY OR STREETS.

GENERAL NOTES:

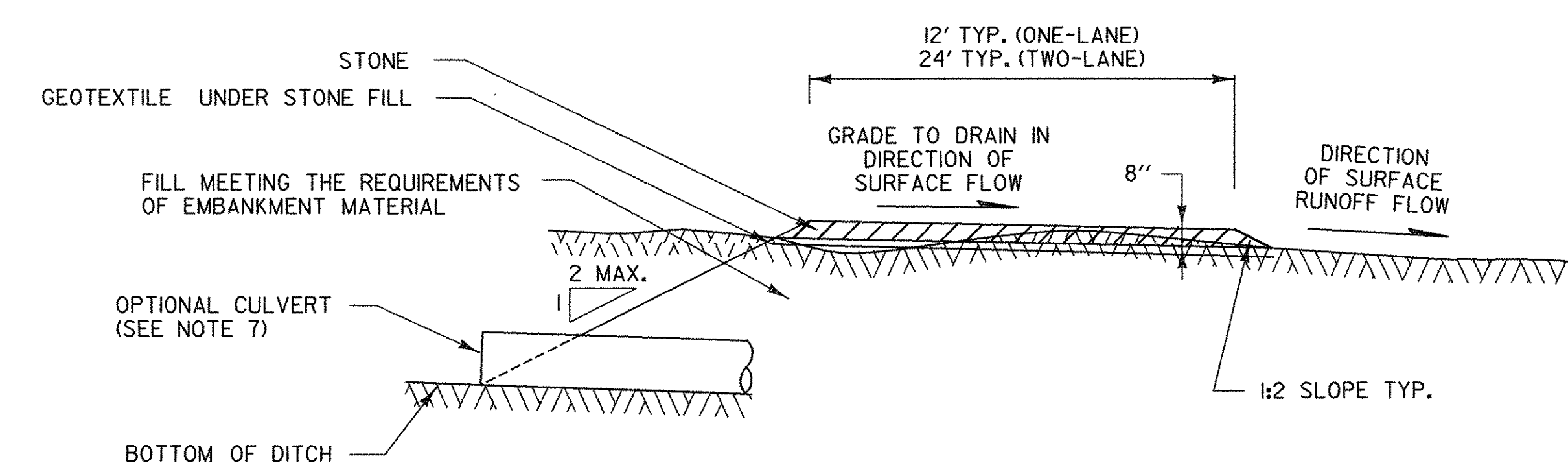
1. STONE SIZE - USE CLEAN STONE WITH GRADATION BETWEEN 2 INCHES AND 4 INCHES .
2. LENGTH - 50 FEET (MIN)
3. THICKNESS - 18 INCHES (MIN)
4. WIDTH - 12 FEET (MIN)
5. GEOTEXTILE UNDER STONE WILL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING OF STONE.
6. SURFACE WATER - ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE AS DIRECTED BY THE ENGINEER. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 5:1 SLOPES WILL BE PERMITTED.
7. PROPOSED DRAINAGE PIPES SHALL BE SIZED WITH SUFFICIENT CAPACITY TO CARRY DITCH FLOWS. ALTERNATIVE WAYS OF TRANSPORTING DITCH DRAINAGE ACROSS CONSTRUCTION ENTRANCES MAY BE PROPOSED BY THE CONTRACTOR FOR APPROVAL BY THE ENGINEER.
8. WHEN WASHING OF VEHICLE IS NECESSARY, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.
9. MEASURES SHALL BE INSPECTED EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF A STORM EVENT GREAT ENOUGH TO CAUSE WATER TO LEAVE THE CONSTRUCTION SITE.
10. MAINTENANCE- THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. ALL SEDIMENT SPILLED, DROPPED, WASHED, OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY.
11. AT THE TIME OF REMOVAL OF THE STABILIZED CONSTRUCTION ENTRANCE THE DISTURBED AREA SHALL BE REPAIRED AND STABILIZED.
12. PAYMENT FOR THE STABILIZED CONSTRUCTION ENTRANCE SHALL BE MADE UNDER STONE FILL, TYPE I (MOD. - CONSTRUCTION ENTRANCE).
13. PAYMENT FOR MONITORING STABILIZED CONSTRUCTION ENTRANCES SHALL BE MADE UNDER THE MONITORING EROSION PREVENTION & SEDIMENT CONTROL PLAN ITEM.
14. PAYMENT FOR MAINTAINING THE CONSTRUCTION ENTRANCE SHALL BE MADE UNDER THE MAINTENANCE OF EROSION PREVENTION & SEDIMENT CONTROL PLAN ITEM, UNLESS MAINTENANCE IS REQUIRED DUE TO POOR INSTALLATION PRACTICES.



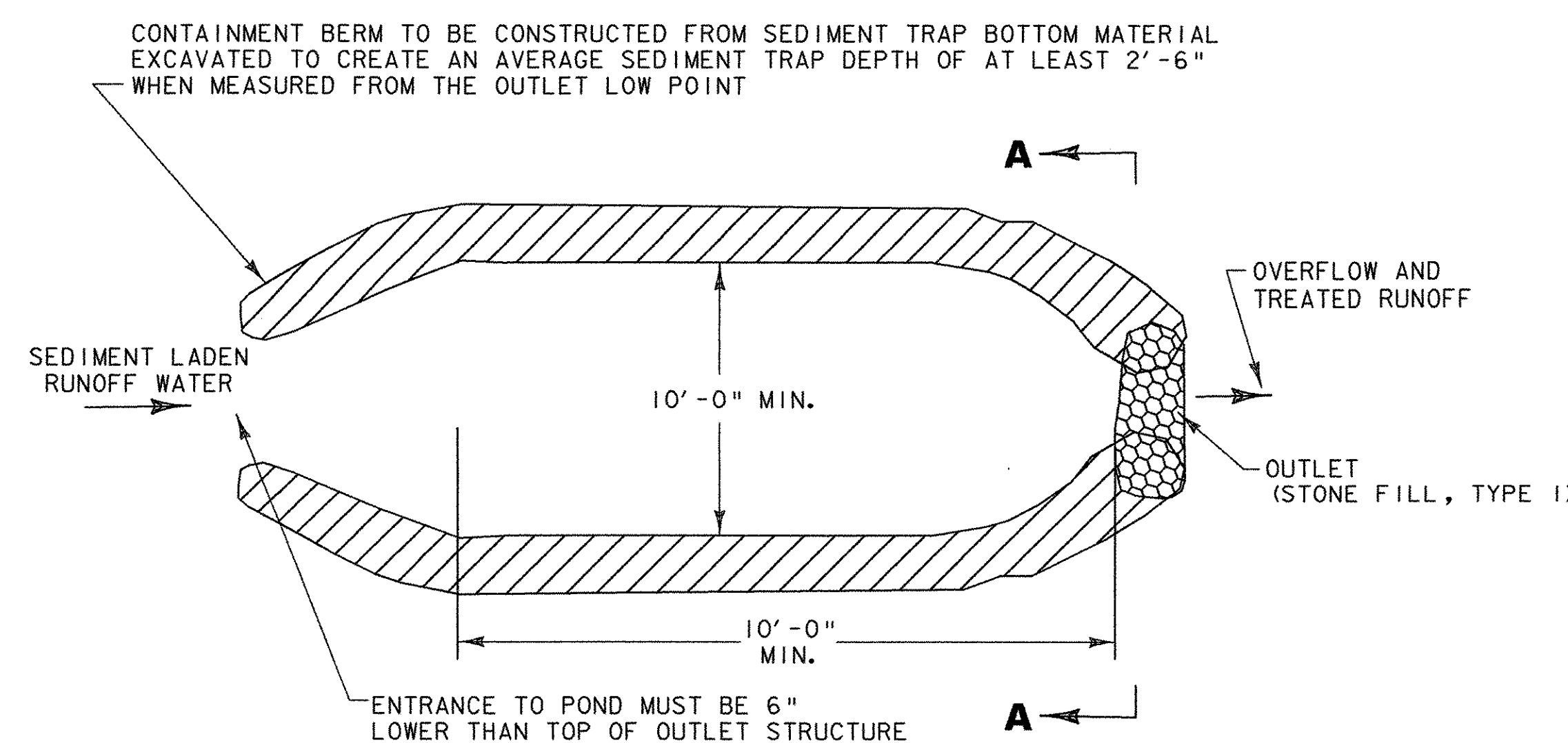
TYPICAL CONSTRUCTION ENTRANCE PROFILE
(CUT AND DITCH SECTIONS)



TYPICAL CONSTRUCTION ENTRANCE PLAN
(CUT/DITCH AND FILL SECTIONS)

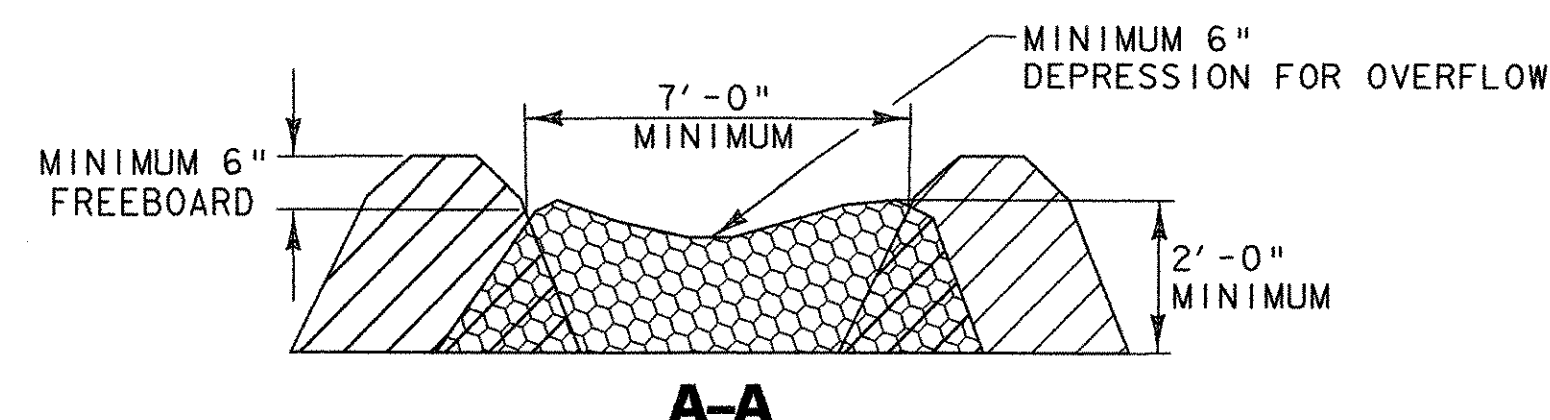


TYPICAL CONSTRUCTION ENTRANCE SECTION



PLAN VIEW

SINGLE CHAMBER TEMPORARY SEDIMENT TRAP



NOTES:

1. ONCE THE POND IS FULL, INFLOW RATES WILL EQUAL OUTFLOW RATES.
2. DISCHARGE MUST BE TO UNDISTURBED GROUND AND FLOW AWAY FROM THE CONSTRUCTION SITE.
3. SEDIMENT CAPTURED BY THE TEMPORARY SEDIMENT TRAP MUST BE REMOVED AND STABILIZED AS PER THE SPECIFICATIONS AND NOTES.

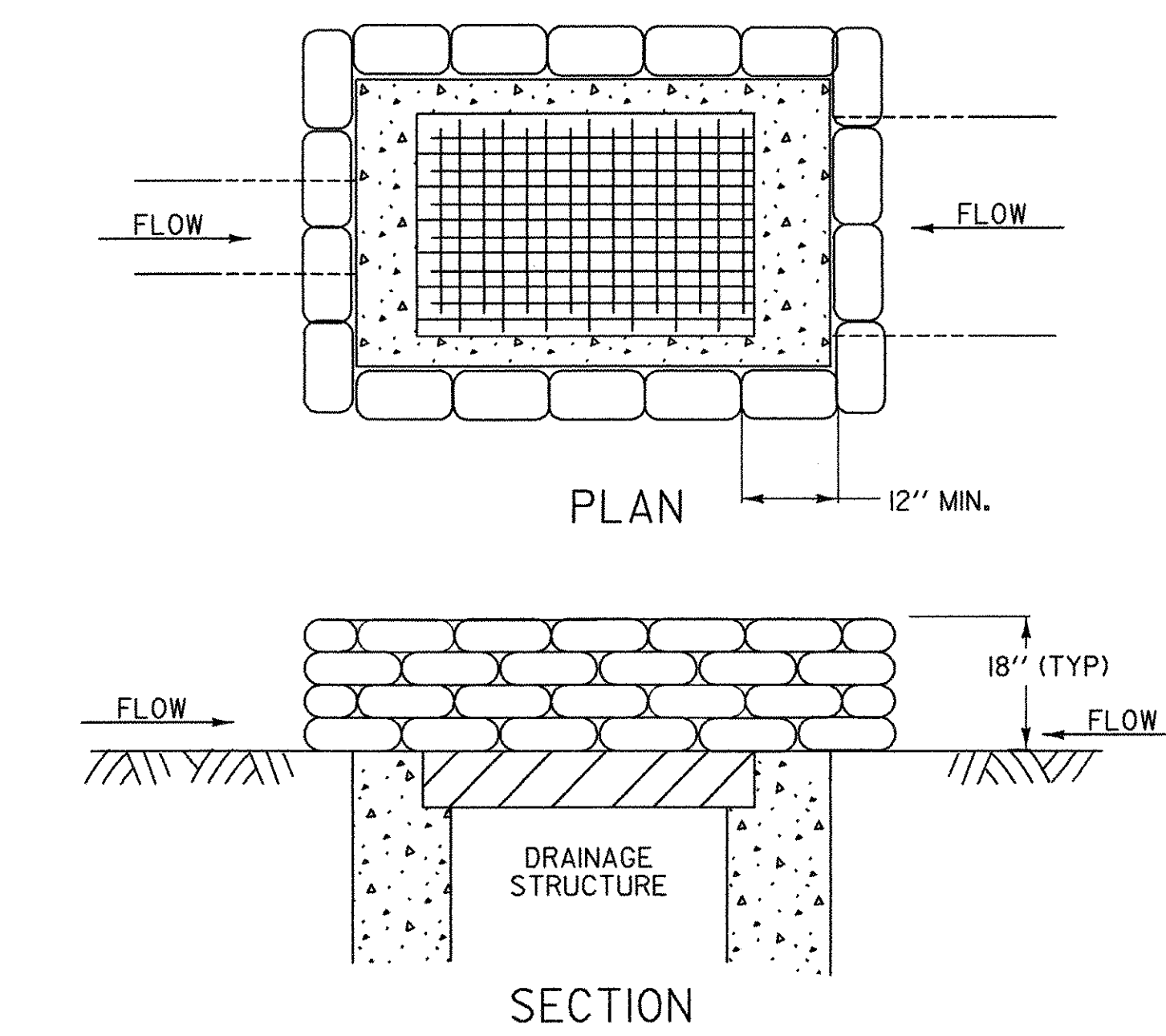
DROP INLET PROTECTION

APPLICATION NOTES:

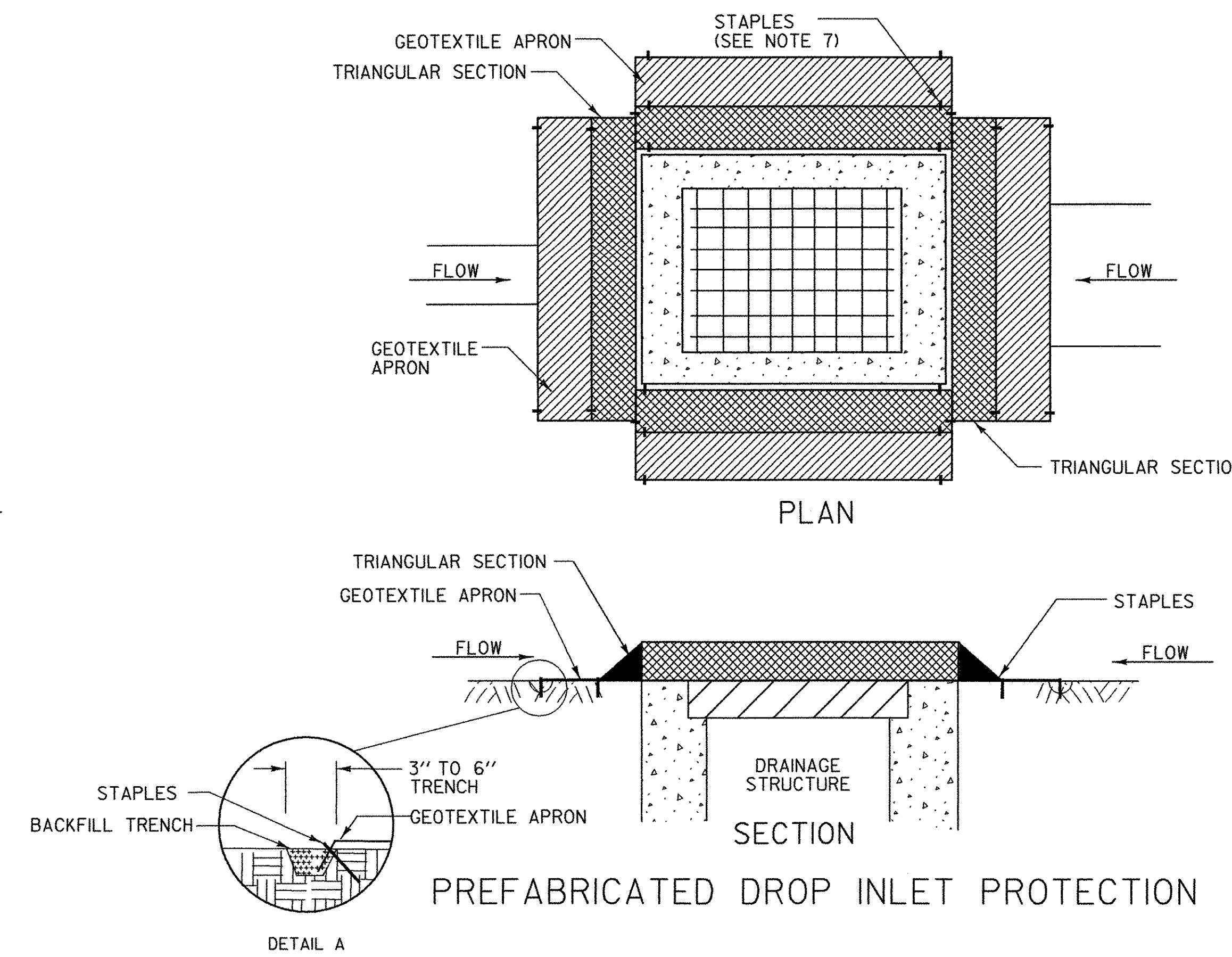
- A. THE PRIMARY PURPOSE OF DRAINAGE STRUCTURE INLET PROTECTION IS TO PREVENT SEDIMENT FROM ENTERING A DRAINAGE SYSTEM BY PONDING WATER WHICH ALLOWS SEDIMENT TO FALL OUT OF SUSPENSION.
- B. THESE EXAMPLES OF DROP INLET PROTECTION ARE NOT INTENDED FOR USE ON GRADES. ON GRADE THEY MAY CAUSE WATER TO BYPASS THE STRUCTURE, CREATING ADDITIONAL EROSION OR FLOODING.
- C. POSSIBLE MODIFICATIONS FOR USE ON GRADE INCLUDE ADDING A BERM DOWNSTREAM OF THE INLET TO CREATE PONDING. CHECK DAMS MAY ALSO BE USED UPSTREAM OF THE INLET TO SLOW VELOCITIES.
- D. PREFABRICATED DROP INLET PROTECTION SPECIFICATIONS SHALL BE PROVIDED TO THE ENGINEER FOR APPROVAL PRIOR TO USE.

GENERAL NOTES:

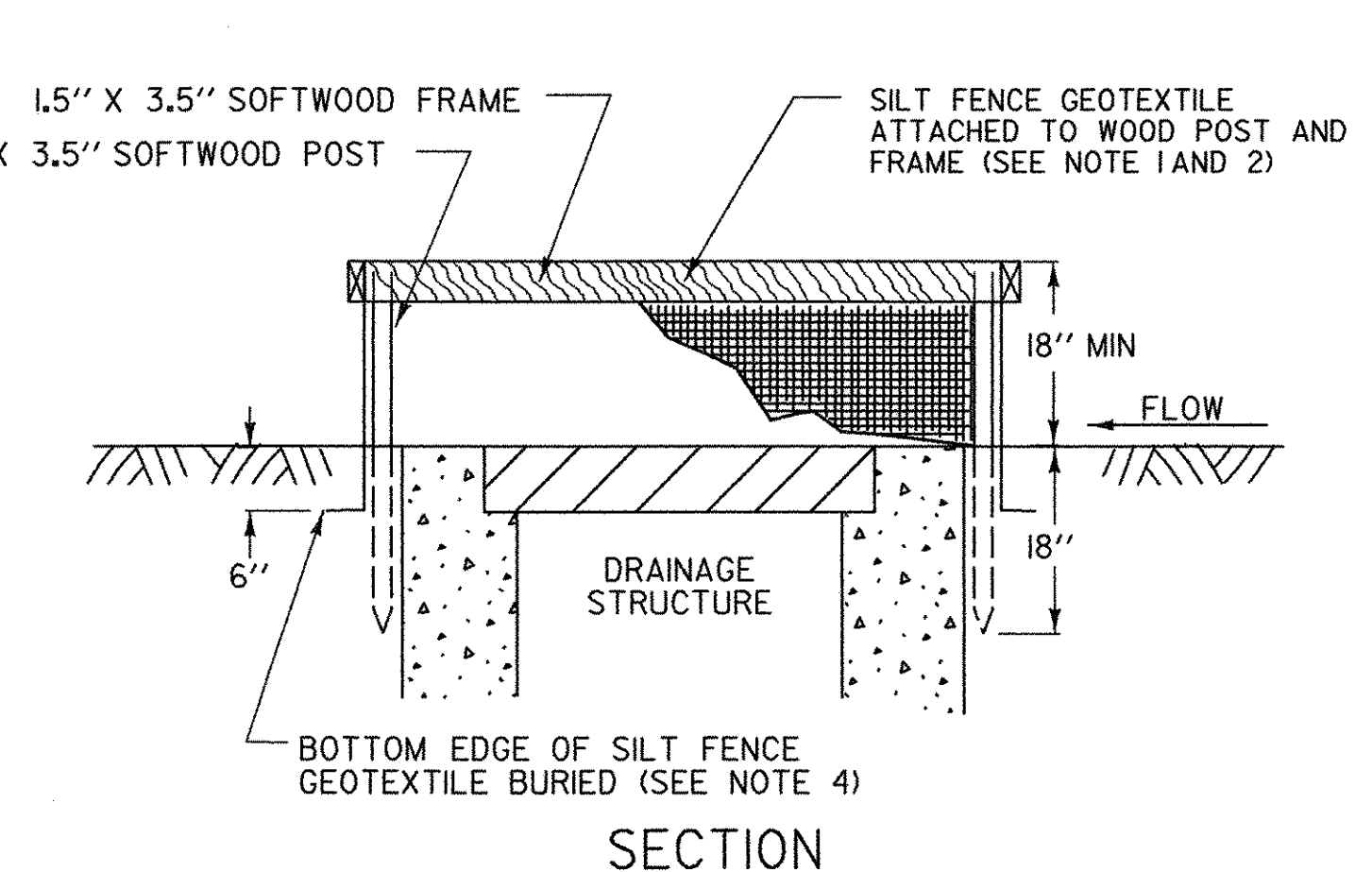
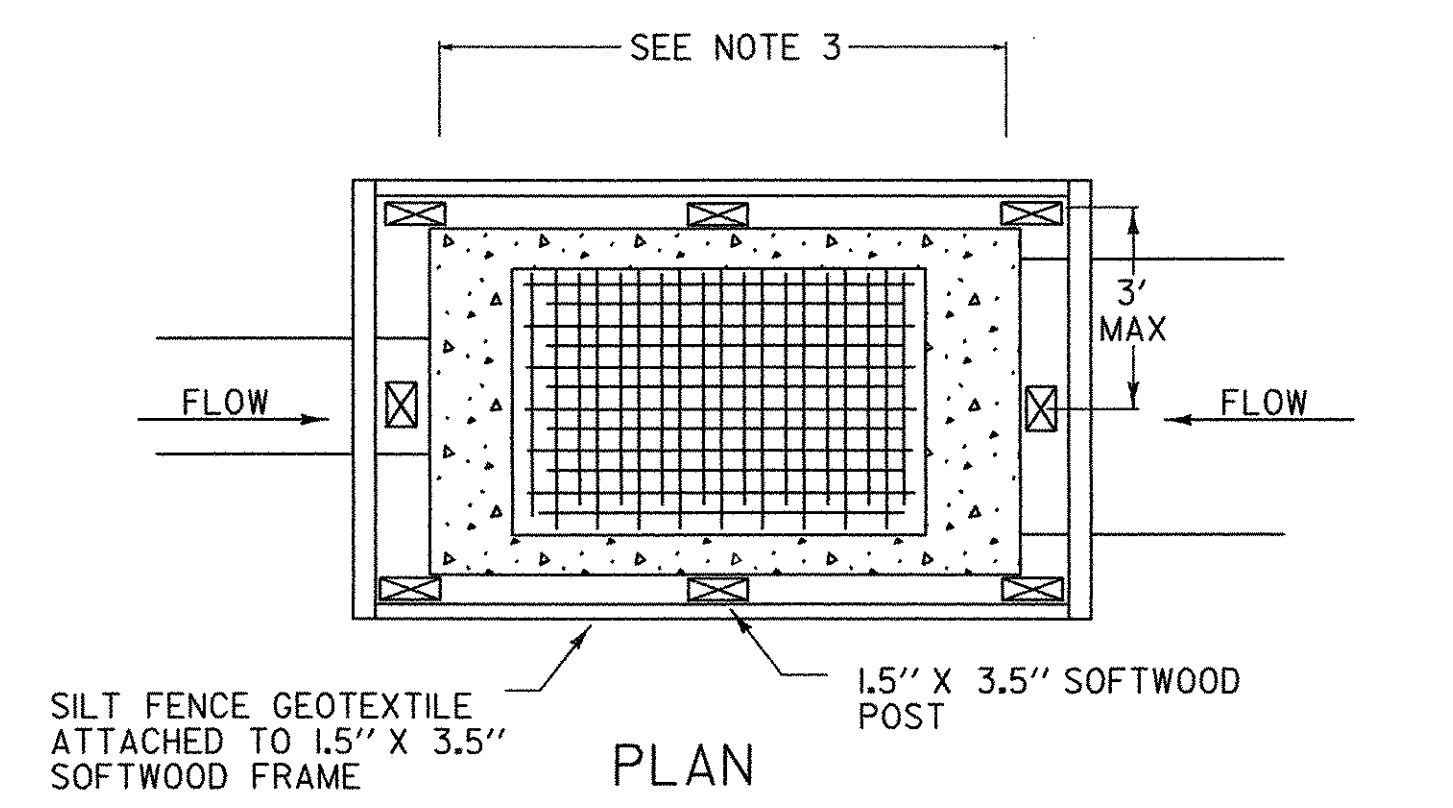
1. THE TOP OF THE INLET PROTECTION SHALL BE SET AT THE MAXIMUM DESIRED WATER LEVEL, BASED ON FIELD LOCATION AND CONDITIONS.
2. SILT FENCE GEOTEXTILE SHALL BE A SINGLE CONTINUOUS PIECE TO ELIMINATE JOINTS.
3. SPACE SILT FENCE POSTS EVENLY AROUND INLET WITH A MAXIMUM SPACING OF 3 FEET. DRIVE POSTS A MINIMUM OF 18 INCHES INTO GROUND. WIRE MESH MAY BE REQUIRED BEHIND GEOTEXTILE TO PROVIDE SUPPORT.
4. SILT FENCE GEOTEXTILE SHALL BE EMBEDDED A MINIMUM OF 6 INCHES AND BACKFILLED. GEOTEXTILE SHALL BE SECURELY FASTENED TO POSTS AND FRAME.
5. GRAVEL BAGS SHALL BE FILLED WITH CLEAN STONE, RATHER THAN SAND, TO PREVENT SEDIMENT FROM ENTERING A DRAINAGE SYSTEM IF BAGS ARE DAMAGED DURING USE.
6. GRAVEL BAGS SHALL BE INDIVIDUALLY TIED, DOUBLE BAGGED AND INVERSELY INSERTED. GRAVEL BAGS SHALL LAP THE JOINTS BETWEEN THE BAGS IN THE LAYER BELOW.
7. SECURE THE ENDS OF THE APRON FOR THE PREFABRICATED DRAINAGE STRUCTURE INLET PROTECTION WITH STAPLES AS DETAILED IN THE PLAN VIEW OR AS RECOMMENDED BY THE MANUFACTURERS LITERATURE.
8. MEASURES SHALL BE INSPECTED EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF A STORM EVENT GREAT ENOUGH TO CAUSE WATER TO LEAVE THE CONSTRUCTION SITE.
9. MEASURES SHALL BE CLEANED AND REPAIRED AS NEEDED, SEDIMENT SHALL BE REMOVED WHEN ACCUMULATION REACHES ONE-HALF OF THE MEASURE HEIGHT. SEDIMENT SHALL BE DISPOSED OF AS UNSUITABLE MATERIAL.
10. PAYMENT FOR INLET PROTECTION SHALL BE MADE UNDER STONE FILL, TYPE I (MOD. - INLET PROTECTION)
11. PAYMENT FOR MONITORING INLET PROTECTION SHALL BE MADE UNDER THE MONITORING EROSION PREVENTION & SEDIMENT CONTROL PLAN ITEM.
12. PAYMENT FOR MAINTAINING INLET PROTECTION SHALL BE MADE UNDER THE MAINTENANCE OF EROSION PREVENTION & SEDIMENT CONTROL PLAN ITEM, UNLESS MAINTENANCE IS REQUIRED DUE TO POOR INSTALLATION PRACTICES.



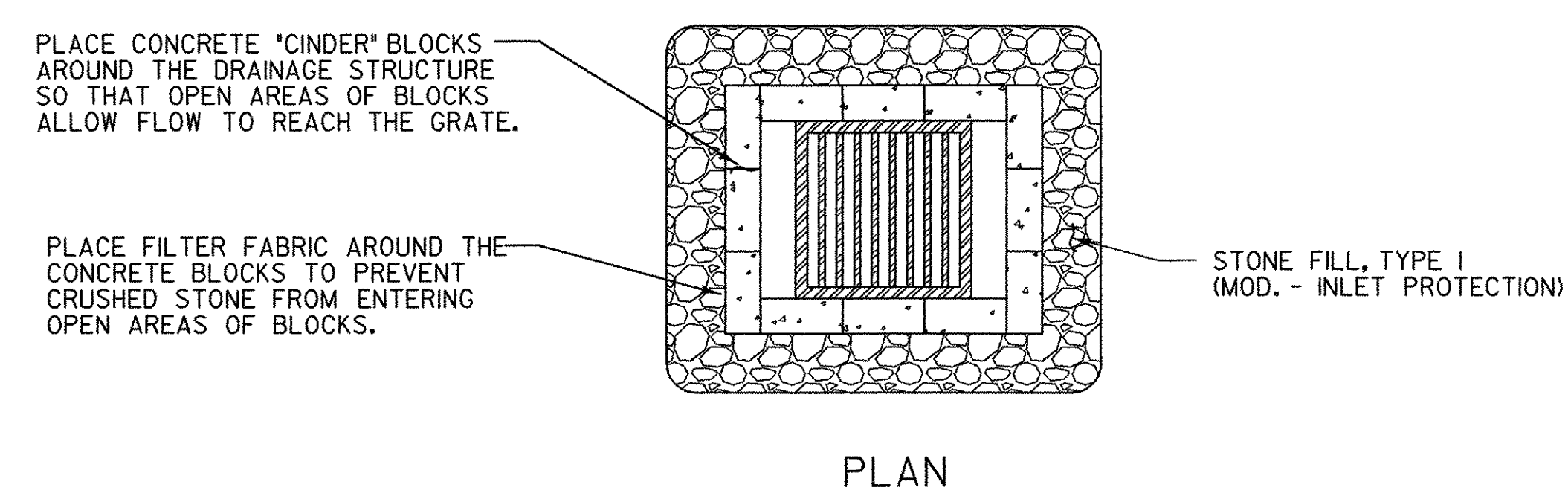
GRAVEL BAG DROP INLET PROTECTION



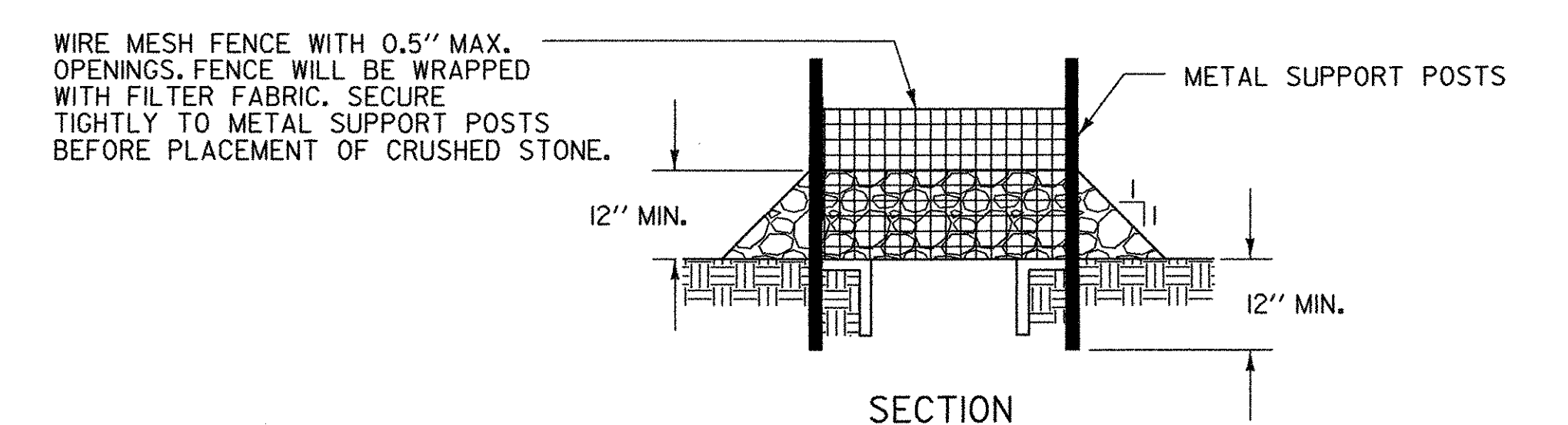
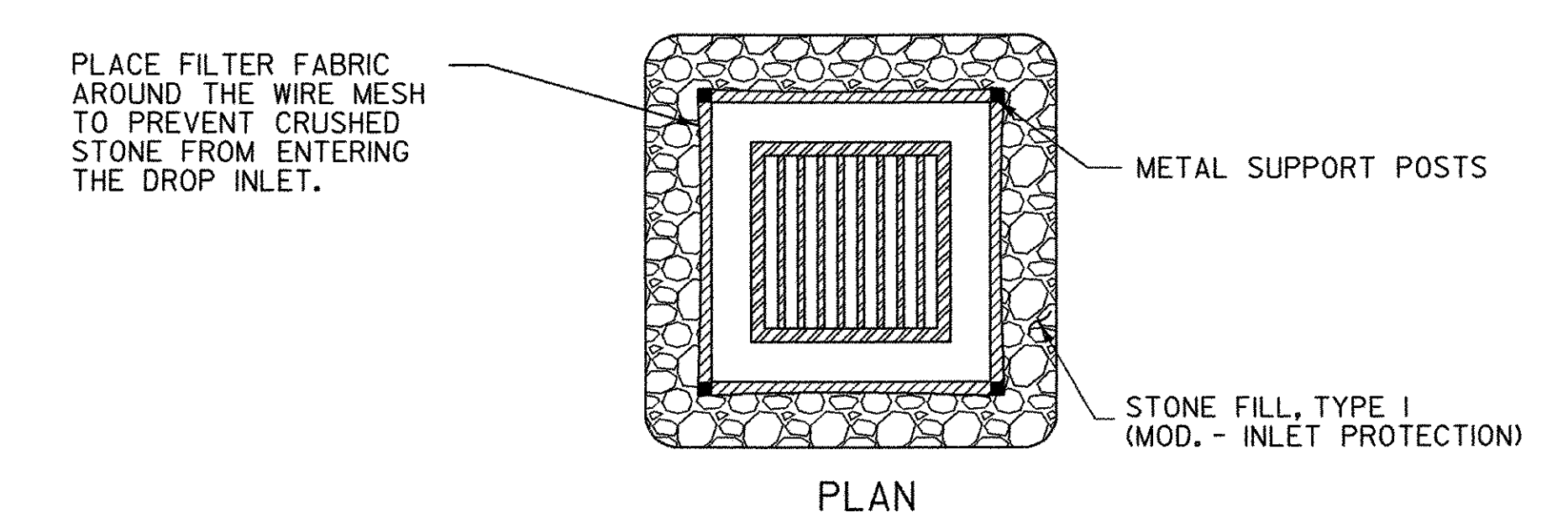
PREFABRICATED DROP INLET PROTECTION



SILT FENCE DROP INLET PROTECTION



ROCK BARRIER DROP INLET PROTECTION
TEMPORARY PAVED AREAS



ROCK BARRIER INLET PROTECTION
TEMPORARY UNPAVED AREAS



PROJECT NAME:	HUNTINGTON
PROJECT NUMBER:	BRO 1445 (29)
FILE NAME:	...Cadd\Trans\z01j302frm2.dgn
DATE:	1/12/2006
PROJECT LEADER:	M. CHENETTE
DRAWN BY:	T. KNIGHT
DESIGNED BY:	T. KNIGHT
CHECKED BY:	M. CHENETTE
EROSION CONTROL DETAILS	
SHEET 18 OF 63	

EROSION PREVENTION AND SEDIMENT CONTROL GENERAL NOTES

1. CONTRACTOR'S RESPONSIBILITIES FOR EROSION PREVENTION AND SEDIMENT CONTROL

- A. PREVENT OR MINIMIZE SOIL EROSION OF DISTURBED LAND AND PREVENT THE DISCHARGE OF SEDIMENT AND OTHER CONSTRUCTION RELATED POLLUTANTS TO WATERS OF THE STATE.
- B. FURNISH, INSTALL, INSPECT AND MAINTAIN EROSION PREVENTION AND SEDIMENT CONTROL MATERIALS IN CONJUNCTION WITH THE GENERAL CLEARING, GRADING AND EXCAVATION OF THIS SITE.
- C. ESTABLISH LIMITS OF SOIL DISTURBANCE; LOCATION(S) OF TOPSOIL STOCKPILES; CONSTRUCTION STAGING AREAS; STORAGE AREAS; REFUELING AND MAINTENANCE AREAS.
- D. ESTABLISH AND MARK BOUNDARIES FOR ANY UNDISTURBED RIPARIAN BUFFER ZONES AND MAINTAIN ALL EXISTING STREAMS AND RIPARIAN BUFFER ZONES IN THEIR NATURAL CONDITION.
- E. LOCATE AREAS FOR DISPOSAL OF STUMPS, EXCESS SOILS AND COLLECTED SEDIMENT AND OTHER POLLUTANTS, AND DISPOSE OF THESE MATERIALS IN A MANNER THAT WILL NOT RESULT IN SEDIMENTS AND POLLUTANTS ENTERING WATERS OF THE STATE.
- F. SEQUENCE CONSTRUCTION ACTIVITIES TO MINIMIZE THE EXTENT OF DISTURBED SOILS LEFT OPEN TO EROSION AT ANY GIVEN TIME AS DETAILED IN THE EROSION PREVENTION AND SEDIMENT CONTROL PLANS.
- G. AVOID ALL LAND DISTURBANCES WITHIN 50 FEET OF ALL WATER BODIES, MEASURED FROM THE TOP OF BANK, AND WETLANDS, EXCEPT WHERE NECESSARY FOR THE RECONSTRUCTION OF EXISTING ROADS AND THE CONSTRUCTION OF BRIDGES, STREAM CROSSINGS, AND COMPONENTS OF STORMWATER MANAGEMENT SYSTEMS WHICH BY NECESSITY MUST BE LOCATED IN THIS ZONE.
- H. MAINTAIN AND PRESERVE TO THE EXTENT POSSIBLE THE SITE'S NATURAL DRAINAGE WAYS THAT CONVEY STORMWATER TO STREAMS, RIVERS, LAKES, PONDS AND WETLANDS.
- I. PREVENT OFF-SITE STORMWATER FROM ENTERING AREAS OF DISTURBED SOIL ON-SITE.
- J. PREVENT THE OFF-SITE DISCHARGE OF SEDIMENT MOBILIZED ON THE CONSTRUCTION SITE, INCLUDING OFF-SITE TRACKING OF SEDIMENT ONTO PAVED PUBLIC OR PRIVATE ROADWAYS BY CONSTRUCTION VEHICLES. THIS SHALL BE ACCOMPLISHED BY THE USE OF VEHICLE TRACKING PADS, POWER BROOM SWEEPING ON PAVED AND CONCRETE SURFACES, AND OTHER MEANS AS DIRECTED BY THE RESIDENT ENGINEER.
- K. DISPOSE OF SEDIMENTS AND OTHER POLLUTANTS WHICH HAVE BEEN COLLECTED AND REMOVED IN THE COURSE OF STORMWATER TREATMENT IN A MANNER THAT WILL NOT RESULT IN THE SEDIMENTS AND POLLUTANTS ENTERING WATERS OF THE STATE. DISPOSAL SITES REQUIRE RELATIVELY LEVEL TERRAIN WITH AN ISOLATION DISTANCE OF AT LEAST 100 FEET FROM ANY SURFACE WATERS, INCLUDING WETLANDS.

2. LIMITATIONS AND PROHIBITIONS

- A. THE CONTRACTOR SHALL SCHEDULE EARTHWORK COMPLETION, SITE STABILIZATION, ESTABLISHMENT OF PERENNIAL COVER AND INSTALLATION OF NON-VEGETATIVE PROTECTION MEASURES NO LATER THAN OCTOBER 15. TO ASSURE ESTABLISHMENT OF VEGETATED COVER, SEEDING AND MULCHING ACTIVITIES SHALL BE COMPLETED BY SEPTEMBER 15.

FOR PROJECTS EXTENDING BEYOND OCTOBER 15, LIMIT EXPOSURE OF SOILS AND MINIMIZE ADDITIONAL EARTHWORKS. ANY PROPOSED SOIL DISTURBANCE AND EARTHWORKS BETWEEN OCTOBER 15 AND MAY 1 WILL REQUIRE DEVELOPMENT OF A SPECIAL WINTER EROSION AND SEDIMENT CONTROL PLAN ADDRESSING THE SPECIFIC CONCERNS OF WINTER CONSTRUCTION. THIS PLAN MUST BE FILED WITH, AND APPROVED BY, THE PERMITTING AUTHORITY BY SEPTEMBER 15. IF IT IS DETERMINED BY THE ENGINEER OR THE PERMITTING AUTHORITY THAT WINTER CONSTRUCTION WOULD PRESENT A SIGNIFICANT RISK TO WATER QUALITY, THE CONTRACTOR WILL NEED TO REQUEST A WINTER SHUTDOWN IN ACCORDANCE WITH THE PROVISIONS OF THE CONTRACT DOCUMENTS.
- B. DISCHARGES OF ANY MATERIAL OTHER THAN STORMWATER, SUCH AS VEHICLE AND EQUIPMENT MAINTENANCE SPILLS, FUELS, WASH WATER, CONSTRUCTION DEBRIS, OIL, WET CONCRETE AND OTHER SUBSTANCES, ARE PROHIBITED. WASHOUT WATER FROM CONCRETE BATCH TRUCKS OR EQUIPMENT USED TO MIX CONCRETE SHALL BE DISCHARGED IN DESIGNATED AREAS APPROVED BY THE ONSITE COORDINATOR AND THE RESIDENT ENGINEER.
- C. NO SILT FENCE SHALL BE UTILIZED IN AREAS OF CONCENTRATED FLOWS, SUCH AS CHANNELS OR DITCHES.
- D. DISPOSAL OF SEDIMENT IN A WETLAND OR ANY CORRECTIVE ACTION UNDERTAKEN TO REMOVE SEDIMENT FROM A WETLAND IS PROHIBITED.
- E. THE FAILURE TO PROMPTLY ABATE THE DISCHARGE OF SEDIMENT OR ANY OTHER WASTE WHICH CAUSES A VISIBLE DISCOLORATION OF SURFACE WATERS (INCLUDING WETLANDS), OR IS FOUND TO BE EXCEEDING WATER QUALITY STANDARDS BASED ON MONITORING, IS PROHIBITED.

3. GENERAL CONSTRUCTION NOTES

- A. SEE THE EROSION CONTROL PLAN FOR CONSTRUCTION NOTES.
- B. VEHICLE AND EQUIPMENT STORAGE AREAS OR AREAS ADJACENT TO CONSTRUCTION TRAILER OR OTHER HIGH TRAFFIC AREAS SHALL BE COVERED WITH GEOTEXTILE FABRIC AND 12 INCHES OF GRAVEL OR CRUSHED STONE AS DIRECTED BY THE RESIDENT ENGINEER. FOLLOWING COMPLETION OF CONSTRUCTION, ALL NON-NATIVE MATERIALS SHALL BE REMOVED FROM THE STAGING AREA. COMPACTED, RUTTED, OR OTHERWISE DISTURBED SOILS SHALL BE TILLED, RAKED, SEEDED AND MULCHED.
- C. ERODIBLE MATERIALS STOCKPILED WITHIN THE MATERIAL STORAGE AREAS SHALL BE ISOLATED WITH FILTER FABRIC. SOIL STOCKPILED ON THE SITE SHALL BE SEEDED AND MULCHED.
- D. ALL DISTURBED AREAS SHALL BE SEEDED AND MULCHED OR MATTED WITHIN 24 HOURS OF BEING STRIPPED OR BACKFILLED AND FINAL GRADED. AREAS WHICH ARE CONTINUOUSLY BEING DISTURBED SHALL BE TEMPORILY PROTECTED WITH MULCH OR EROSION MATTING, ESPECIALLY WHEN A RAIN EVENT IS EXPECTED.
- E. STOCKPILES SHALL BE MULCHED OR MATTED IF THEY WILL BE UNDISTURBED FOR MORE THAN 24 HOURS.

4. INSPECTION

- A. THE ONSITE COORDINATOR SHALL INSPECT ALL EROSION PREVENTION AND SEDIMENT CONTROL STRUCTURES AND MEASURES AT LEAST ONCE EVERY SEVEN (7) CALENDAR DAYS AND NO LATER THAN 24 HOURS AFTER ANY STORM EVENT WHICH GENERATES A DISCHARGE OF STORMWATER RUNOFF FROM THE CONSTRUCTION SITE, TO ENSURE THEY ARE OPERATING CORRECTLY.
- B. THE ONSITE COORDINATOR SHALL INSPECT ANY SITES THAT HAVE BEEN TEMPORARILY OR FINALLY STABILIZED A MINIMUM OF ONCE A MONTH.
- C. THE CONTRACTOR SHALL INSPECT CHANNEL LININGS, EMBANKMENTS AND CHANNEL BEDS DAILY FOR ANY SIGN OF EROSION.
- D. THE CONTRACTOR SHALL INSPECT DISCHARGE POINTS DAILY TO VISUALLY ASSESS WHETHER EROSION PREVENTION AND SEDIMENT CONTROL MEASURES ARE EFFECTIVE IN PREVENTING IMPACTS TO RECEIVING WATERS.
- E. IN THE CASE OF SOIL DISTURBANCE OR EARTHWORK OCCURRING OVER THE WINTER PERIOD (OCT. 15 TO MAY 1), DAILY MONITORING OF ALL EROSION PREVENTION, SEDIMENT CONTROL AND CONSTRUCTION ACTIVITIES SHALL BE REQUIRED IN AREAS WHERE SUCH SOIL DISTURBANCE, EARTHWORK OR ACTIVITIES ARE ONGOING. IN AREAS THAT HAVE BEEN SHUT DOWN FOR THE WINTER, THE ONSITE COORDINATOR SHALL INSPECT EROSION PREVENTION AND SEDIMENT CONTROL DEVICES IN THE FIELD MONTHLY, NO LATER THAN 24 HOURS AFTER ANY STORM EVENT WHICH GENERATES A DISCHARGE OF STORMWATER RUNOFF FROM THE CONSTRUCTION SITE, OR DURING A THAW. THE CONTRACTOR SHALL BE DIRECTED TO MAKE REPAIRS OR INSTALL ADDITIONAL MEASURES AS NECESSARY.
- F. THE ONSITE COORDINATOR AND THE CONTRACTOR SHALL INSPECT FOR THE EVIDENCE OF, OR THE POTENTIAL FOR, SEDIMENT LEAVING FROM ALL DISTURBED AREAS OR MATERIAL STORAGE AREAS.
- G. AN EROSION PREVENTION AND SEDIMENT CONTROL MONITORING REPORT FORM COMPLETED BY THE ONSITE COORDINATOR STATING THE DATE OF REVIEW AND DESCRIBING THE EROSION AND SEDIMENT CONTROL AND STORMWATER MANAGEMENT MEASURES REVIEWED, THE EFFECTIVENESS OF THEIR OPERATION, ANY DEFICIENCIES, AND CORRECTIVE ACTION TO BE UNDERTAKEN SHALL BE PREPARED AFTER EACH REVIEW. A COPY SHALL BE PROVIDED TO THE ENGINEER AND MAINTAINED ON FILE AT THE PROJECT SITE.

5. MAINTENANCE

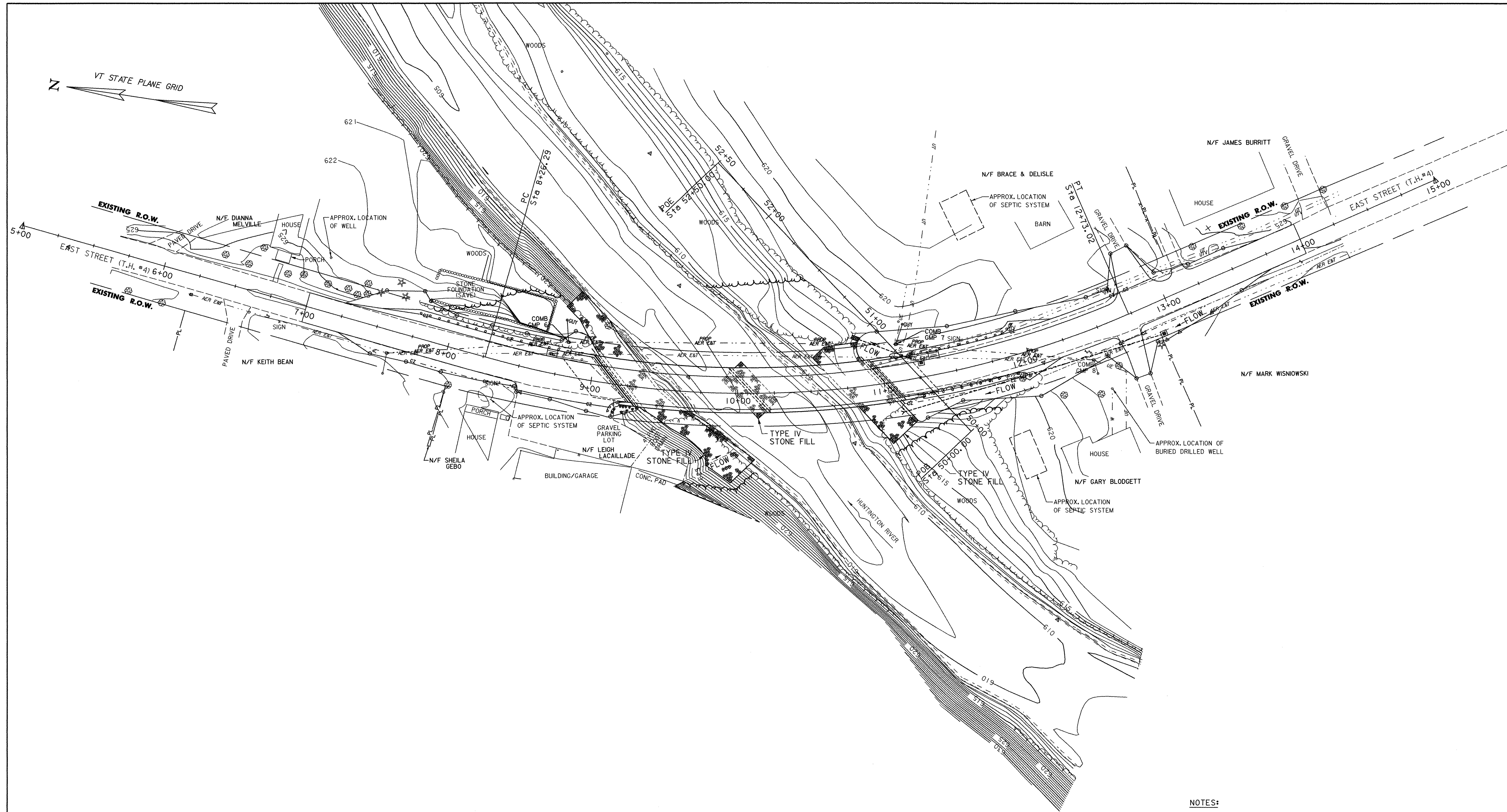
- A. THE CONTRACTOR SHALL KEEP ALL SEEDED AREAS WATERED AND IN GOOD CONDITION, RE-SEEDING IF AND WHEN NECESSARY UNTIL A GOOD, HEALTHY, UNIFORM GROWTH IS ESTABLISHED OVER THE ENTIRE AREA SEEDED.
- B. THE CONTRACTOR SHALL REPAIR ALL EROSION PREVENTION AND SEDIMENT CONTROL STRUCTURES AND MEASURES THAT ARE DETERMINED TO BE FAILING, OR NOT FUNCTIONING AS DESIGNED, WITHIN 24 HOURS OF INSPECTION.
- C. THE CONTRACTOR SHALL REMOVE ACCUMULATED SEDIMENT FROM CONTAINMENT SYSTEMS AND OTHER SEDIMENT CONTROL STRUCTURES AS REQUIRED, SUCH THAT PERFORMANCE OF THESE SYSTEMS IS NOT COMPROMISED OR IN ANY WAY IMPAIRED.
- D. THE CONTRACTOR SHALL REMOVE ALL DEBRIS AND REPAIR ALL DAMAGES CAUSED BY SOIL EROSION OR CONSTRUCTION EQUIPMENT AT OR BEFORE THE END OF EACH WORKING DAY.

6. CORRECTIVE ACTION

- A. THE CONTRACTOR SHALL NOTIFY THE RESIDENT ENGINEER AS SOON AS POSSIBLE, BUT WITHIN 24 HOURS, OF ANY EVIDENCE OF MEASURABLE AMOUNTS OF SEDIMENT OR SEDIMENT-LADEN WATER LEAVING THE CONSTRUCTION SITE OR ANY VISIBLE DISCOLORATION OF SURFACE WATERS (INCLUDING WETLANDS).
- B. THE CONTRACTOR SHALL TAKE IMMEDIATE ACTION TO CORRECT THE DISCHARGE, INCLUDING HALTING OR REDUCING CONSTRUCTION ACTIVITIES AS NECESSARY UNTIL THE DISCHARGE AND/OR THE CONDITION IS FULLY CORRECTED.

PROJECT NAME: HUNTINGTON
PROJECT NUMBER: BRO 1445 (29)

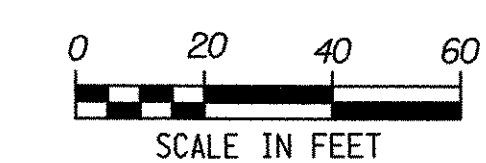
FILE NAME: ...\\Cadd\Trans\z01j302frm2.dgn PLOT DATE: 1/12/2006
PROJECT LEADER: M. CHENETTE DRAWN BY: J. OAKMAN
DESIGNED BY: D. ALTERI CHECKED BY: D. ALTERI
EROSION CONTROL NOTES SHEET 19 OF 63



NOTES:
 1. PLEASE REFER TO THE PROFILE (SHEET 12), ROADWAY AND CHANNEL CROSS SECTIONS (SHEETS 56-63) FOR FINAL GRADING.

SURVEYED BY: VERMONT SURVEY AND ENGINEERING, INC.
 SURVEYED DATE: 7/2001

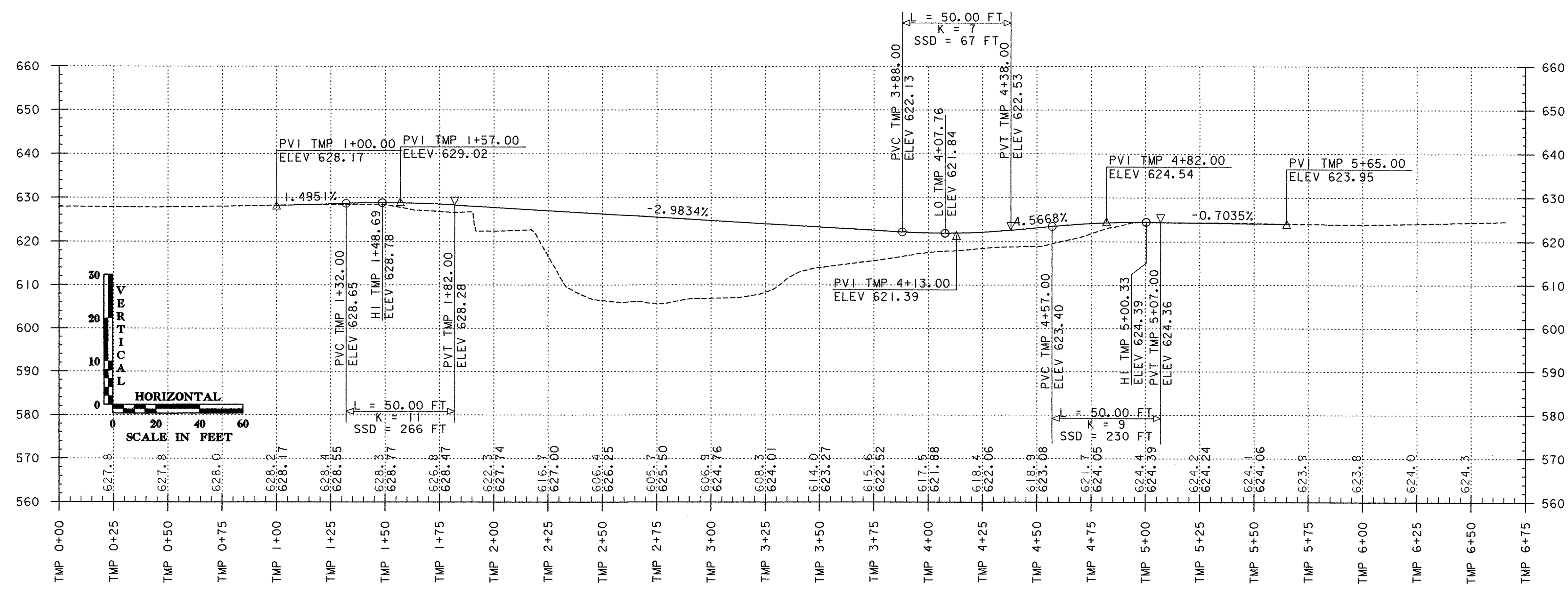
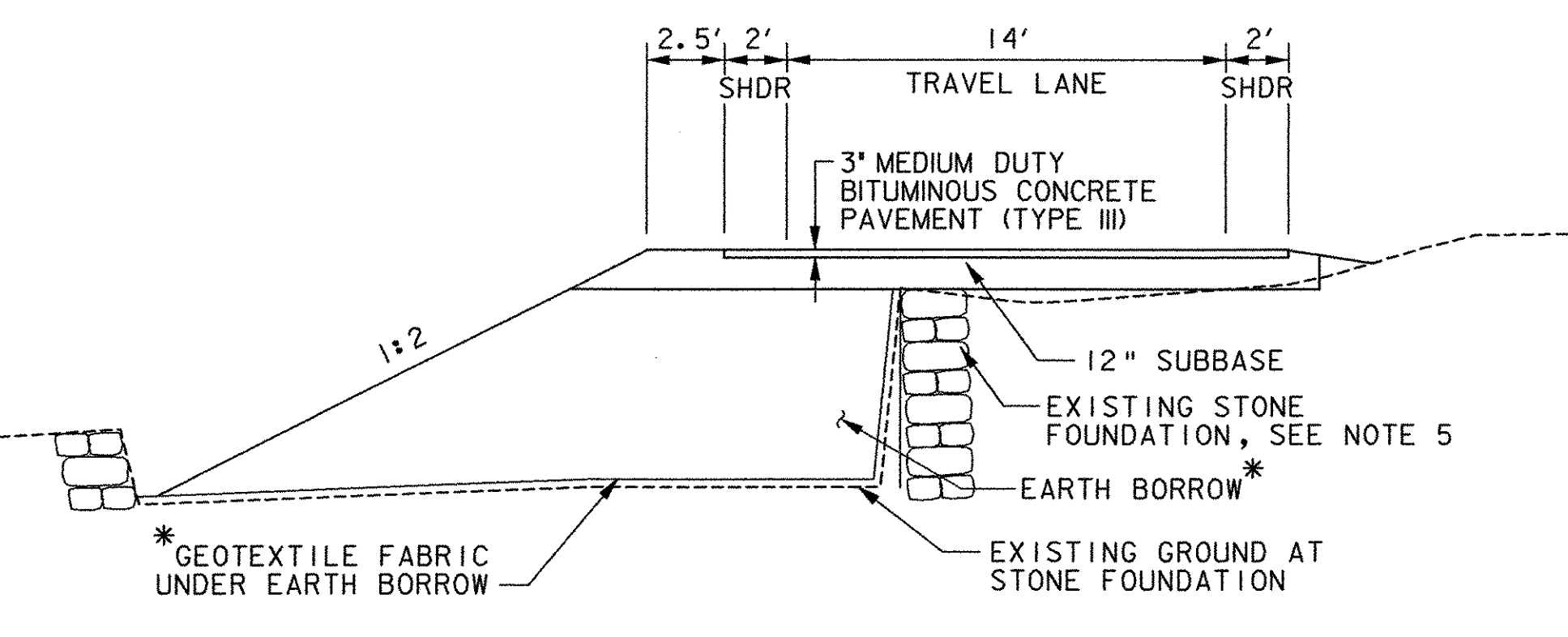
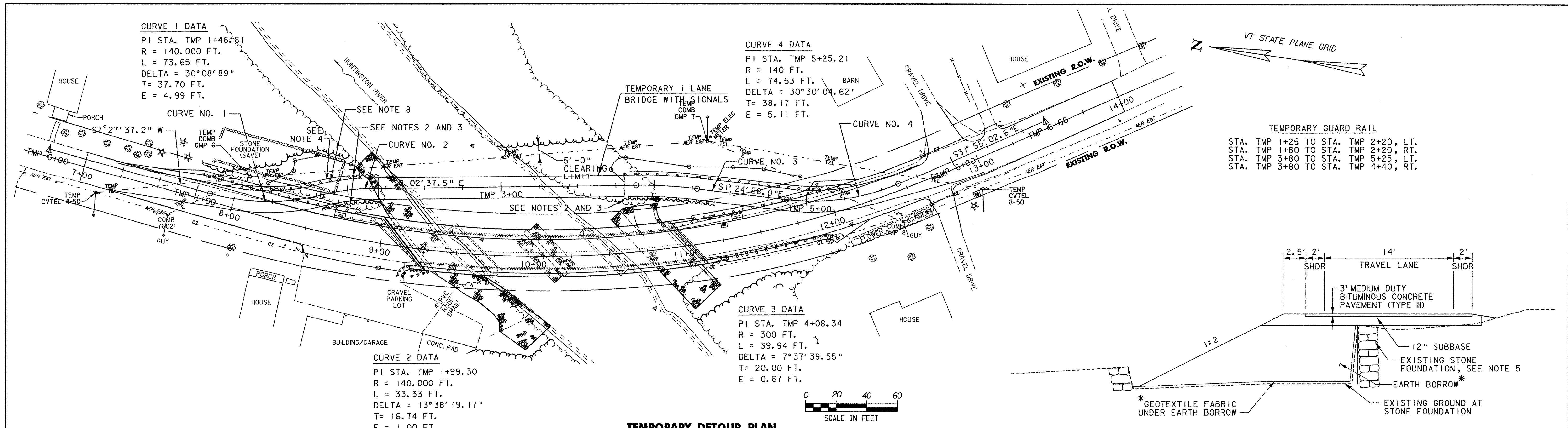
DATUM
 VERTICAL: NAVD88 (US FEET)
 HORIZONTAL: NAD83 (1996) SPC VT (US FEET)



PROJECT NAME: HUNTINGTON
 PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...\\Cadd\Trans\z01J302finalpln.dgn PLOT DATE: 1/30/2006
 PROJECT LEADER: M. CHENETTE DRAWN BY: S. BURBANK
 DESIGNED BY: D-H CHECKED BY: M. CHENETTE

FINAL CONDITIONS SITE PLAN SHEET 20 OF 63



- NOTES:**
- SEE TEMPORARY TRAFFIC SIGNAL PLAN FOR TEMPORARY DETOUR SIGNALING, STRIPING AND SIGNAL INFORMATION.
 - THE TEMPORARY RETAINING STRUCTURES SHALL BE DESIGNED AND FURNISHED AS PART OF ITEM 528.10, ONE-WAY TEMPORARY BRIDGE (MOD.).
 - THE TEMPORARY GUARD RAIL BETWEEN STATIONS TMP 1+80 TO TMP 2+20 RT. & TMP 2+00 TO TMP 2+20 LT. AND TMP 3+80 TO 4+40 RT. SHALL BE MOUNTED TO THE TEMPORARY RETAINING STRUCTURE FOR THE ABUTMENT IN A MANNER APPROVED BY THE ENGINEER.
 - THE CONTRACTOR MAY SPAN THE EXISTING STONE FOUNDATION AS PART OF THE TEMPORARY BRIDGE, IN LIEU OF FILLING IT IN AS SHOWN ABOVE.
 - THE CONTRACTOR SHALL PLACE AND REMOVE FILL IN THE AREA OF THE EXISTING STONE FOUNDATION WITH EXTREME CARE, TO AVOID DAMAGE TO THE STONE WALL. THE METHODS FOR PLACING AND REMOVING FILL SHALL BE APPROVED BY THE ENGINEER, PRIOR TO THE START OF WORK.
 - THE CONTRACTOR SHALL REMOVE ALL FILL FOR THE TEMPORARY DETOUR AND FABRIC FROM THE FOUNDATION AT THE CONCLUSION OF THE WORK AND RESTORE THE FOUNDATION AND GENERAL AREA TO THE ORIGINAL CONDITION, EXCLUDING LANDSCAPING.
 - THE CONTRACTOR MAY MAKE MINOR ADJUSTMENTS TO THE TEMPORARY DETOUR PROFILE, SUBJECT TO THE APPROVAL OF THE ENGINEER, PROVIDED THAT ALL WORK REMAINS WITHIN THE LIMITS SHOWN AND THE MINIMUM BEAM ELEVATIONS AND SPAN LENGTH ARE MAINTAINED.
 - GUARD RAIL POSTS FOR TEMPORARY GUARD RAIL BETWEEN STATIONS TMP 1+50 LT. TO TMP 2+00 LT. SHALL BE PLACED IN DRILLED OR HAND DUG HOLES. LOCATE POSTS TO AVOID CONFLICTS OR DAMAGE TO THE EXISTING STONE FOUNDATION.

PROJECT NAME: HUNTINGTON
 PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...\\Cadd\Trans\z01J302\sl.dgn
 PROJECT LEADER: M. CHENETTE
 DESIGNED BY: D. ALTERI

PLOT DATE: 1/30/2006
 DRAWN BY: J. OAKMAN
 CHECKED BY: D. ALTERI

TRAFFIC CONTROL PLAN
 SHEET 21 OF 63





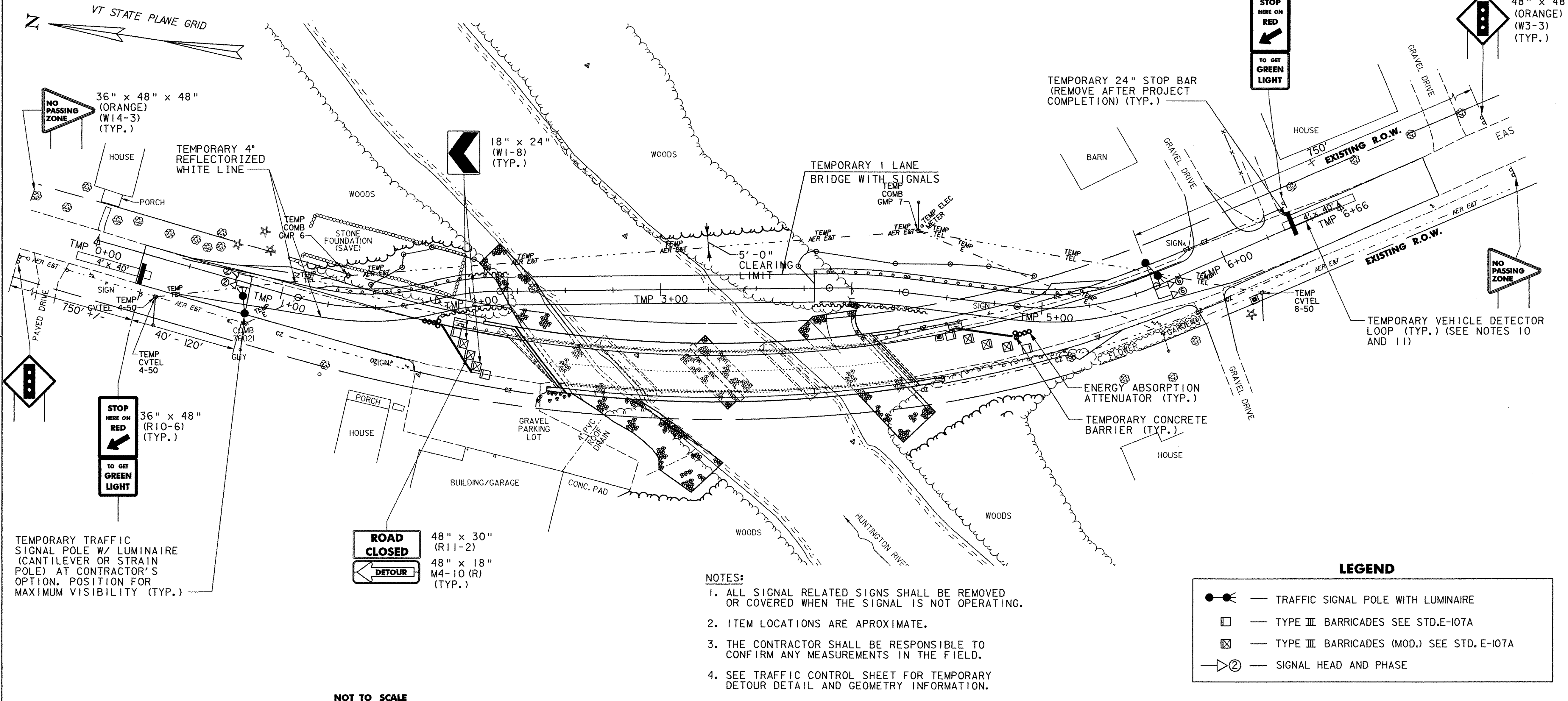
WHITE REFL. BACKGROUND MATERIALS: PER STD. E-142

PHASING DIAGRAM

PHASE	2			6		
	G	Y	R	G	Y	R
MINIMUM	8	4	25	8	4	25
EXTENSION	2			2		
MAXIMUM	12			12		
HEAD 2	G	Y	R	R	R	R
HEAD 6	R	R	R	G	Y	R

SPECIAL REQUIREMENTS

APPROACH	TEMPORARY VEHICLE DETECTOR	FLASHING BEACON ON ADVANCED WARNING SIGN
2	✓	
6	✓	



- NOTES:**
1. ALL SIGNAL RELATED SIGNS SHALL BE REMOVED OR COVERED WHEN THE SIGNAL IS NOT OPERATING.
 2. ITEM LOCATIONS ARE APPROXIMATE.
 3. THE CONTRACTOR SHALL BE RESPONSIBLE TO CONFIRM ANY MEASUREMENTS IN THE FIELD.
 4. SEE TRAFFIC CONTROL SHEET FOR TEMPORARY DETOUR DETAIL AND GEOMETRY INFORMATION.

LEGEND

	TRAFFIC SIGNAL POLE WITH LUMINAIRE
	TYPE III BARRICADES SEE STD. E-107A
	TYPE III BARRICADES (MOD.) SEE STD. E-107A
	SIGNAL HEAD AND PHASE

GENERAL TEMPORARY TRAFFIC SIGNAL NOTES

1. DESIGN OF THE SIGNAL SUPPORT(S) AND ANY REQUIRED GUYING IS THE RESPONSIBILITY OF THE CONTRACTOR.
2. SIGNAL TIMING/TIMING ADJUSTMENTS REQUESTED BY THE RESIDENT ENGINEER SHALL BE ACCOMPLISHED WITHIN A 48 HOUR PERIOD AND PAYMENT SHALL BE INCIDENTAL TO THE TRAFFIC SIGNAL ITEM. THE ALL-RED CLEARANCE INTERVAL IS BASED ON AN ASSUMED SPEED OF 10-20 MPH, THE RESIDENT ENGINEER SHALL MAKE SEVERAL TRIAL RUNS TO DETERMINE THE PROPER ALL-RED CLEARANCE INTERVAL.
3. SIGNAL FACES SHALL CONSIST OF 12" LENSES. (RED, YELLOW, AND GREEN)
4. THE BOTTOM OF THE HOUSING OF A SIGNAL FACE SUSPENDED OVER A ROADWAY SHALL NOT BE LESS THAN 16 1/2 FEET NOR MORE THAN 19 FEET ABOVE THE PAVEMENT GRADE AT THE CENTER OF THE ROADWAY. THE BOTTOM OF A SIGNAL FACE NOT MOUNTED OVER A ROADWAY, SHALL NOT BE LESS THAN 8 FEET NOR MORE THAN 15 FEET ABOVE THE GROUND. CAUTION SHOULD BE USED TO INSURE COMPLIANCE WITH THE HEIGHT REQUIREMENTS IN THE EVENT THE NEW APPROACH GRADES DIFFER SIGNIFICANTLY FROM THE OLD ROAD GRADE.
5. SIGNAL FACES FOR ANY ONE APPROACH SHALL NOT BE LESS THAN 8 FEET APART MEASURED HORIZONTALLY BETWEEN CENTER OF FACES.
6. SIGNAL HEADS MAY BE HUNG ON A SPAN WIRE OR ON A CANTILEVER MAST ARM. AT LEAST ONE SIGNAL HEAD SHALL BE UNMISTAKABLY IN LINE WITH THE CENTER OF APPROACHING TRAFFIC AT ALL TIMES. THE SECOND SIGNAL HEAD MAY BE POST MOUNTED, LOCATED AT A DISTANCE NO GREATER THAN 14 1/2 FEET FROM THE CENTER OF THE APPROACH LANE WHEN THE STOP BAR IS 40 FEET FROM THE SIGNAL HEAD. CONSULT THE M.U.T.C.D. 2000 MILLENNIUM EDITION FOR ADDITIONAL INFORMATION CONCERNING SIGNAL PLACEMENT.
7. SIGNAL HEAD PLACEMENT IS CRITICAL. HEADS SHALL BE ADJUSTED TO REFLECT LANE LOCATION CHANGES.
8. THE SIGNAL SYSTEM SHALL CONSIST OF POLES, SIGNS AND POSTS, WARNING SIGN, LUMINARIES, FLASHING BEACONS, AND SIGNAL EQUIPMENT TO PROVIDE FOR AN ADEQUATE DESIGN. IT ALSO INCLUDES PERMITS AND COST ASSOCIATED WITH PROVIDING ELECTRICAL POWER.
9. THE CONTRACTOR SHALL PROVIDE AN ACTUATED CONTROLLER. THE APPROACHES NOTED SHALL HAVE A TEMPORARY VEHICLE DETECTOR. THE TYPE OF DETECTION SHALL BE AT THE OPTION OF THE CONTRACTOR. LOOPS ARE SHOWN FOR PLACEMENT PURPOSES ONLY. THE CONTROLLER, DETECTOR AND ALL OTHER SIGNAL EQUIPMENT SHALL MEET OR EXCEED ALL NEMA STANDARDS.
10. WHEN USED, VEHICLE DETECTOR LOOPS SHALL BE 4' X 40' FOR PRESENCE DETECTION AT THE STOP BAR WITH THE NEAR PORTION LOCATED 5 FEET BEYOND THE STOP BAR. VEHICLE DETECTOR LOOPS SHALL BE CAPABLE OF DETECTING MOTORCYCLES.
11. INTERVAL TIMING SHOWN IN SECONDS.
12. INSTALL WIRING BETWEEN SIGNAL POLES BY WHATEVER MEANS POSSIBLE OR CONVENIENT TO PROVIDE FOR A SAFE INSTALLATION. ATTACHMENT TO UTILITY POLES TO BE COORDINATED BY THE CONTRACTOR WITH UTILITY COMPANY.
13. PLACE TEMPORARY POLES BEHIND GUARDRAIL WHERE POSSIBLE.
14. POLES SUPPORTING SPAN WIRES AND/OR MAST ARMS SHALL BE ADEQUATELY BRACED OR GUYED AND SHALL NOT BE PLACED SO AS TO CREATE A HAZARD TO THE TRAVELLING PUBLIC.
15. ALL TEMPORARY SIGNAL EQUIPMENT, SIGNS, TYPE III BARRICADES, ETC., SHALL BELONG TO THE CONTRACTOR AT THE END OF THE PROJECT AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR THEIR REMOVAL, INCLUDING ANY TEMPORARY PAVEMENT MARKINGS, UTILITY POLES, WIRES, ETC.
16. A 250 WATT MER/150 WATT HPS LUMINAIRE AND MAST ARM SHALL BE PROVIDED ON A POLE ON EACH APPROACH AT A MOUNTING HEIGHT OF 30' ABOVE ROADWAY CENTERLINE. THE INTENT IS TO LIGHT UP THE AREA AROUND THE SIGNAL HEADS AND STOP BAR FOR INCREASED VISIBILITY. THE RESIDENT ENGINEER SHALL DETERMINE THE ADEQUACY OF THE LIGHTING AND DIRECT CHANGES IF THE LIGHTING IS INSUFFICIENT.
17. STOP BARS SHALL BE LOCATED A MINIMUM OF 40' AND A MAXIMUM OF 120' FROM THE NEAREST SIGNAL HEAD.
18. PAYMENT FOR THE VEHICLE DETECTORS SHALL BE FOR EACH UNIT INSTALLED.
19. TYPE III BARRICADES, REFLECTORIZED PLASTIC DRUMS, SIGNS AND POSTS AS SHOWN ON THIS SHEET AND NOTED BELOW ARE INCIDENTAL TO THE TRAFFIC CONTROL SIGNAL ITEMS ('STOP HERE ON RED', 'SIGNAL AHEAD', 'NO PASSING ZONE', 'NO TURN ON RED', AND 'TO GET GREEN LIGHT', ETC.). THE TEMPORARY STOP BARS WILL BE PAID FOR UNDER THE TEMPORARY 24' STOP BAR ITEM.
20. THE 'NO PASSING' SIGN SHALL BE USED TO PREVENT PASSING FOR 750' IN ADVANCE OF THE STOP BAR. THE SIGNAL SHALL BE PER STANDARD E-102.
21. SEE STD. E-140 FOR 'STOP HERE ON RED' SIGN DETAIL, E-101 FOR 'SIGNAL AHEAD' SYMBOL SIGN AND CHEVRON SYMBOL SIGN. SEE STANDARD E-102A FOR 'ROAD CLOSED' SIGN. SEE STANDARD E-121 FOR SIGN PLACEMENT. SEE STANDARD E-171A AND E-172 FOR ADDITIONAL INFORMATION ON SIGNALS AND DETECTORS.
22. ALL ELECTRICAL WORK SHALL MEET THE REQUIREMENTS OF THE NATIONAL ELECTRICAL CODE AND STATE INSPECTOR.
23. ALL STOP SIGNS AND ANY TRAFFIC SIGNS MADE IRRELEVANT DUE TO THE TEMPORARY SIGNAL SHALL BE COVERED WITH A NON TRANSLUCENT WEATHER RESISTANT MATERIAL. THESE SIGNS SHOULD ONLY BE COVERED DURING THE OPERATION OF THE TEMPORARY SIGNAL OR AT THE DISCRETION OF THE ENGINEER. THE COSTS OF COVERING AND UNCOVERING THESE SIGNS SHALL BE INCLUDED IN THE TRAFFIC CONTROL SIGNAL ITEM.
24. CONSTRUCTION APPROACH SIGNS SHALL BE PROVIDED ON EACH APPROACH PER STANDARD E-107. ADDITIONAL CONSTRUCTION APPROACH SIGNS SHALL BE INSTALLED AS REQUIRED BY THE RESIDENT ENGINEER PER STANDARD E-100, E-101, E-102 & E-102A. PAYMENT FOR THESE SIGNS SHALL BE PAID FOR AS APART OF THE 'TRAFFIC CONTROL' ITEM.
25. PAYMENT FOR THE TEMPORARY BARRIER USED SHALL BE MADE UNDER THE APPROPRIATE ITEM.
26. WHEN A TEMPORARY BARRIER IS USED, A BARRIER ENDING FACING ONCOMING TRAFFIC SHALL BE TAPERED BEYOND THE CLEAR ZONE, OR PROTECTED WITH AN APPROVED END TREATMENT DESIGNED FOR THE 85TH PERCENTILE SPEED OR THE POSTED SPEED LIMIT OF THE ROADWAY.

STDS. REQUIRED	E-100, E-101, E-102, E-102A, E-106, E-107, E-107A, E-110, E-121, E-140 E-142, E-170, E-171A, E-171B, E-171C, E-172, E-175
PROJECT NAME:	HUNTINGTON
PROJECT NUMBER:	BRO 1445 (29)
FILE NAME:	...\\Cadd\Trans\201302ts2.dgn
PROJECT LEADER:	M. CHENETTE
DESIGNED BY:	D. ALTERI
TRAFFIC SIGNAL PLAN	
PLOT DATE:	1/30/2006
DRAWN BY:	J. OAKMAN
CHECKED BY:	D. ALTERI
SHEET	22 OF 63



SOIL CLASSIFICATION

AASHTO

A1	Gravel and Sand
A3	Fine Sand
A2	Silty or Clayey Gravel and Sand
A4	Silty Soil - Low Compressibility
A5	Silty Soil - Highly Compressible
A6	Clayey Soil - Low Compressibility
A7	Clayey Soil - Highly Compressible

ROCK QUALITY DESIGNATION

R.Q.D. (%)	ROCK DESCRIPTION
<25	Very Poor
25 to 50	Poor
51 to 75	Fair
76 to 90	Good
>90	Excellent

SHEAR STRENGTH

UNDRAINED SHEAR STRENGTH IN P.S.F.	CONSISTENCY
<250	Very Soft
250-500	Soft
500-1000	Med. Stiff
1000-2000	Stiff
2000-4000	Very Stiff
>4000	Hard

CORRELATION GUIDE OF "N" TO DENSITY/CONSISTENCY

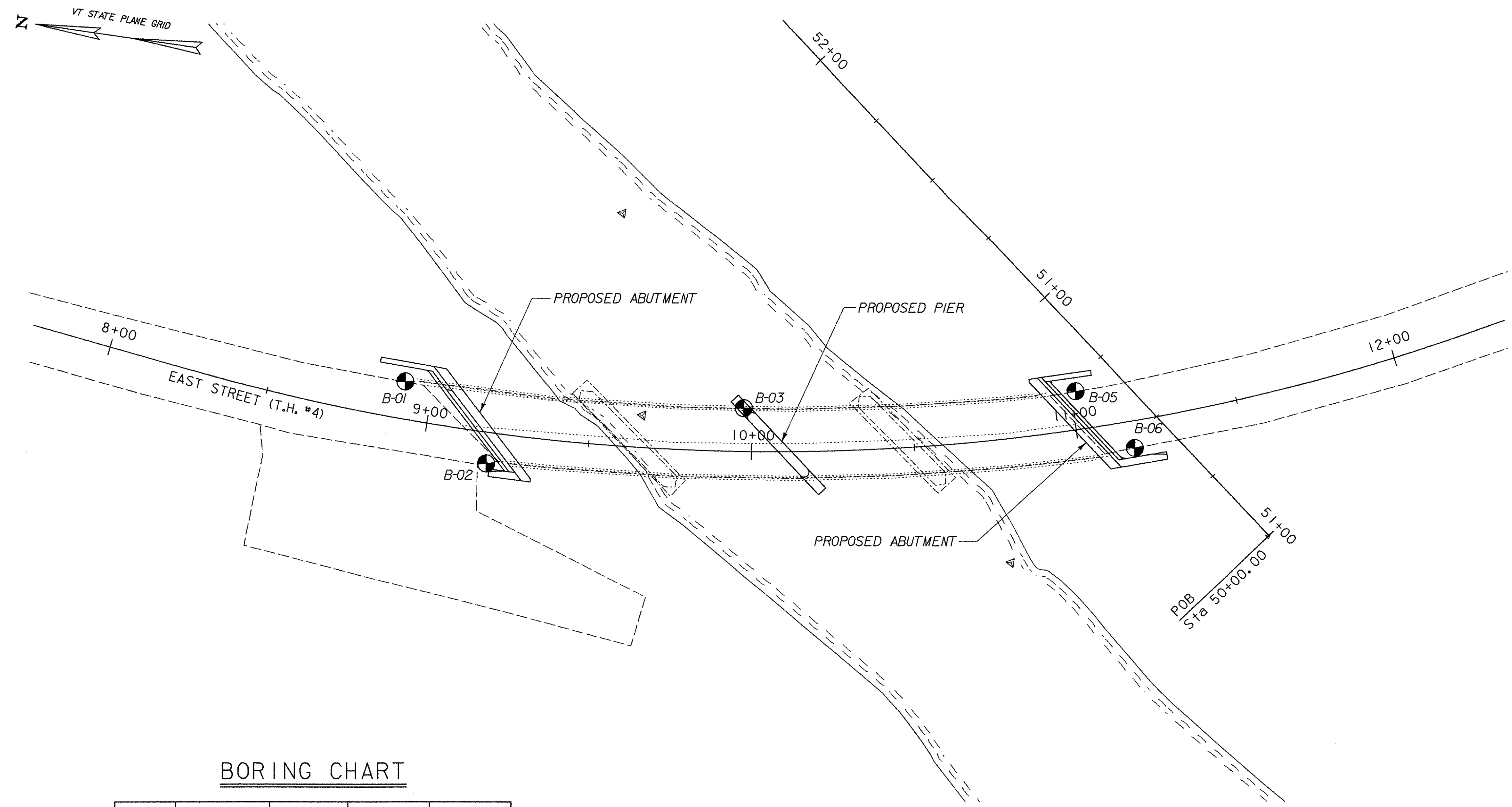
DENSITY (GRANULAR SOILS)		CONSISTENCY (COHESIVE SOILS)	
N	DESCRIPTIVE TERM	N	DESCRIPTIVE TERM
<5	Very Loose	<2	Very Soft
5-10	Loose	2-4	Soft
11-24	Med. Dense	5-8	Med. Stiff
25-50	Dense	9-15	Stiff
>50	Very Dense	16-30	Very Stiff
		31-60	Hard
		>60	Very Hard

COMMONLY USED SYMBOLS

▼	Water Elevation
⊕	Standard Penetration Boring
⊗	Auger Boring
⊙	Rod Sounding
S	Sample
N	Standard Penetration Test
	Blow Count Per Foot For:
	2" O.D. Sampler
	1 3/8" I.D. Sampler
	Hammer Weight Of 140 Lbs.
	Hammer Fall Of 30"
VS	Field Vane Shear Test
US	Undisturbed Soil Sample
B	Blast
DC	Diamond Core
MD	Mud Drill
WA	Wash Ahead
HSA	Hollow Stem Auger
AX	Core Size 1 1/8"
BX	Core Size 1 3/8"
NX	Core Size 2 1/8"
M	Double Tube Core Barrel Used
LL	Liquid Limit
PL	Plastic Limit
PI	Plasticity Index
NP	Non Plastic
w	Moisture Content (Dry Wgt. Basis)
D	Dry
M	Moist
MTW	Moist To Wet
W	Wet
Sat	Saturated
Bo	Boulder
Gr	Gravel
Sa	Sand
SI	Silt
Cl	Clay
HP	Hardpan
Le	Ledge
NLTD	No Ledge To Depth
CNPF	Can Not Penetrate Further
TLOB	To Ledge Or Boulder
NR	No Recovery
Rec.	Recovery
1/2 Rec.	Percent Recovery
RQD	Rock Quality Designation
CBR	California Bearing Ratio
<	Less Than
>	Greater Than
R	Refusal (N > 100)

COLOR

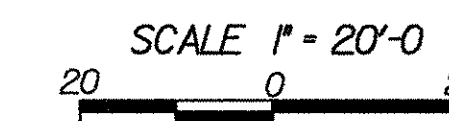
bik	Black	pnk	Pink
bl	Blue	pu	Purple
brn	Brown	rd	Red
dk	Dark	tn	Tan
gry	Gray	wh	White
gn	Green	yel	Yellow
lt	Light	mltc	Multicolored
or	Orange		



BORING CHART

HOLE NO.	SURV. STATION	OFFSET	GROUND ELEV.	ELEV. TLOB
B-01	8+91.20	11.4' LT	628.16	NLTD
B-02	9+19.29	9.5' RT	628.10	551.10
B-03	9+97.52	13.3' LT	607.30	NLTD
B-04	ELIMINATED			
B-05	11+01.52	11.7' LT	626.64	NLTD
B-06	11+16.76	8.2' RT	626.18	NLTD

BORING LAYOUT



DEFINITIONS (AASHTO)

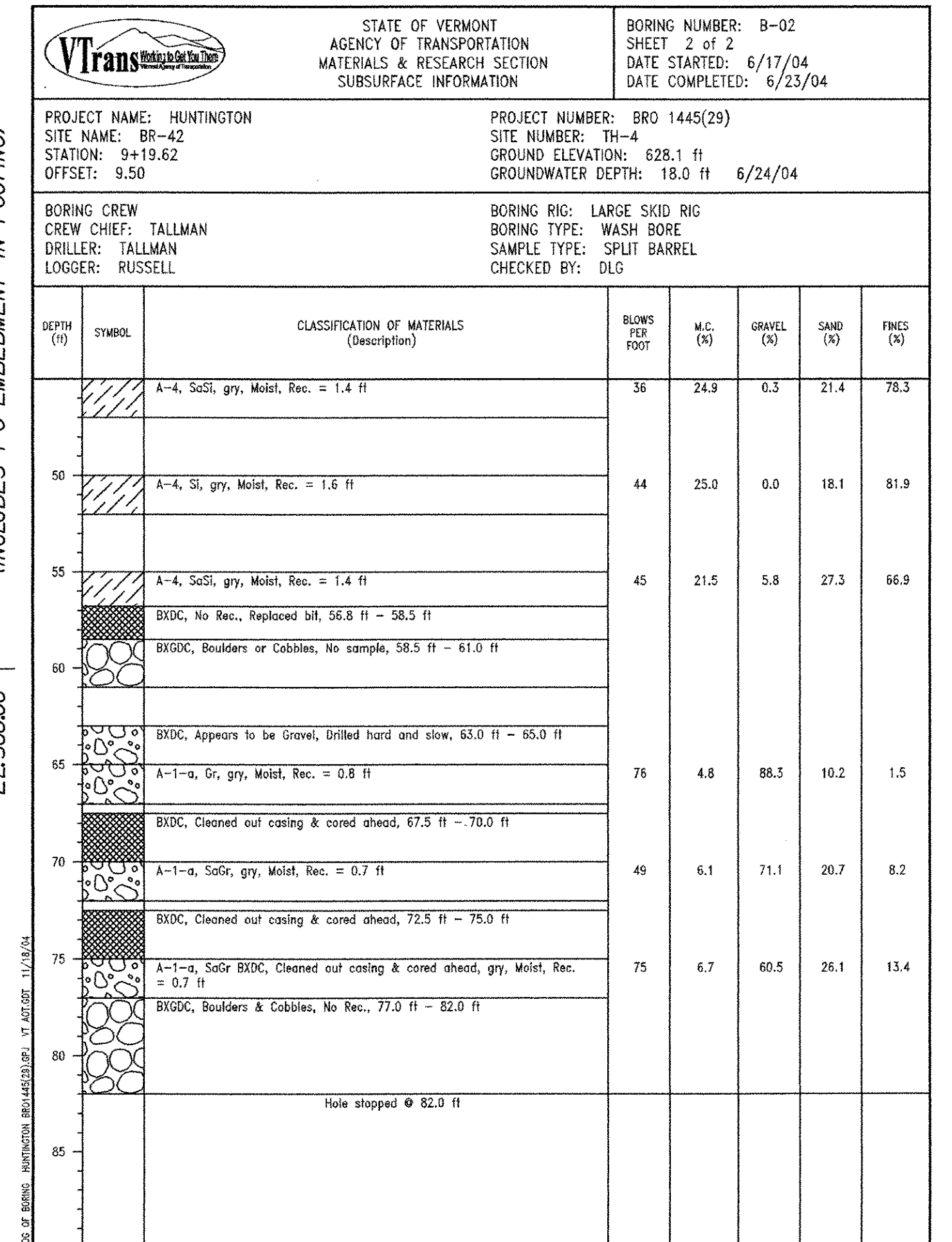
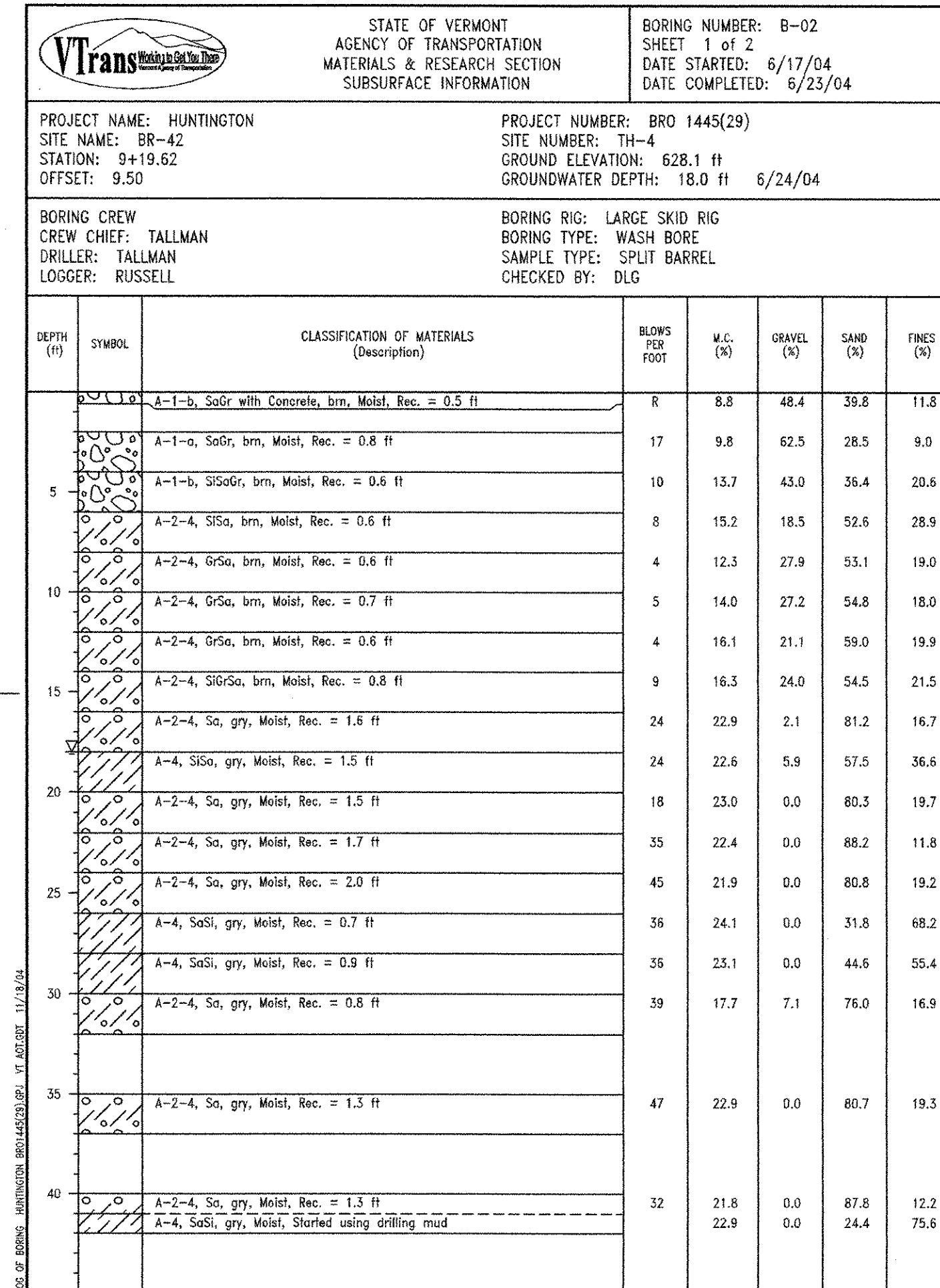
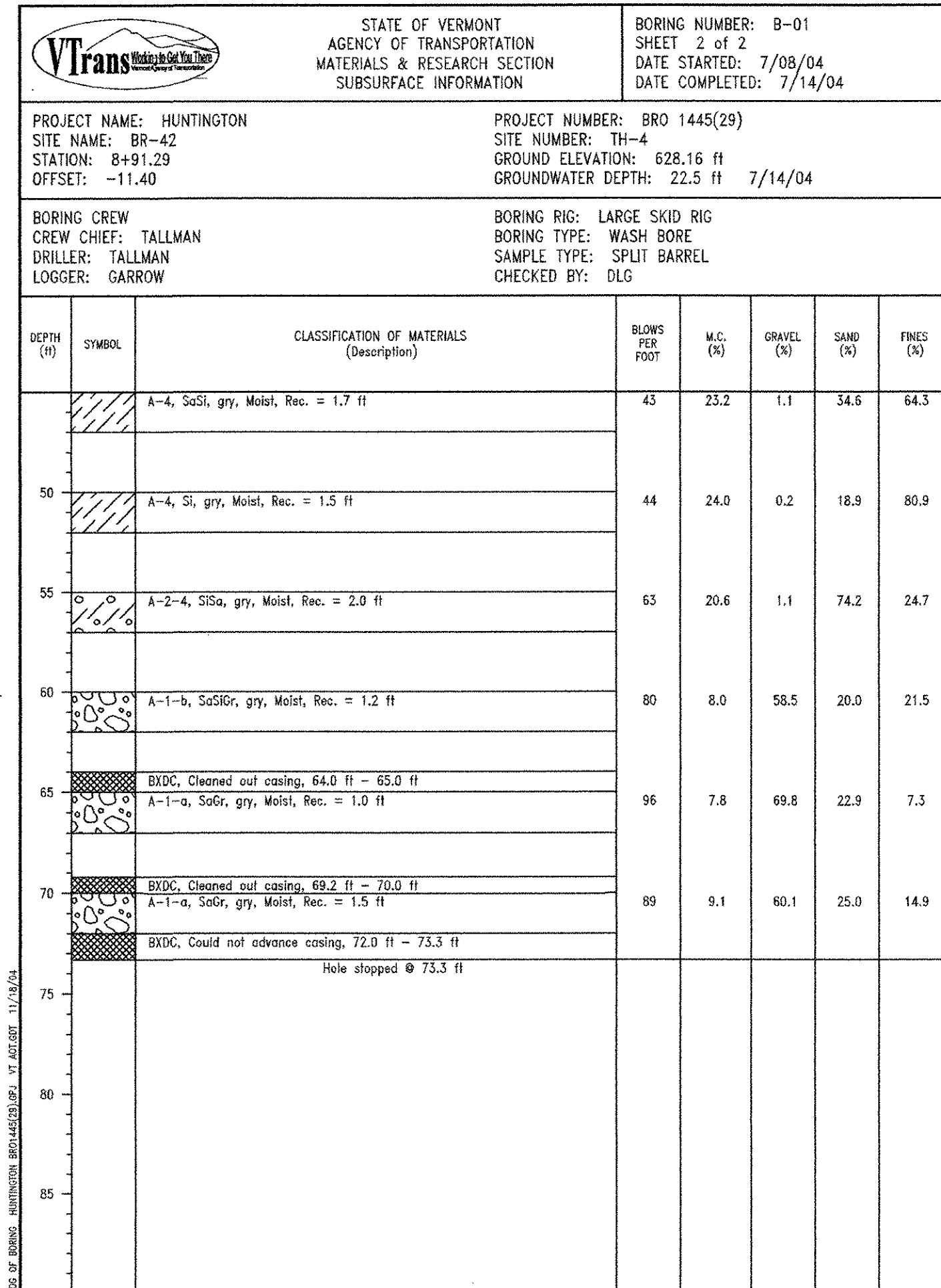
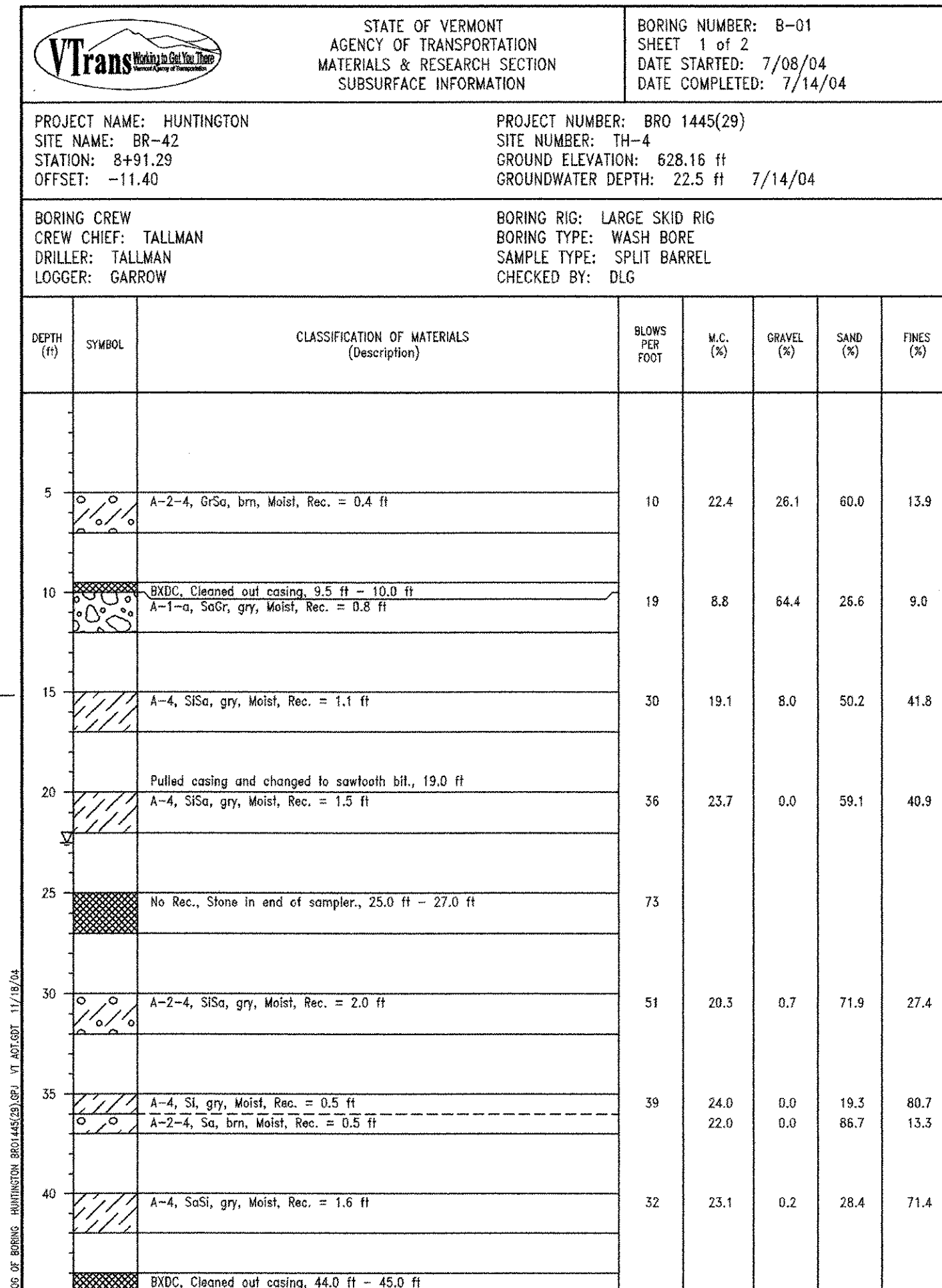
BEDROCK (LEDGE) - Rock in its native location of rock > 12 inches.	VARVED - Alternate layers of silt and clay.
BOULDER - A rock fragment with an average dimension > 12 inches.	HARDPAN - Extremely dense soil, cemented layer, not softened when wet.
COBBLE - Rock fragments with an average dimension between 3 and 12 inches.	MUCK - Soft organic soil (containing > 10% organic material).
GRAVEL - Rounded particles of rock < 3" and > 0.075" (#10 sieve).	MOISTURE CONTENT - Weight of water divided by dry weight of soil.
SAND - Particles of rock < 0.075" (#10 sieve) and > 0.0025" (#200 sieve).	FLOWING SAND - Granular soil so saturated (loose) that it flows into drill casing during extraction of wash rod.
SILT - Soil < 0.0025" (#200 sieve), non or slightly plastic and exhibits no strength when air-dried.	STRIKE - Angle from magnetic north to line of intersection of bed with a horizontal plane.
CLAY - Fine grained soil, exhibits plasticity when moist and considerable strength when air-dried.	DIP - Inclination of bed with a horizontal plane.

GENERAL NOTES

- The subsurface explorations shown herein were made between 06/17/04 and 10/05/04 by the Agency.
- Soil and rock classifications, properties and descriptions are based on engineering interpretation from available subsurface information by the Agency and may not necessarily reflect actual variations in subsurface conditions that may be encountered between individual boring or sample locations.
- Observed water levels and/or conditions indicated are as recorded at the time of exploration and may vary according to the prevailing rainfall, methods of exploration and other factors.
- Engineering judgement was exercised in preparing the subsurface information presented herein. Analysis and interpretation of subsurface data was performed and interpreted for Agency design and estimating purposes. Presentation of the information in the Contract is intended to provide the Contractor access to the same data available to the Agency. The subsurface information is presented in good faith and is not intended as a substitute for personal investigation, independent interpretation, independent analysis or judgement by the Contractor.
- Pictorial structure details shown on the boring plan layout or soils profile are for illustrative purposes only and may not accurately portray final contract details.
- Terminology used on boring logs to describe the hardness, degree of weathering, and spacing of fractures, joints and other discontinuities in the bedrock is defined in the AASHTO Manual on Subsurface Investigations, 1988.

STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
BORING INFORMATION SHEET 1			
Designed By	T. KNIGHT	Drawn By	D. HARRINGTON
Checked By	M. CHENETTE	Date	06/04
		Bridge Design Supervisor	M. CHENETTE Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn: ... \Cadd\Trans\201302borpln.dgn		Plot Date:	1/12/2006
Bridge Sheet No.		Sheet	23 of 63



STATE OF VERMONT
AGENCY OF TRANSPORTATION

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
BORING INFORMATION SHEET 2			
Designed By	T. KNIGHT	Drawn By	S. BURBANK
Checked By	M. CHENETTE	Date	12/04
		Bridge Design Supervisor	M. CHENETTE Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn:\...\Cadd\Trans\z01J302bor1.dgn		Plot Date:	1/12/2006
Bridge Sheet No.		Sheet	24 of 63

VT		STATE OF VERMONT AGENCY OF TRANSPORTATION MATERIALS & RESEARCH SECTION SUBSURFACE INFORMATION		BORING NUMBER: B-03 SHEET 1 of 2 DATE STARTED: 9/28/04 DATE COMPLETED: 10/05/04			
PROJECT NAME: HUNTINGTON SITE NAME: BR-42 STATION: 9+95.50 OFFSET: -13.30		PROJECT NUMBER: BRO 1445(29) SITE NUMBER: TH-4 GROUND ELEVATION: 607.30 ft GROUNDWATER DEPTH:		BORING CREW CREW CHIEF: TALLMAN DRILLER: TALLMAN LOGGER: RUSSELL			
BORING RIG: DOZER BORING TYPE: WASH BORE SAMPLE TYPE: SPLIT BARREL CHECKED BY: DLG							
DEPTH (ft)	SYMBOL	CLASSIFICATION OF MATERIALS (Description)	BLOWS PER FOOT	W.C. (%)	GRAVEL (%)	SAND (%)	FINES (%)
5		BXDC, Cleaned out casing, 4.6 ft - 5.0 ft A-2-4, So, gry, Moist, Rec. = 1.4 ft	33	23.0	1.7	86.3	12.0
10		A-4, SoSl, gry, Moist, Rec. = 1.7 ft	33	25.4	0.5	41.9	57.8
15		A-4, SiSo, gry, Moist, Rec. = 1.7 ft	80	25.3	0.7	63.8	35.5
20		A-3, So, gry, Moist, Rec. = 1.8 ft BXDC, Casing plugged up, Used drillers mud, 22.0 ft - 25.0 ft	45	22.8	0.3	90.2	9.5
25		A-2-4, So, gry, Moist, Rec. = 2.0 ft, Casing broke off, Moved Hole 2 ft, Went down 30' before sampling again.	53	16.7	13.1	71.4	15.5
30		A-4, SoSl, gry, Moist, Rec. = 2.0 ft	47	21.5	2.7	42.4	54.9
35		A-4, SoSl, gry, Moist, Rec. = 1.8 ft	48	25.3	3.8	41.7	54.5

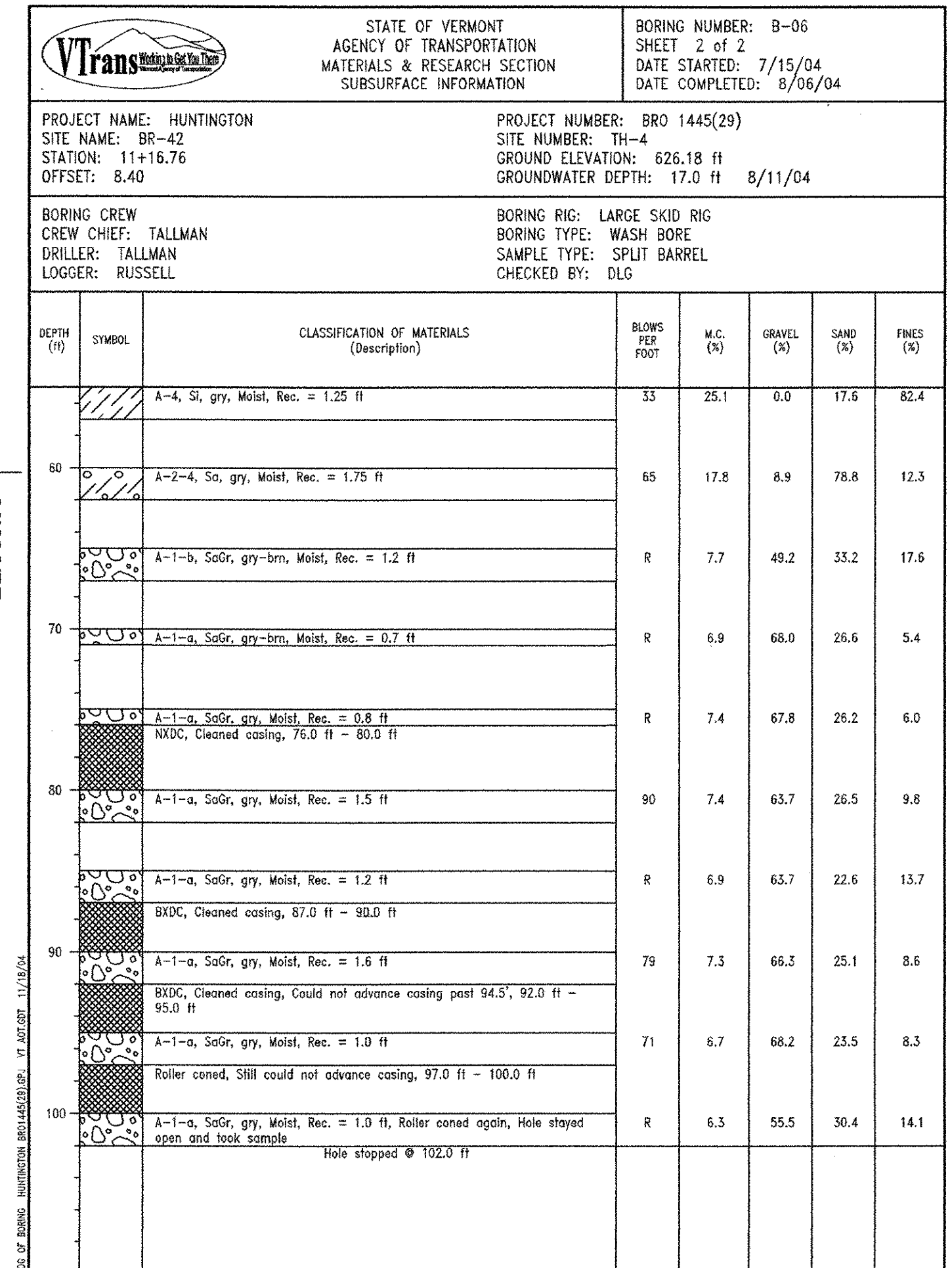
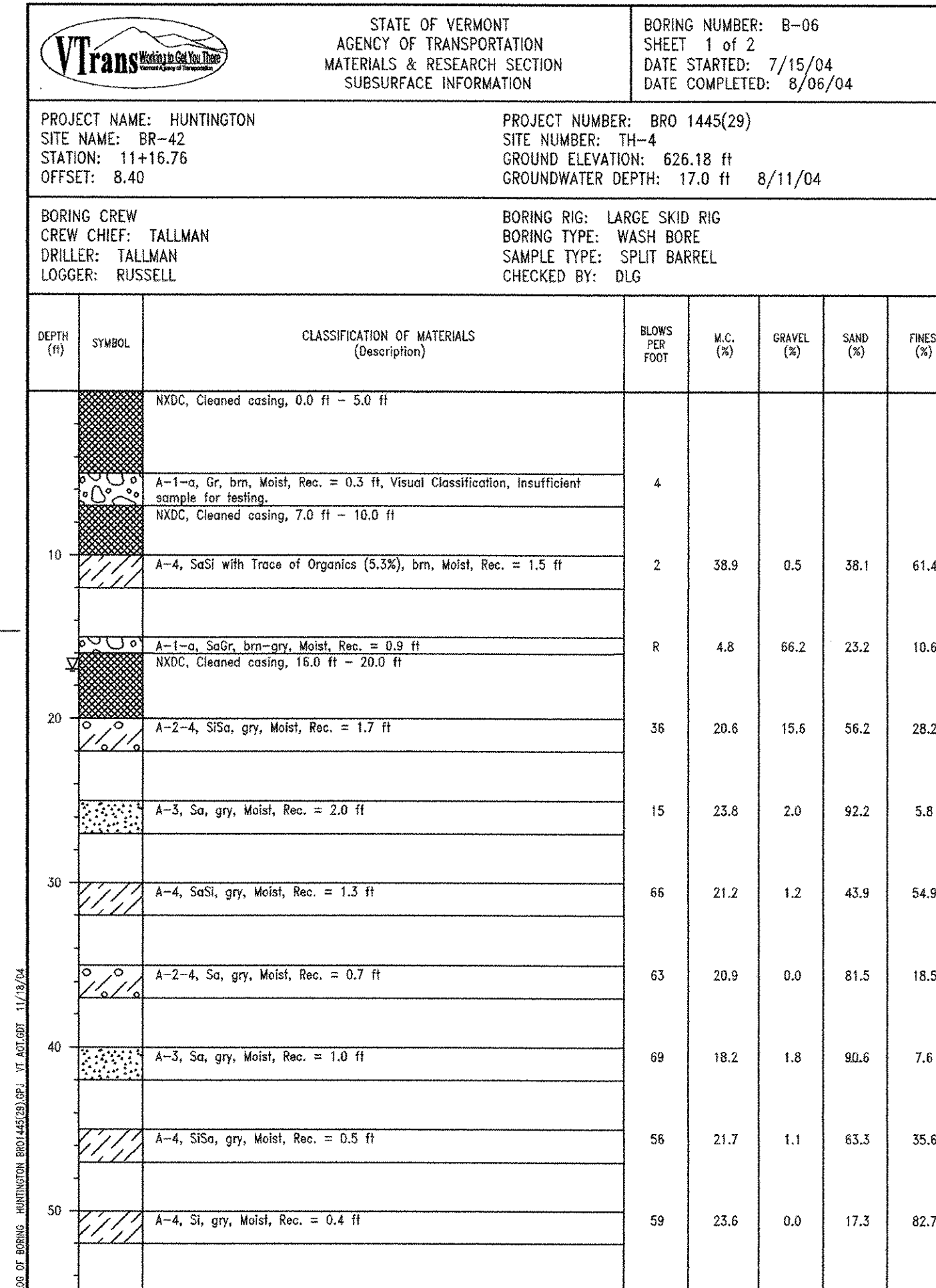
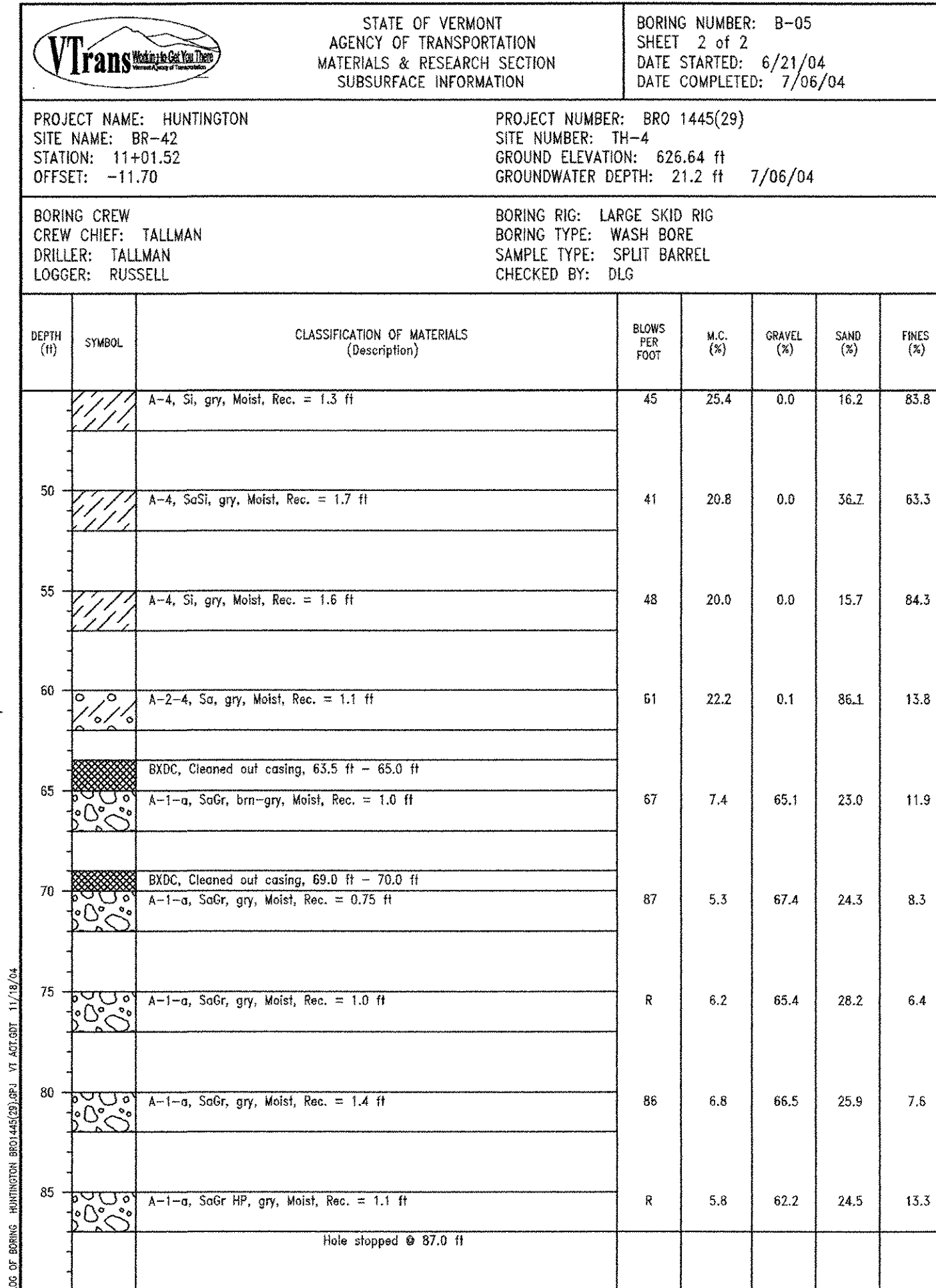
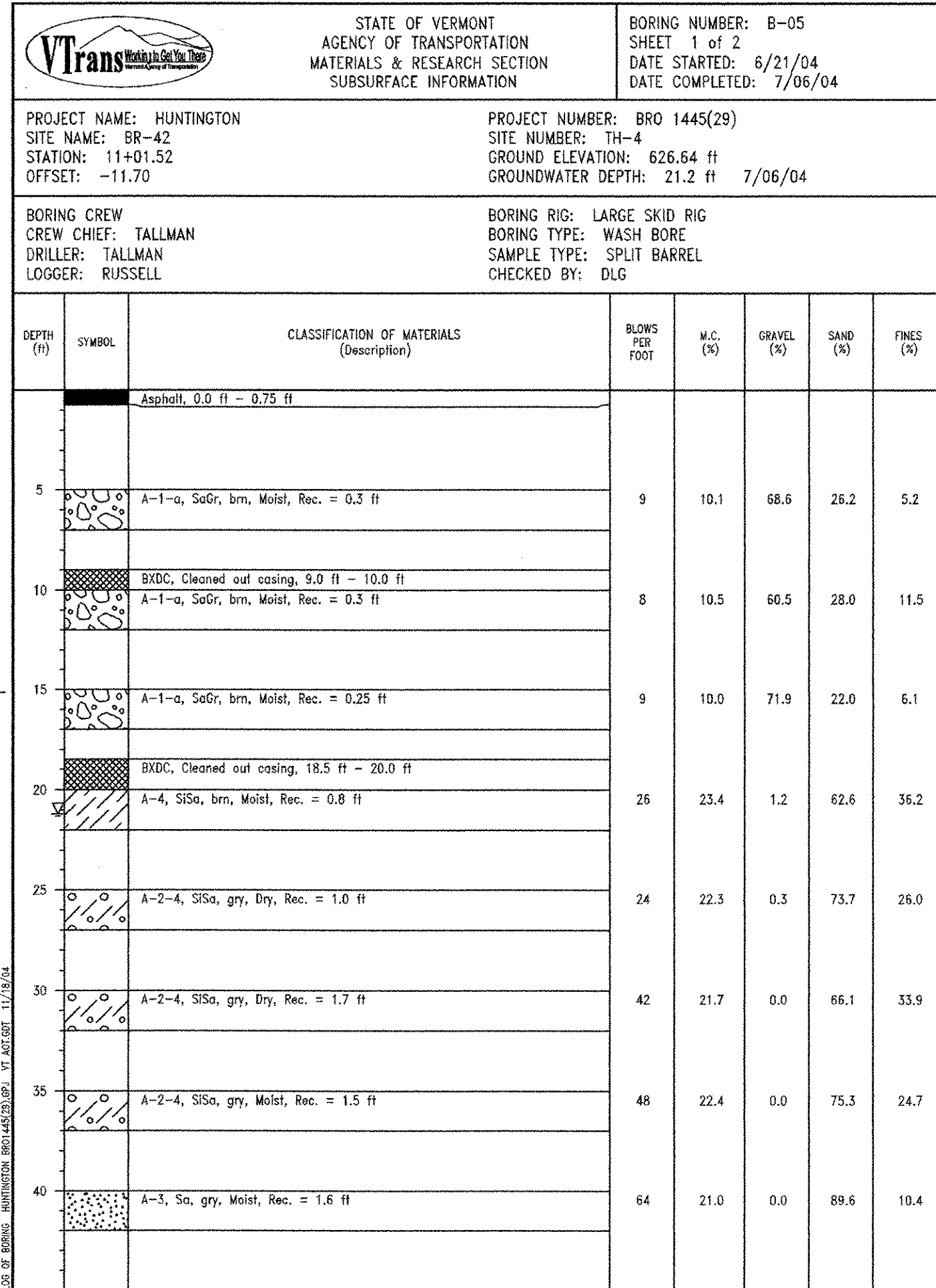
PIER BOTTOM OF FOOTING
 EL. 599.00
 ESTIMATED PILE LENGTH = 53'-0"
 (INCLUDES 1'-0" EMBEDMENT IN FOOTING)
 U.S. OF MILES: APPROXIMATE BEARING CAPACITY AT 50% OF SPT VALUE

VT		STATE OF VERMONT AGENCY OF TRANSPORTATION MATERIALS & RESEARCH SECTION SUBSURFACE INFORMATION		BORING NUMBER: B-03 SHEET 2 of 2 DATE STARTED: 9/28/04 DATE COMPLETED: 10/05/04			
PROJECT NAME: HUNTINGTON SITE NAME: BR-42 STATION: 9+95.50 OFFSET: -13.30		PROJECT NUMBER: BRO 1445(29) SITE NUMBER: TH-4 GROUND ELEVATION: 607.30 ft GROUNDWATER DEPTH:		BORING CREW CREW CHIEF: TALLMAN DRILLER: TALLMAN LOGGER: RUSSELL			
BORING RIG: DOZER BORING TYPE: WASH BORE SAMPLE TYPE: SPLIT BARREL CHECKED BY: DLG							
DEPTH (ft)	SYMBOL	CLASSIFICATION OF MATERIALS (Description)	BLOWS PER FOOT	W.C. (%)	GRAVEL (%)	SAND (%)	FINES (%)
45		A-4, SoSl, gry, Moist, Rec. = 1.2 ft A-2-4, SiSo, gry, Moist, Rec. = 0.5 ft	81	23.0	0.1	21.7	78.2
45		BXDC, Cleaned out and then hit a big aquifer, 44.5 ft - 45.0 ft A-1-a, Gr, gry, Moist, Rec. = 0.8 ft	80	7.8	77.5	17.1	5.4
50		BXDC, Cleaned out casing, 49.5 ft - 50.0 ft A-1-a, Gr, gry, Moist, Rec. = 0.2 ft	R	5.6	84.0	14.9	1.1
55		BXDC, Cleaned out casing, 54.5 ft - 55.0 ft A-1-a, SoGr, gry, Moist, Rec. = 0.8 ft	R	9.5	66.6	29.7	3.7
60		BXDC, Cleaned out casing, 59.25 ft - 60.0 ft A-1-a, SoGr, gry, Moist, Rec. = 1.5 ft	R	8.4	67.5	26.6	5.9
65		A-1-a, SoGr, gry, Moist, Rec. = 1.2 ft, Casing would not advance due to bit being worn out. Hole stopped @ 67.0 ft	R	7.9	71.2	22.5	6.3
DRILLER'S NOTES: A large aquifer is at 45 feet and pushes a full casing of water. Water was still pushing out of the casing at 55 feet. Water would stop flowing out of the casing with 6 feet of casing showing above the river. Water also stopped flowing while retreating the casing at 30 feet.							

APPROXIMATE
 BOTTOM OF PILE
 EL. 547.00
 U.S. OF MILES: APPROXIMATE BEARING CAPACITY AT 50% OF SPT VALUE

**STATE OF VERMONT
AGENCY OF TRANSPORTATION**

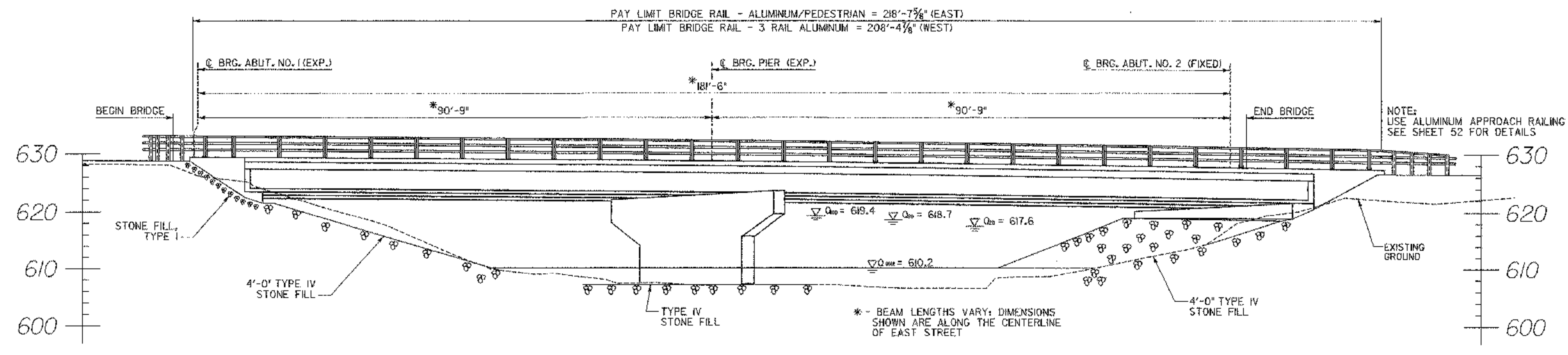
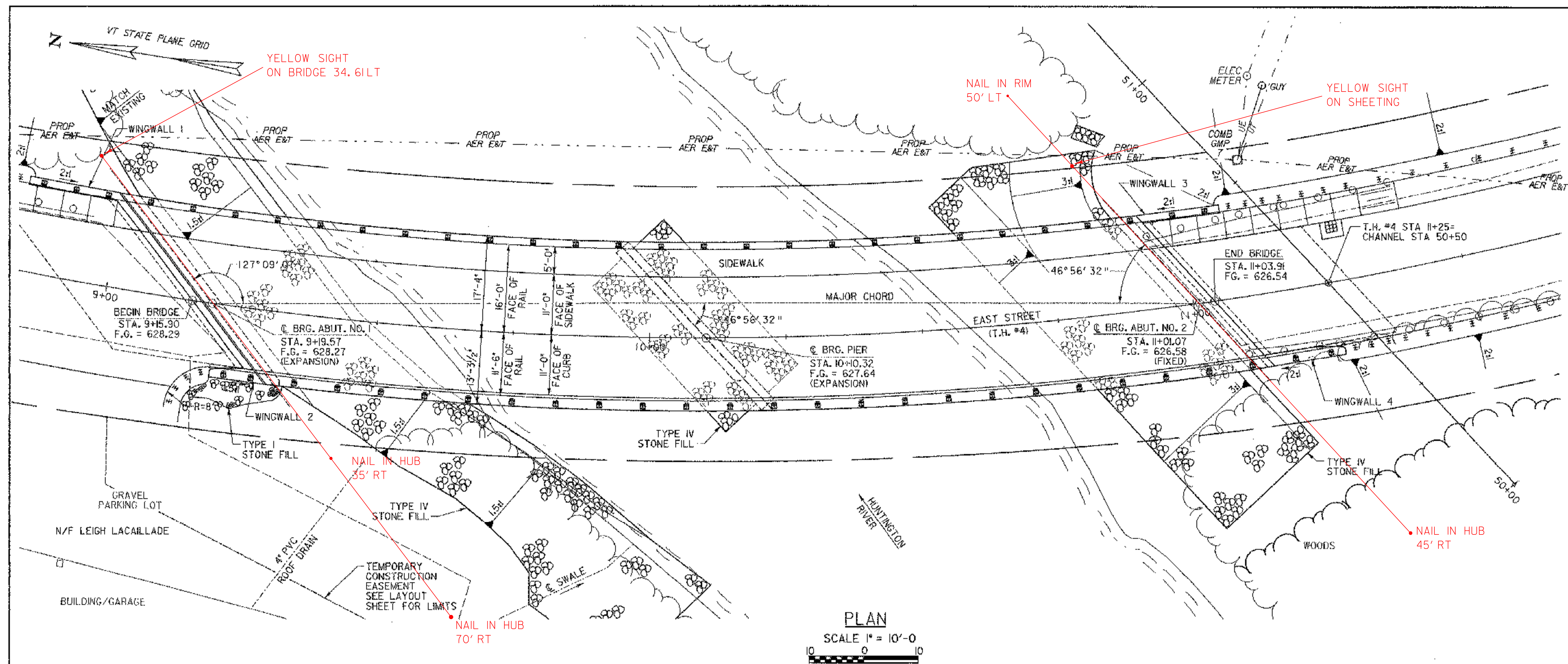
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
BORING INFORMATION SHEET 3			
Designed By	T. KNIGHT	Drawn By	S. BURBANK
Checked By	M. CHENETTE	Date	12/04
		Bridge Design Supervisor	M. CHENETTE Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn:...	Cadd\Trans\z01j302bor2.dgn	Plot Date:	1/12/2006
Bridge Sheet No.		Sheet	25 of 63



STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of HUNTINGTON		Bridge No. 42
Highway No. T.H. 4		Log Sta.
		Surv. Sta.
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER		
BORING INFORMATION SHEET 4		
Designed By T. KNIGHT	Drawn By S. BURBANK	
Checked By M. CHENETTE	Date 12/04	Bridge Design Supervisor M. CHENETTE Date 09/05
PROJECT HUNTINGTON		PROJECT NO. BRO 1445 (29)
DH Dgn:\... \Cadd\Trans\z01302bor3.dgn		Plot Date: 1/12/2006
Bridge Sheet No.	Sheet 26 of 63	





NOTE:
USE ALUMINUM APPROACH RAILING
SEE SHEET 52 FOR DETAILS

STATE OF VERMONT AGENCY OF TRANSPORTATION			
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
PLAN AND ELEVATION			
Designed By	F. KNIGHT	Drawn By	D. HARRINGTON
Checked By	M. CHENETTE	Date	08/05
		Bridge Design Supervisor	M. CHENETTE
		Date	09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgnw ... \Cadd\Trans\201302plan.dgn		Plot Date:	1/30/2006
Bridge Sheet No.		Sheet	27 of 63



PROJECT NOTES

GENERAL

1. ALL MATERIALS AND CONSTRUCTION SHALL CONFORM TO STATE OF VERMONT, AGENCY OF TRANSPORTATION, 2001 STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, AND ITS LATEST REVISIONS, AND THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, DATED 2002, AND ITS LATEST REVISIONS.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY PROJECT SIGNING. THE COSTS WILL BE INCIDENTAL TO THE ITEM 641.10, "TRAFFIC CONTROL".
3. DURING CONSTRUCTION, TRAFFIC WILL BE MAINTAINED ON A ONE-WAY TEMPORARY BRIDGE LOCATED DOWNSTREAM OF THE EXISTING STRUCTURE. THE BRIDGE SHALL SPAN THE STREAM IN ACCORDANCE WITH THE REQUIREMENTS INDICATED ON SHEET 2.
4. ACCESS TO ALL EXISTING SIDE ROADS, DRIVES, AND PARKING AREAS SHALL BE MAINTAINED AT ALL TIMES DURING CONSTRUCTION.
5. ALL DIMENSIONS SHOWN IN THE PLANS ARE HORIZONTAL OR VERTICAL AT 68 DEGREES FAHRENHEIT.
6. IN-STREAM CONSTRUCTION SHALL BE CONDUCTED ONLY DURING THE PERIOD OF JUNE 1 TO OCTOBER 1, UNLESS THE CONTRACTOR OBTAINS PERMISSION FROM THE AGENCY OF NATURAL RESOURCES TO WORK OUTSIDE OF THAT TIME FRAME.

EARTHWORK AND RELATED ITEMS

7. ITEM 529.20 "REMOVAL, PARTIAL, OF STRUCTURE" IS RECORDED ON CONTRACT ITEM 529.15, "REMOVAL OF STRUCTURE" SHALL BE USED FOR REMOVAL OF EXISTING SUPERSTRUCTURE AND ANY PORTIONS OF THE SUBSTRUCTURE NOT REMOVED UNDER THE ITEMS 203.27, "UNCLASSIFIED CHANNEL EXCAVATION" OR 204.25, "STRUCTURE EXCAVATION". THE ABUTMENTS SHALL BE REMOVED IN THEIR ENTIRETY. THE WOOD PILES SHALL REMAIN IN PLACE UNLESS THEY ARE REMOVED UNDER THE ITEMS 203.27 OR 204.25. THE PIERS SHALL BE REMOVED TO THE TOP OF THEIR FOOTINGS. REMOVAL AND DISPOSAL OF THE BRIDGE PAVEMENT SHALL BE PAID FOR UNDER ITEM 529.10, "REMOVAL OF BRIDGE PAVEMENT". THE EXISTING STRUCTURAL STEEL ON THIS PROJECT WAS PAINTED WITH A MATERIAL WHICH MAY CONTAIN LEAD. THE REMOVED STRUCTURAL STEEL IS THE PROPERTY OF THE CONTRACTOR. THE CONTRACTOR SHALL INDEMNIFY AND HOLD THE STATE, ITS OFFICERS, AND EMPLOYEES HARMLESS CONCERNING THE CONTRACTOR'S USE OR DISPOSITION OF THE STRUCTURAL STEEL.
8. THE "STONE FILL, TYPE IV" UNDER THE BRIDGE SHALL BE PLACED BEFORE THE GIRDERS ARE SET.

CONCRETE AND REINFORCING STEEL

9. CONCRETE FOR THE DECK, SIDEWALK AND CURB SHALL BE HIGH PERFORMANCE CLASS A AND WILL BE PAID FOR UNDER ITEM 501.33, "CONCRETE, HIGH PERFORMANCE CLASS A". ALL OTHER CONCRETE SHALL BE HIGH PERFORMANCE CLASS B AND WILL BE PAID FOR UNDER ITEM 501.34, "CONCRETE, HIGH PERFORMANCE CLASS B" UNLESS OTHERWISE NOTED.
10. SURFACES OF BRIDGE SEATS UNDER BEARING DEVICES SHALL BE LEVEL. OTHER BRIDGE SEAT AREAS SHALL BE SLOPED $\frac{1}{2}$ " PER FOOT. THE ABUTMENT SEATS SHALL BE SLOPED FULL WIDTH TOWARD MIDSPAN. THE ENTIRE BRIDGE SEAT SURFACE SHALL BE GIVEN A MAGNESIUM FLOAT FINISH.

11. NO CONCRETE SHALL BE PLACED IN THE ABUTMENTS OR WINGWALLS ABOVE THE ADJACENT BEAM SEAT ELEVATIONS UNTIL THE BEAMS HAVE BEEN PROFILED AND THE FINISHED GRADE OF THE DECK HAS BEEN DETERMINED BY THE ENGINEER.
12. NO CONSTRUCTION TRAFFIC SHALL BE ALLOWED ON THE NEW DECK UNTIL THE CURE PERIOD IS UP AND THE 28-DAY DESIGN STRENGTH IS ATTAINED, AS EVIDENCED BY TEST CYLINDERS CURED UNDER FIELD CONDITIONS.
13. THE KEY IN CONCRETE CONSTRUCTION JOINTS SHALL BE MONOLITHIC AND CONTINUOUS FOR THE FULL LENGTH OF THE JOINT. UPWARD KEYS SHALL BE PLACED INTEGRALLY WITH THE CONCRETE BELOW THE JOINT.
14. JOINTS AND SCORE MARKS IN CONCRETE SHALL BE CONSTRUCTED AS INDICATED ON THE PLANS OR AS DIRECTED BY THE ENGINEER.
15. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED ONE (1) INCH.
16. ITEM 514.10, "WATER REPELLENT (MOD.-SILANE)" SHALL BE APPLIED TO ALL EXPOSED CONCRETE SURFACES EXCEPT THE UNDERSIDE OF DECK BETWEEN DRIP BEADS.
17. ALL REINFORCING STEEL SHALL BE DETAILED AND FABRICATED USING PROCEDURES AND TOLERANCES IN ACCORDANCE WITH AND APPLICABLE PUBLICATIONS OF THE "CONCRETE REINFORCING STEEL INSTITUTE."
18. MINIMUM COVER FOR REINFORCING STEEL IN THE SUBSTRUCTURES SHALL BE TWO (2) INCHES ALONG WALL FACES AGAINST EARTH, AND THREE (3) INCHES ELSEWHERE, UNLESS DETAILED OTHERWISE.
19. ALL REINFORCING STEEL IN THE CONCRETE DECK, BRIDGE SIDEWALK, APPROACH SLABS, BACKWALLS AND BRIDGE CURB SHALL BE EPOXY COATED AND PAID FOR UNDER ITEM 507.17, "EPOXY COATED REINFORCING STEEL". WHEN EPOXY COATED REINFORCING STEEL IS TO BE CUT, THE UNCOATED ENDS SHALL BE REPAIRED WITH MATERIALS AND PROCEDURES APPROVED BY THE COATING MANUFACTURER. FLAME CUTTING OF EPOXY COATED REINFORCING STEEL WILL NOT BE PERMITTED.
20. REINFORCING PLACEMENT TOLERANCES SHALL BE:
SPACING $\pm 1"$
CLEARANCE $\pm \frac{1}{4}"$

STRUCTURAL STEEL

21. ALL STRUCTURAL STEEL PAID FOR UNDER ITEM 506.55, "STRUCTURAL STEEL (PLATE GIRDER)" SHALL CONFORM TO AASHTO M270 GRADE 50W UNLESS OTHERWISE NOTED.
22. THE CHARPY V-NOTCH TEST IS REQUIRED ONLY FOR THOSE MEMBERS DESIGNATED AS SUCH ON THE PLANS AS SPECIFIED IN SUBSECTION 714.01 OF THE STANDARD SPECIFICATIONS.
23. AFTER THE SUPERSTRUCTURE STEEL HAS BEEN ERECTED, ELEVATIONS SHALL BE TAKEN ALONG THE TOP OF EACH GIRDER UNDER THE DIRECTION OF THE ENGINEER. THESE ELEVATIONS SHALL BE USED IN DETERMINING FINAL GRADE.
24. ALL FIELD CONNECTIONS IN UNPAINTED AREAS SHALL BE MADE USING $\frac{7}{8}"$ DIAMETER BOLTS, CONFORMING TO AASHTO M 164 TYPE 3. IN PAINTED AREAS USE $\frac{3}{8}"$ DIAMETER BOLTS CONFORMING TO AASHTO M 164 TYPE 1. HOLES SHALL BE $\frac{15}{16}"$ DIAMETER, UNLESS OTHERWISE NOTED. CONNECTIONS NOT DESIGNED SHALL BE DETAILED BY THE FABRICATOR AND SUBMITTED TO THE ENGINEER FOR APPROVAL.

25. FASCIA OVERHANG BRACKETS OR SIMILAR FALSE WORK SHALL BE SPACED AT A MAXIMUM OF FOUR (4) FEET. THE DESIGN OF THE FALSE WORK SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
26. ANY HOLES IN THE FASCIA BEAMS NOT OTHERWISE FILLED SHALL BE FITTED WITH BUTTON HEAD OR HEX HEAD BOLTS CONFORMING TO AASHTO M 164 TYPE 3 (UNPAINTED AREA) OR TYPE 1 (PAINTED AREA). THE BOLTS SHALL BE TIGHTENED IN ACCORDANCE WITH SUBSECTION 506.19 OF THE STANDARD SPECIFICATIONS.
27. ALL STRUCTURAL STEEL INCLUDING, GIRDERS, CROSS FRAMES, BEARING PLATES AND GUSSET PLATES WITHIN A DISTANCE OF 10 FEET FROM THE EXPANSION END OF THE GIRDERS WILL BE COATED WITH A PROTECTIVE PAINT SYSTEM AND GREASE RUSTPROOFING COMPOUND. THE FINAL PAINT COAT TO BE DARK BROWN, FEDERAL COLOR CHIP NO. 20059. THIS WORK SHALL BE PAID FOR UNDER ITEM 513.25, "STRUCTURAL PAINTING, SHOP APPLIED" AND 513.40, "SURFACE PREPARATION". SEE SUPPLEMENTAL SPECIFICATION 513 FOR PAINTING STRUCTURAL STEEL.

STEEL PILES

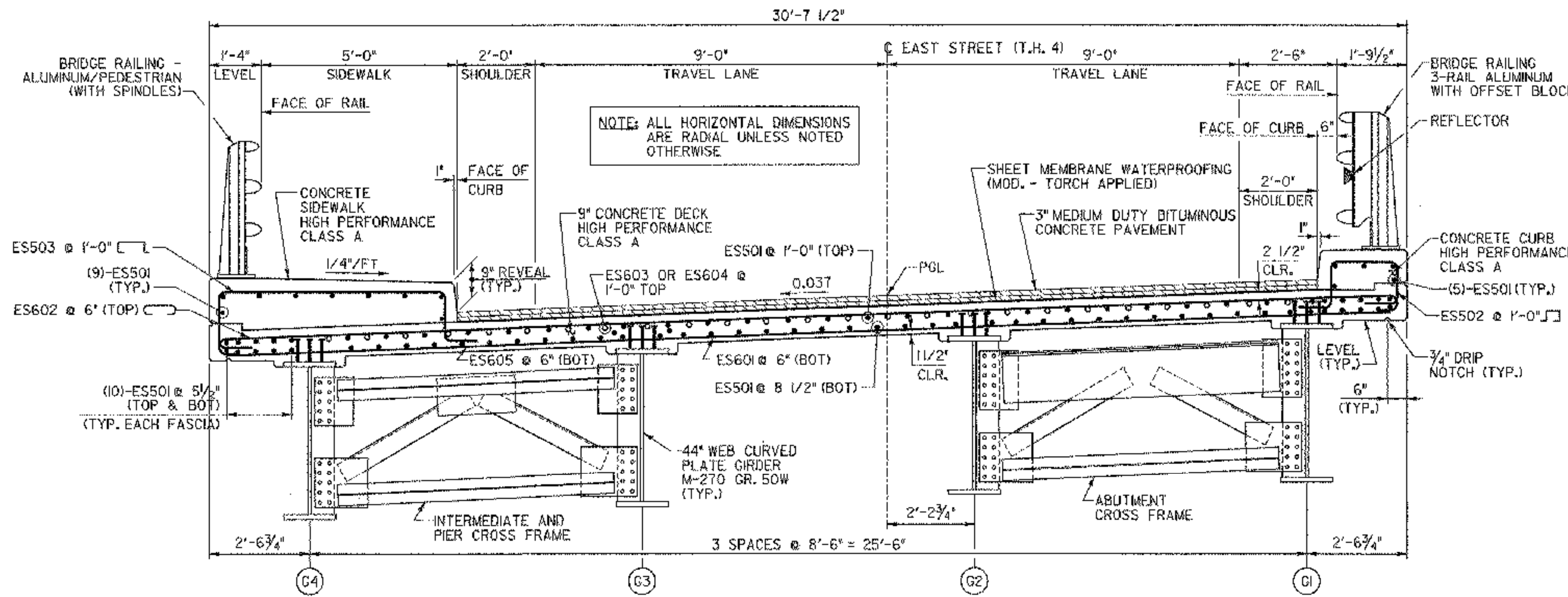
28. PILES SHALL BE HPI2x74 ASTM A572, GRADE 36. THE ESTIMATED LENGTH OF EACH PILE AT ABUTMENT NO. 1 IS 46 FEET AT ABUTMENT NO. 2 IS 47 FEET, AND AT THE PIER 53 FEET, INCLUDING THE 1 FOOT EMBEDMENT IN THE CONCRETE PILE CAP. NO SUBSTITUTIONS FOR THE NUMBER, SIZE AND GRADE OF THE PILES WILL BE ALLOWED.
29. THE DRIVING POINT OF ALL PILES SHALL BE REINFORCED. POINT REINFORCEMENT SHALL BE CAST STEEL AND SHALL CONFORM TO SUBSECTIONS 505.04 (E) AND 730.01 OF THE STANDARD SPECIFICATIONS.
30. THE PILES SHALL BE DRIVEN TO AN ULTIMATE AXIAL CAPACITY OF 326 KIPS AT THE ABUTMENTS AND 439 KIPS AT THE PIER, AS DIRECTED BY THE RESIDENT ENGINEER.
31. DYNAMIC MONITORING SHALL BE PERFORMED FOR THE FIRST PILE DRIVEN AT EACH SUBSTRUCTURE LOCATION TO DETERMINE THE DRIVING CRITERIA REQUIRED TO ACHIEVE THE SPECIFIED ULTIMATE AXIAL CAPACITY AND PREVENT DAMAGE TO THE PILES DURING DRIVING OPERATIONS. THIS WORK SHALL BE PERFORMED IN ACCORDANCE WITH SUBSECTION 505.04 (C) -2 OF THE STANDARD SPECIFICATIONS. PAYMENT FOR PILE TESTING SHALL BE MADE UNDER ITEM 505.45, "DYNAMIC PILE LOADING TEST".
32. PILE HEAD CUT-OFF ELEVATION SHALL BE WITHIN 2 INCHES OF THE ELEVATION DETAILED IN THE PLANS. AT THE CUT-OFF ELEVATION, THE PILE SHALL BE FREE FROM DRIVING DAMAGE AS DETERMINED BY THE RESIDENT ENGINEER.

STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			

PROJECT NOTES

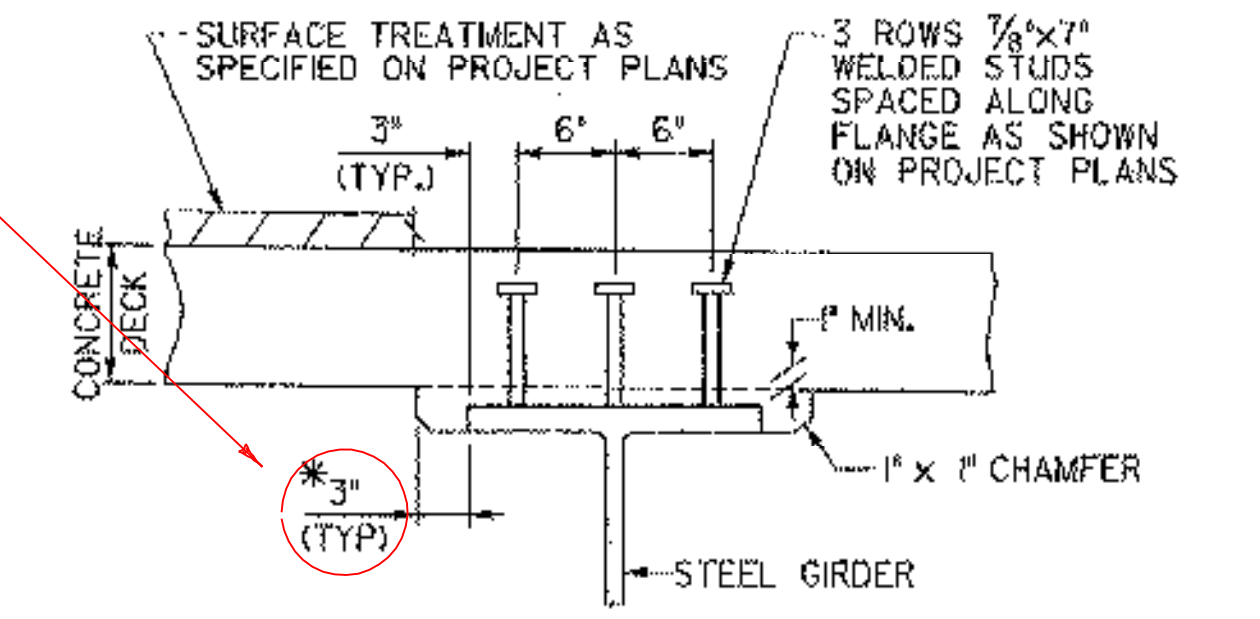
Designed By	S. BURBANK	Drawn By	S. BURBANK
Checked By	Date	Bridge Design Supervisor	Date
M. CHENETTE	03/05	M. CHENETTE	09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn: ... \Cadd\Trans\2014\302notes.dgn		Plot Date: 1/12/2006	
Bridge Sheet No.		Sheet 28 of 63	



TYPICAL BRIDGE SECTION

SCALE 1/2" = 1'-0"

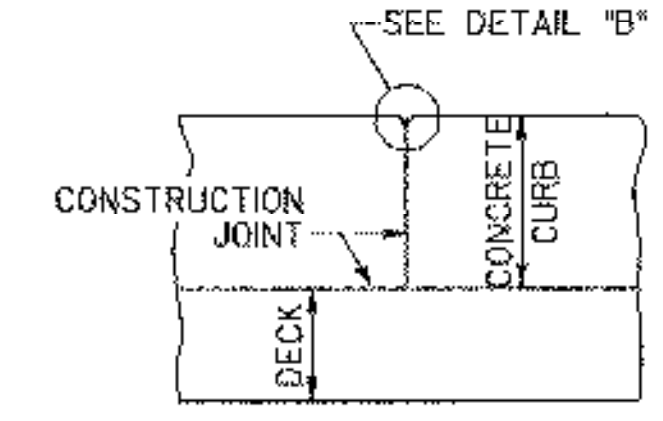
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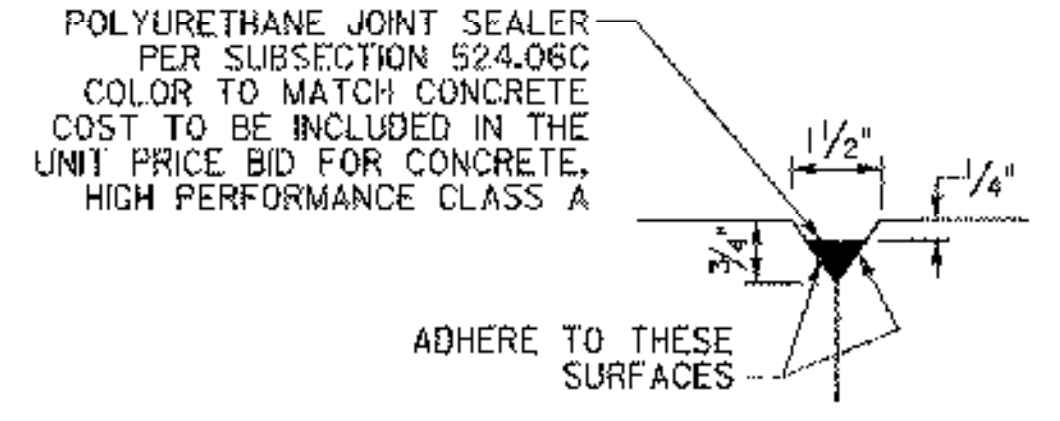
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HAUNCH AND SHEAR CONNECTOR DETAILS

NOT TO SCALE



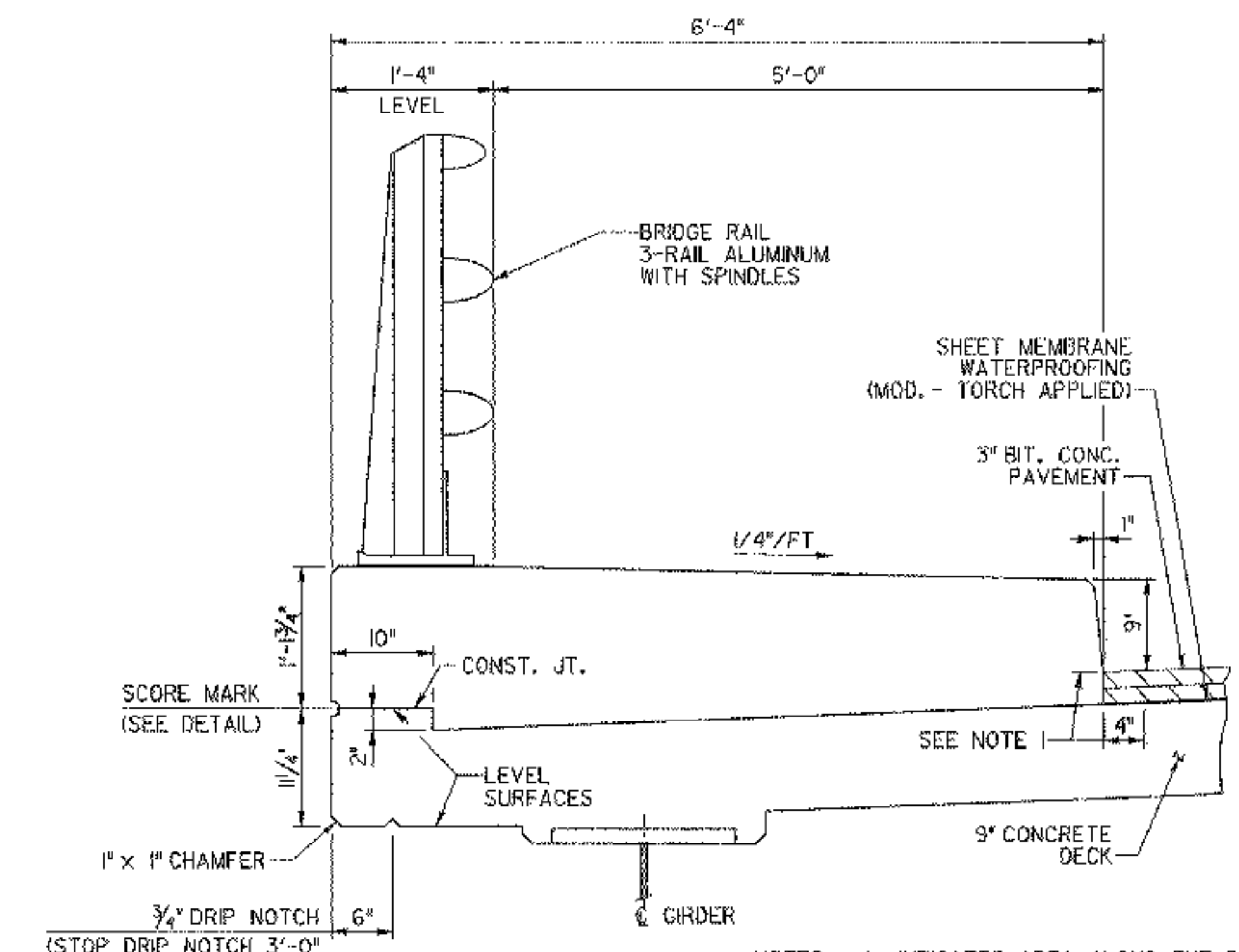
SECTION A-A



DETAIL B

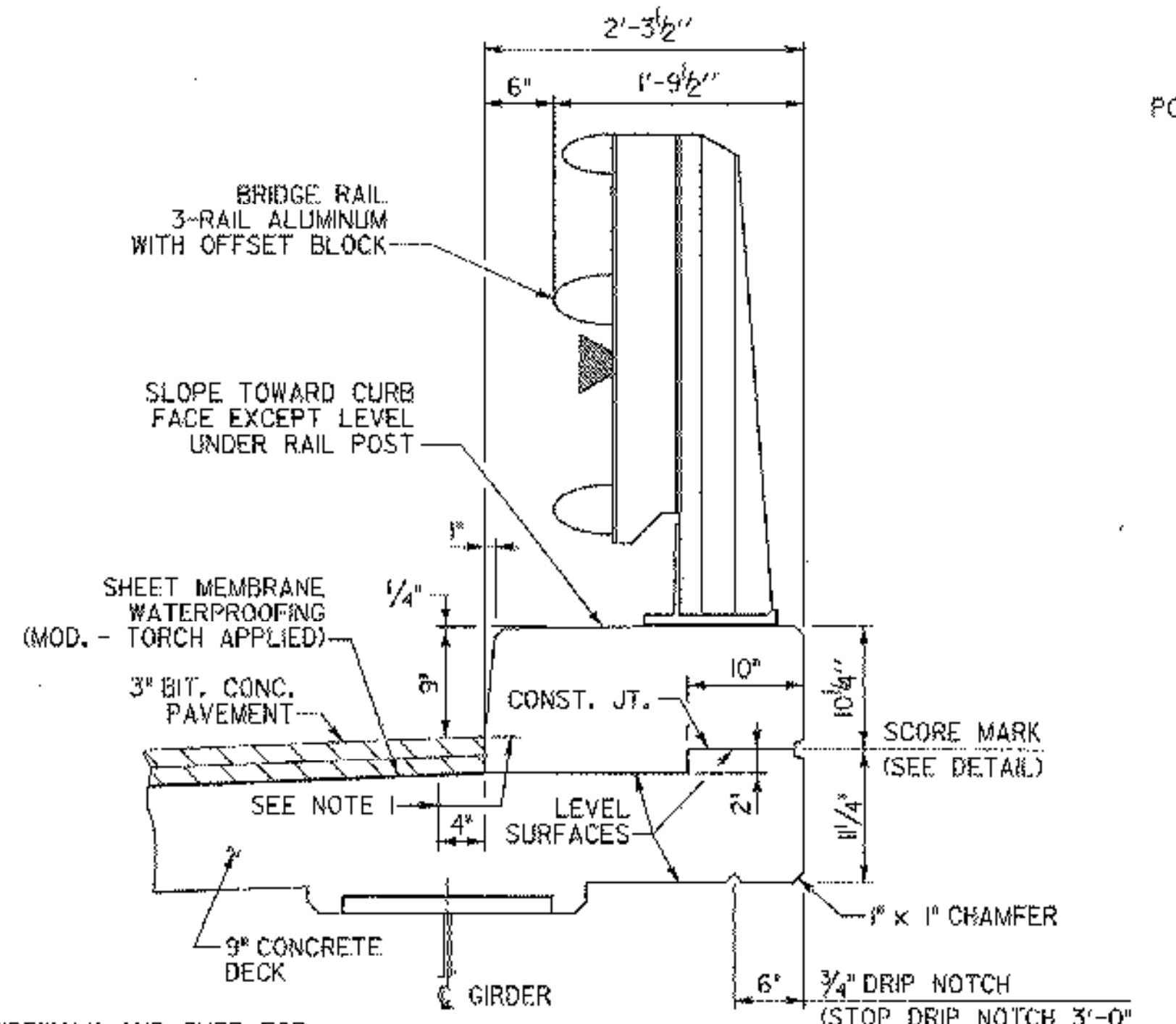
POLYURETHANE JOINT SEALER PER SUBSECTION 524.06G COLOR TO MATCH CONCRETE COST TO BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE, HIGH PERFORMANCE CLASS A

ADHERE TO THESE SURFACES



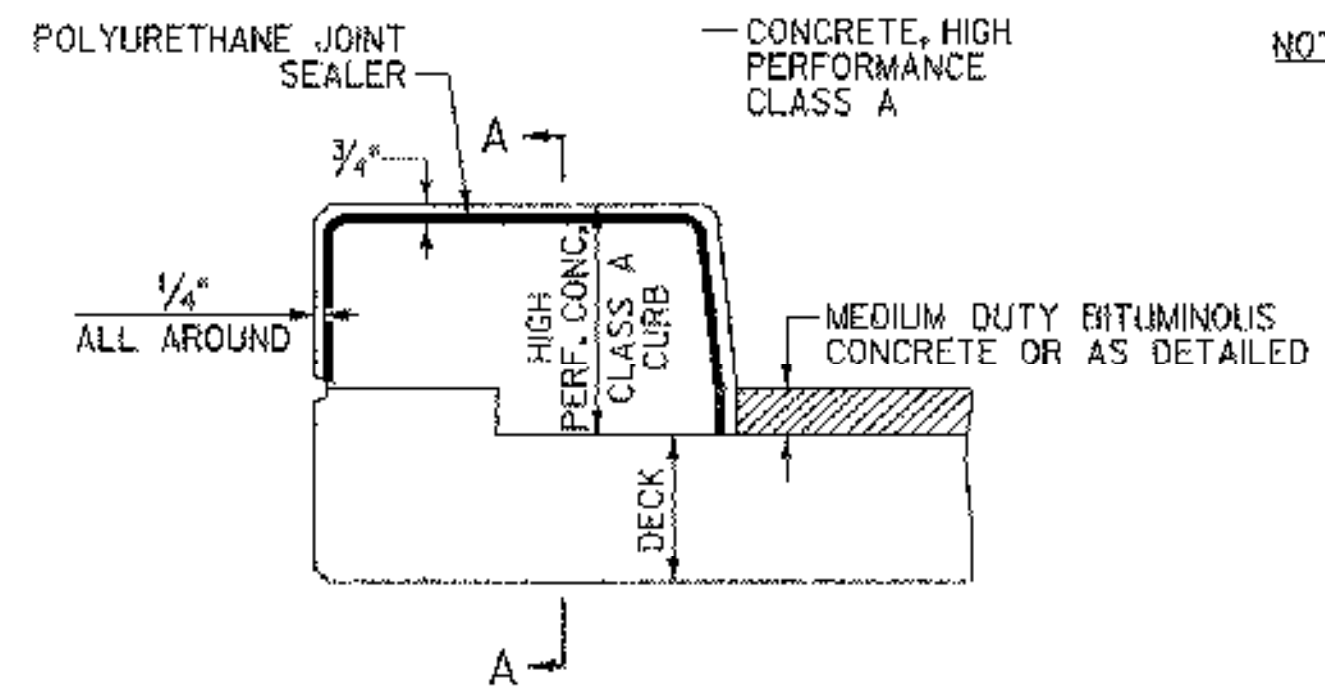
CURB SECTION - LEFT FASCIA

SCALE 1" = 1'-0"



CURB SECTION - RIGHT FASCIA

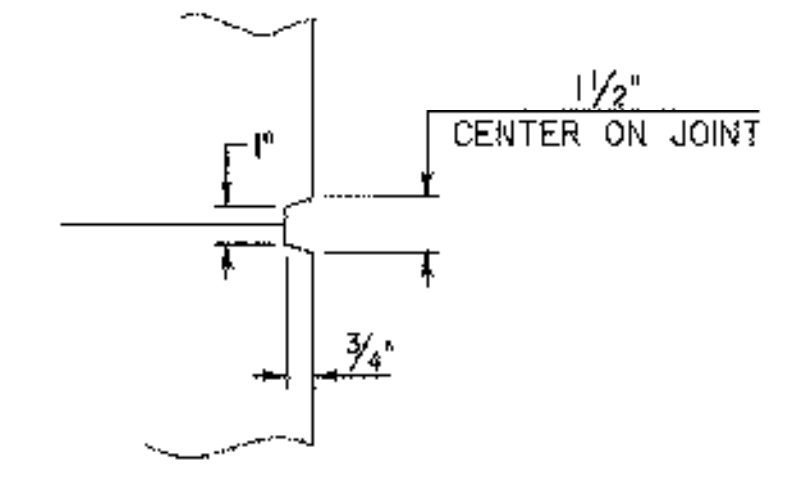
SCALE 1" = 1'-0"



TYPICAL SECTION THROUGH CONCRETE CURB CONSTRUCTION JOINT

NOT TO SCALE

- NOTES:
1. CONSTRUCTION JOINTS THROUGH CONCRETE CURBS SHALL BE SPACED MAXIMUM 15'-0" CENTER TO CENTER AND SHALL BE 1'-6" MINIMUM FROM THE CENTER OF THE NEAREST BRIDGE RAIL POST. CONCRETE SHALL BE PLACED IN ALTERNATING SECTIONS WITH A MINIMUM OF 48 HOURS DELAY BETWEEN ADJACENT POURS.
 2. LONGITUDINAL REINFORCING SHALL PASS THROUGH CONCRETE CURB CONSTRUCTION JOINTS.
 3. CONSTRUCTION JOINTS THROUGH SIDEWALKS SHALL BE SIMILAR TO CONCRETE CURB CONSTRUCTION JOINTS.



SCORE MARK DETAIL

NOT TO SCALE

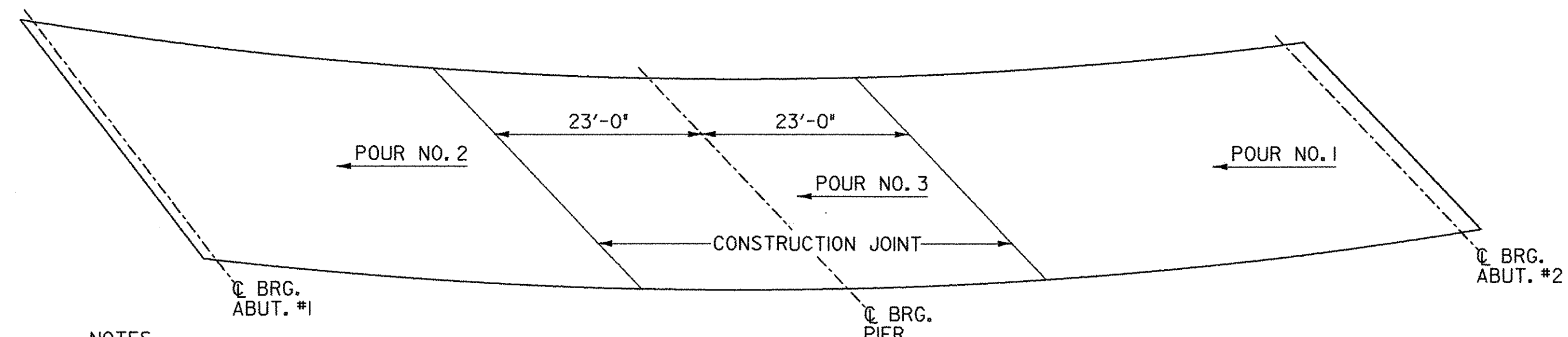
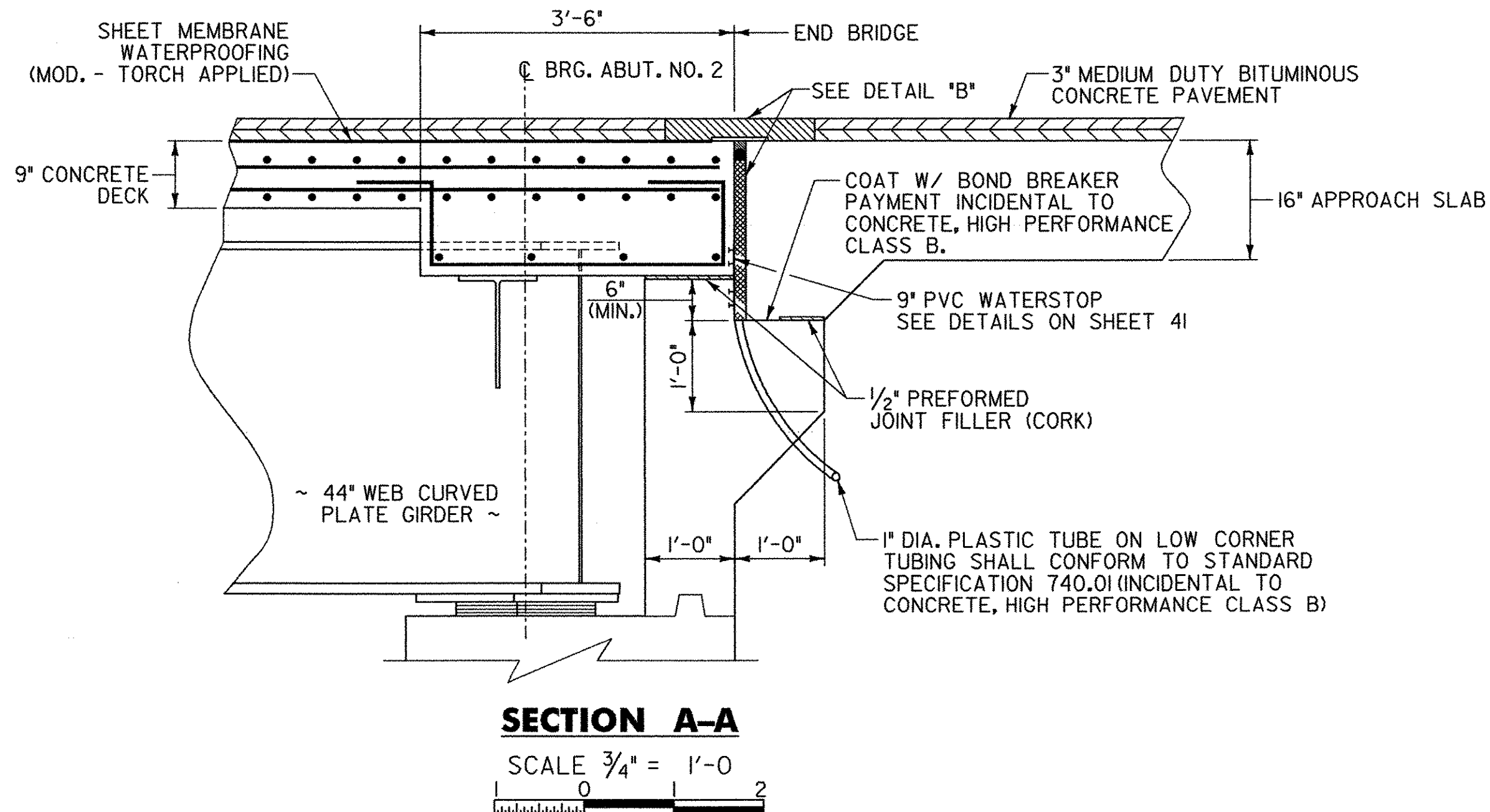
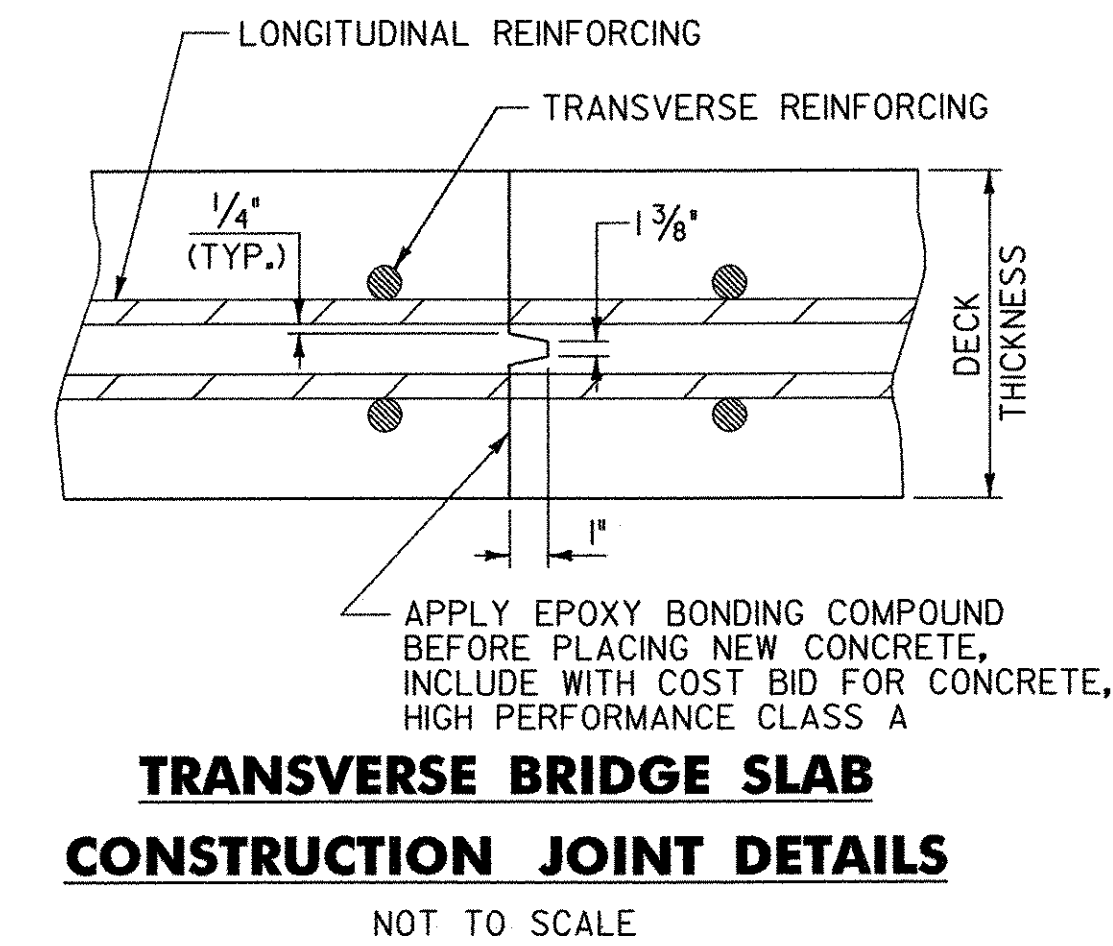
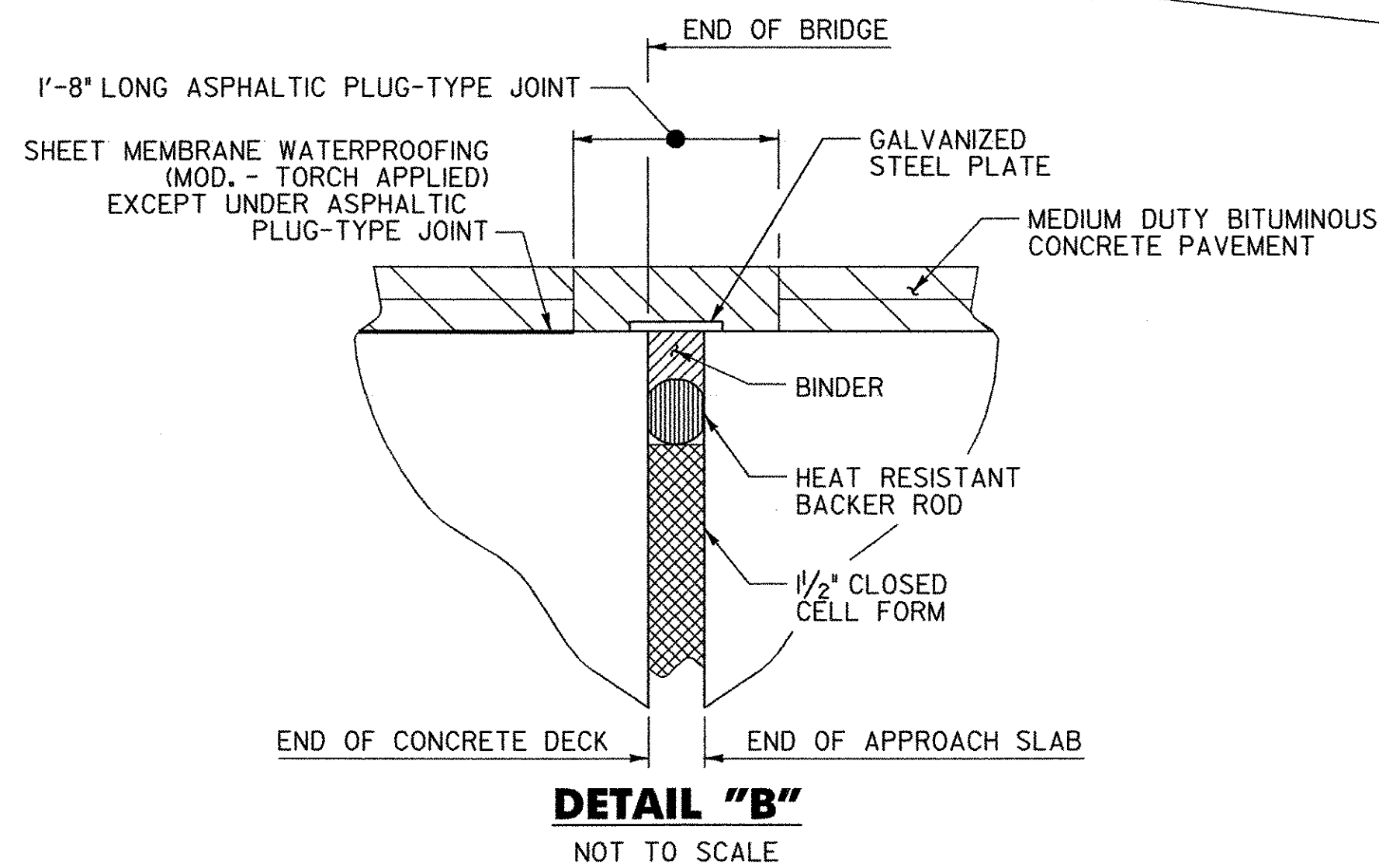
STATE OF VERMONT AGENCY OF TRANSPORTATION			
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
TYPICAL BRIDGE SECTION			
Designed By	S. BURBANK	Drawn By	S. BURBANK
Checked By	Date	Bridge Design Supervisor	
G. BOGUE	08/05	M. CHENETTE	Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn.s \...Cadd\Trans\201302typ.dgn		Plot Date:	1/12/2006
Bridge Sheet No.		Sheet 29 of 63	





DECK REINFORCING PLAN

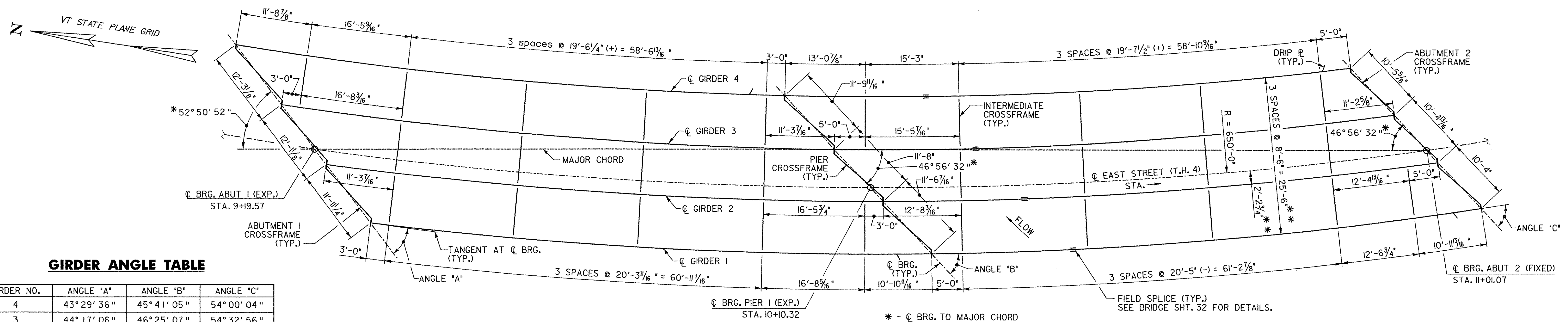
SCALE 1/8" = 1'-0"
 10 2 4 6 8



NOTES:

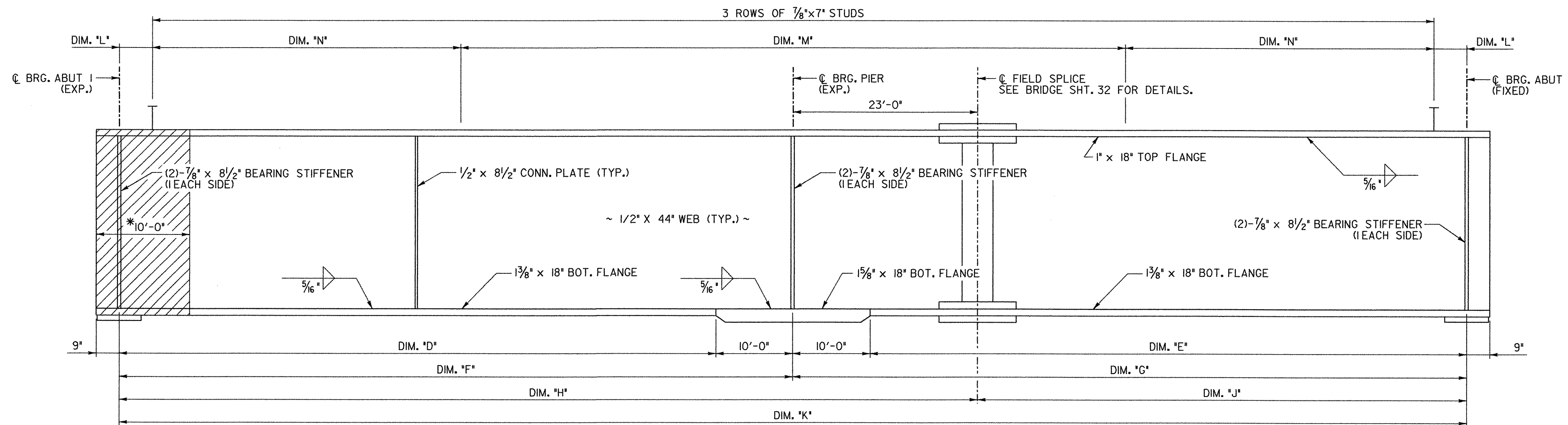
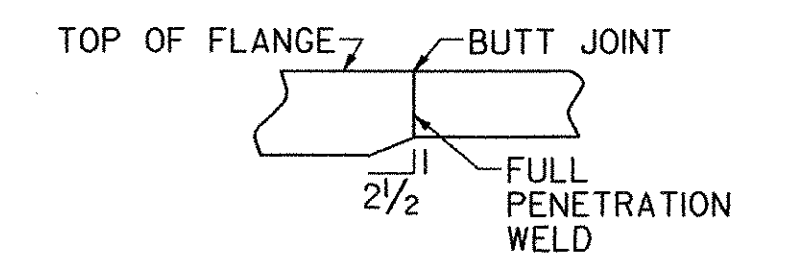
1. THE ARROWS INDICATE THE DIRECTION OF THE CONCRETE PLACEMENT.
2. THE CONCRETE SHALL BE DEPOSITED PARALLEL TO THE CENTERLINE OF BEARING SO AS TO LOAD THE GIRDERS EQUALLY.
3. TRANSVERSE BRIDGE SLAB CONSTRUCTION JOINTS, AS SHOWN ON THIS SHEET, SHALL BE USED BETWEEN ADJACENT DECK POURS.
4. IF APPROVED BY THE ENGINEER, POURS NO. 1 AND NO. 2 MAY BE POURED THE SAME DAY PROVIDED THAT THE MAXIMUM TIME SPAN OF THE POURS IS EIGHT HOURS, THE INDICATED POURING SEQUENCE SHALL BE FOLLOWED AND A RETARDING ADMIXTURE USED SO THAT THE DECK CONCRETE WILL NOT SET UP UNTIL ALL THE CONCRETE HAS BEEN POURED. THE RETARDING ADMIXTURE SHALL BE INCIDENTAL TO ITEM 501.33, CONCRETE HIGH PERFORMANCE CLASS A.
5. A NINETY-SIX (96) HOUR DELAY BETWEEN THE COMPLETION OF ONE DAY'S POUR AND THE BEGINNING OF ANY OTHER POUR SHALL BE OBSERVED.
6. THE CONTRACTOR MAY SUBMIT, IN WRITING, AN ALTERNATE POUR SEQUENCE TO THE ENGINEER FOR REVIEW.

STATE OF VERMONT AGENCY OF TRANSPORTATION			
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
DECK REINFORCING PLAN			
Designed By	S. BURBANK	Drawn By	S. BURBANK
Checked By	T. KNIGHT	Date	08/05
		Bridge Design Supervisor	M. CHENETTE Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn: ... \Cadd\Trans\z01\302\deck.dgn		Plot Date:	03/30/2006
Bridge Sheet No.		Sheet	30 of 63



GIRDER ANGLE TABLE

GIRDER NO.	ANGLE "A"	ANGLE "B"	ANGLE "C"
4	43° 29' 36"	45° 41' 05"	54° 00' 04"
3	44° 17' 06"	46° 25' 07"	54° 32' 56"
2	45° 02' 44"	47° 07' 31"	55° 04' 43"
1	45° 46' 36"	47° 48' 21"	55° 35' 30"



NOTES:

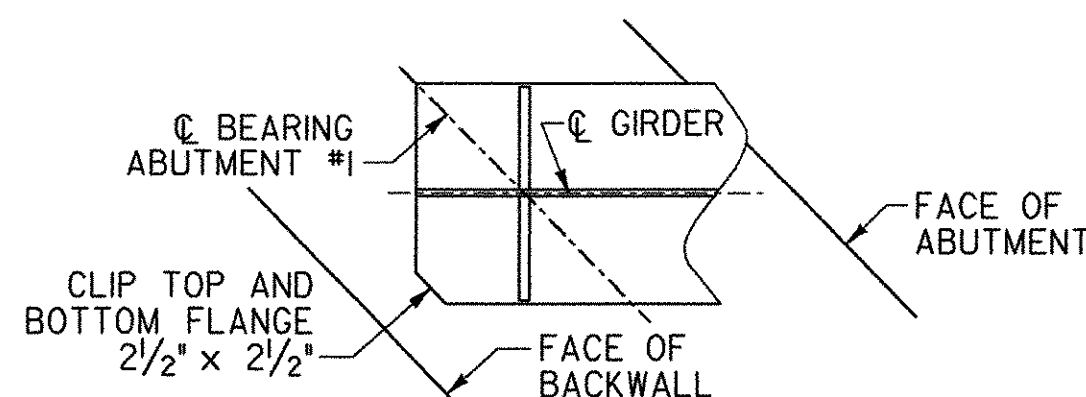
- GIRDER DIMENSIONS ARE ALONG C. OF GIRDER.
- FOR CAMBER AND DEAD LOAD DEFLECTION DIAGRAMS, SEE SHEET 32.
- THE INTERMEDIATE CROSSFRAMES ARE PLACED ALONG LINES THAT ARE RADIAL TO THE CURVE.
- THE INTERMEDIATE CROSSFRAMES' SPACING NOT SHOWN ALONG GIRDERS 2 AND 3 ARE PROPORTIONAL TO THE SPACINGS GIVEN ALONG GIRDERS 1 AND 4.
- FOR CROSSFRAME DETAILS, SEE SHEET 33.
- THE ENDS OF GIRDERS SHALL BE VERTICAL UNDER DEAD LOAD.
- THE BEARING STIFFENERS SHALL BE PERPENDICULAR TO THE FLANGES AND WEB IN THE FINAL POSITION.
- CHARPY V-NOTCH TESTING REQUIRED ON ALL STRUCTURAL STEEL, EXCEPT IN THE DOWNSPOUTS.

* - THE EXPANSION ENDS OF THE GIRDERS SHALL BE PAINTED AND GREASED SEE NOTE 27 ON SHEET 28

GIRDER ELEVATION
NOT TO SCALE

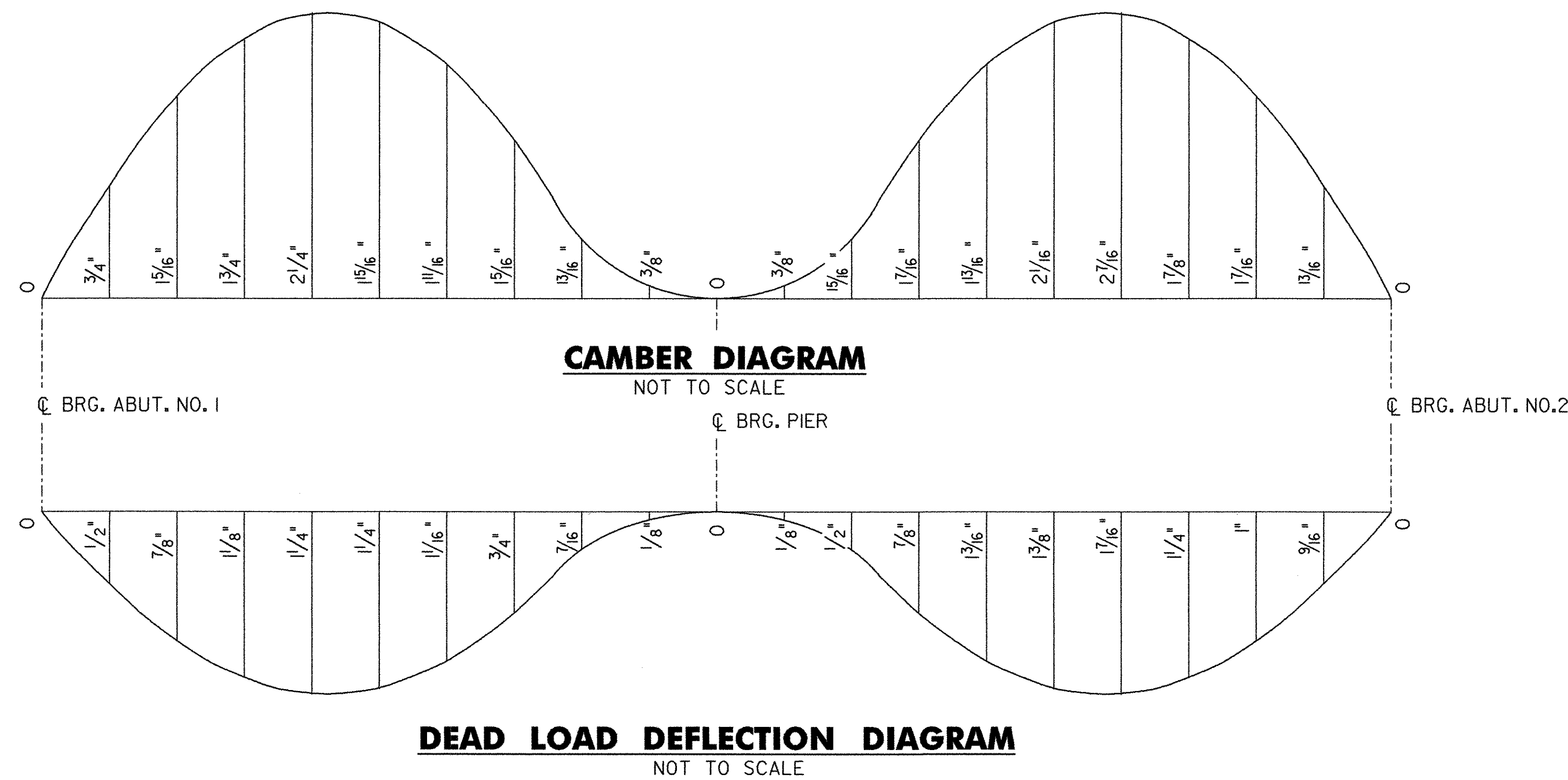
GIRDER DIMENSION TABLE

GIRDER NO.	RADIUS	DIM "D"	DIM "E"	DIM "F"	DIM "G"	DIM "H"	DIM "J"	DIM "K"	DIM "L"	DIM "M"	DIM "N"
4	635.229'	79'-9 1/4"	82'-2 7/16"	89'-9 1/4"	92'-2 7/16"	112'-9 1/4"	69'-2 7/16"	181'-11 1/16"	3 3/8" (-)	105 SPACES @ 9" = 78'-9"	56 SPACES @ 11" = 51'-4"
3	643.729'	80'-3 5/16"	81'-4 1/8"	90'-3 5/16"	91'-4 1/8"	113'-3 5/16"	68'-4 1/8"	181'-8 1/16"	1 1/2" (+)	105 SPACES @ 9" = 78'-9"	56 SPACES @ 11" = 51'-4"
2	652.229'	80'-10 7/8"	80'-6 7/16"	90'-10 7/8"	90'-6 7/16"	113'-10 7/8"	67'-6 7/16"	181'-5 5/16"	4 5/8" (+)	104 SPACES @ 9" = 78'-0"	56 SPACES @ 11" = 51'-4"
1	660.729'	81'-6 1/16"	79'-9 9/16"	91'-6 1/16"	89'-9 9/16"	114'-6 1/16"	66'-9 9/16"	181'-3 1/2"	3 3/4" (+)	104 SPACES @ 9" = 78'-0"	56 SPACES @ 11" = 51'-4"

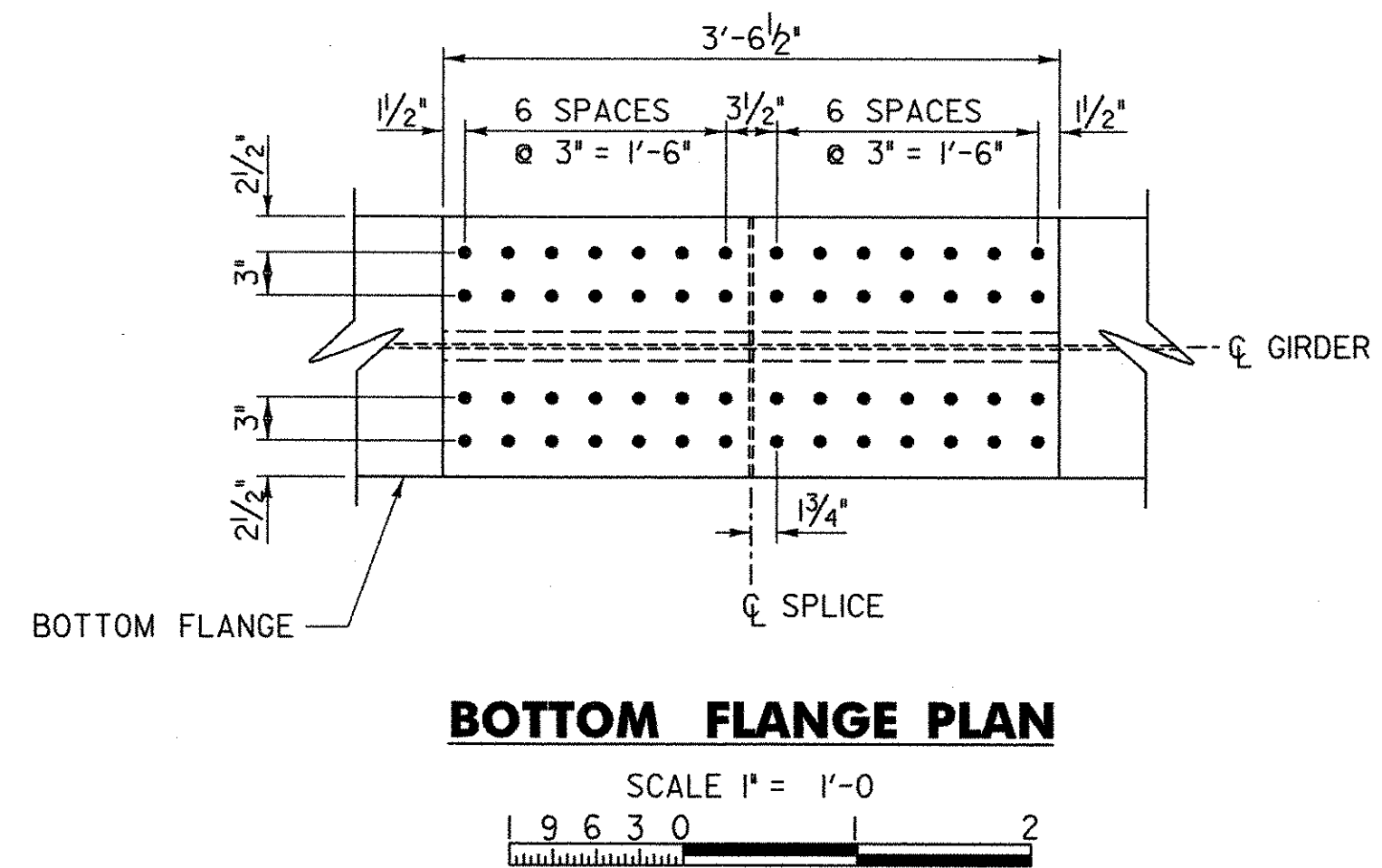
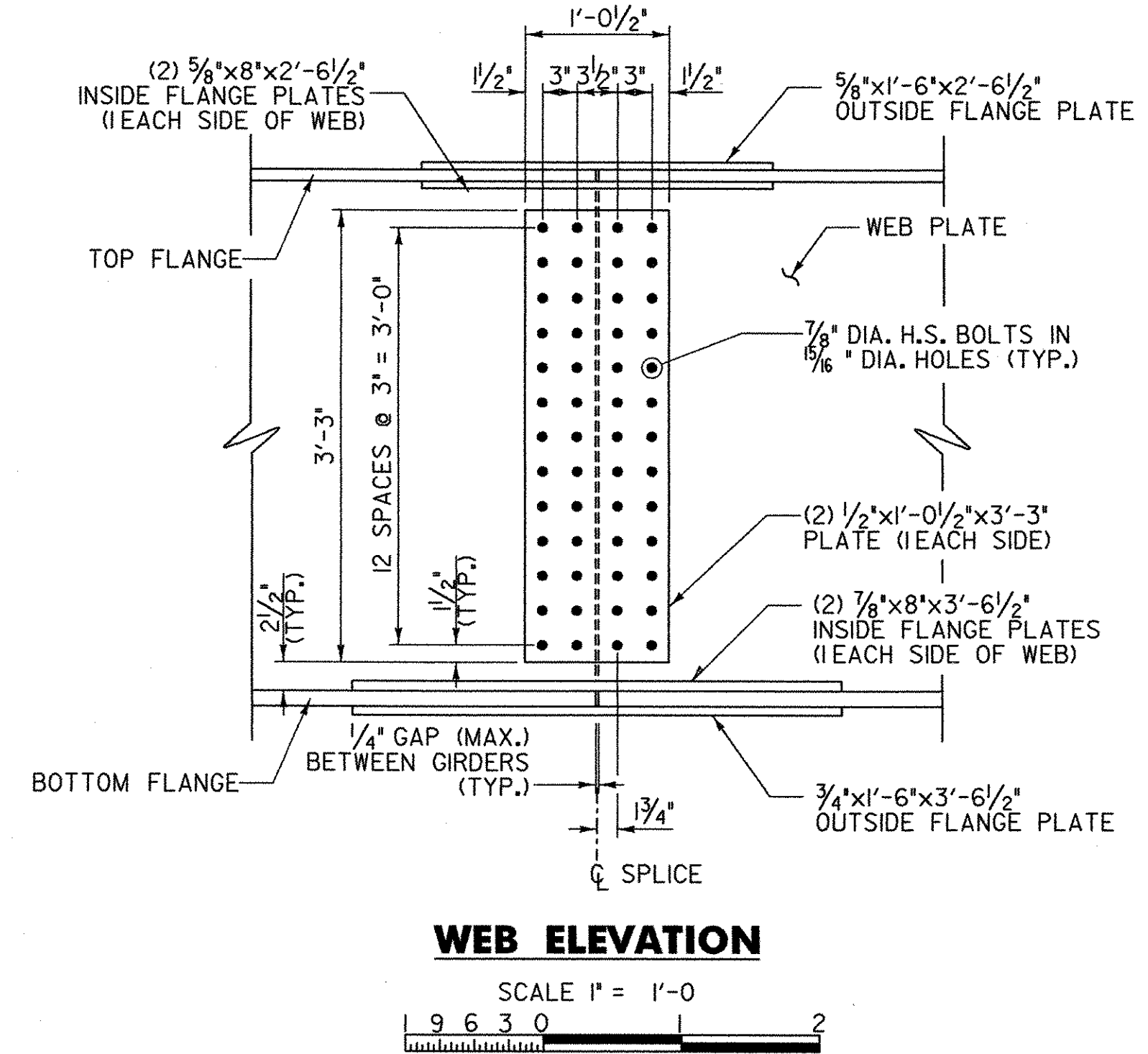
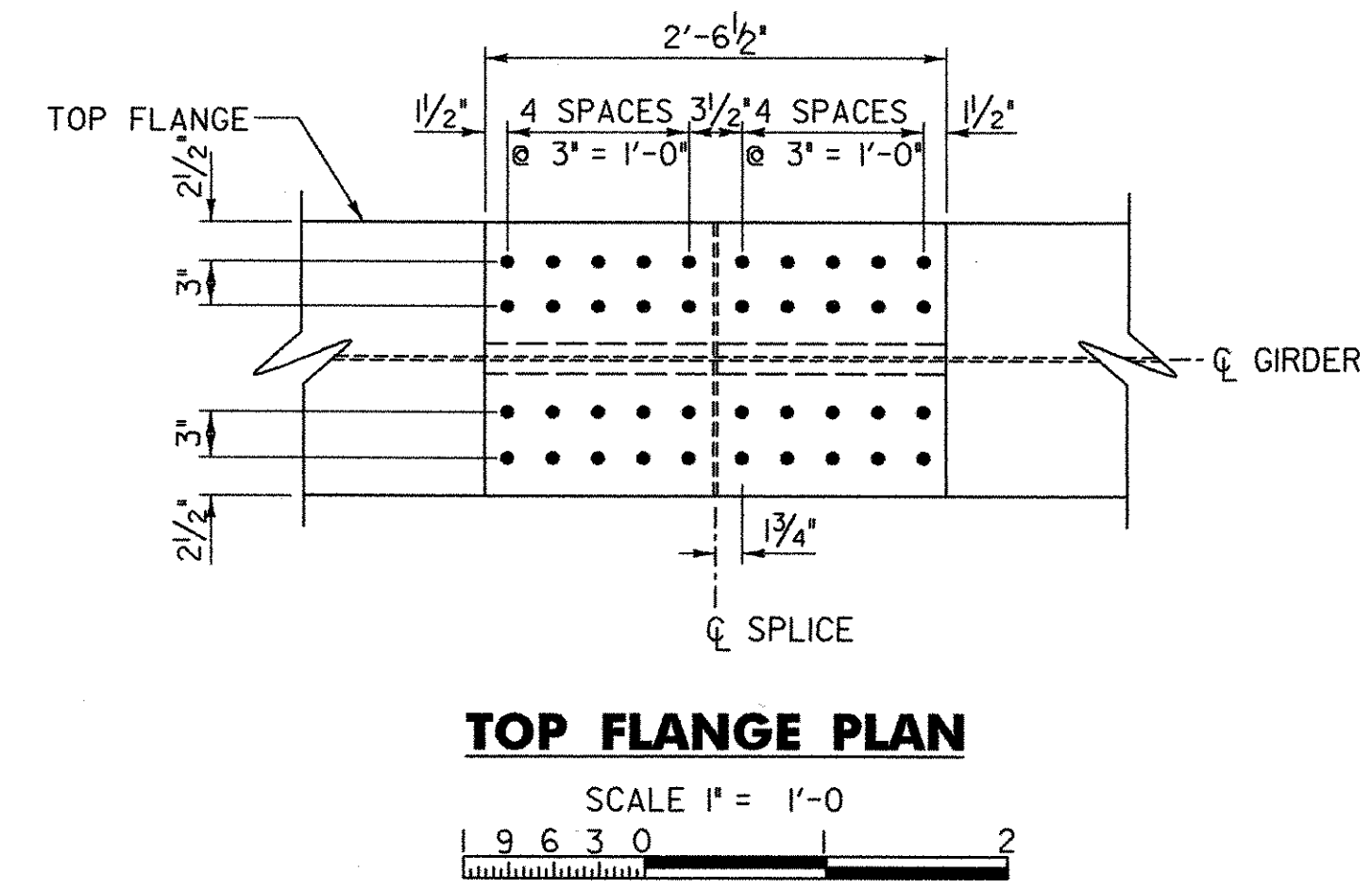


STATE OF VERMONT
AGENCY OF TRANSPORTATION

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
FRAMING PLAN			
Designed By	G. BOGUE	Drawn By	T. KNIGHT
Checked By	Date	Bridge Design Supervisor	
S. BURBANK	03/06	M. CHENETTE	Date 03/06
PROJECT	HUNTINGTON		PROJECT NO.
			BRO 1445 (29)
DH Dgn.: ...Cadd\Trans\201302\fra.dgn	Plot Date: 3/28/2006		
Bridge Sheet No.			Sheet 31 of 63



- NOTES:
1. CAMBER AND DEAD LOAD MEASUREMENTS ARE GIVEN IN INCHES AT SPAN TENTH POINTS.
 2. DEAD LOAD DEFLECTIONS SHOWN ARE FOR ALL DEAD LOADS AND SUPERIMPOSED DEAD LOADS INCLUDING BEAM AND DIAPHRAGM WEIGHTS. DESIGN DEAD LOAD DEFLECTIONS SHOWN ARE BASED ON GIRDER 2. ACTUAL DEAD LOAD DEFLECTIONS FOR OTHER GIRDERS VARY SLIGHTLY.



**STATE OF VERMONT
AGENCY OF TRANSPORTATION**

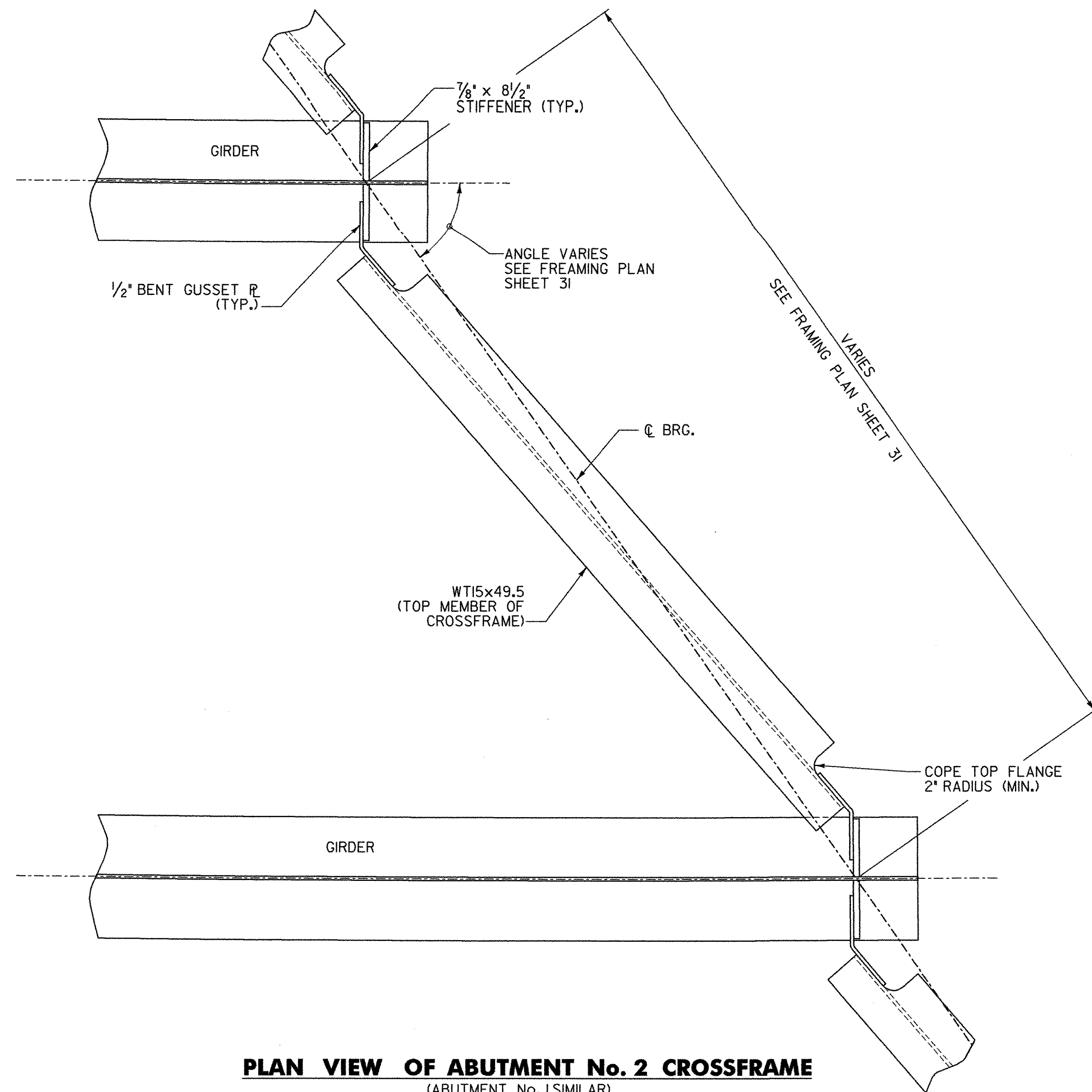
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	

EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

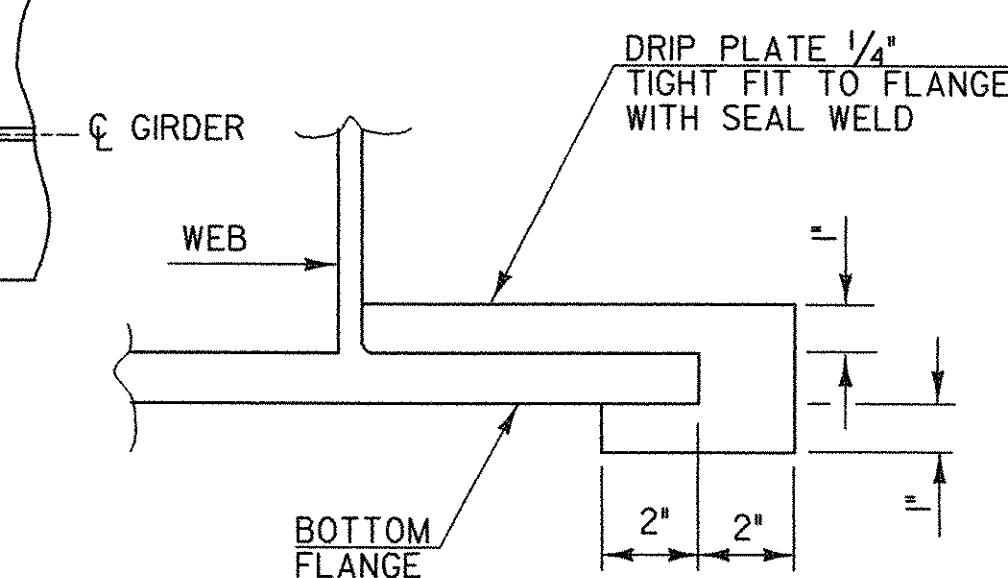
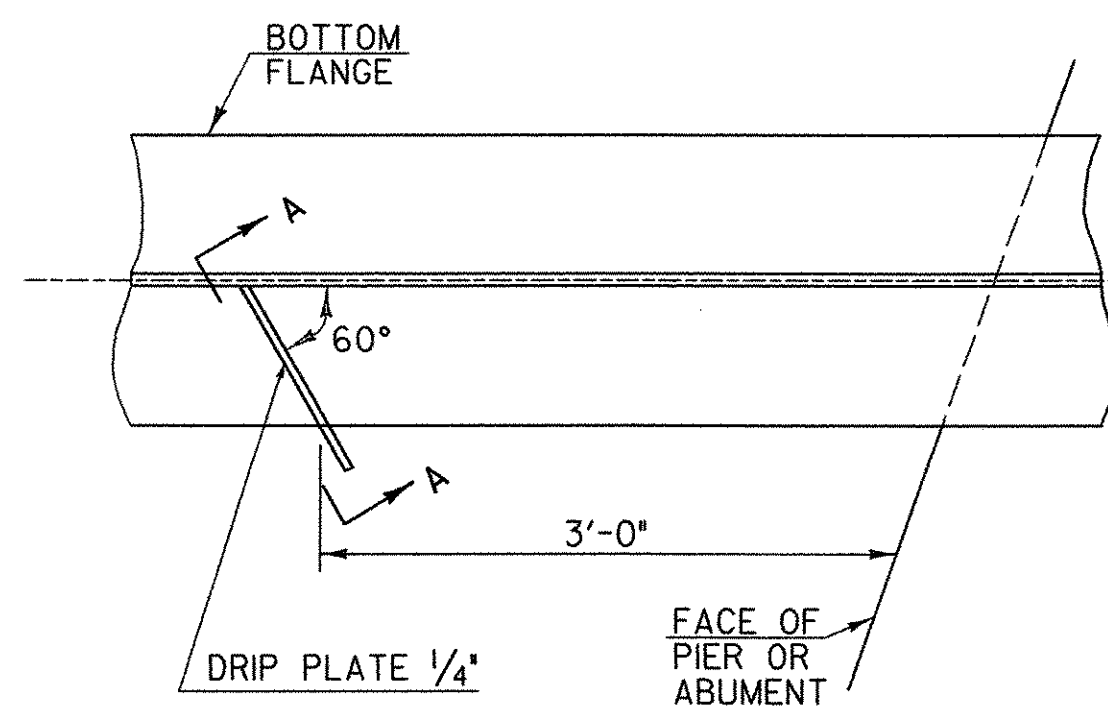
MISCELLANEOUS GIRDER DETAILS

Designed By	G. BOGUE	Drawn By	J. SOTER
Checked By	Date	Bridge Design Supervisor	Date
S. BURBANK	08/05	M. CHENETTE	09/05

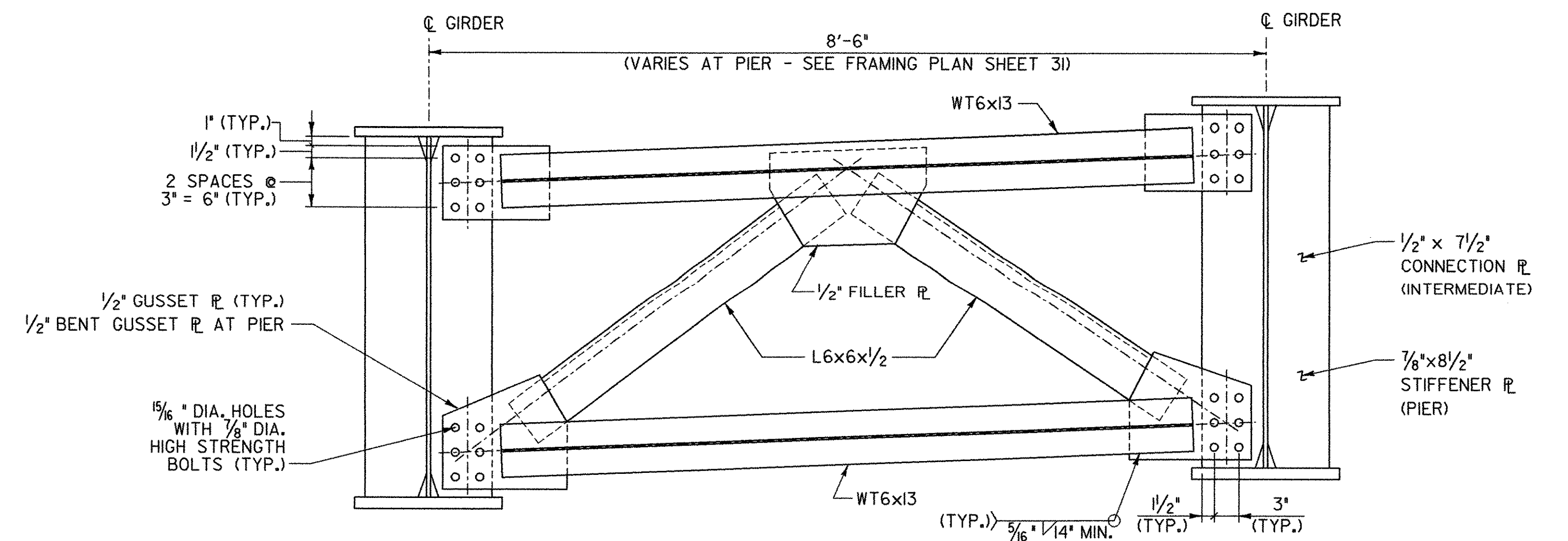
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
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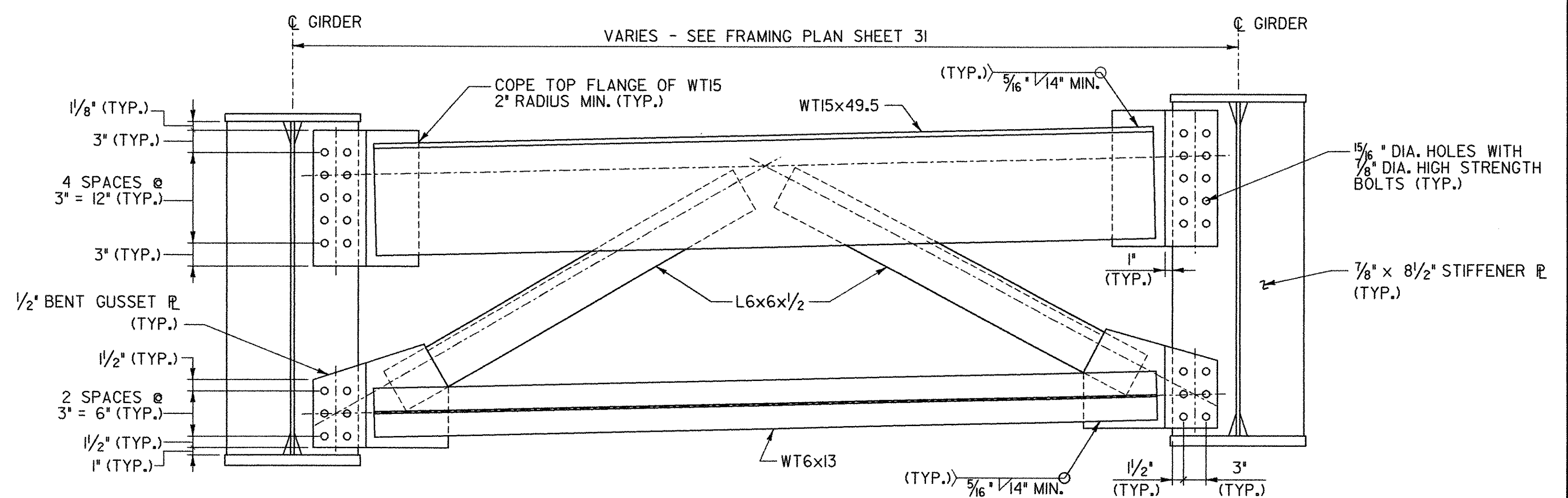
SCALE 1" = 1'-0"
1 9 6 3 0 2



NOTE: DRIP PLATES SHALL BE PLACED ON OUTSIDE EDGE OF FASCIA GIRDERS ON THE HIGH SIDE OF ALL PIERS AND ABUTMENTS OR AS INDICATED ON PROJECT PLANS.



SCALE 1" = 1'-0"
1 9 6 3 0 2



SCALE 1" = 1'-0"
1 9 6 3 0 2

**STATE OF VERMONT
AGENCY OF TRANSPORTATION**

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	

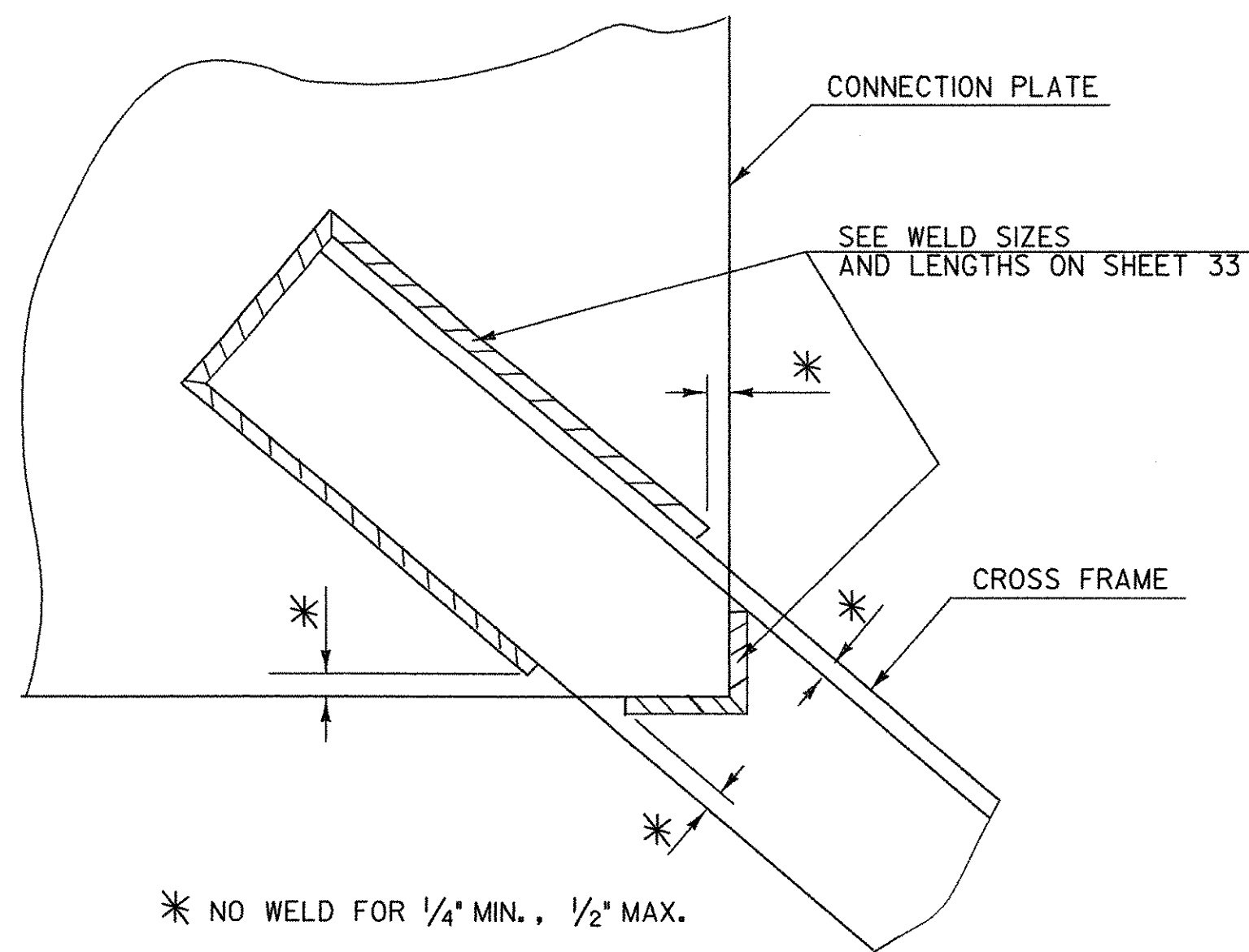
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

CROSSFRAME DETAILS

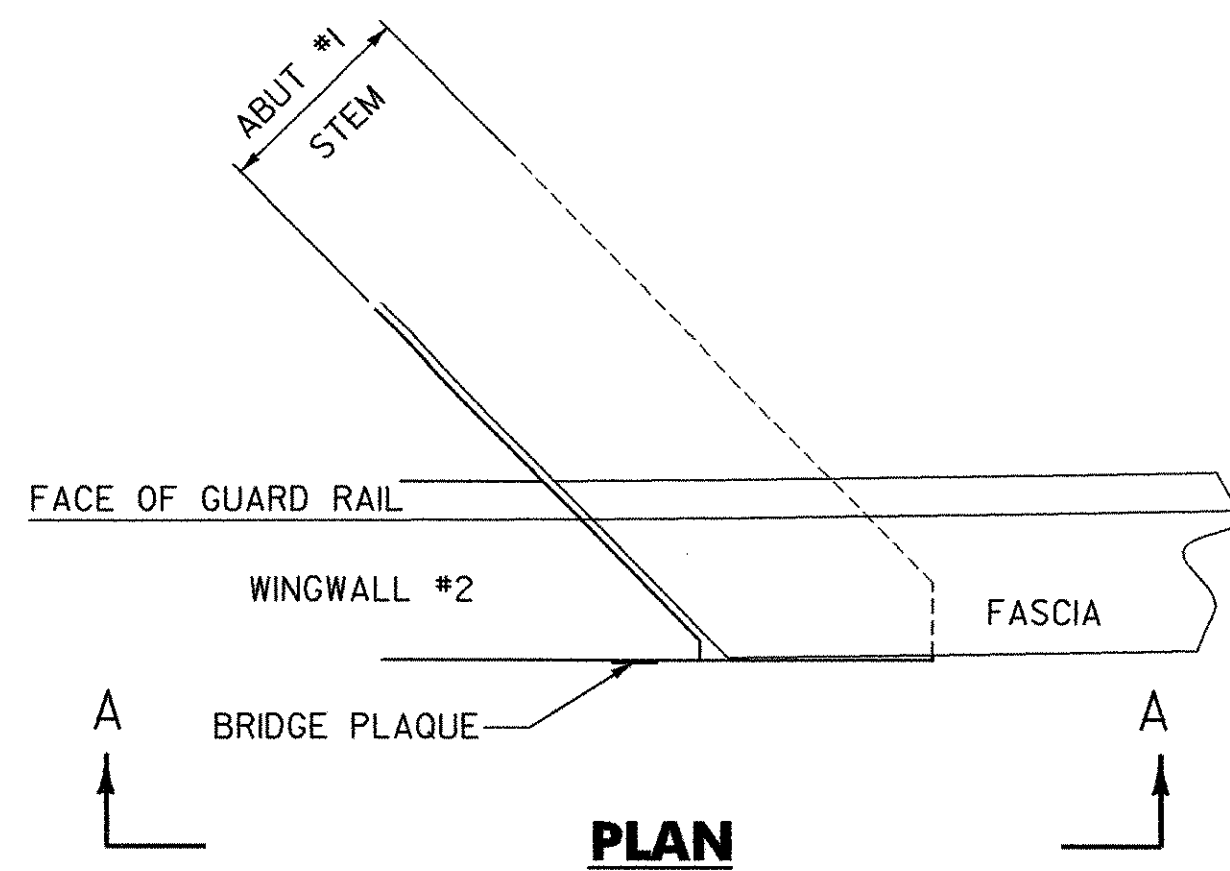
Designed By	G. BOGUE	Drawn By	J. SOTER
Checked By	S. BURBANK	Date	06/05
		Bridge Design Supervisor	M. CHENETTE
		Date	09/05

PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
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DH Dgn: ...Cadd\Trans\201302xfdet.dgn Plot Date: 1/12/2006

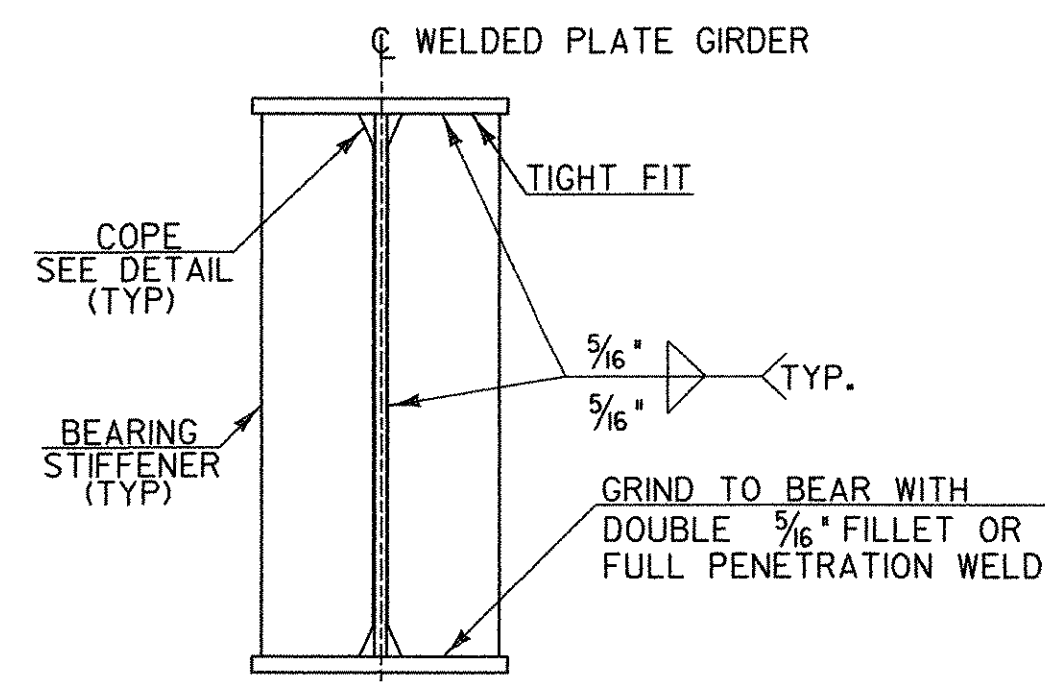


WELD LOCATION DETAIL AT CROSS FRAMES

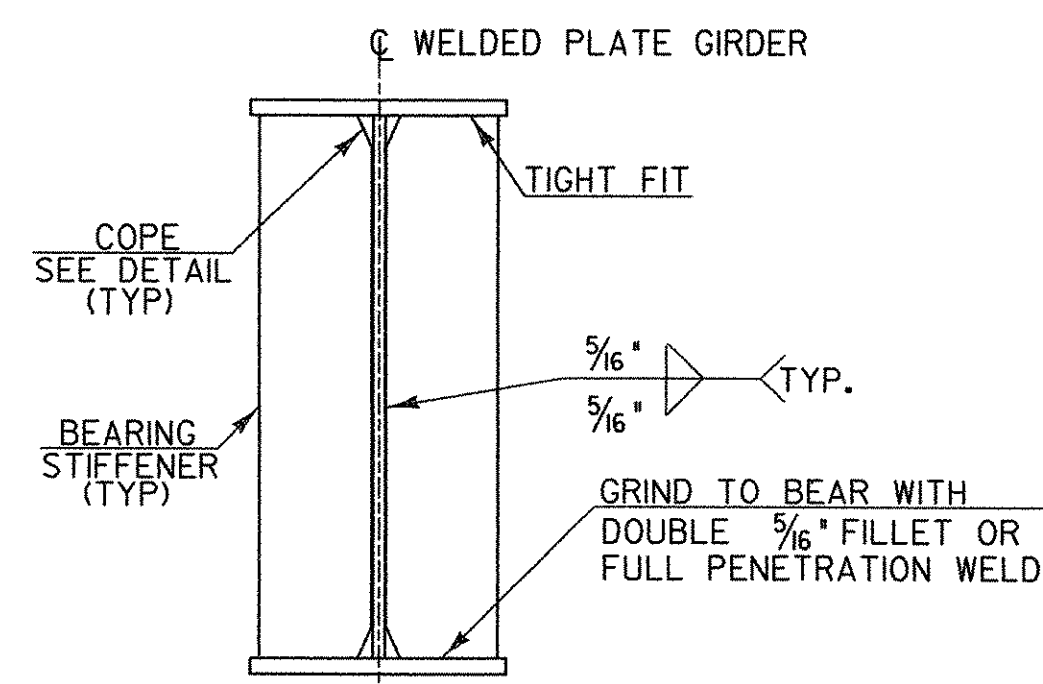


LOCATE BRIDGE PLAQUE

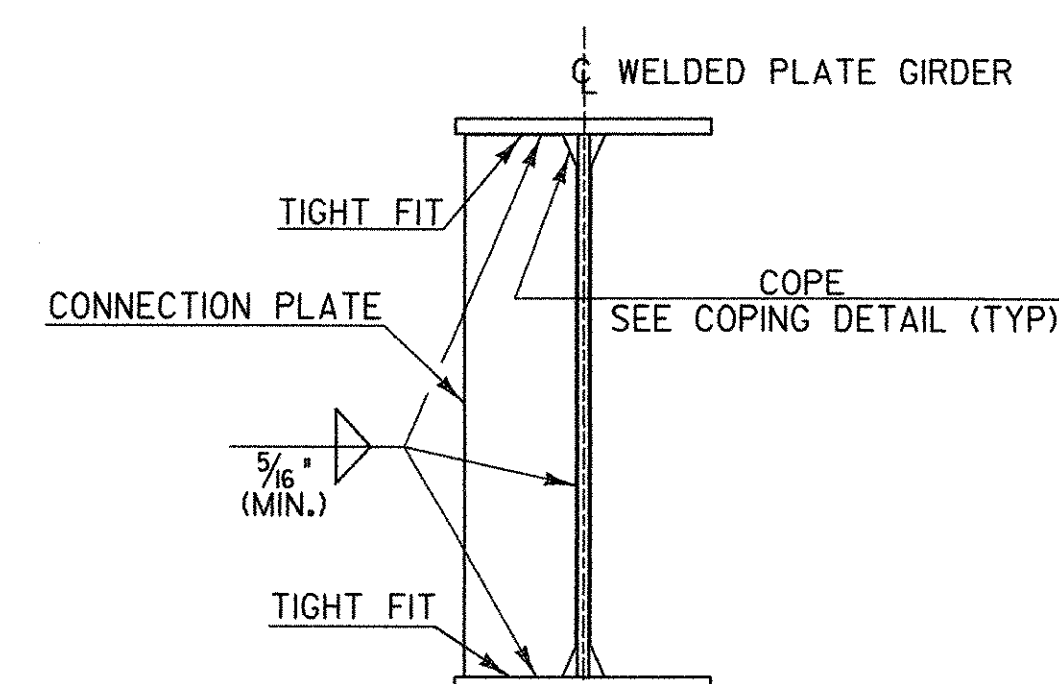
THE BRIDGE PLAQUE WILL BE SUPPLIED BY THE AGENCY OF TRANSPORTATION AND SHALL BE INSTALLED BY THE CONTRACTOR AT ABUTMENT #1 ON THE RIGHT SIDE AS SHOWN OR AS DIRECTED BY ENGINEER.



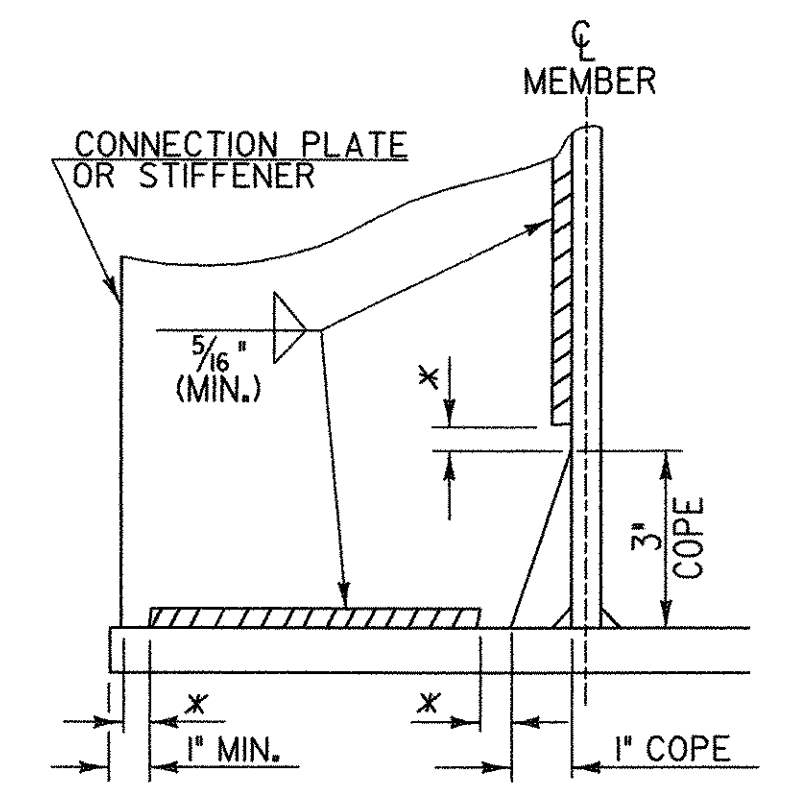
ABUTMENT BEARING STIFFENERS FOR WELDED PLATE GIRDERS



PIER BEARING STIFFENERS FOR WELDED PLATE GIRDERS

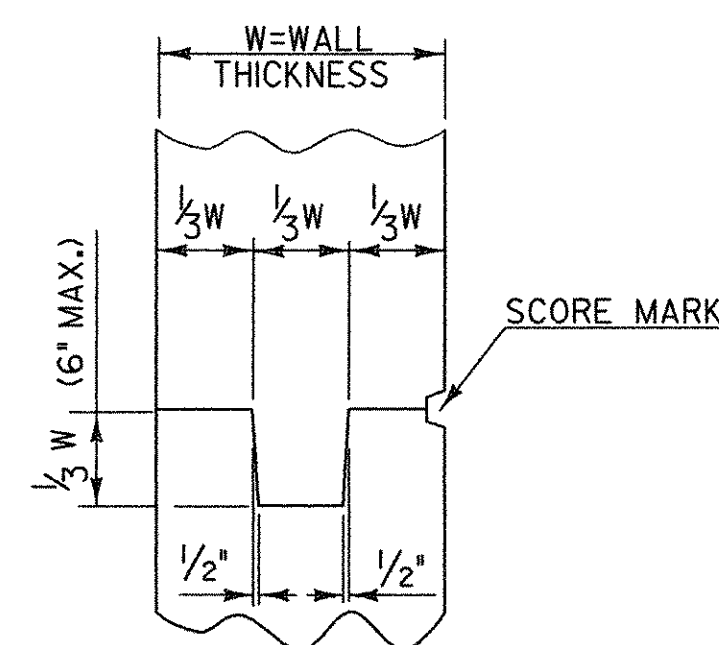


INTERMEDIATE CONNECTION PLATES FOR WELDED PLATE GIRDERS

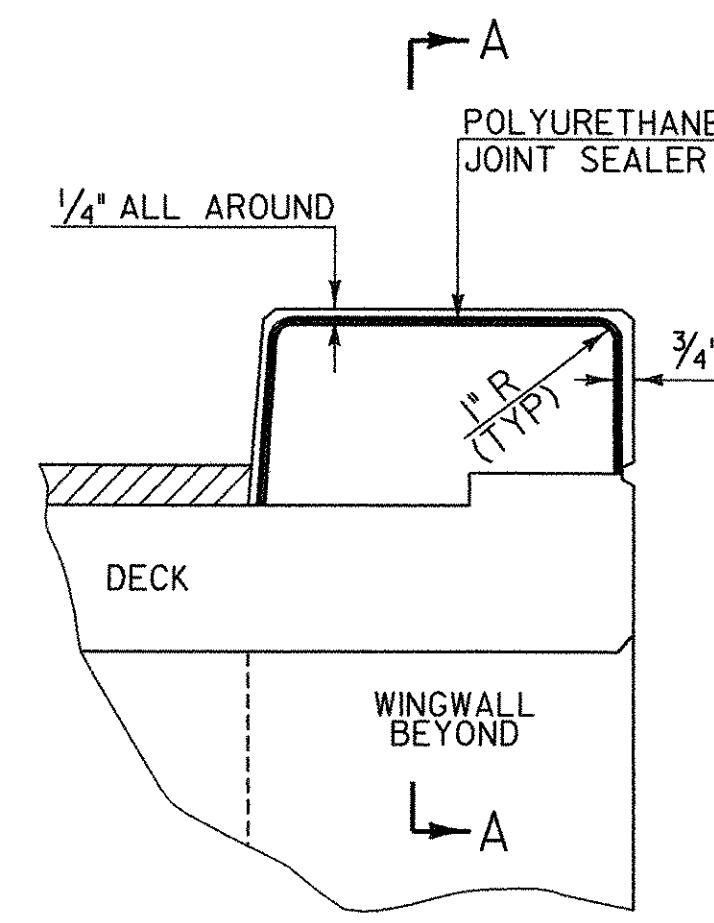


WELD TERMINATION AND COPING DETAILS FOR STEEL MEMBERS

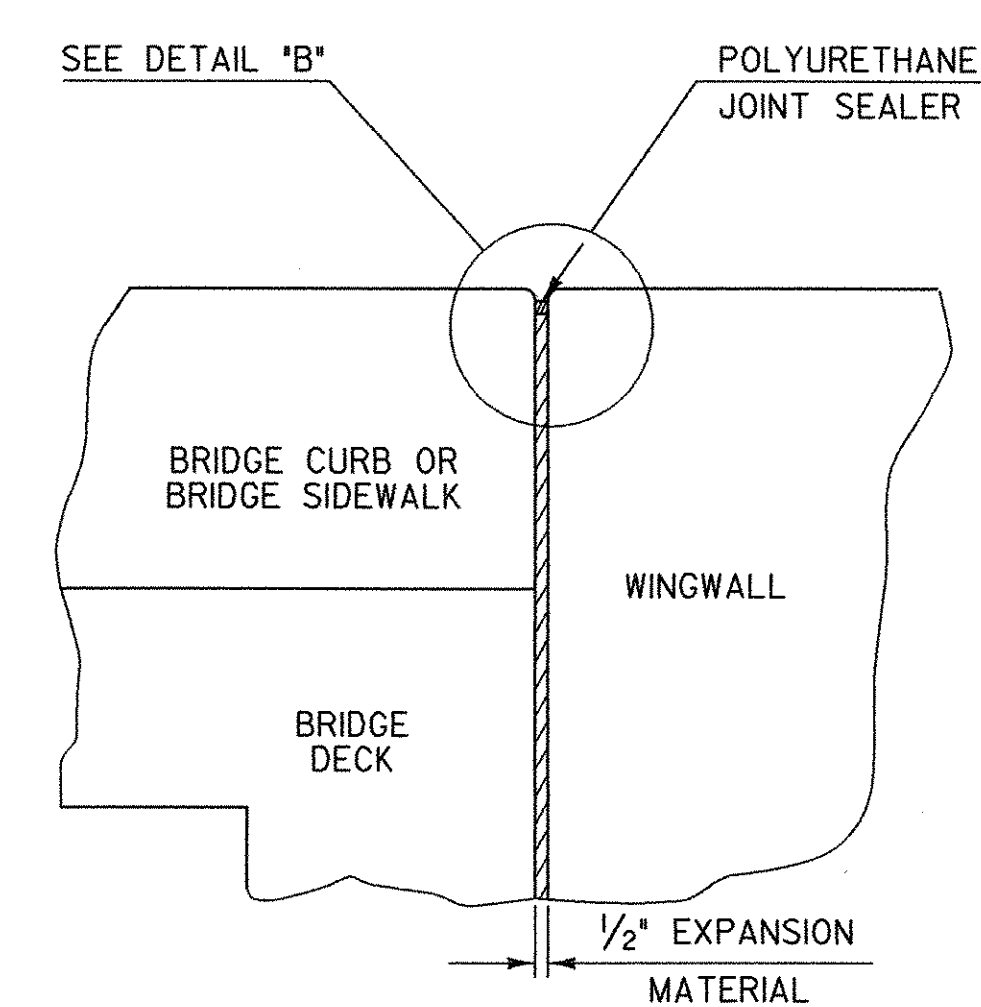
*NO WELD FOR 3/8" MIN., 7/8" MAX. (EXCEPT MUST MAINTAIN 1" MINIMUM FROM EDGE OF FLANGE)



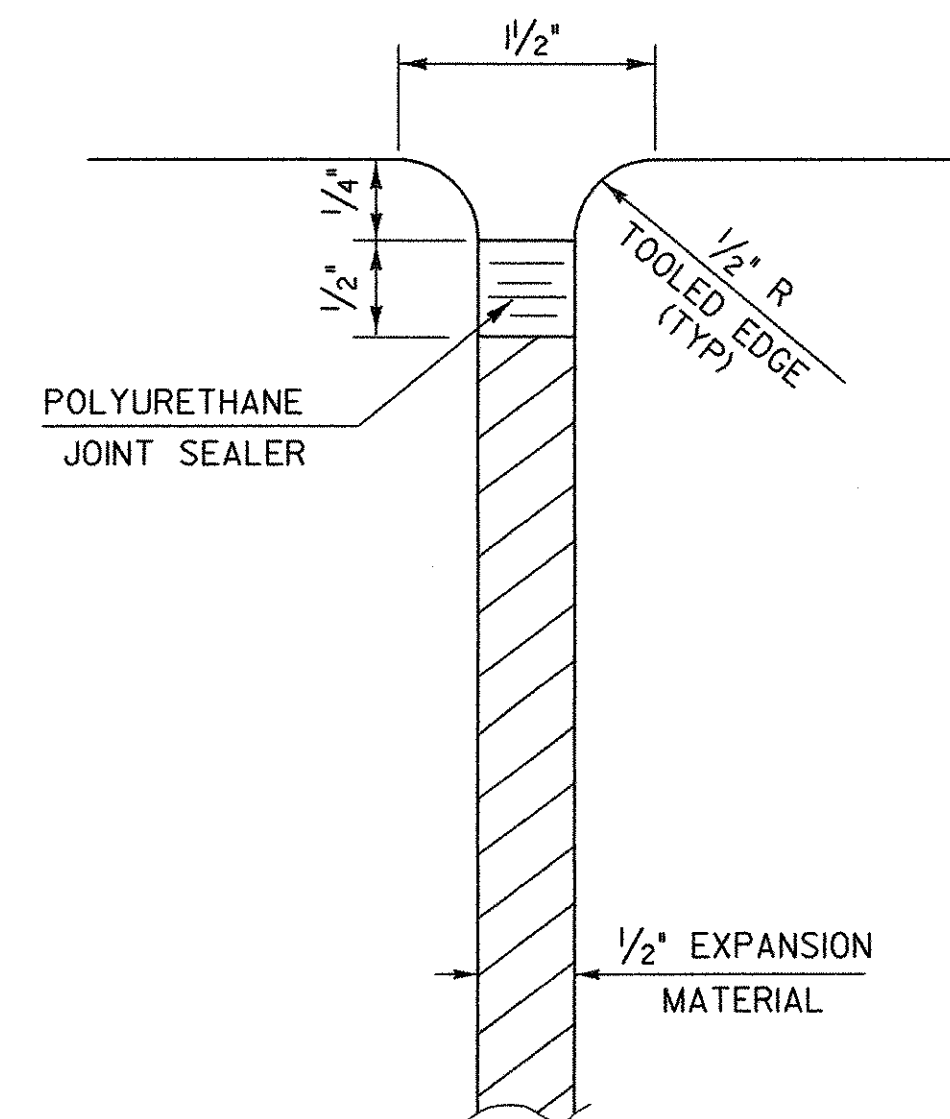
TYPICAL CONCRETE CONSTRUCTION JOINT



TYPICAL SECTION THROUGH CURB OR SIDEWALK JOINT AT FIXED END OF BRIDGE

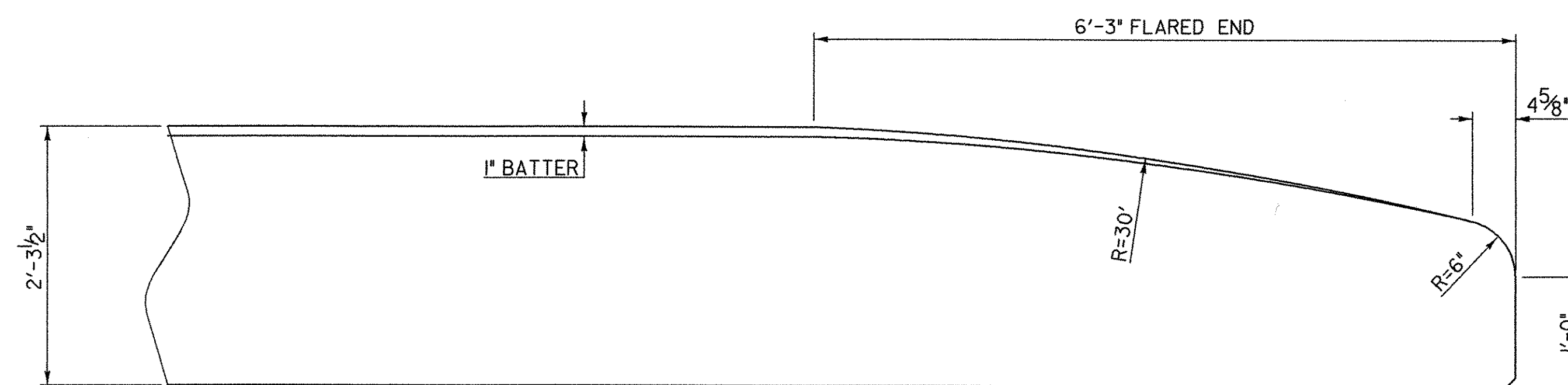


SECTION A - A



DETAIL "B"

FOR DETAILS OF POLYURETHANE JOINT SEALER SEE SUBSECTION 524.06



FLARED END FOR WINGWALL CURB

CONCRETE CURB
HIGH PERFORMANCE CLASS A

**STATE OF VERMONT
AGENCY OF TRANSPORTATION**

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	

EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

MISCELLANEOUS DETAILS

Designed By	G. BOGUE	Drawn By	J. SOTER
Checked By	S. BURBANK	Bridge Design Supervisor	M. CHENETTE
Date	06/05	Date	09/05

PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
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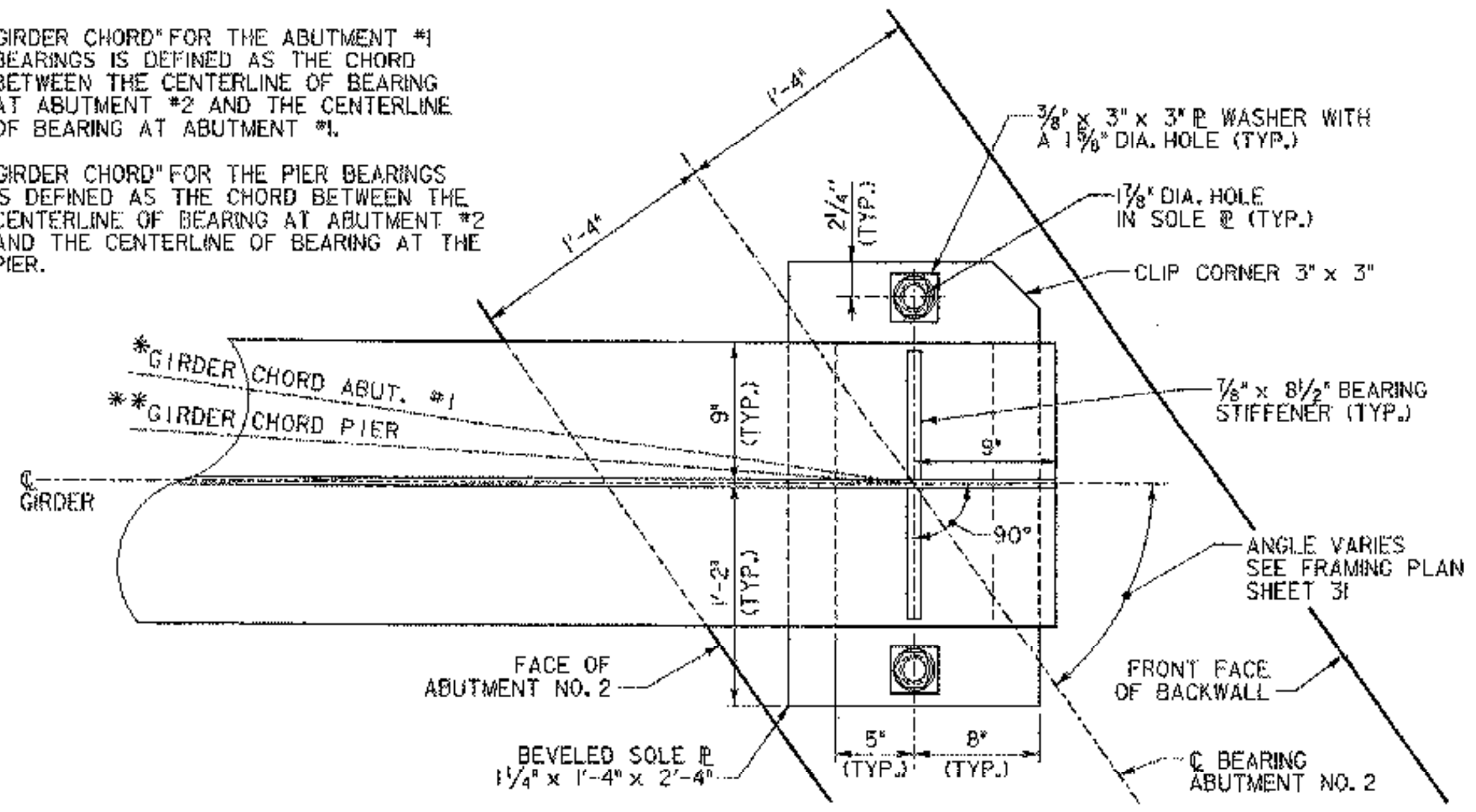
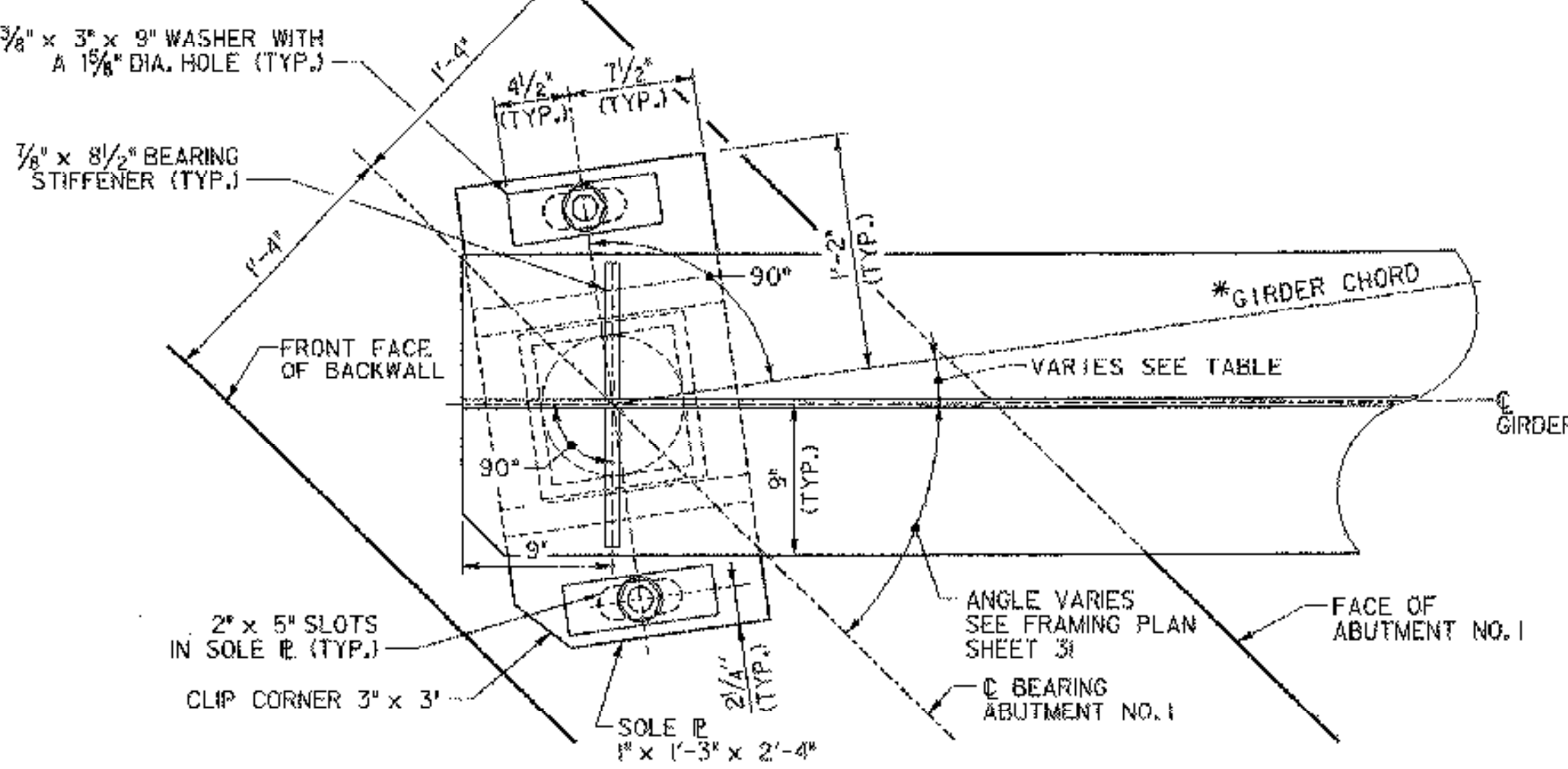
DH Dgn: ... \Cadd\Trans\201302miscdet.dgn Plot Date: 1/12/2006

GIRDER CHORD ANGLE TABLE

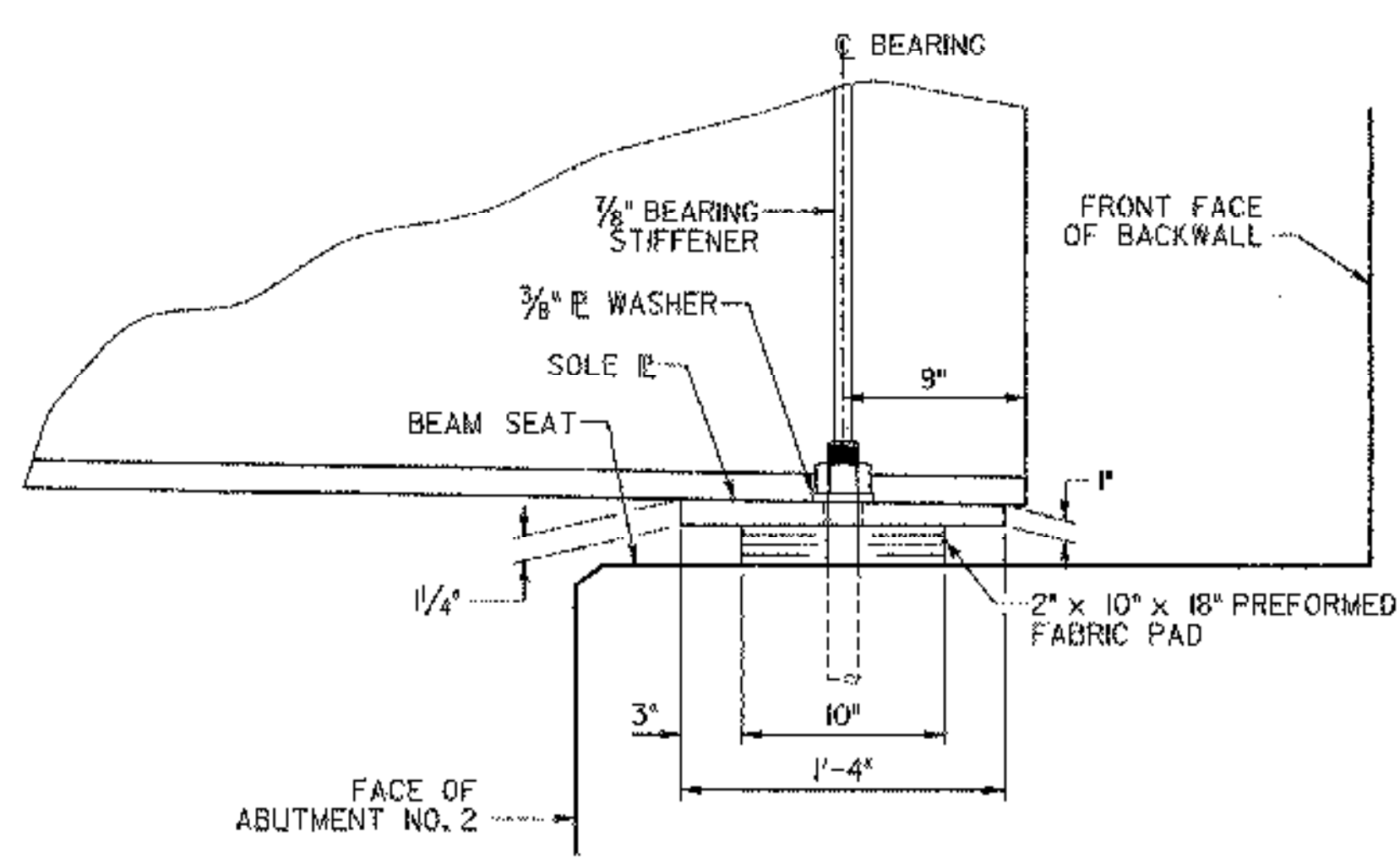
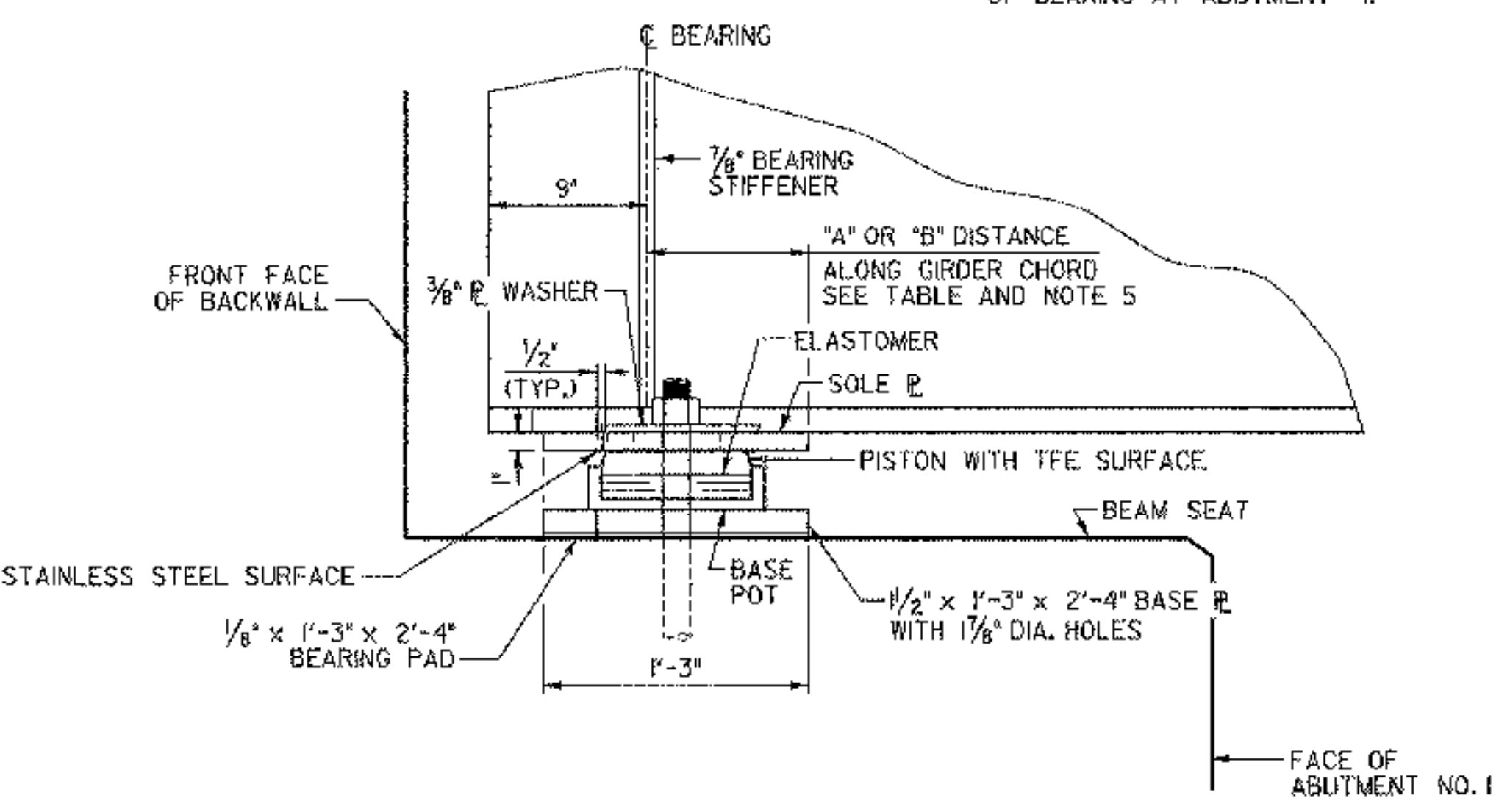
LOCATION	ANGLE BETWEEN GIRDER CHORD AND THE TANGENT AT C BRG. FOR ABUTMENT
GIRDER NO. 4	8° 12' 24"
GIRDER NO. 3	8° 05' 05"
GIRDER NO. 2	7° 58' 11"
GIRDER NO. 1	7° 51' 37"

1" PER TOM KNIGHT

* - "GIRDER CHORD" FOR THE ABUTMENT #1 BEARINGS IS DEFINED AS THE CHORD BETWEEN THE CENTERLINE OF BEARING AT ABUTMENT #2 AND THE CENTERLINE OF BEARING AT ABUTMENT #1.
 ** - "GIRDER CHORD" FOR THE PIER BEARINGS IS DEFINED AS THE CHORD BETWEEN THE CENTERLINE OF BEARING AT ABUTMENT #2 AND THE CENTERLINE OF BEARING AT THE PIER.



PLAN
 SCALE 1 1/2" = 1'-0"
 1 2 3 4 5 6 7 8 9 10



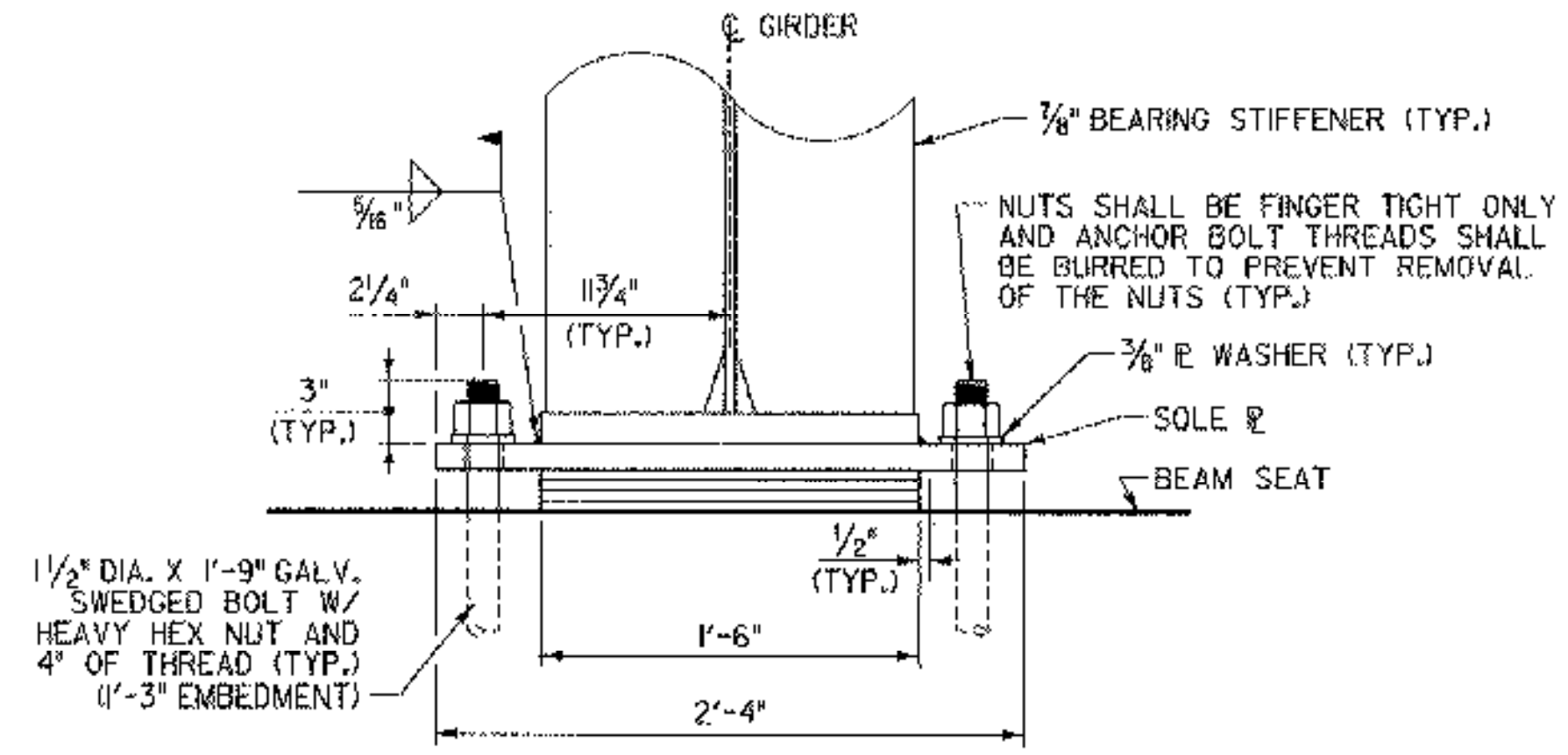
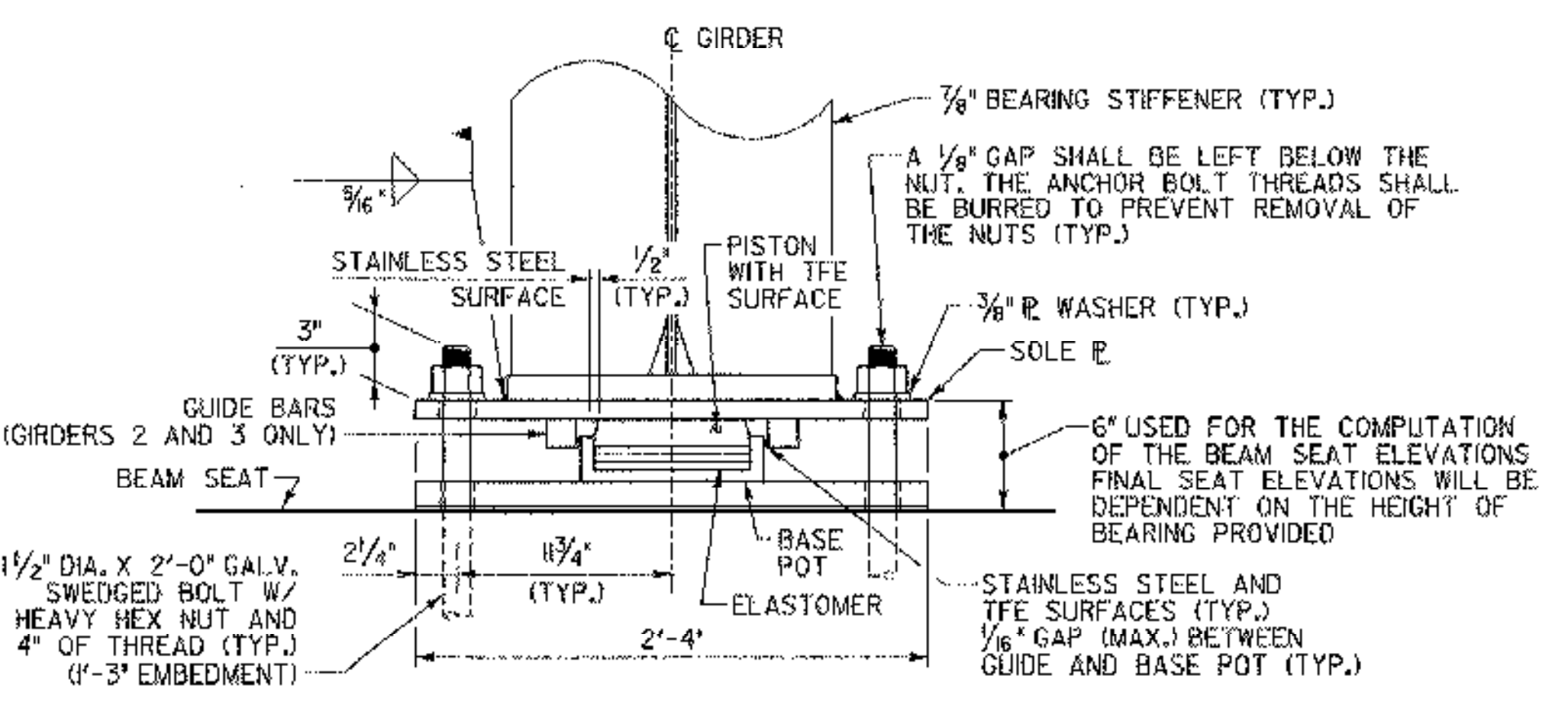
SIDE ELEVATION
 SCALE 1 1/2" = 1'-0"
 1 2 3 4 5 6 7 8 9 10

TEMPERATURE SETTING TABLE

TEMP.	"A" DIST.	"B" DIST.
0° F	8 5/8"	8 1/16"
15° F	8 7/16"	7 7/8"
30° F	8 1/4"	7 11/16"
45° F	8 1/16"	7 1/2"
60° F	7 7/8"	7 3/16"
75° F	7 11/16"	7 1/8"
90° F	7 1/2"	6 5/16"
105° F	7 5/16"	6 3/4"

SEE NOTE 5 ON SHEET 36

SEE SHEET 36 FOR BEARING NOTES



FRONT VIEW
 SCALE 1 1/2" = 1'-0"
 1 2 3 4 5 6 7 8 9 10

FIXED BEARING DETAILS @ ABUTMENT NO. 2

EXPANSION BEARING DETAILS @ ABUTMENT NO. 1

STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of **HUNTINGTON** Bridge No. **42**

Highway No. **T.H. 4** Log Sta. Surv. Sta.

EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

ABUTMENT BEARING DETAILS

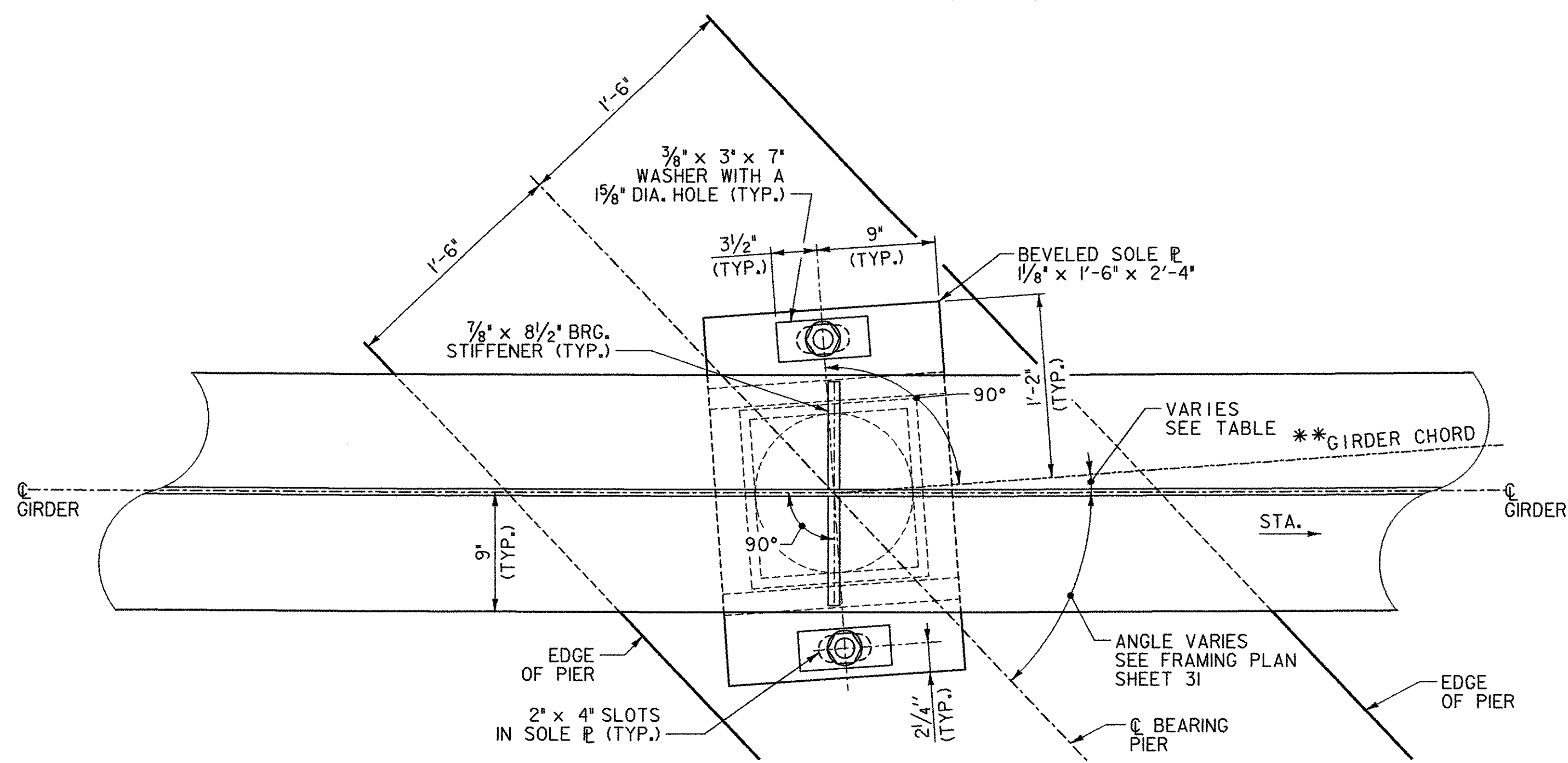
Designed By **T. KNIGHT** Drawn By **S. BURBANK**

Checked By **M. CHENETTE** Date **08/05** Bridge Design Supervisor **M. CHENETTE** Date **09/05**

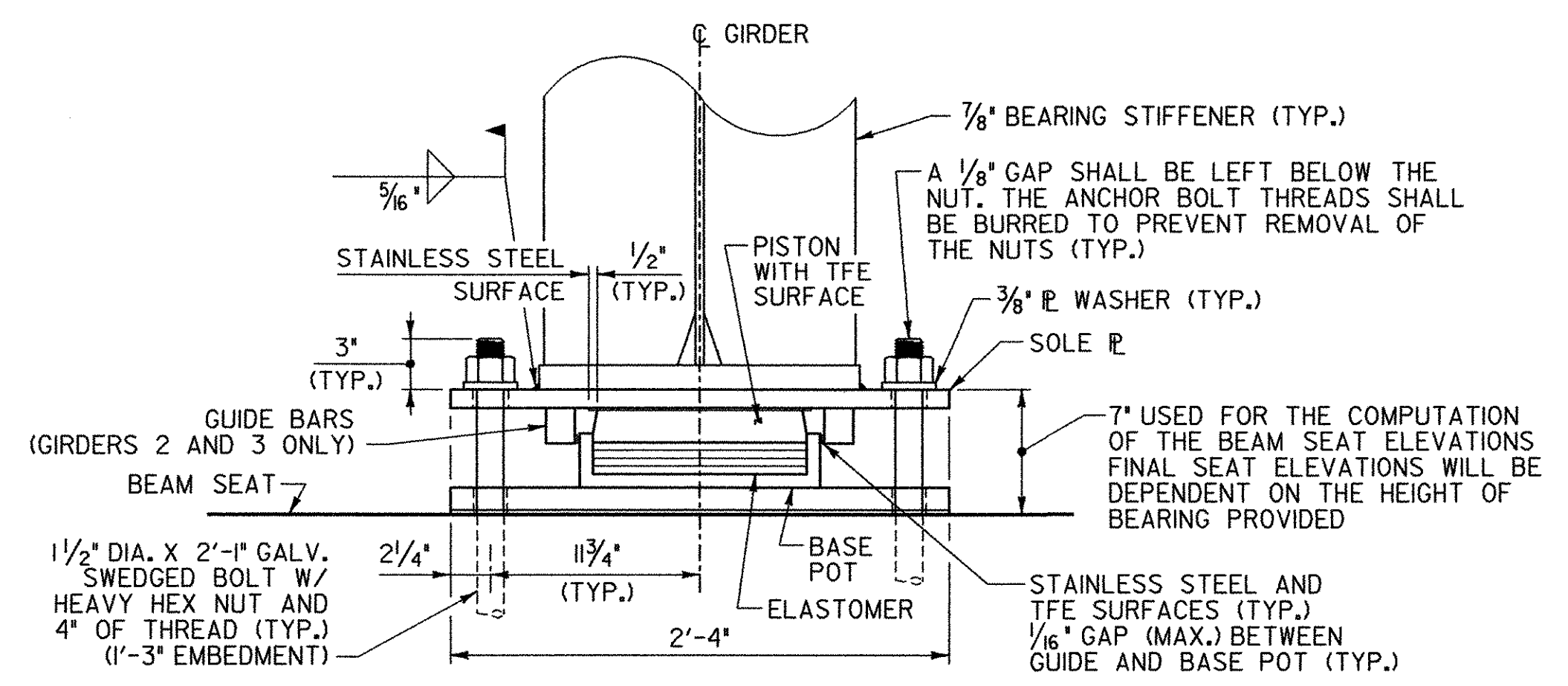
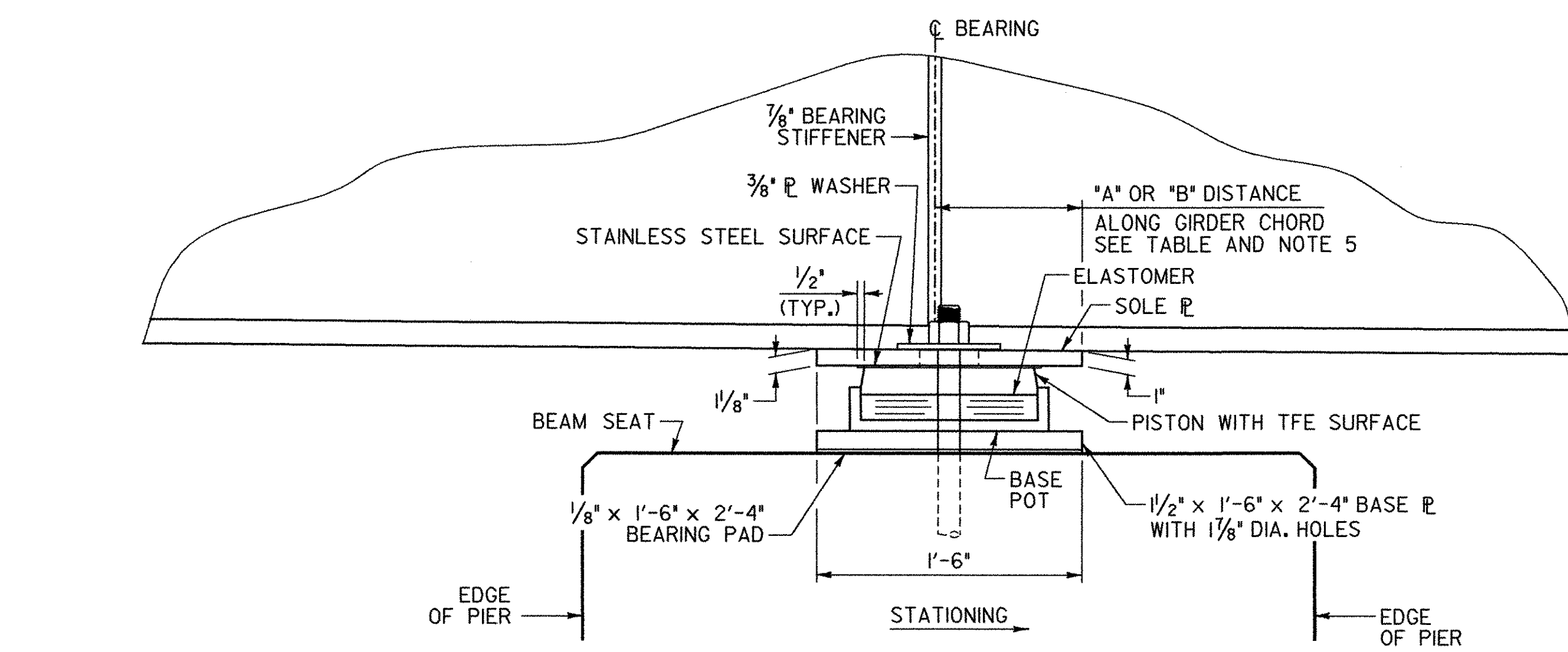
PROJECT **HUNTINGTON** PROJECT NO. **BRO 1445 (29)**

DH Dgn: ...Cadd\Trans\201302brgdet.dgn Plot Date: 01/31/2006

Bridge Sheet No. Sheet **35** of **63**



** - "GIRDER CHORD" FOR THE PIER BEARINGS IS DEFINED AS THE CHORD BETWEEN THE CENTERLINE OF BEARING AT ABUTMENT #2 AND THE CENTERLINE OF BEARING AT THE PIER.



BEARING NOTES:

- BEARINGS SHALL CONFORM TO APPLICABLE SUBSECTIONS OF THE STANDARD SPECIFICATION SECTIONS 531 AND 731
- BEARINGS SHALL BE PAID FOR UNDER THE ITEM 531.0, "BEARING DEVICE ASSEMBLY".
- SHOP DRAWINGS CONFORMING TO STANDARD SPECIFICATION SUBSECTION 513.03 SHALL BE SUBMITTED TO INCLUDE WELDING AND BONDING PROCEDURES.
- THE CONCRETE SURFACE UNDER THE BEARING DEVICE SHALL BE LEVEL.
- THE "A" DISTANCE IS LISTED FOR SETTING THE BEARING AFTER THE STRUCTURAL STEEL IS ERECTED AND BEFORE THE CONCRETE DECK IS POURED. THE "B" DISTANCE IS THE FINAL SETTING FOR THE BEARING PAD AFTER THE CONCRETE SLAB, CURB, SIDEWALK, PAVEMENT, AND BRIDGE RAIL ARE PLACED. THE DIFFERENCE IS THE THEORETICAL ELONGATION OF THE BOTTOM FLANGE DUE TO DEAD LOAD DEFLECTION. THE FINAL "B" DISTANCE, AS SHOWN IN THE TABLE, MUST BE ATTAINED WITHIN 1/8".
- DESIGN CRITERIA:
 - BASE PLATE TO CONCRETE DESIGN PRESSURE = 1000 P.S.I. MAXIMUM.
 - SEE TABLES BELOW FOR DESIGN LOADS.
- ALL STEEL IN BEARING DEVICES (EXCEPT STAINLESS STEEL) SHALL BE AASHTO M-270, (ASTM A-709) GRADE 36.
- ANCHOR BOLTS SHALL HAVE A MINIMUM OF 1'-3" EMBEDMENT INTO THE CONCRETE AND SHALL CONFORM TO STANDARD SPECIFICATION SUBSECTION 714.08. ANCHOR BOLTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-183 (ASTM A-36).
- ALL THE ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. ALL WASHERS SHALL BE 3/8" PLATE (MINIMUM). PAYMENT FOR ANCHOR BOLTS, NUTS AND WASHERS SHALL BE INCLUDED IN THE UNIT BID PRICE FOR "BEARING DEVICE ASSEMBLY".
- BEARING DEVICES SHALL BE GALVANIZED OR METALIZED AS PER STANDARD SPECIFICATION SUBSECTIONS 531.04(b) AND 506.15(d) AND (b). AREAS OF DAMAGED GALVANIZING SHALL BE COATED IN ACCORDANCE WITH ASTM A 760/A 760M AND A780. AREAS OF DAMAGED METALIZING SHALL BE COATED WITH THE SAME SEALANT USED BY THE BEARING SUPPLIER.

ABUTMENT NO. 1 BEARING DESIGN LOAD TABLE

LOCATION	DESCRIPTION	* VERTICAL LOAD CAPACITY (KIPS)	TRANSVERSE LOAD CAPACITY (KIPS)	LONGITUDINAL LOAD CAPACITY (KIPS)	** MOVEMENT CAPACITY (INCHES)	ROTATION CAPACITY (RADIAN)
GIRDER NO. 4	NON-GUIDED EXP. BEARING	170	N/A	N/A	2 1/2"	0.020
GIRDER NO. 3	GUIDED EXP. BEARING (LONG.)	170	34	N/A	2 1/2"	0.020
GIRDER NO. 2	GUIDED EXP. BEARING (LONG.)	170	34	N/A	2 1/2"	0.020
GIRDER NO. 1	NON-GUIDED EXP. BEARING	170	N/A	N/A	2 1/2"	0.020

PIER BEARING DESIGN LOAD TABLE

LOCATION	DESCRIPTION	* VERTICAL LOAD CAPACITY (KIPS)	TRANSVERSE LOAD CAPACITY (KIPS)	LONGITUDINAL LOAD CAPACITY (KIPS)	** MOVEMENT CAPACITY (INCHES)	ROTATION CAPACITY (RADIAN)
GIRDER NO. 4	NON-GUIDED EXP. BEARING	335	N/A	N/A	1 1/2"	0.015
GIRDER NO. 3	GUIDED EXP. BEARING (LONG.)	335	67	N/A	1 1/2"	0.015
GIRDER NO. 2	GUIDED EXP. BEARING (LONG.)	335	67	N/A	1 1/2"	0.015
GIRDER NO. 1	NON-GUIDED EXP. BEARING	335	N/A	N/A	1 1/2"	0.015

ABUTMENT NO. 2 BEARING DESIGN LOAD TABLE

LOCATION	DESCRIPTION	* VERTICAL LOAD CAPACITY (KIPS)	TRANSVERSE LOAD CAPACITY (KIPS)	LONGITUDINAL LOAD CAPACITY (KIPS)	MOVEMENT CAPACITY (INCHES)	ROTATION CAPACITY (RADIAN)
GIRDER NO. 4	FIXED	150	15	15	0	0.016
GIRDER NO. 3	FIXED	150	15	15	0	0.016
GIRDER NO. 2	FIXED	150	15	15	0	0.016
GIRDER NO. 1	FIXED	150	15	15	0	0.016

* DESIGN LOAD SHOWN IS THE MAXIMUM ACTUAL LOAD FOR ALL GIRDERS.
** MOVEMENT IS IN THE DIRECTION OF GIRDER CHORDS.

GIRDER CHORD ANGLE TABLE

LOCATION	ANGLE BETWEEN GIRDER CHORD AND THE TANGENT AT C. BRG. FOR THE PIER
GIRDER NO. 4	4° 09' 30"
GIRDER NO. 3	4° 03' 54"
GIRDER NO. 2	3° 58' 36"
GIRDER NO. 1	3° 53' 35"

TEMPERATURE SETTING TABLE

TEMP.	"A" DIST.	"B" DIST.
0° F	9 5/8"	9 3/8"
15° F	9 1/2"	9 1/4"
30° F	9 3/8"	9 1/8"
45° F	9 1/4"	9"
60° F	9 1/8"	8 7/8"
75° F	9"	8 3/4"
90° F	8 7/8"	8 5/8"
105° F	8 3/4"	8 1/2"

SEE NOTE 5 THIS SHEET

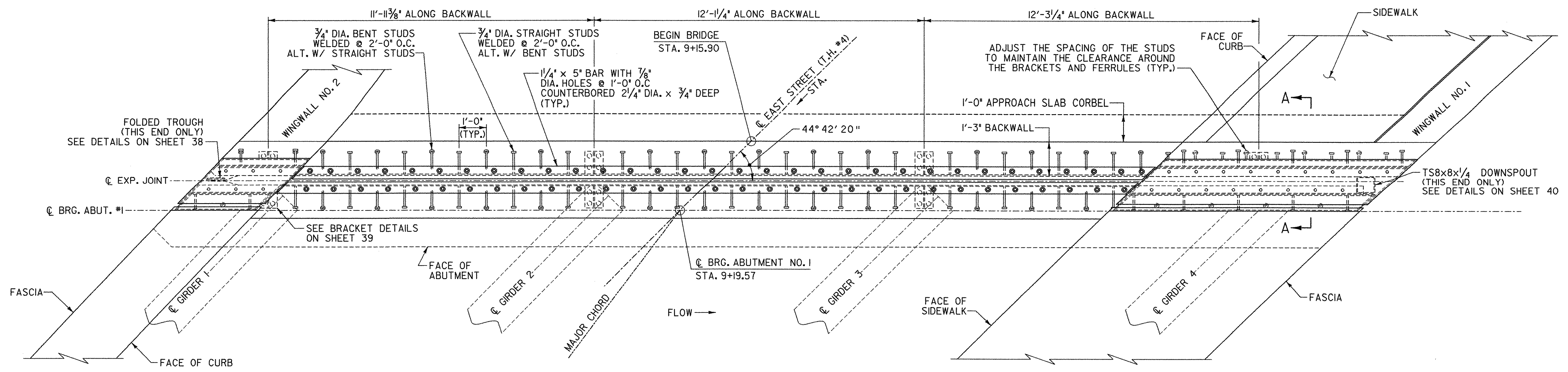
STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of HUNTINGTON Bridge No. 42
 Highway No. T.H. 4 Log Sta. Surv. Sta.
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

PIER BEARING DETAILS

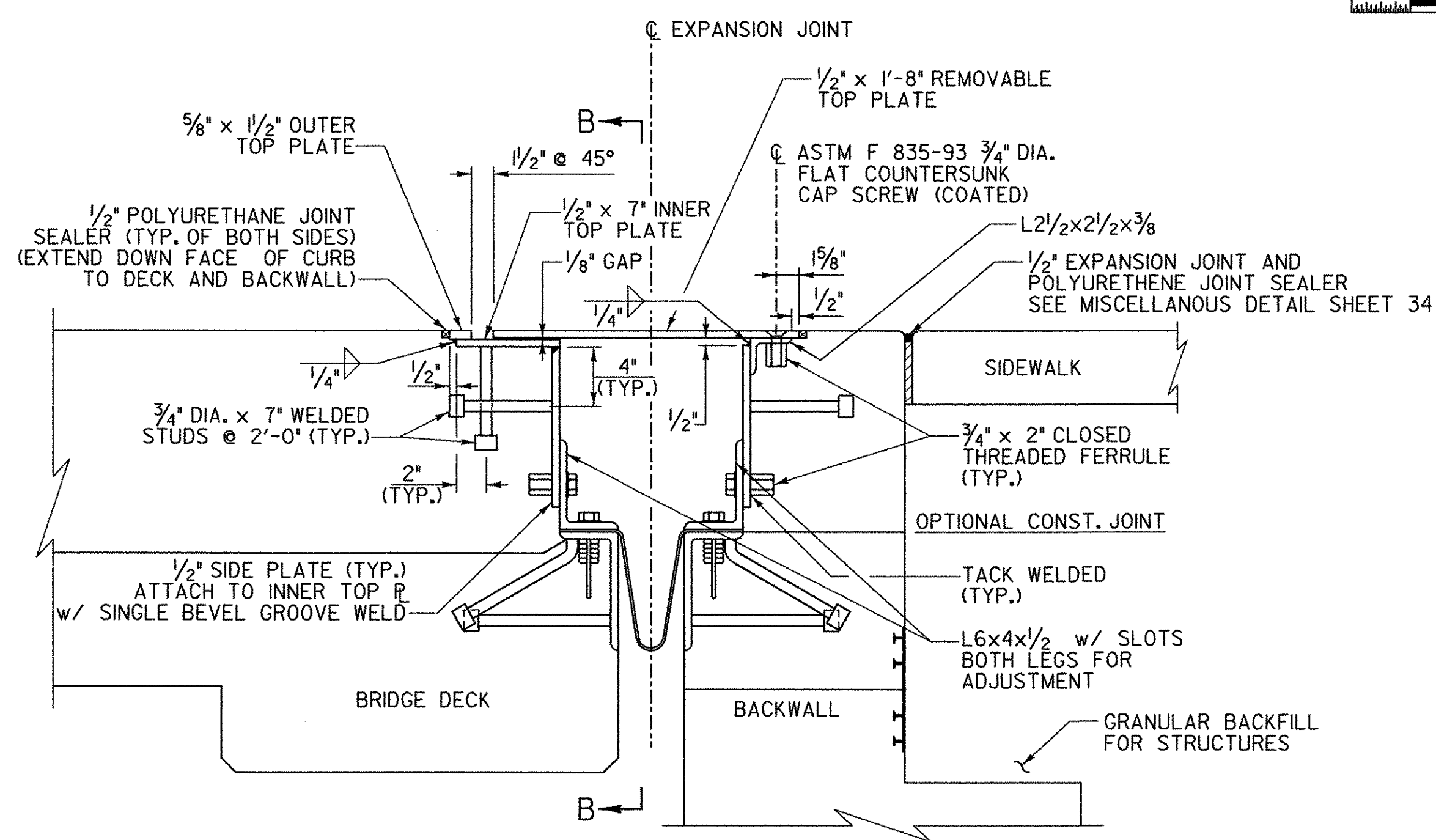
Designed By G. BOGUE Drawn By S. BURBANK
 Checked By M. CHENETTE Date 08/05 Bridge Design Supervisor M. CHENETTE Date 09/05
 PROJECT HUNTINGTON PROJECT NO. BRO 1445 (29)
 DH Dgn: ...\\cadd\Trans\201302brgs2.dgn Plot Date: 01/31/2006
 Bridge Sheet No. Sheet 36 of 63





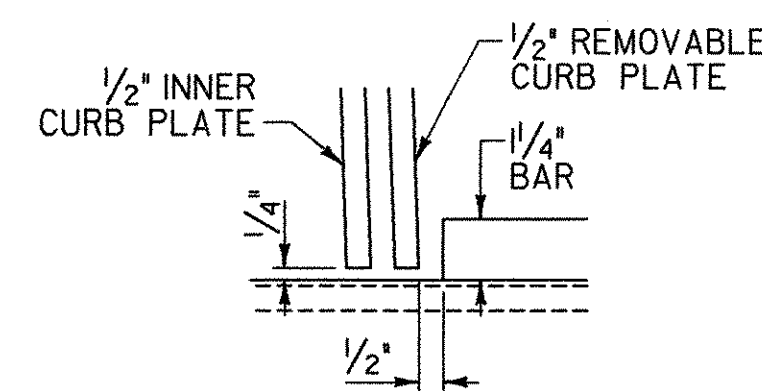
PLAN VIEW

SCALE 1/2" = 1'-0"
0 1 2



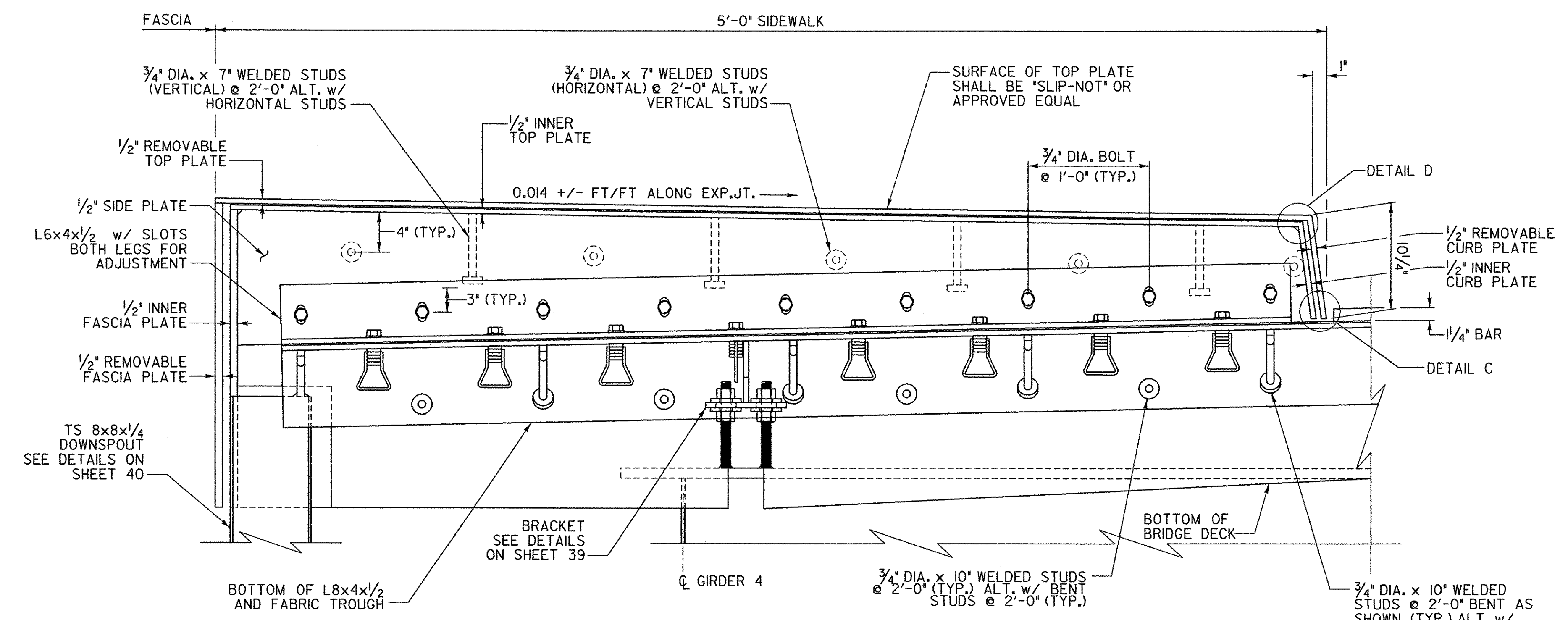
SECTION A-A

SCALE 1/2" = 1'-0"



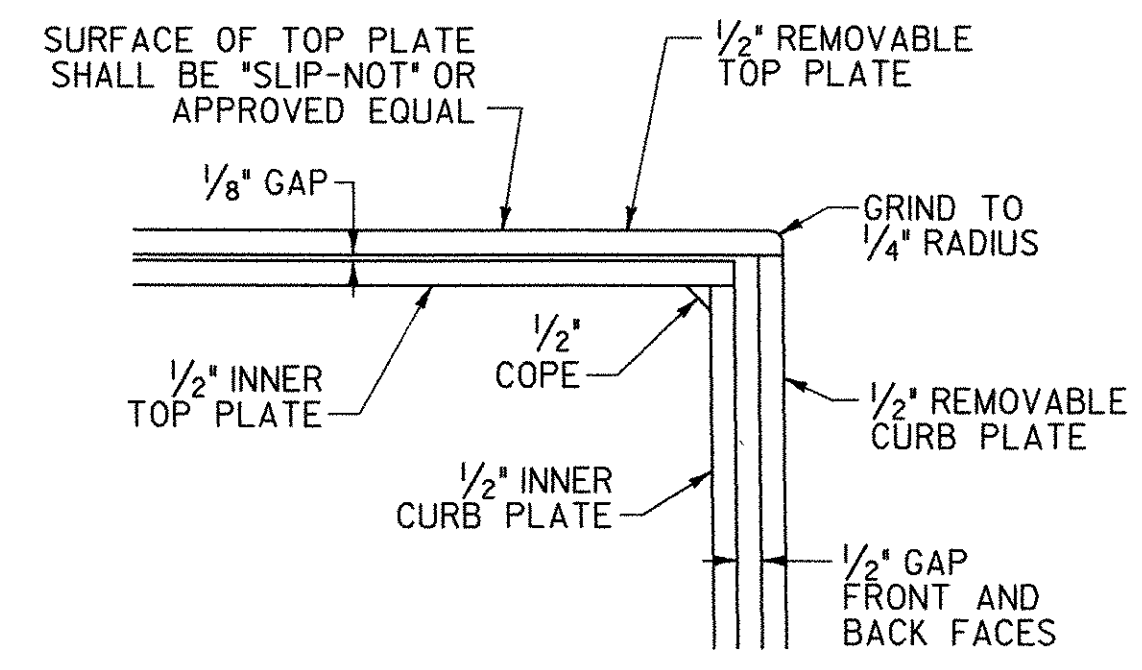
DETAIL C

SCALE 3" = 1'-0"



SECTION B-B

SCALE 1/2" = 1'-0"



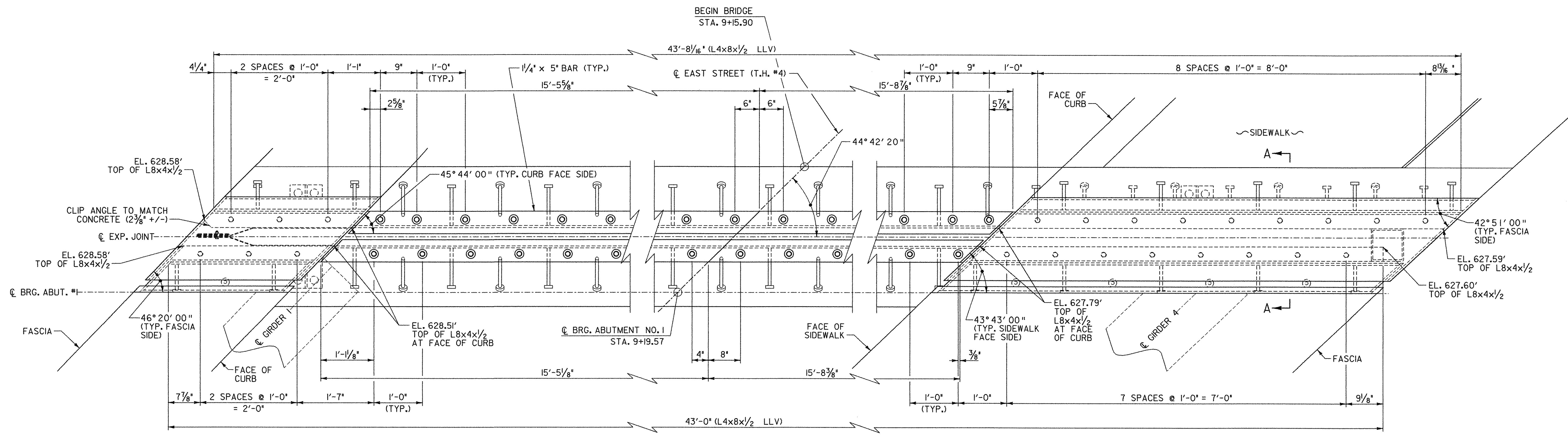
DETAIL D

SCALE 3" = 1'-0"



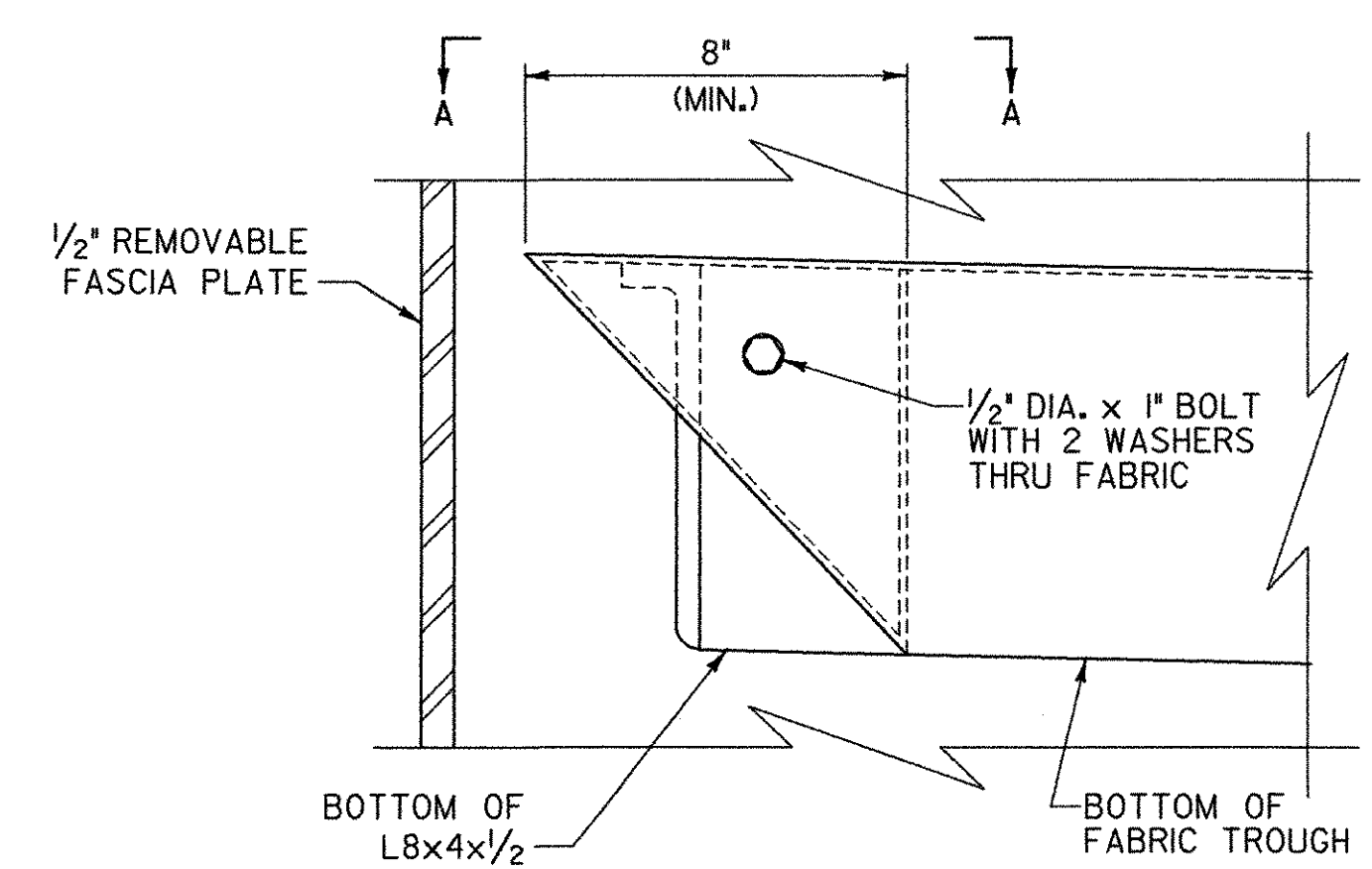
STATE OF VERMONT AGENCY OF TRANSPORTATION		
Town Of	HUNTINGTON	Bridge No. 42
Highway No.	T.H. 4	Log Sta. Surv. Sta.
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER		
EXPANSION JOINT PLAN		
Designed By	S. BURBANK	Drawn By S. BURBANK
Checked By	M. CHENETTE	Bridge Design Supervisor
Date	08/05	Date 09/05
PROJECT	HUNTINGTON	PROJECT NO. BRO 1445 (29)
DH Dgn.: ...Cadd\Trans\201302expjtl.dgn		Plot Date: 1/2/2006
Bridge Sheet No.	Sheet 37 of 63	



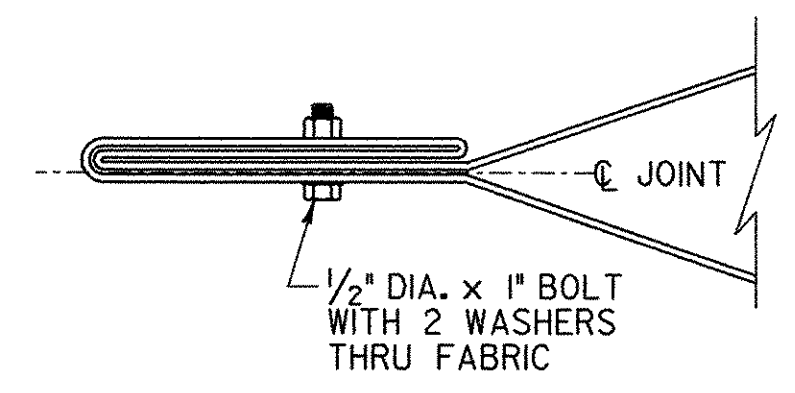


PARTIAL PLAN VIEW

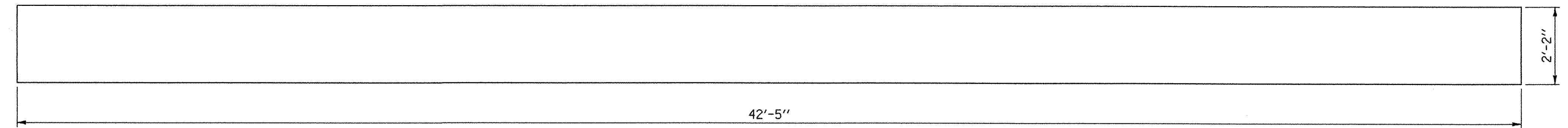
SCALE 1" = 1'-0"



ELEVATION FOLDED TROUGH END DETAILS
 NOT TO SCALE



VIEW A-A
 NOT TO SCALE



TROUGH LAYOUT

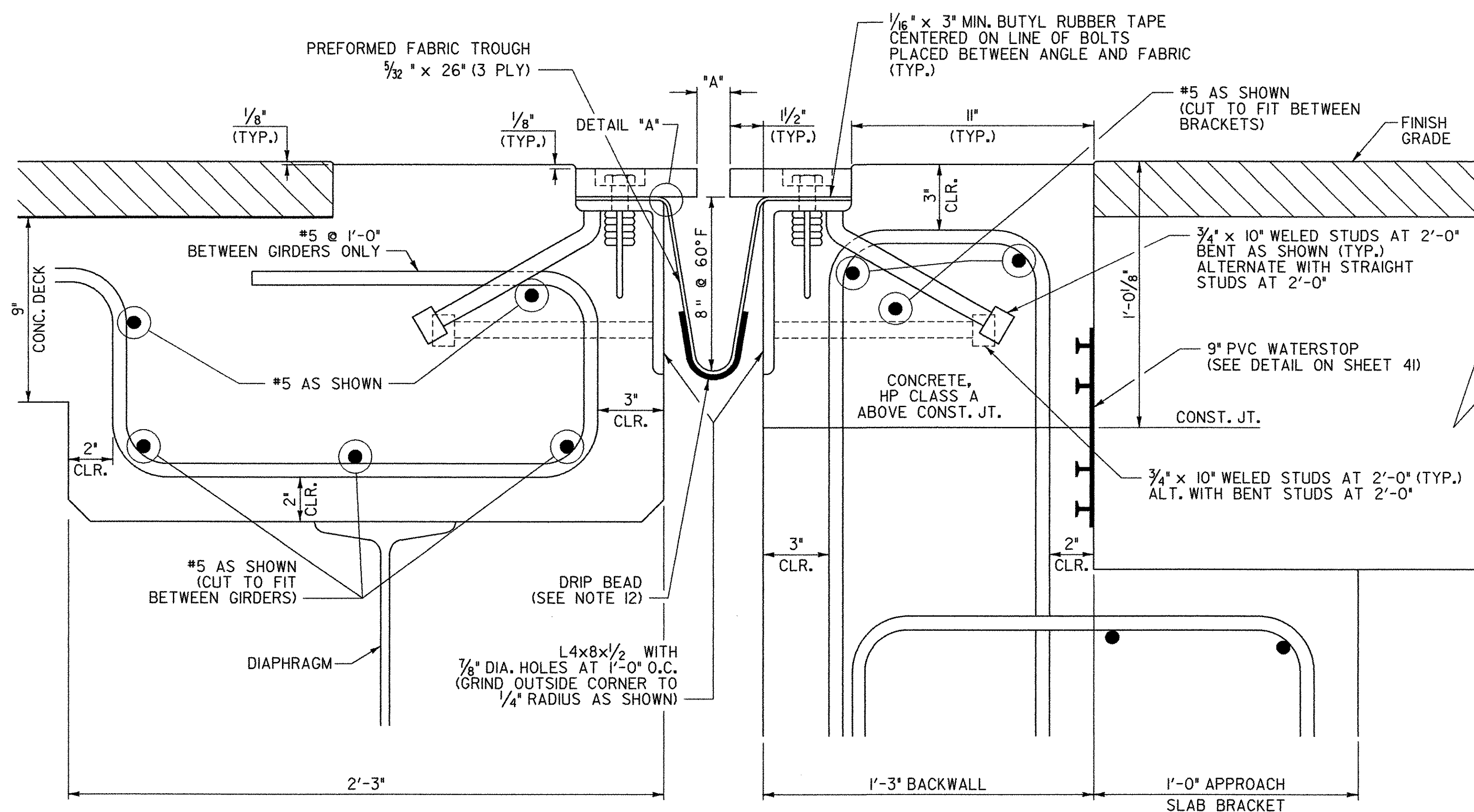
SCALE 1/2" = 1'-0"

SEE PREVIOUS SHEET FOR SECTION A-A

**STATE OF VERMONT
 AGENCY OF TRANSPORTATION**

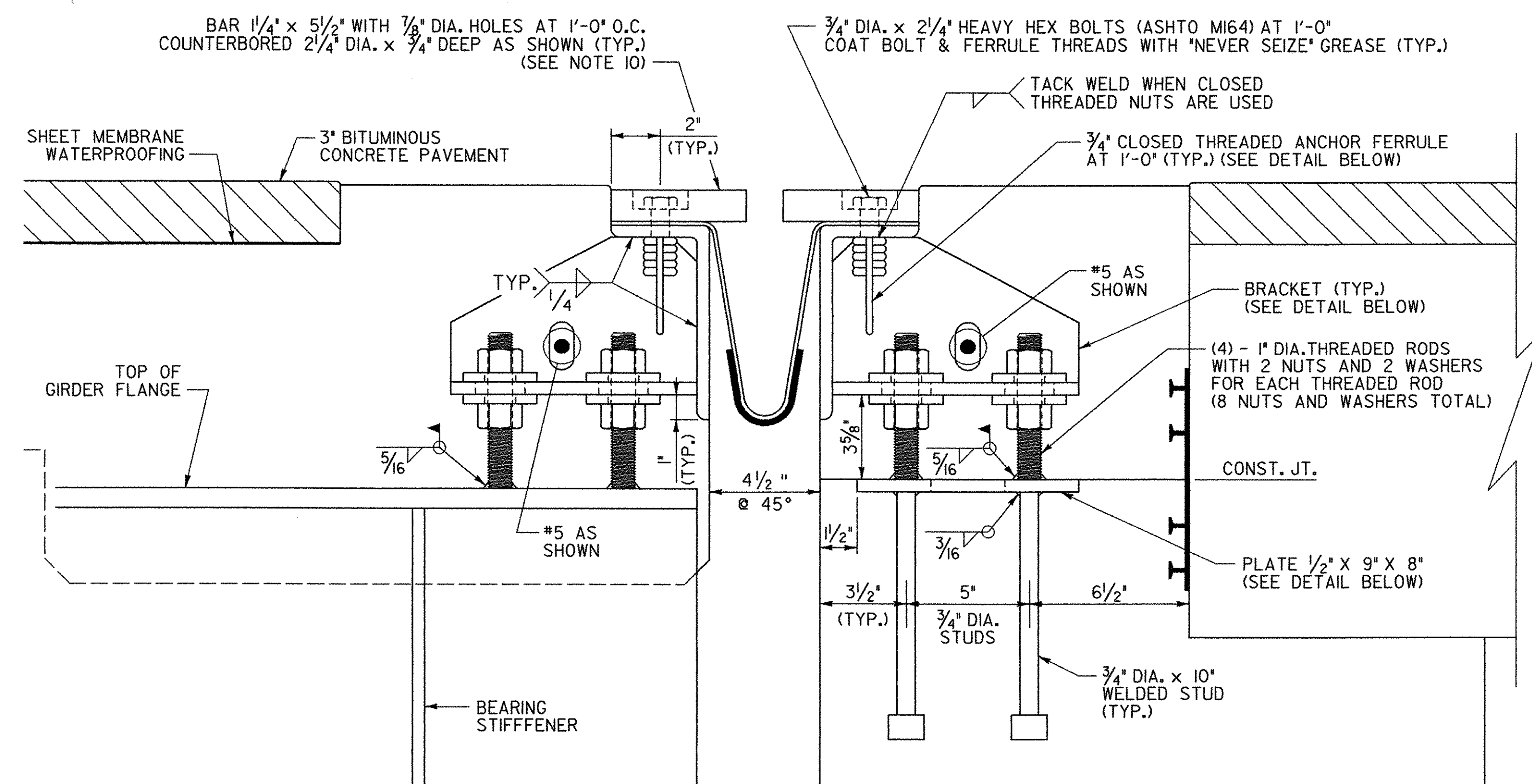
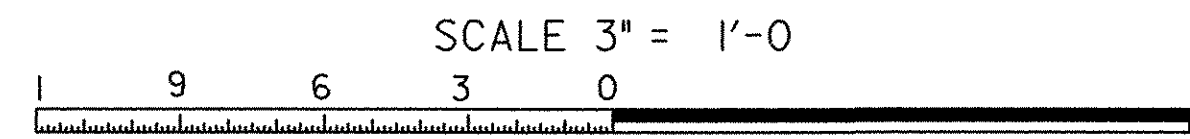
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
EXPANSION JOINT DETAILS SHEET 1			
Designed By	S. BURBANK	Drawn By	S. BURBANK
Checked By	Date	Bridge Design Supervisor	
M. CHENETTE	08/05	M. CHENETTE	Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn: ...Cadd\Trans\z01302expjt2.dgn Plot Date: 1/12/2006		Bridge Sheet No.	Sheet 38 of 63





TYPICAL SECTION BETWEEN GIRDERS

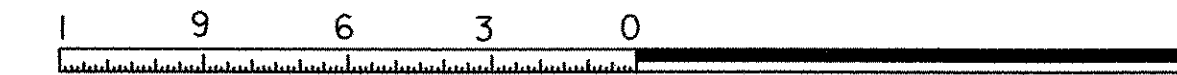
(NORMAL TO \odot BEARING)



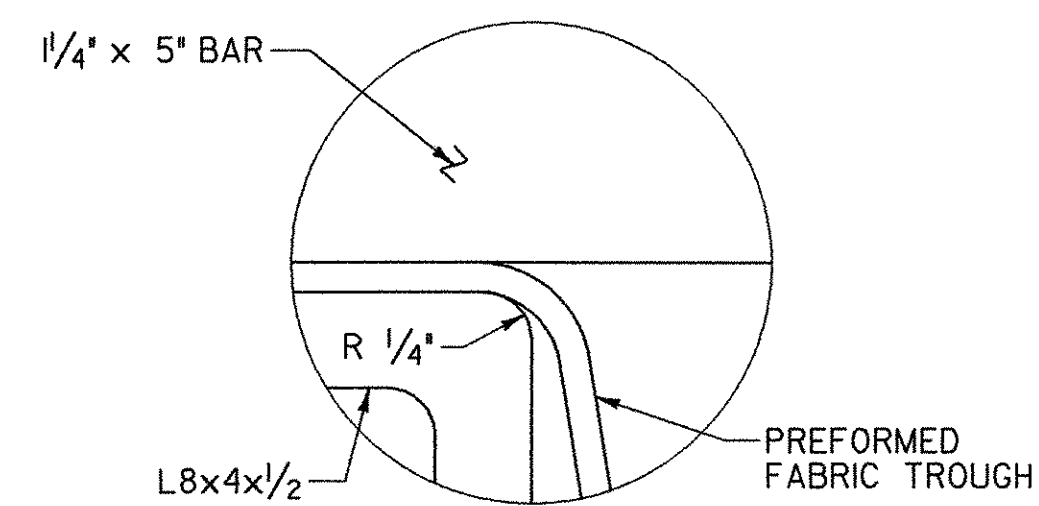
TYPICAL SECTION AT GIRDERS

(NORMAL TO \odot BEARING)

SCALE 3" = 1'-0"



1. DETAILS ON THIS SHEET AND THE EXPANSION JOINT PLAN SHEET ARE FOR ITEM 516.10 "BRIDGE EXPANSION JOINT (VERMONT)".
2. PREFORMED FABRIC MATERIAL SHALL BE CONTINUOUS AND SHALL CONFORM TO SUBSECTION 707.07 OF STANDARD SPECIFICATIONS.
3. BUTYL RUBBER TAPE SHALL CONFORM TO AASHTO SPECIFICATION M-198, TYPE II.
4. THE FINAL FINISH OF THE EXPANSION DEVICE SHALL BE COVERED DURING THE PLACING OF BRIDGE DECK CONCRETE.
5. ALL STEEL COMPONENTS SHALL BE AASHTO M270 GRADE 36 GALVANIZED OR METALIZED AS PER SUBSECTION 506.15(a) OR (b) OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE SPECIFIED. THREADED RODS SHALL CONFORM TO ASTM A307, GRADE C. THE NUTS FOR THE THREADED RODS SHALL BE ASTM A563.
6. THE ITEM "BRIDGE EXPANSION JOINT (VERMONT)" SHALL INCLUDE THE FABRICATION AND ERECTION OF THE COMPLETE JOINT ASSEMBLY INCLUDING ALL STEEL PLATES, BRACKETS, ANGLES, WELDED STUDS OR RODS, PREFORMED FABRIC DRAIN TROUGH MATERIAL, BUTYL RUBBER TAPE AND ANY OTHER MISCELLANEOUS MATERIAL NECESSARY TO INSTALL JOINT.
7. THE 4" x 8" x 1/2" ANGLES SHALL BE FURNISHED AS ONE CONTINUOUS PIECE. THE 1 1/4" x 5 1/2" BARS EACH SIDE OF THE JOINT SHALL BE PROVIDED IN TWO EQUAL LENGTHS.
8. COAT CONCRETE CONTACT SURFACES WITH EPOXY BONDING COMPOUND MEETING THE REQUIREMENTS OF SUBSECTION 719.02 OF THE STANDARD SPECIFICATIONS. PAYMENT SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEM 516.10 "BRIDGE EXPANSION JOINT (VERMONT)".
9. FILL COUNTERBORED HOLES WITH HOT POURED JOINT SEALER AFTER BOLT INSTALLATION. PAYMENT FOR THE WORK SHALL BE INCIDENTAL TO ITEM 516.10.
10. PAYMENT FOR PVC WATERSTOP SHALL BE INCIDENTAL TO THE CONCRETE PAY ITEM.
11. A DRIP BEAD OF 1/4" x 7" STRIP OF PREFORMED MATERIAL SHALL BE CEMENTED TO THE BOTTOM OF THE FABRIC TROUGH USING AN ADHESIVE APPROVED BY THE MANUFACTURER. THE DRIP BEAD SHALL BE APPLIED 1" FROM THE DOWNSPOUT END OF THE TROUGH.
12. THE FABRIC TROUGH SHALL BE THOROUGHLY CLEANED AND FLUSHED AFTER THE PAVING OPERATION.
13. THE EXPANSION JOINT SHALL BE SHOP ASSEMBLED AND SHIPPED AS ONE UNIT.

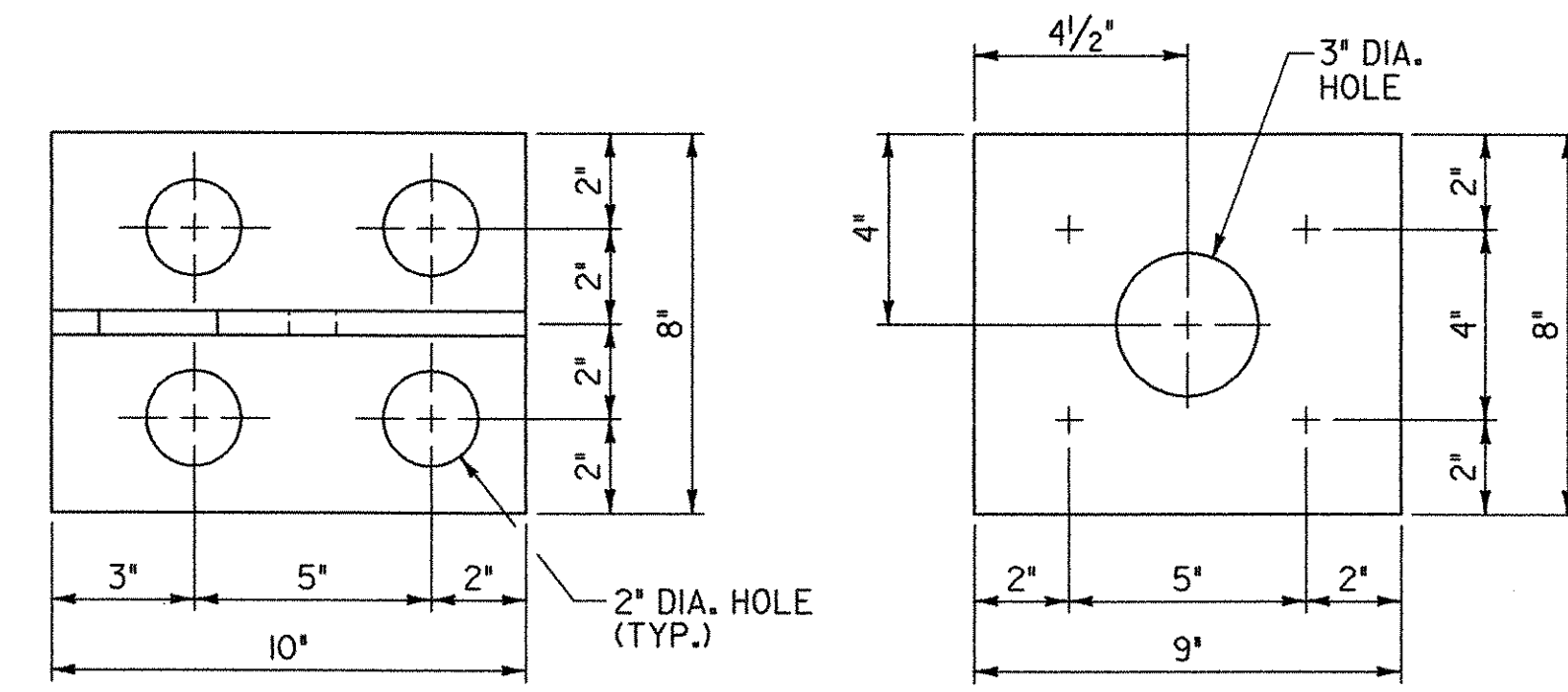


DETAIL "A"

NOT TO SCALE

TEMP	"A" DIST.
0° F	2"
15° F	1 1/8"
30° F	1 1/4"
45° F	1 1/2"
60° F	1 5/8"
75° F	1 3/4"
90° F	1"
105° F	1 1/8"

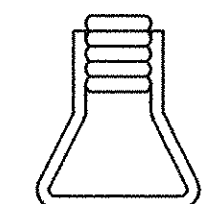
"A" IS THE SETTING BEFORE DEAD LOADS ARE IN PLACE.



PLAN

PLATE PLAN

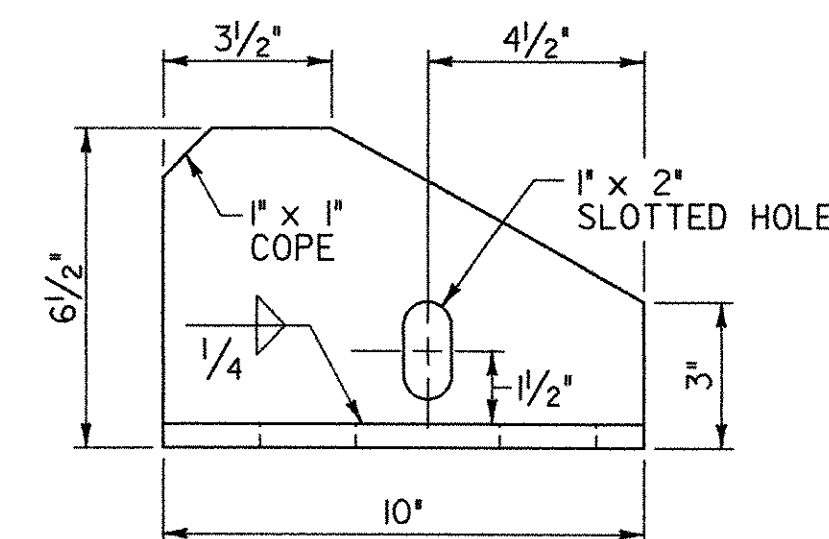
DAYTON/RICHMOND TYPE LF, 3/4" DIA. CLOSED FERRULE OR EQUIVALENT



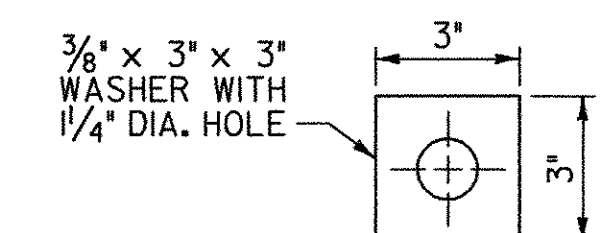
NOTE: CLOSED THREADED FERRULE NUTS WITHOUT ANCHOR LOOPS MAY BE USED.

ANCHOR FERRULE DETAIL

NOT TO SCALE



ELEVATION

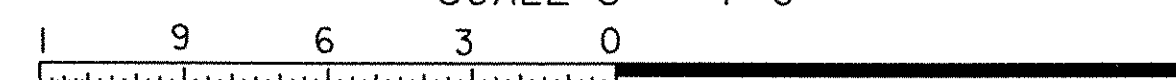


WASHER FOR BRACKET

NOTE: ALL PLATES SHALL BE 1/2" THICK U.N.O.

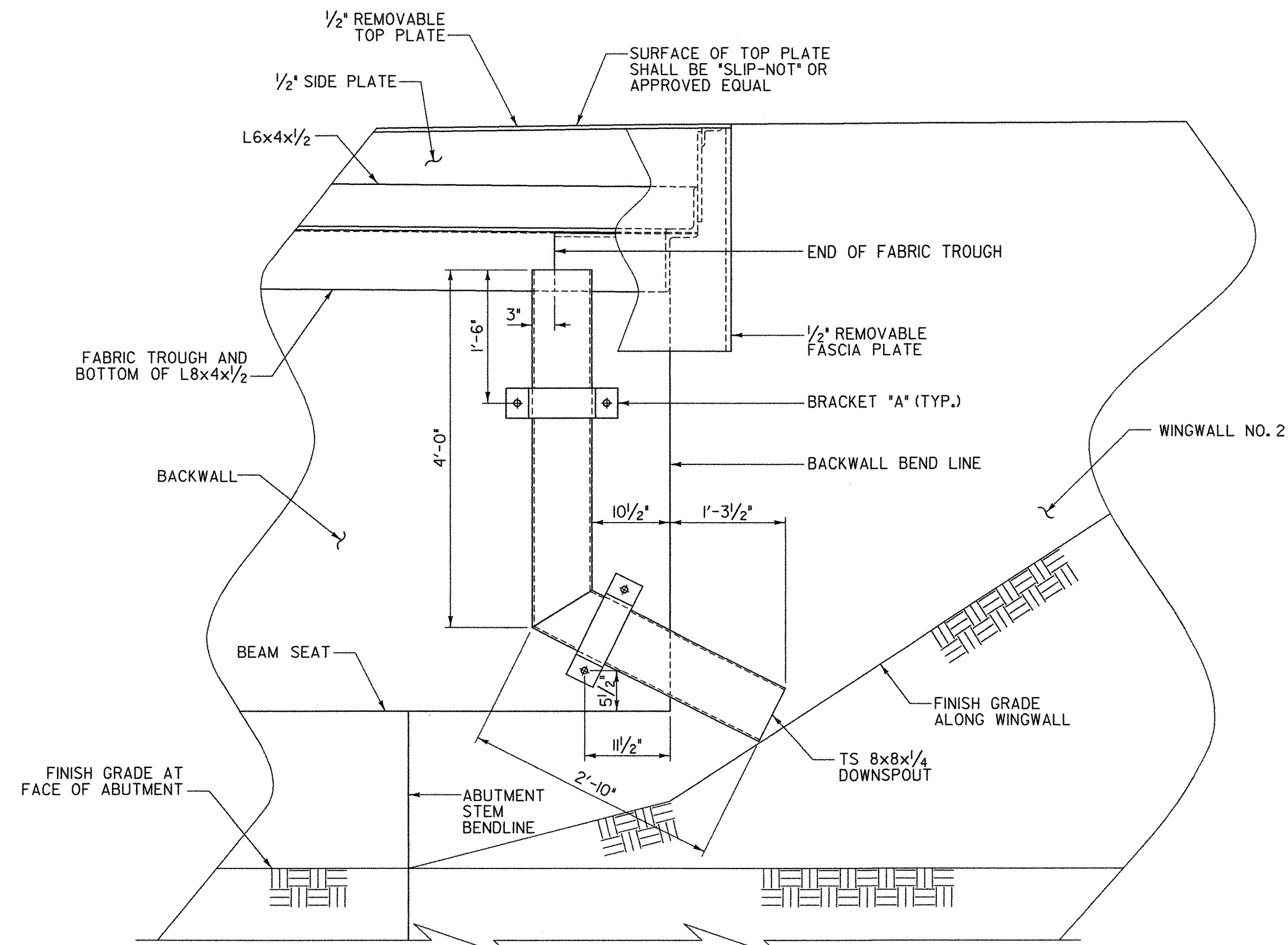
BRACKET DETAILS

SCALE 3" = 1'-0"



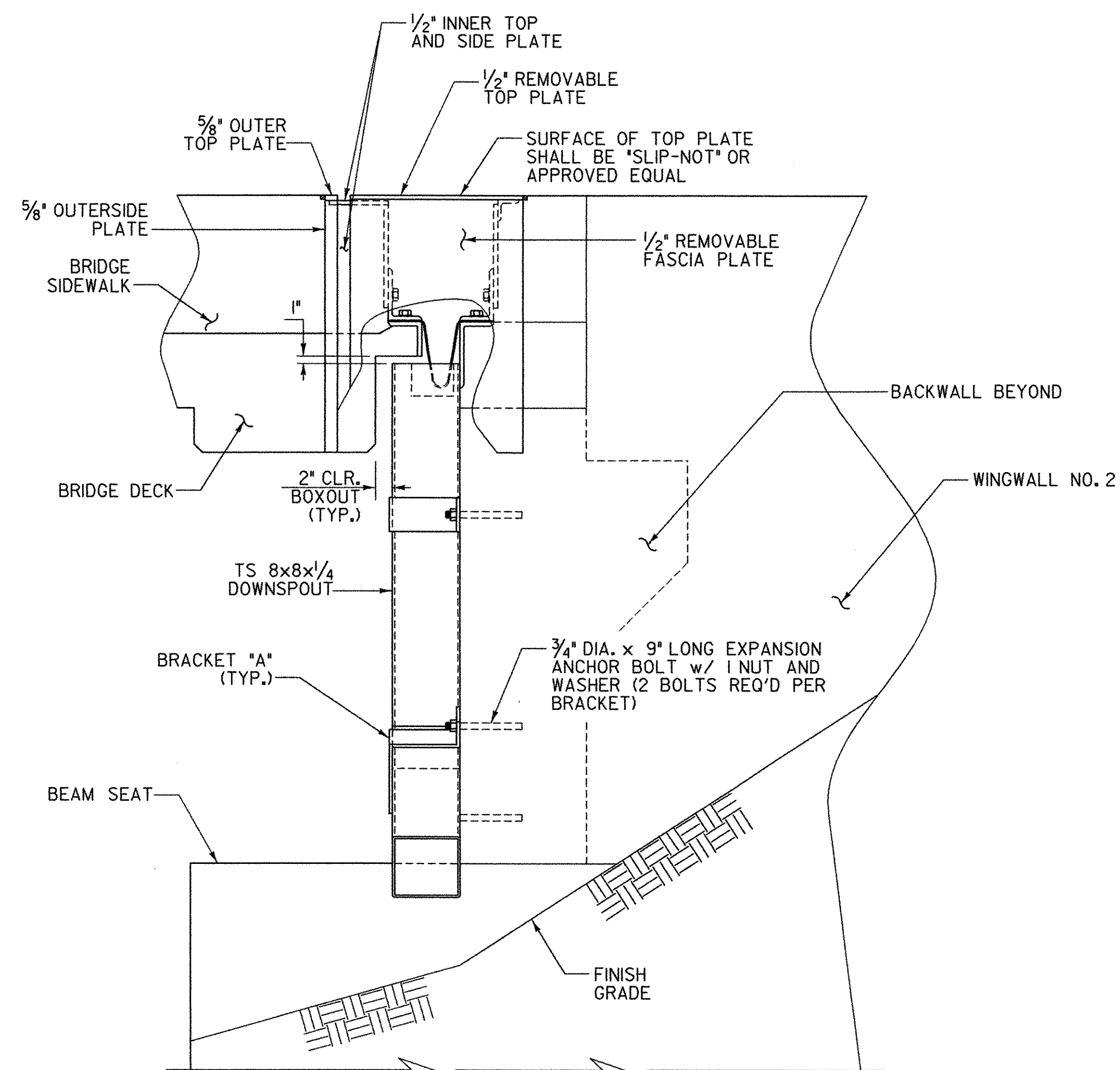
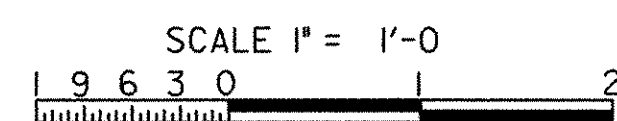
**STATE OF VERMONT
AGENCY OF TRANSPORTATION**

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
EXPANSION JOINT DETAILS SHEET 2			
Designed By	S. BURBANK	Drawn By	S. BURBANK
Checked By	Date	Bridge Design Supervisor	Date
M. CHENETTE	08/05	M. CHENETTE	09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn.: ...Cadd\Trans\201302exp\j3.dgn		Plot Date: 11/2/2006	
Bridge Sheet No.		Sheet	39 of 63



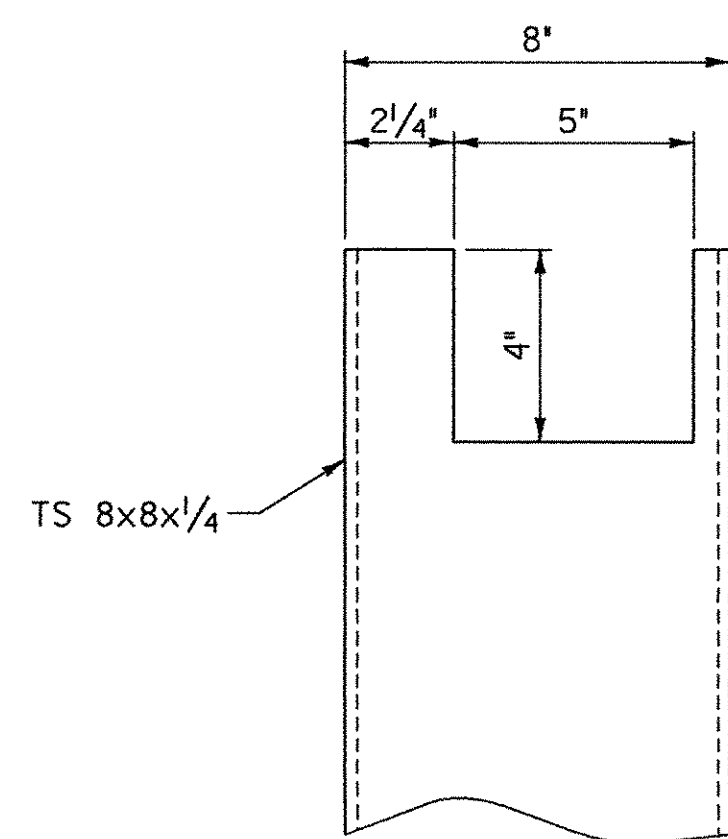
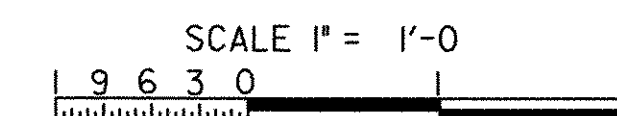
SEE EXPANSION JOINT PLAN FOR INFORMATION NOT SHOWN

DOWNSPOUT FRONT ELEVATION

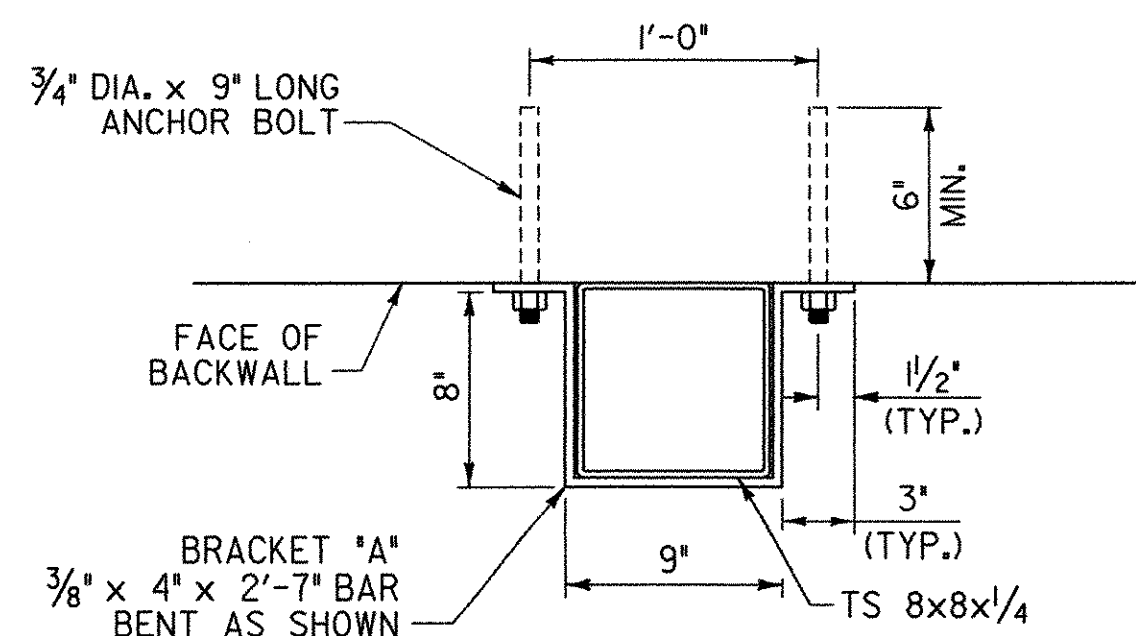
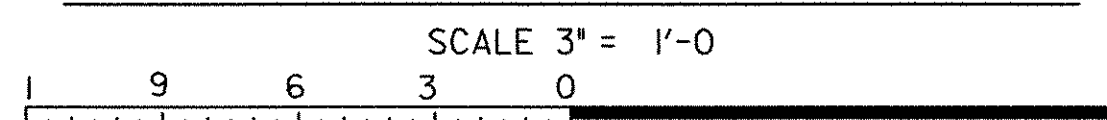


SEE EXPANSION JOINT PLAN FOR INFORMATION NOT SHOWN

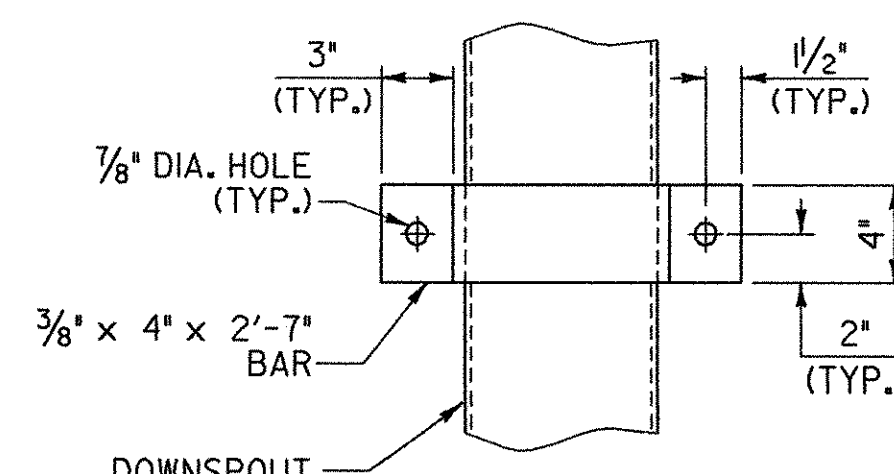
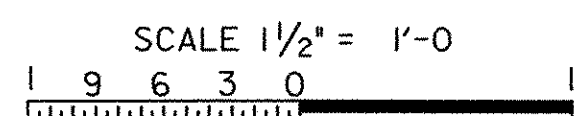
DOWNSPOUT SIDE ELEVATION



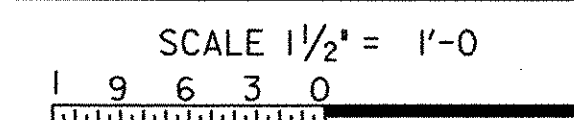
DOWNSPOUT CUTOUT FOR TROUGH



DETAIL FOR ATTACHING DOWNSPOUT TO BACKWALL



ELEVATION VIEW OF BRACKET A



DOWNSPOUT NOTES

- HOLLOW STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A500 OR A501.
- ALL PLATES, BARS, AND ANGLES SHALL CONFORM TO AASHTO M270 GRADE 36.
- DOWNSPOUTS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A-123 AFTER FABRICATION.
- ALL BOLTS AND RELATED HARDWARE SHALL BE ASTM A307 AND SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A513 (AASHTO M232).
- IN AREAS WHERE THE GALVANIZING HAS BEEN REMOVED FROM THE DOWNSPOUT EITHER BY CUTTING, BURNING, WELDING, PLACING, OR ANY OTHER MEANS, IT SHALL BE REPAIRED BY THOROUGHLY CLEANING THE DAMAGED AREAS WITH A WIRE BRUSH AND PAINTING THE DAMAGED AREAS WITH TWO COATS OF AN APPROVED SEALANT.
- ALL MATERIALS AND INSTALLATION COSTS FOR THE DOWNSPOUT, INCLUDING STEEL TUBING, RELATED HARDWARE, AND ANCHOR BOLTS SHALL BE PAID FOR UNDER ITEM 506.55 "STRUCTURAL STEEL (PLATE GIRDER)".

STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	

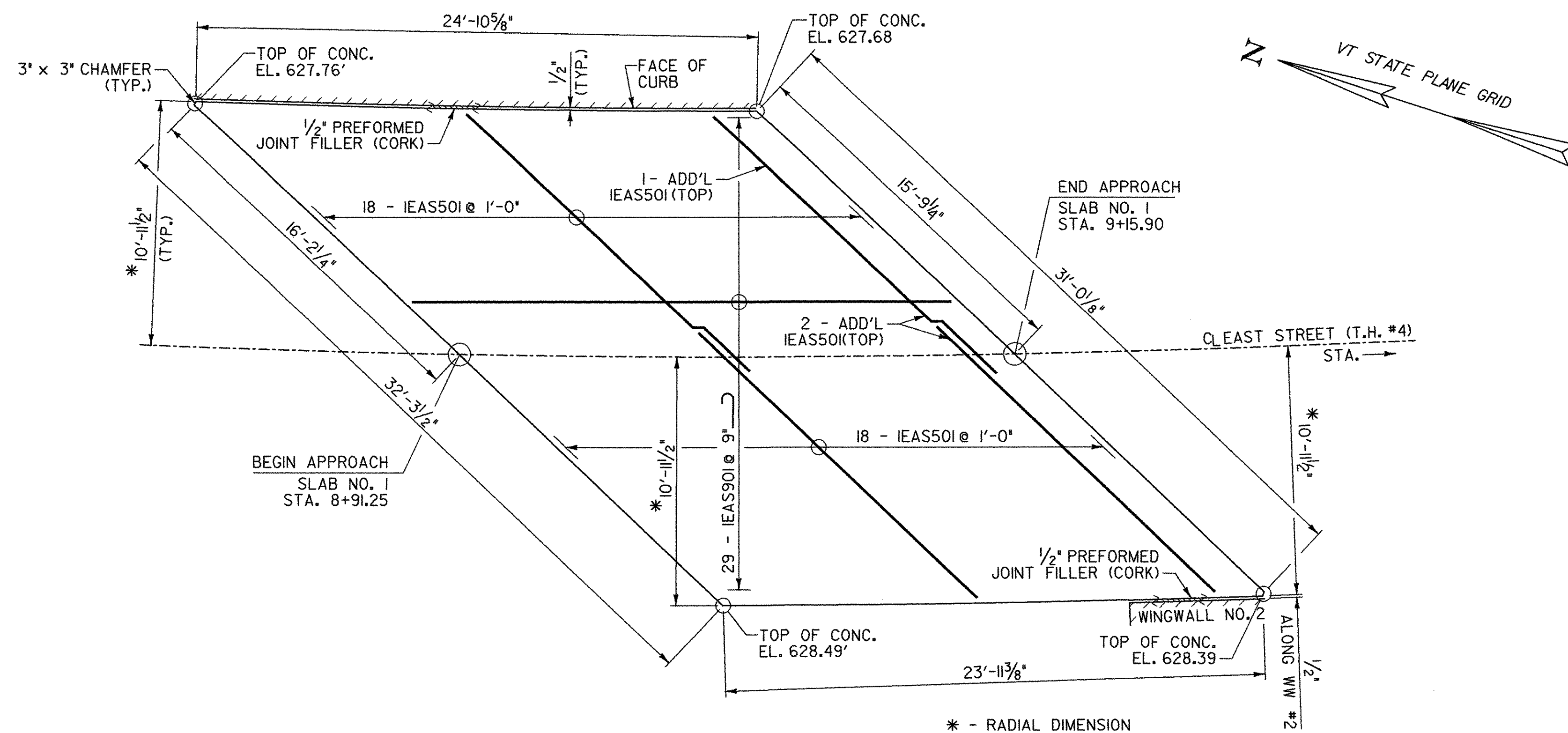
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

DOWNSPOUT DETAILS

Designed By	S. BURBANK	Drawn By	J. SOTER
Checked By	Date	Bridge Design Supervisor	Date
M. CHENETTE	08/05	M. CHENETTE	09/05

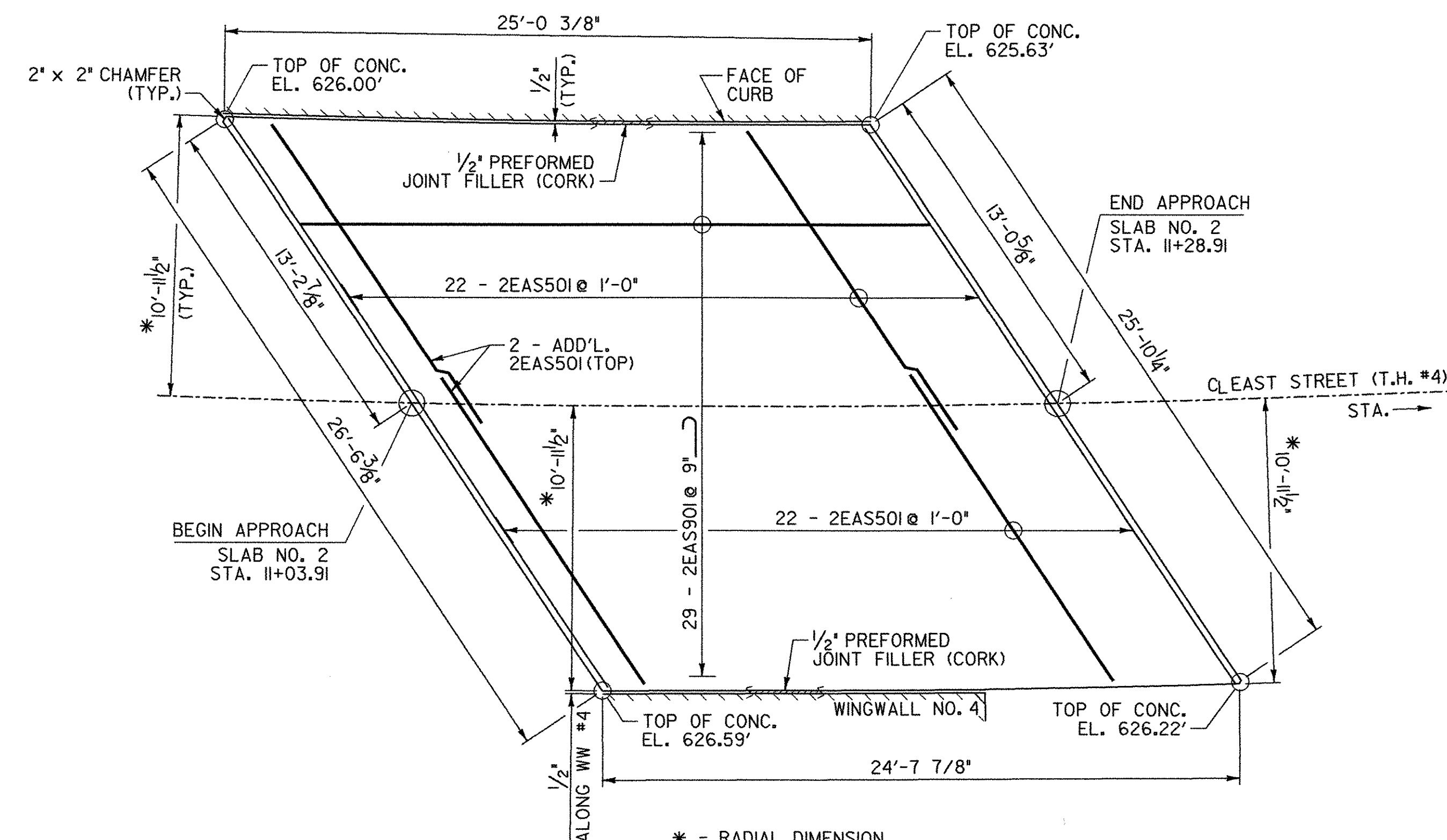
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
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DH Dgn.: ...Cadd\Trans\z01302ds.dgn Plot Date: 1/12/2006



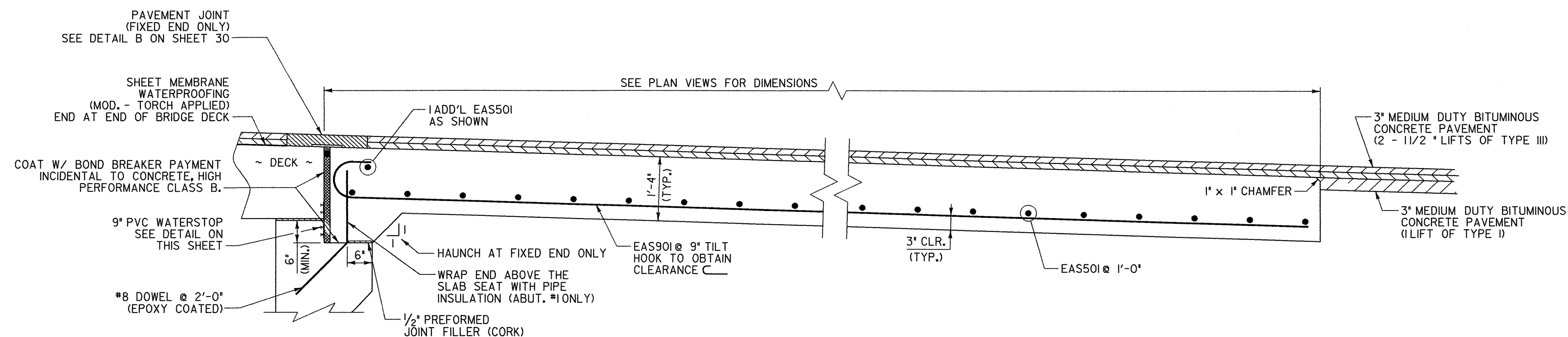
APPROACH SLAB NO. 1

SCALE 1/4" = 1'-0"
 1 0 2 4 6



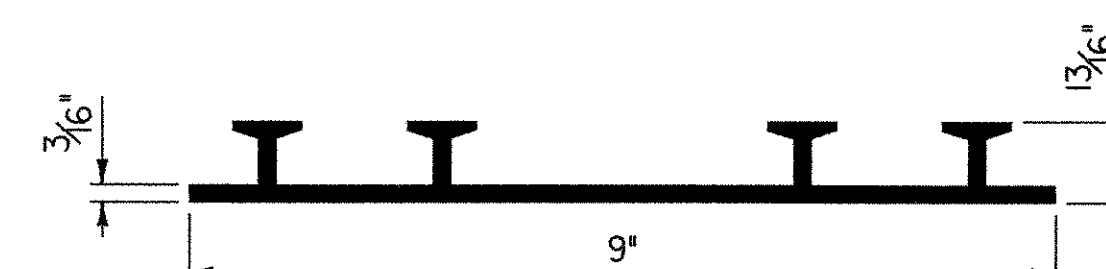
APPROACH SLAB NO. 2

SCALE 1/4" = 1'-0"
 1 0 2 4 6



APPROACH SLAB SECTION

SCALE 3/4" = 1'-0"
 0 1 2



THE COSTS FOR P.V.C. WATERSTOP SHALL BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE, HIGH PERFORMANCE CLASS B. OTHER CONFIGURATIONS MAY BE USED UPON APPROVAL OF THE STRUCTURES ENGINEER.

P. V. C. WATERSTOP FOR CONSTRUCTION JOINTS

NOT TO SCALE

**STATE OF VERMONT
 AGENCY OF TRANSPORTATION**

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	

EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

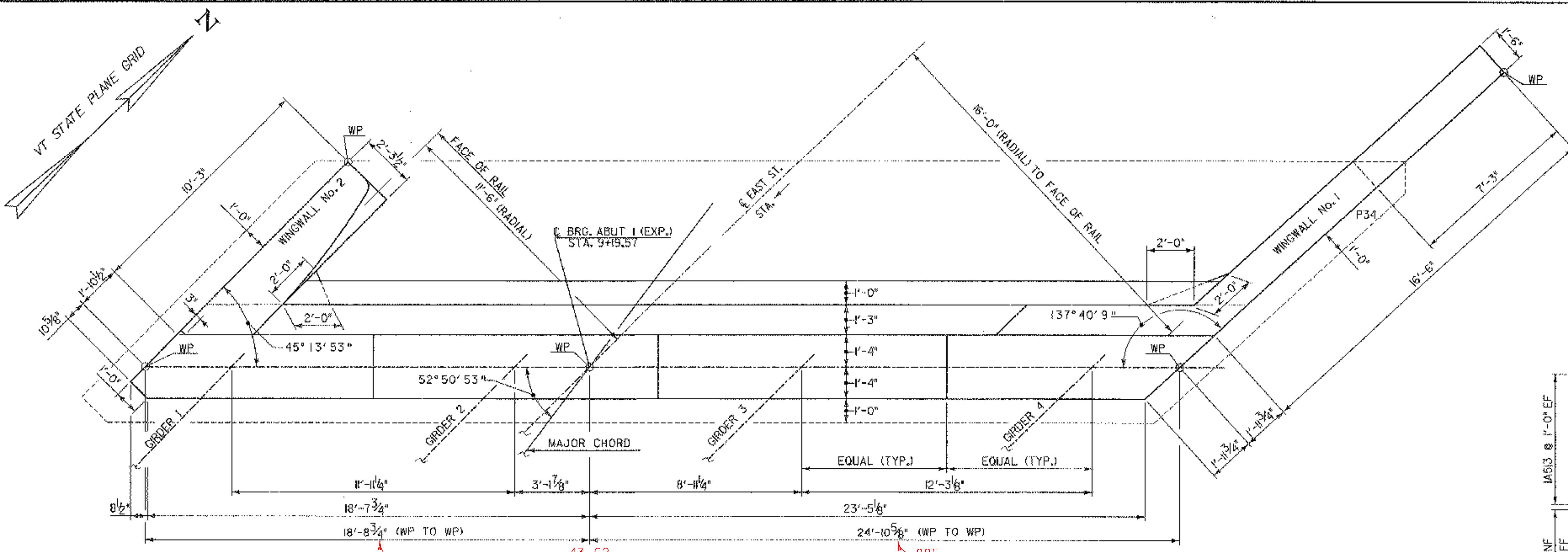
APPROACH SLAB DETAILS

Designed By	S. BURBANK	Drawn By	S. BURBANK
Checked By	Date	Bridge Design Supervisor	Date
M. CHENETTE	08/05	M. CHENETTE	09/05

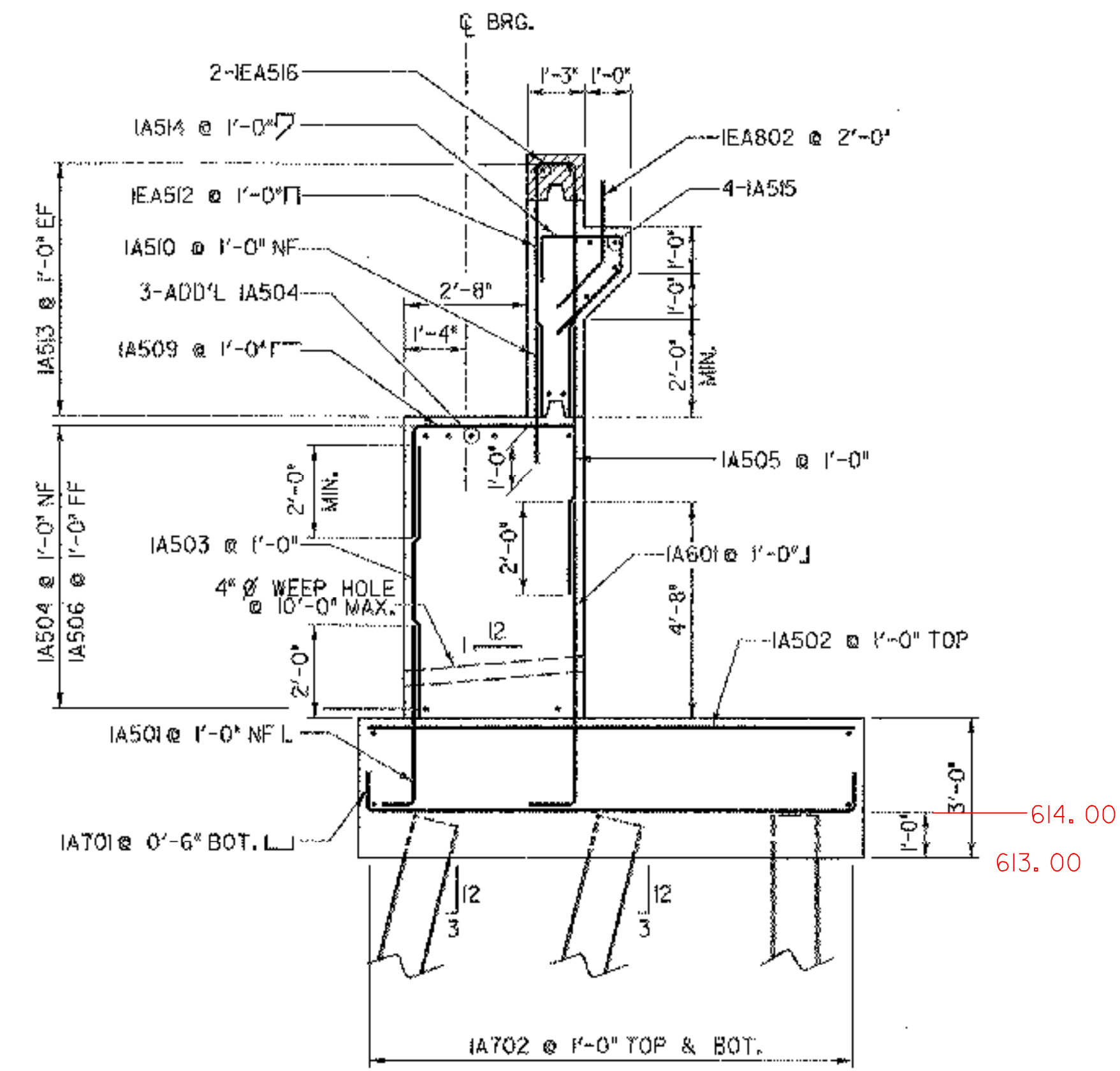
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
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DH Dgn.: ...Cadd\Trans\z01302aprslab.dgn Plot Date: 1/30/2006
 Bridge Sheet No. Sheet 41 of 63

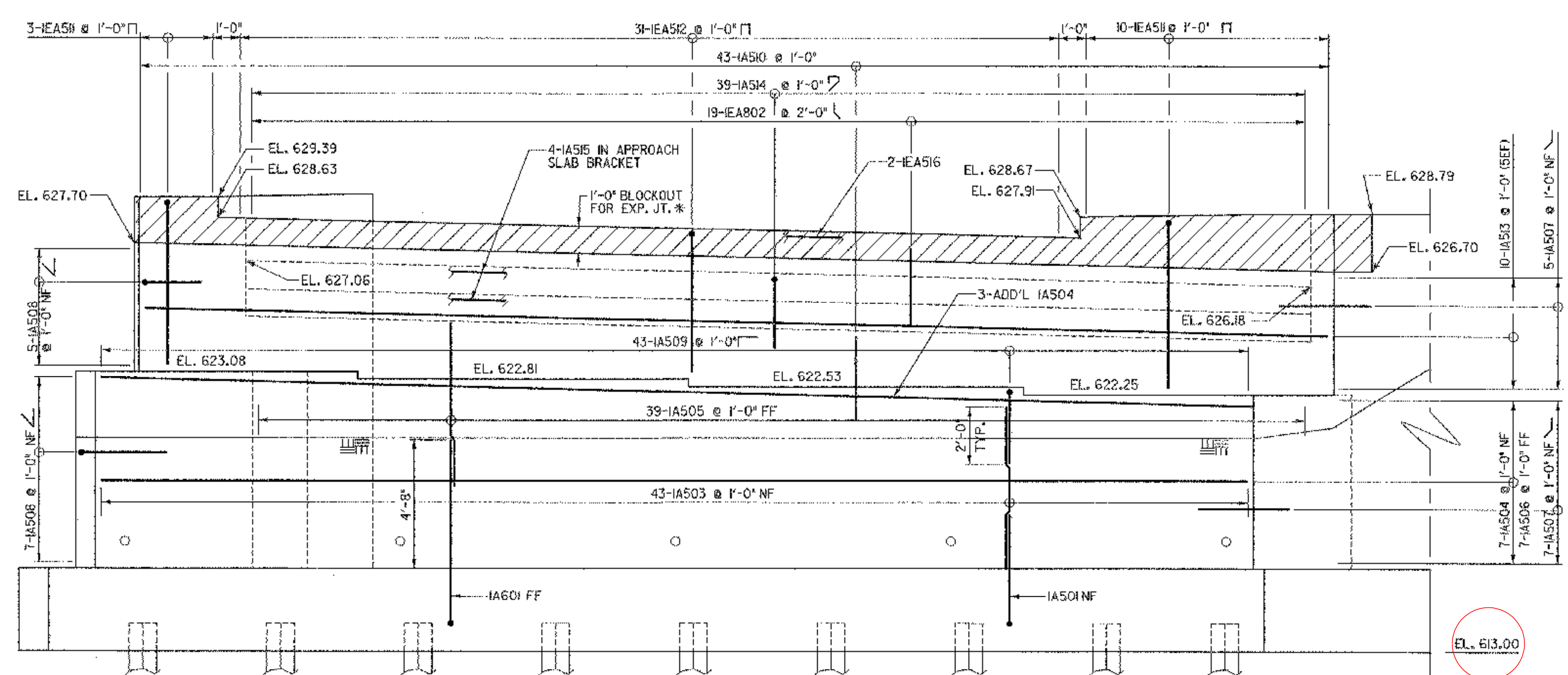




ABUTMENT 1 PLAN
 SCALE 3/8" = 1'-0"
 1 0 1 2 3 4



ABUTMENT NO. 1 TYPICAL SECTION
 SCALE 3/8" = 1'-0"
 1 0 1 2 3 4



ABUTMENT 1 ELEVATION
 SCALE 3/8" = 1'-0"
 1 0 1 2 3 4

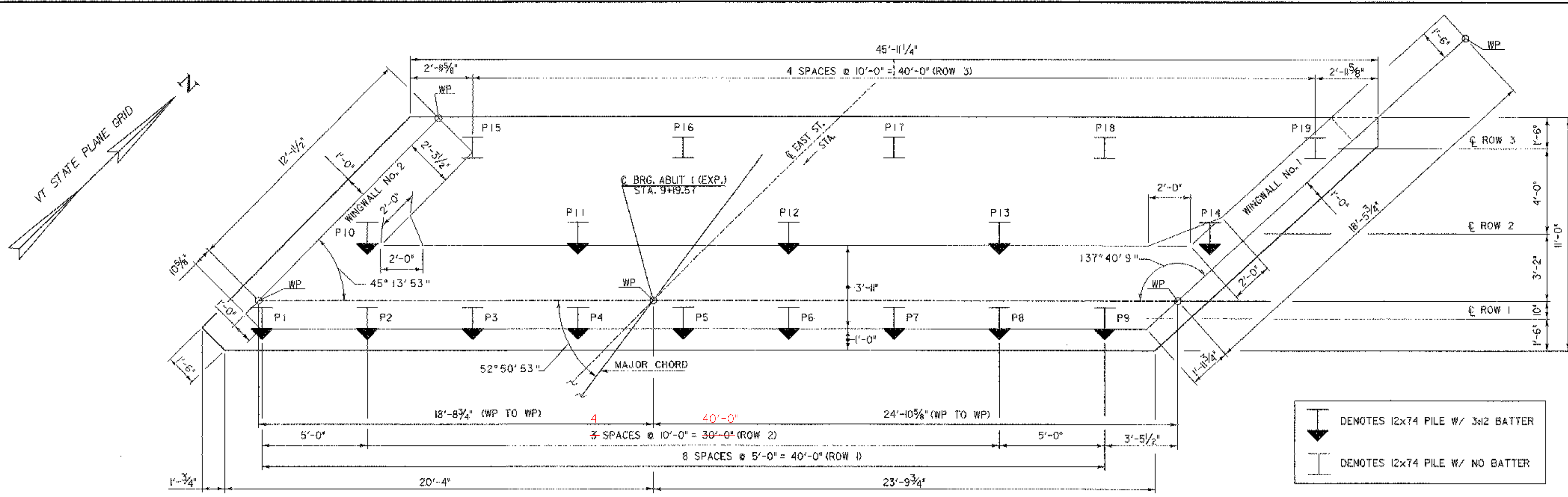
* CONCRETE FOR EXPANSION JOINT BLOCKOUT TO BE HIGH PERFORMANCE CLASS A.

EL. 613.00

STATE OF VERMONT AGENCY OF TRANSPORTATION			
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
ABUTMENT NO. 1 PLAN AND ELEVATION			
Designed By	T. KNIGHT	Drawn By	T. KNIGHT
Checked By	Date	Bridge Design Supervisor	
S. BURBANK	06/05	M. CHENETTE	Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn.:	...Cadd\Trans\z01\302abut1.dgn	Plot Date:	1/12/2006
Bridge Sheet No.		Sheet	42 of 63

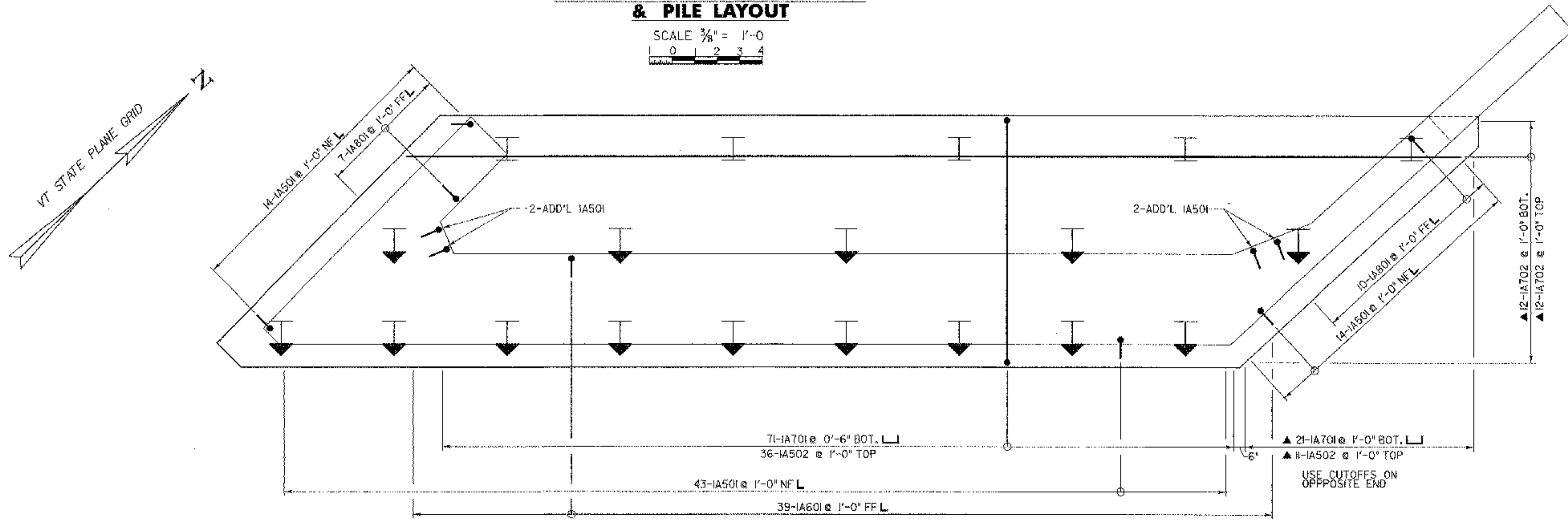
NOTE:

- NF = NEAR FACE
- FF = FAR FACE
- EF = EACH FACE
- ▲ = CUT TO FIT IN FIELD





ABUTMENT 1 - FOOTING PLAN & PILE LAYOUT

SCALE $\frac{3}{8}'' = 1'-0''$
 0 1 2 3 4



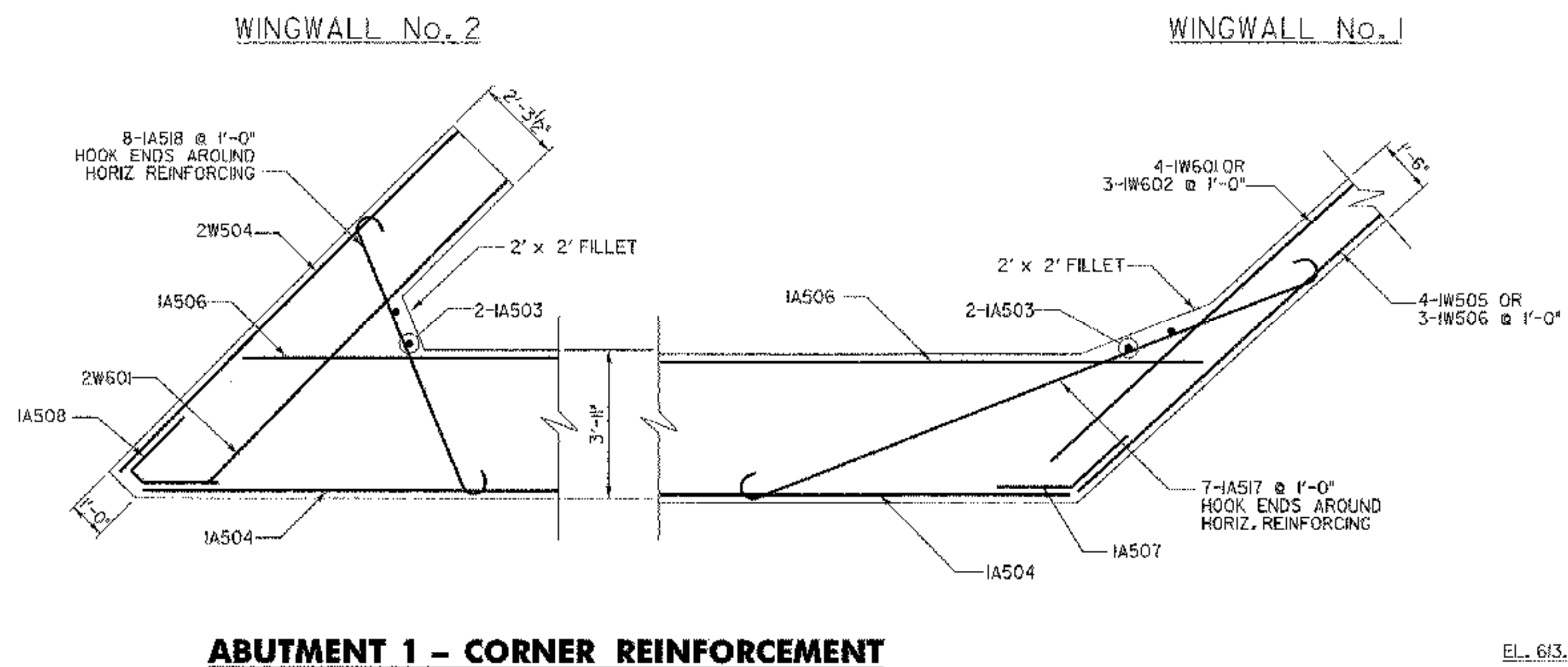
ABUTMENT 1 - FOOTING REINFORCEMENT PLAN

SCALE $\frac{3}{8}'' = 1'-0''$
 0 1 2 3 4

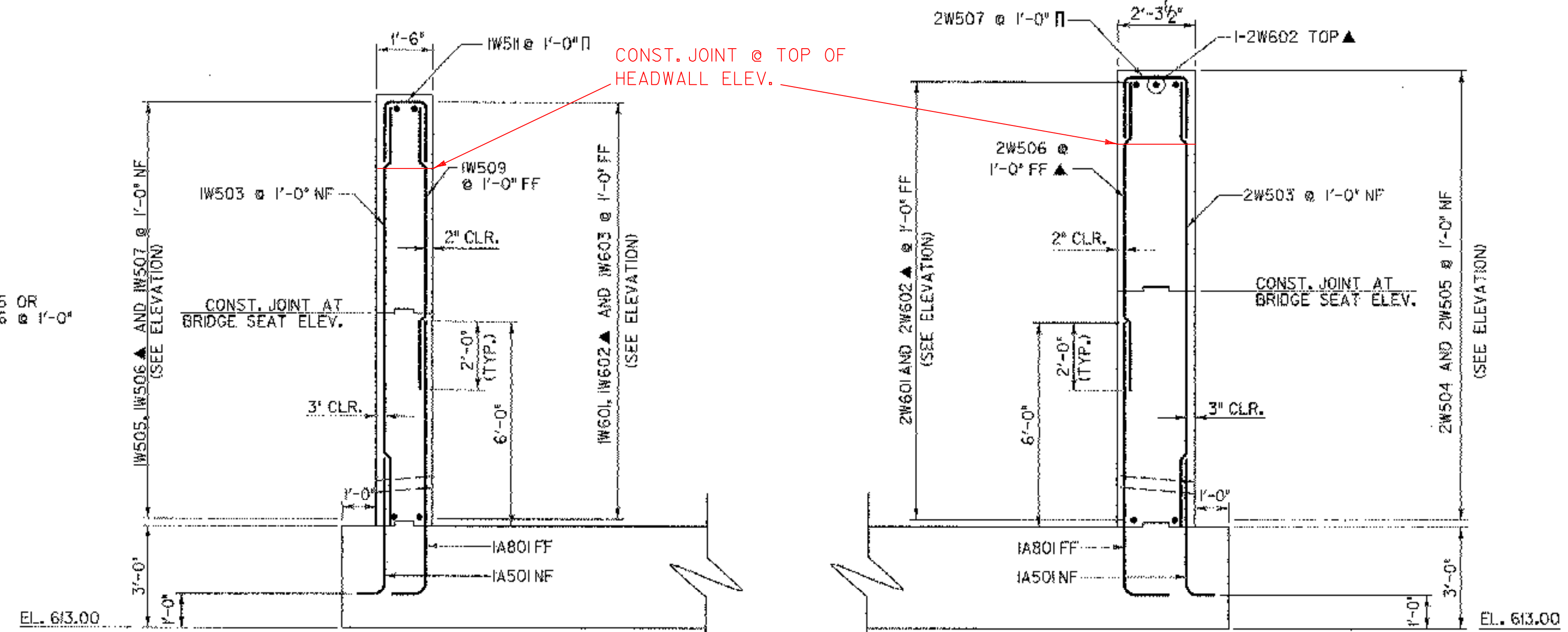
 DENOTES 12x74 PILE W/ 3/4" BATTER
 DENOTES 12x74 PILE W/ NO BATTER

NOTE:
 NF = NEAR FACE
 FF = FAR FACE
 EF = EACH FACE
 ▲ = CUT TO FIT IN FIELD
 3" CLR. UNLESS OTHERWISE SPECIFIED ON THE PLANS.

STATE OF VERMONT AGENCY OF TRANSPORTATION	
Town Of HUNTINGTON	Bridge No. 42
Highway No. T.H. 4	Log Sta. Surv. Sta.
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER	
ABUTMENT NO. 1 FOOTING PLAN	
Designed By T. KNIGHT	Drawn By T. KNIGHT
Checked By S. BURBANK	Bridge Design Supervisor M. CHENETTE
Date 06/05	Date 09/05
PROJECT HUNTINGTON	
BRO 1445 (29)	
DH Dgn. ... \Cadd\Trans\z01302ob+ftgl.dgn Plot Date: 1/12/2006	
Bridge Sheet No.	Sheet 43 of 63

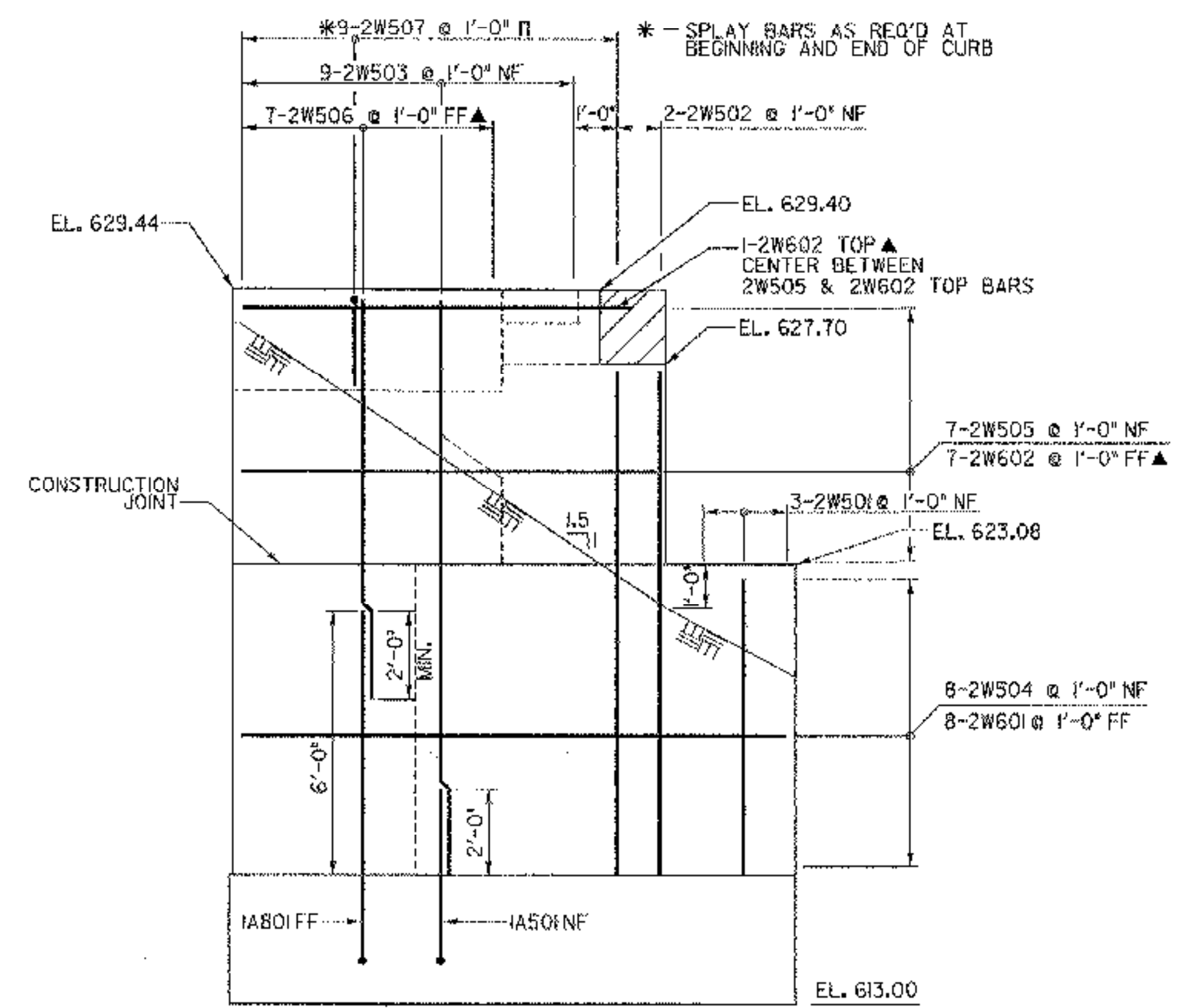


ABUTMENT 1 - CORNER REINFORCEMENT
(BELOW BEAM SEAT)
SCALE 3/8" = 1'-0"
0 1 2 3 4

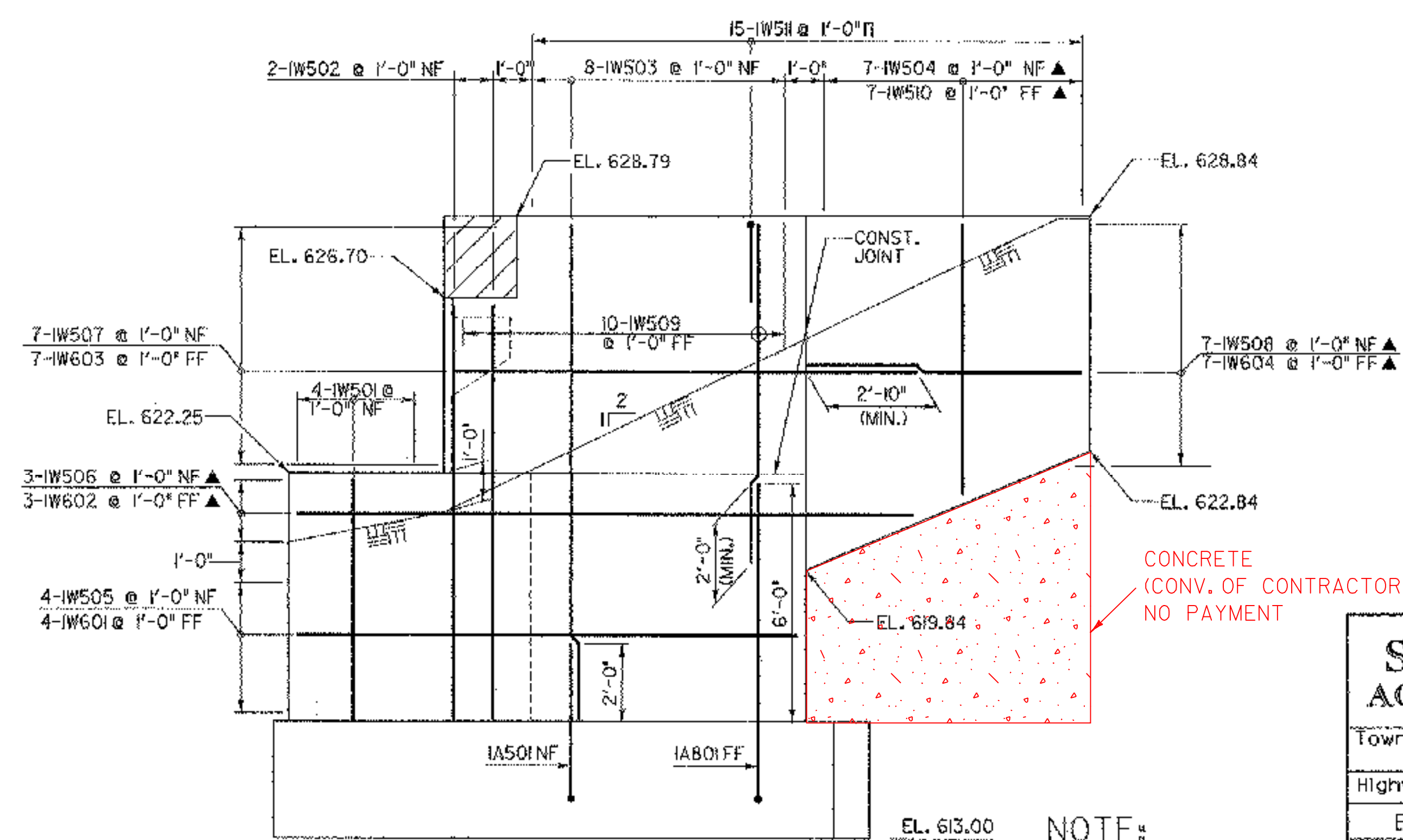


WINGWALL No. 1
SCALE 3/8" = 1'-0"
0 1 2 3 4

WINGWALL No. 2
SCALE 3/8" = 1'-0"
0 1 2 3 4



WINGWALL No. 2 ELEVATION
SCALE 3/8" = 1'-0"
0 1 2 3 4



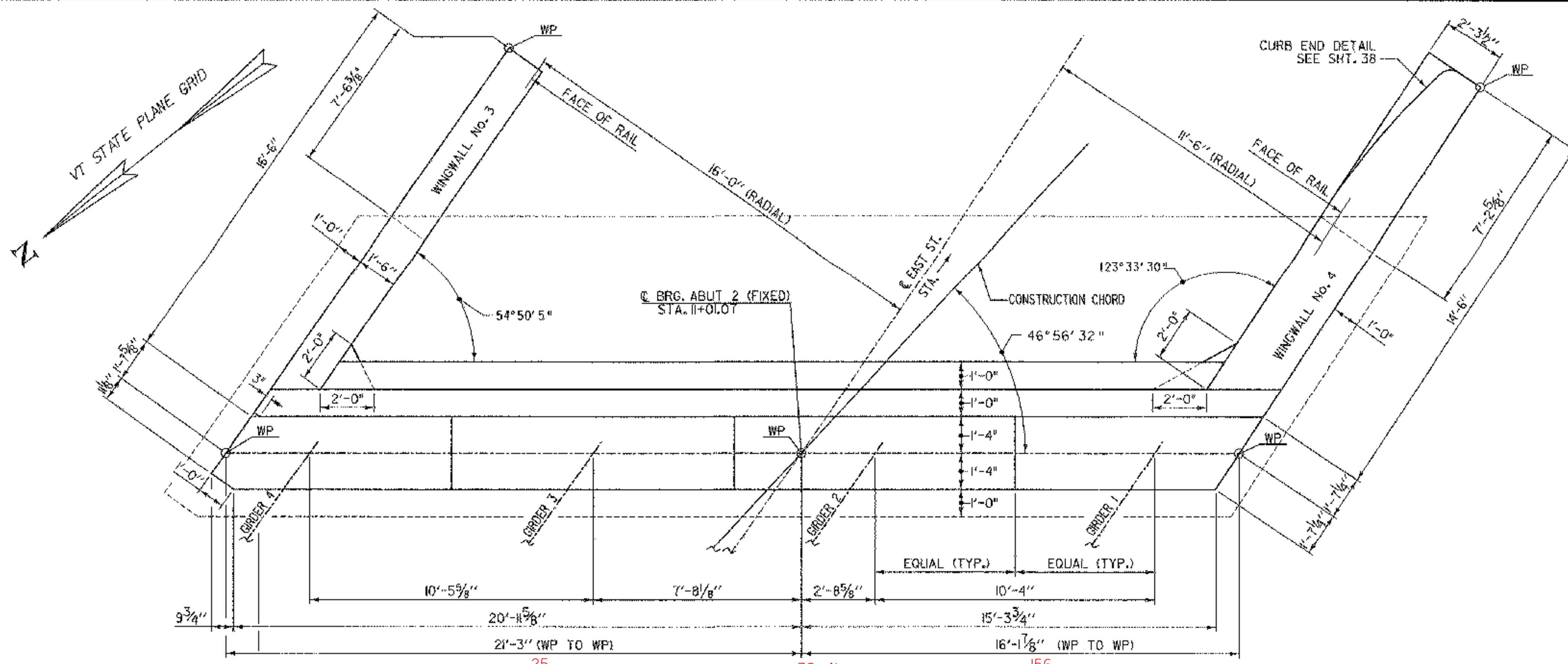
WINGWALL No. 1 ELEVATION
SCALE 3/8" = 1'-0"
0 1 2 3 4

CONCRETE (CONV. OF CONTRACTOR) NO PAYMENT

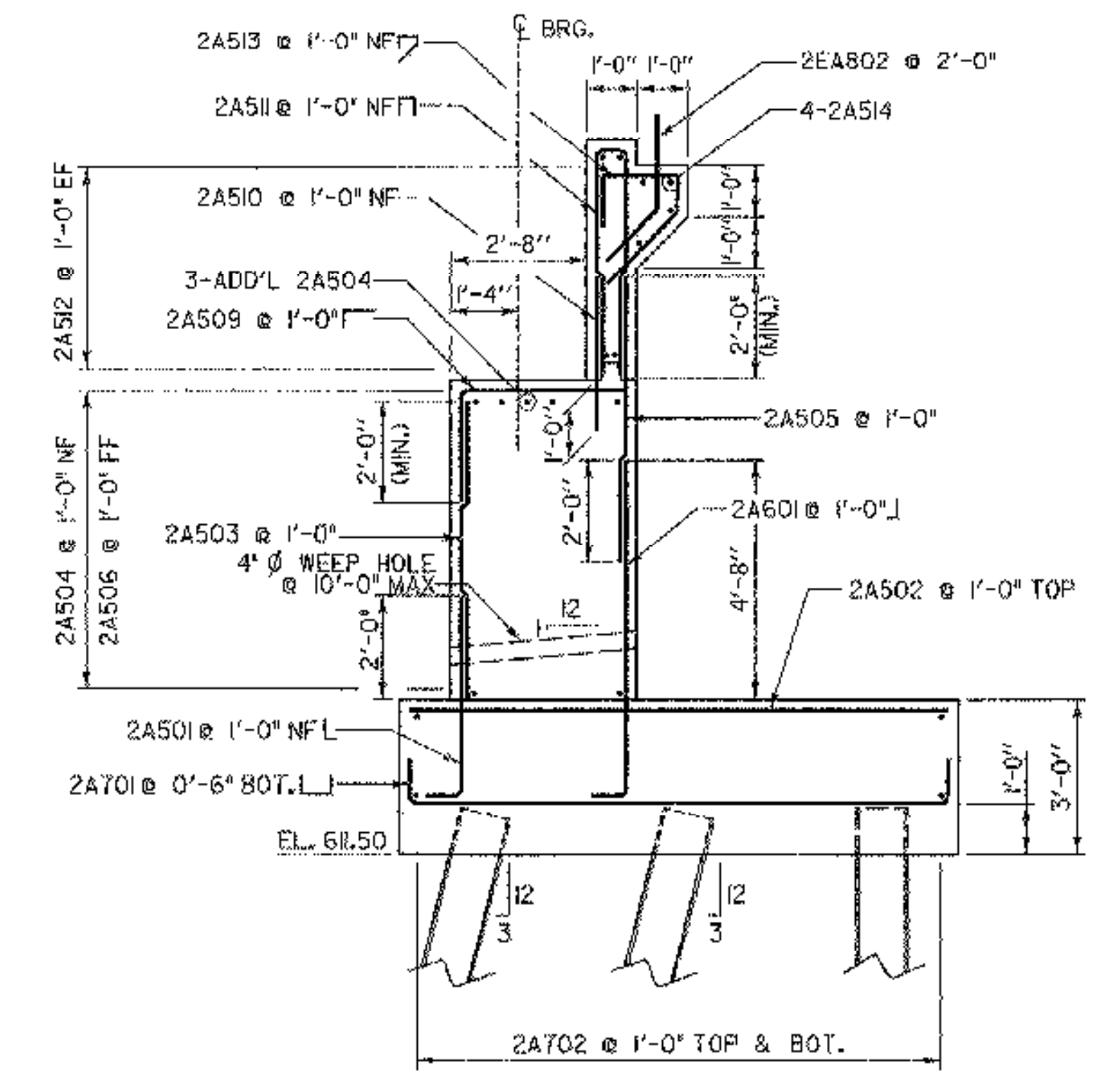
NOTE:
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FF = FAR FACE
EF = EACH FACE
▲ = CUT TO FIT IN FIELD
3" CLR. UNLESS OTHERWISE SPECIFIED ON THE PLANS.

STATE OF VERMONT AGENCY OF TRANSPORTATION	
Town Of HUNTINGTON	Bridge No. 42
Highway No. T.H. 4	Log Sta. Surv. Sta.
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER	
ABUTMENT NO. 1 WINGWALL DETAILS	
Designed By T. KNIGHT	Drawn By J. SOTER
Checked By S. BURBANK	Bridge Design Supervisor M. CHENETTE
Date 06/05	Date 09/05
PROJECT HUNTINGTON	PROJECT NO. BRO 1445 (29)
DH Dgn: ... \cadd\Trans\z01302abufw.dgn Plot Date: 1/12/2006	
Bridge Sheet No.	Sheet 44 of 63

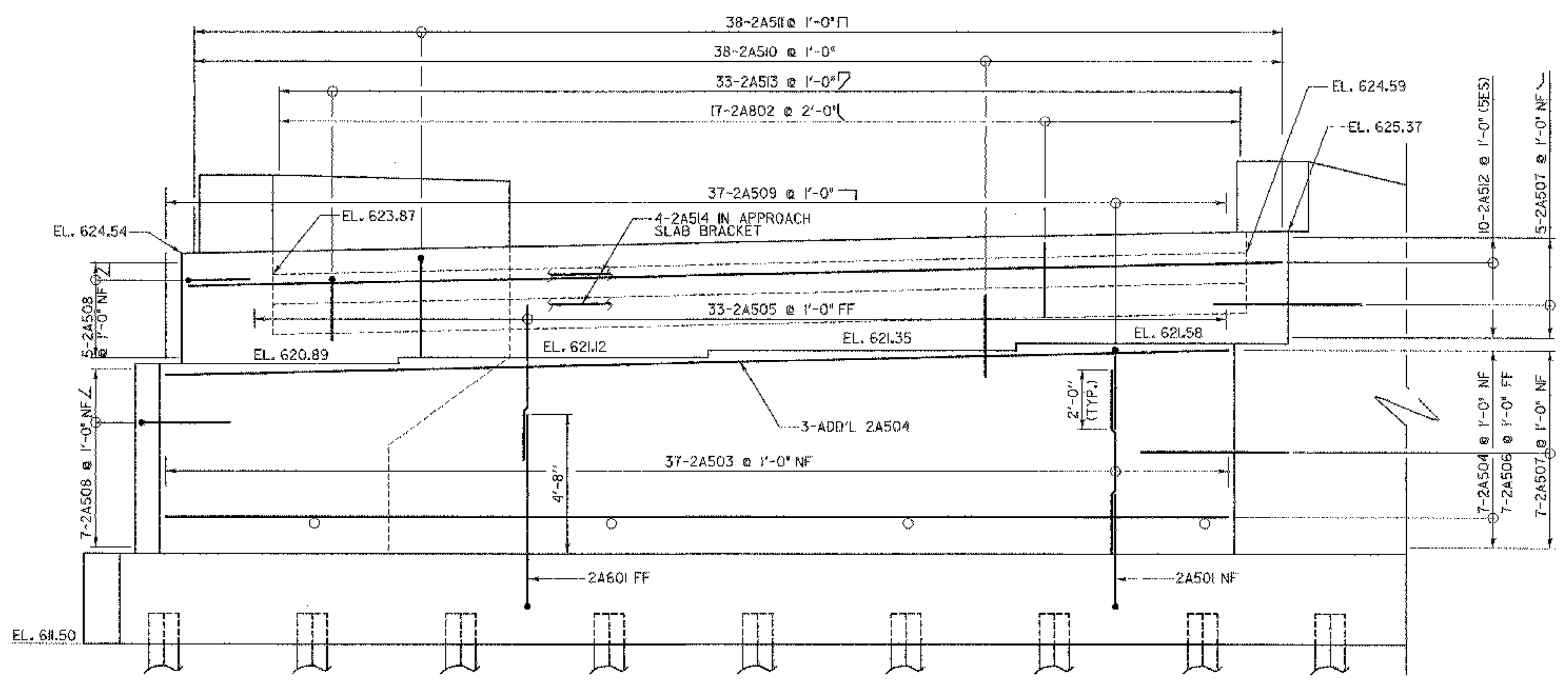




ABUTMENT 2 PLAN
 SCALE 3/8" = 1'-0"
 0 1 2 3 4



ABUTMENT NO. 2 TYPICAL SECTION
 SCALE 3/8" = 1'-0"
 0 1 2 3 4



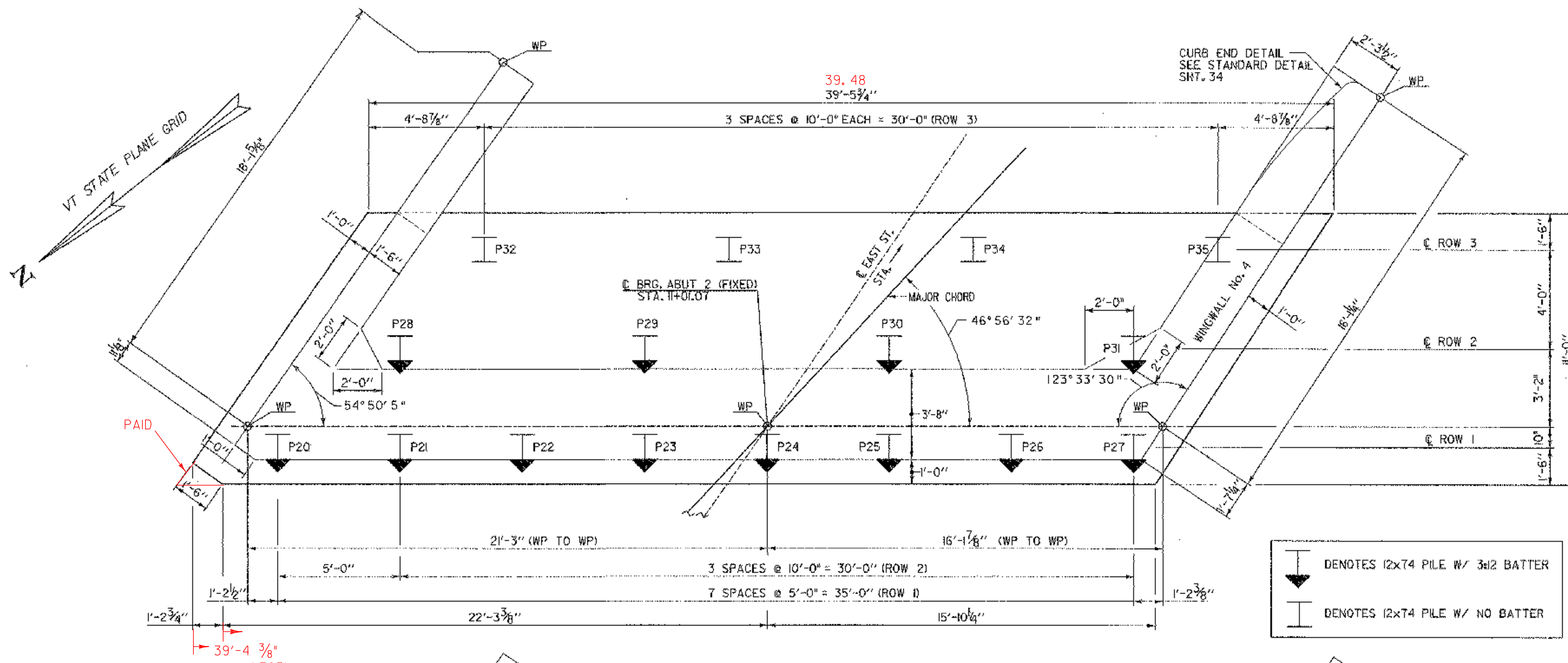
ABUTMENT 2 ELEVATION
 SCALE 3/8" = 1'-0"
 0 1 2 3 4

NOTE:

- NF = NEAR FACE
- FF = FAR FACE
- EF = EACH FACE
- ▲ = CUT TO FIT IN FIELD

STATE OF VERMONT AGENCY OF TRANSPORTATION			
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
ABUTMENT NO. 2 PLAN AND ELEVATION			
Designed By	T. KNIGHT	Drawn By	T. KNIGHT
Checked By	Date	Bridge Design Supervisor	
S. BURBANK	08/05	M. CHENETTE	Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn: ... \Cadd\Trans\201302abut2.dgn Plot Date: 1/12/2006		Bridge Sheet No.	Sheet 45 of 63

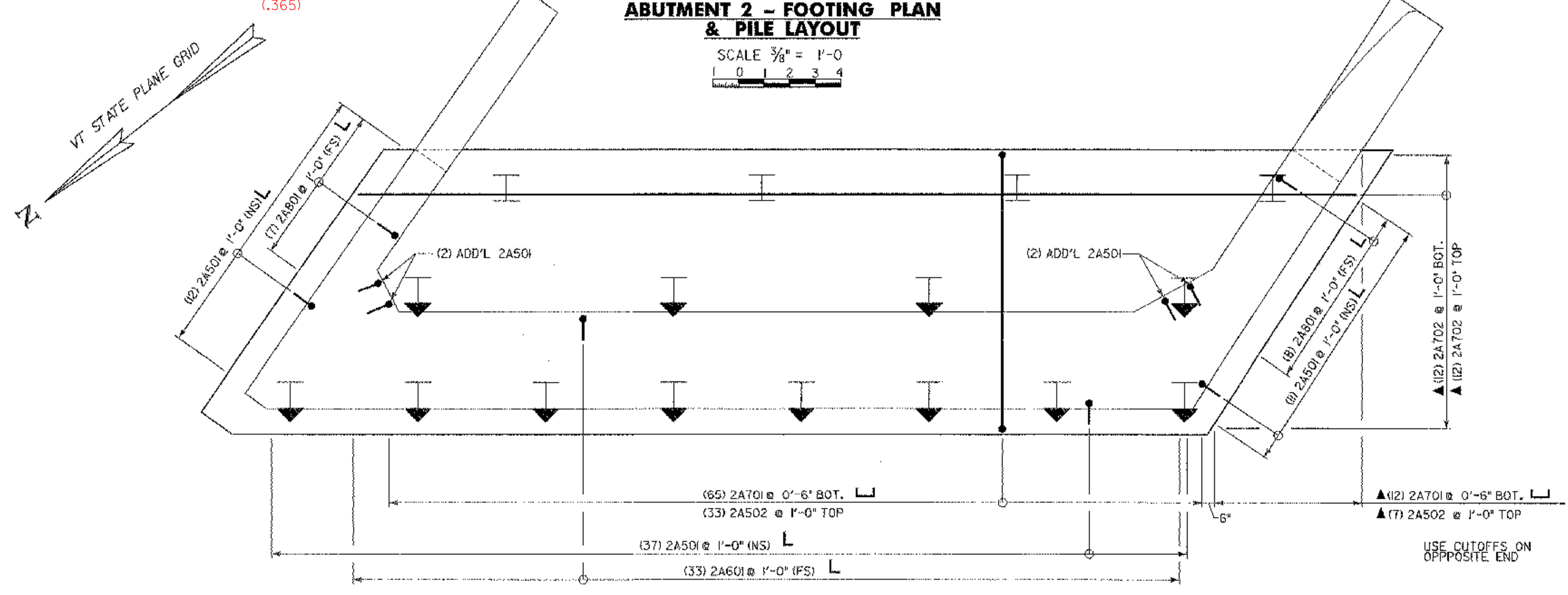




ABUTMENT 2 - FOOTING PLAN & PILE LAYOUT

SCALE 3/8" = 1'-0"
 0 1 2 3 4

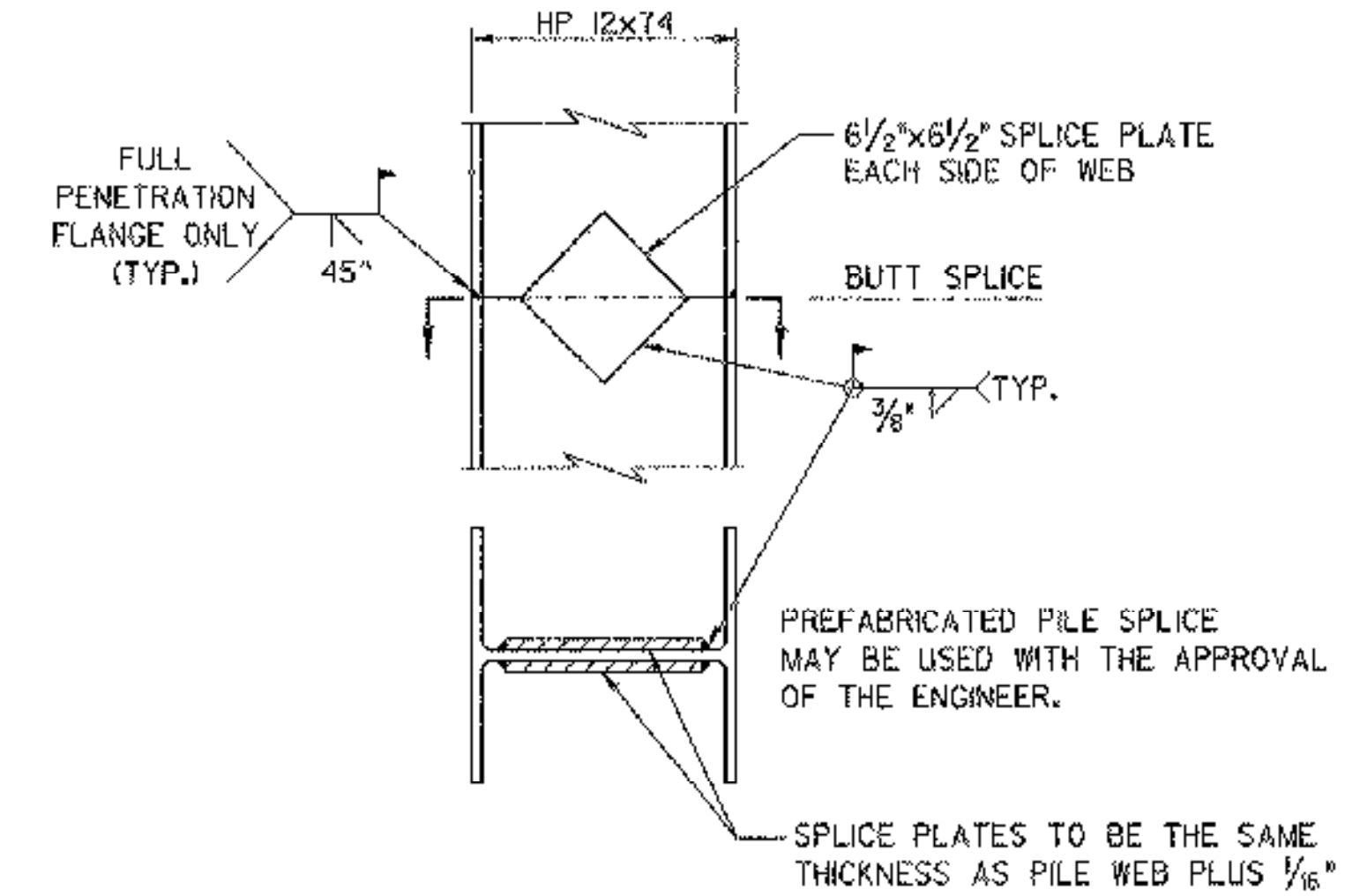
DENOTES 12x74 PILE W/ 3/4" BATTER
 DENOTES 12x74 PILE W/ NO BATTER



ABUTMENT 2 - FOOTING REINFORCEMENT PLAN

SCALE 3/8" = 1'-0"
 0 1 2 3 4

▲ DENOTES BAR TO BE CUT IN FIELD



DETAIL OF TYPICAL PILE SPlice
 NOT TO SCALE

	INFO	DRIVEN	
P.34	TEST PILE	10/5	RETEST-PAY
		CUTOFF @612.5	
P.35		10/5	INCOMPLETE
P.32	48'	10/5	
P.27	46'	10/5	
P.26	46'	10/5	
P.25	50'	10/6	
P.24	46'	10/6	
P.23	46.5'	10/6	
P.22	47'	10/6	
P.33	48'	10/5	
P.21	50'	10/9	
P.20	46'	10/9	
P.35	86'	10/10	
P.28	51'	10/9	
P.29	49'	10/10	
P.30	52'	10/10	
P.31	48'	10/10	
760 TOTAL			

ADD'L PILE LENGTH DUE TO OVER EXCAVATION BY THE CONTRACTOR. EXCAVATION USED TO DETERMINE PILE LENGTH, VS TRANSIT GRADE USED TO DETERMINE CUT OFF LOCATION.

NOTE:

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 FF = FAR FACE
 EF = EACH FACE
 ▲ = CUT TO FIT IN FIELD
 3" CLR. UNLESS OTHERWISE SPECIFIED ON THE PLANS.

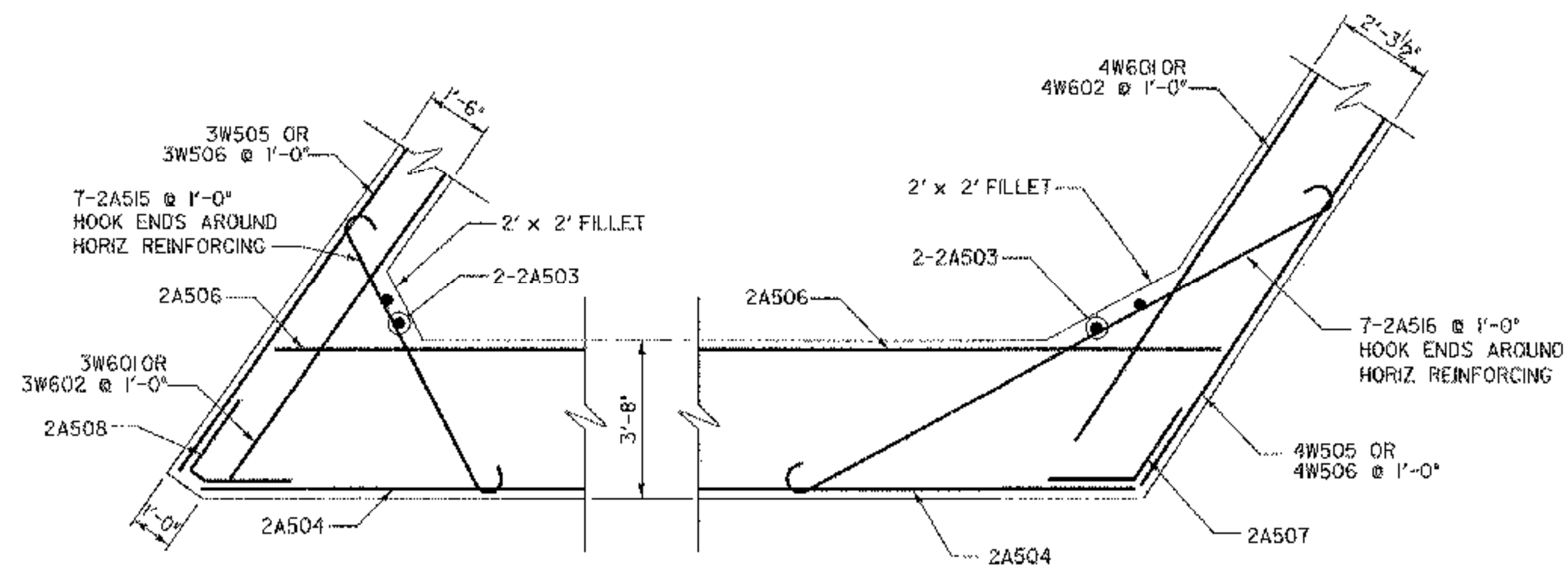


STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
ABUTMENT NO. 2 FOOTING PLAN			
Designed By	T. KNIGHT	Drawn By	T. KNIGHT
Checked By	Date	Bridge Design Supervisor	
S. BURBANK	06/05	M. CHENETTE	Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Lgn: ...Code\Trans\2013020b1ftg2.dgnPlot Date: 1/12/2006		Bridge Sheet No.	Sheet 46 of 63

WINGWALL No. 3

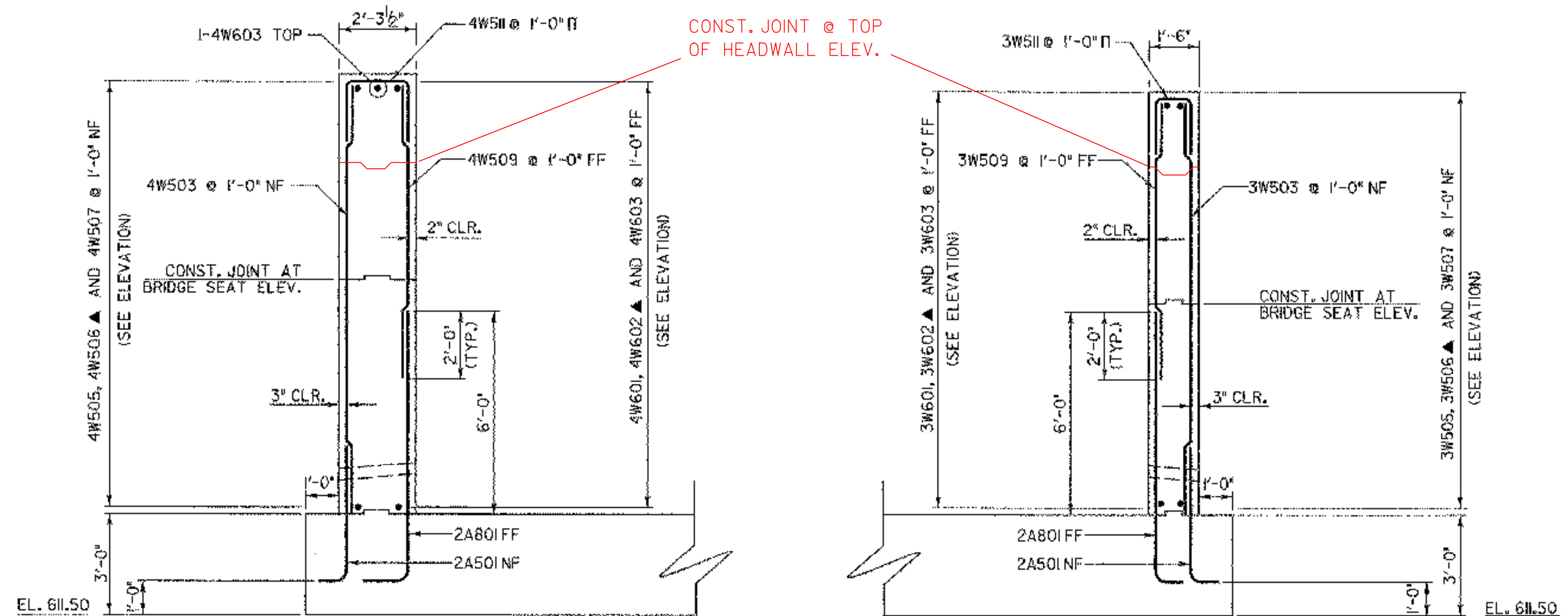
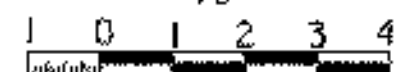
WINGWALL No. 4



ABUTMENT 2 - CORNER REINFORCEMENT

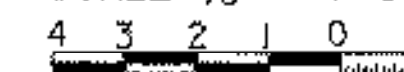
(BELOW BEAM SEAT)

SCALE 3/8" = 1'-0"



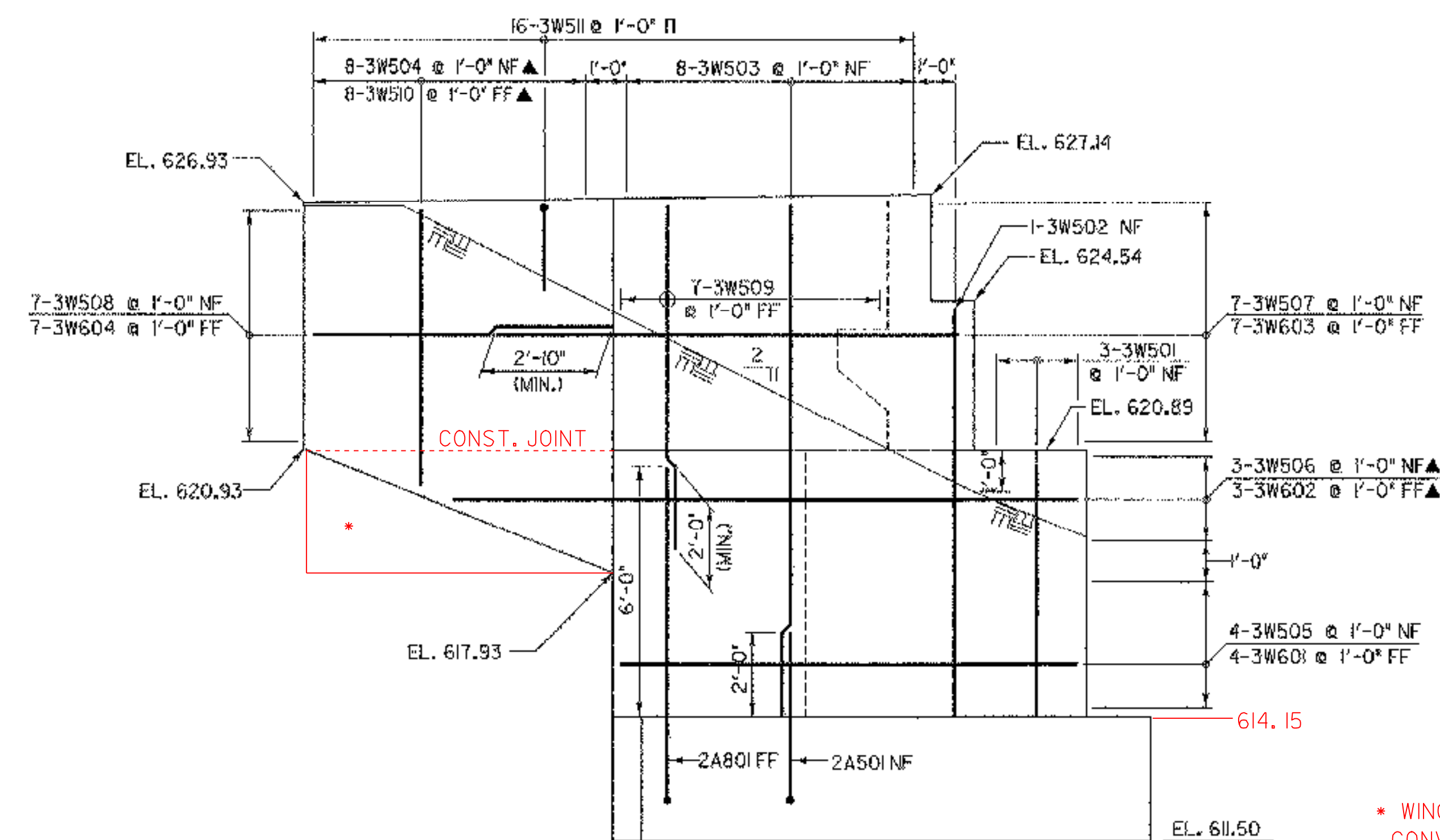
WINGWALL No. 4

SCALE 3/8" = 1'-0"



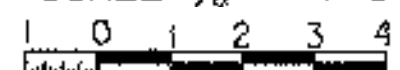
WINGWALL No. 3

SCALE 3/8" = 1'-0"

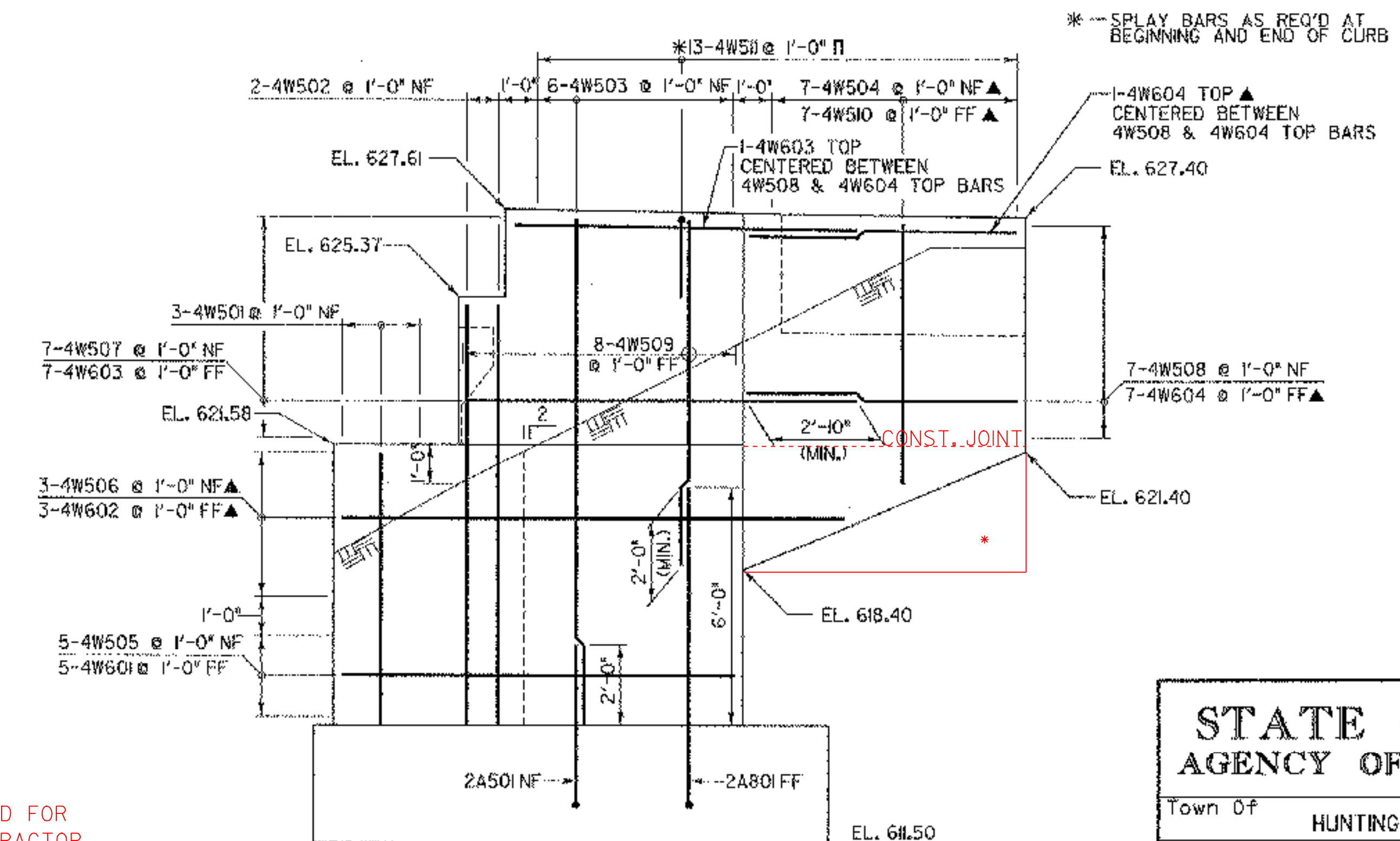


WINGWALL No. 3 ELEVATION

SCALE 3/8" = 1'-0"



* WING FILLET EXCLUDED FOR CONVENIENCE OF CONTRACTOR



WINGWALL No. 4 ELEVATION

SCALE 3/8" = 1'-0"



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EF = EACH FACE
▲ = CUT TO FIT IN FIELD
3" CLR. UNLESS OTHERWISE SPECIFIED ON THE PLANS.

**STATE OF VERMONT
AGENCY OF TRANSPORTATION**

Town Of HUNTINGTON Bridge No. 42
Highway No. T.H. 4 Log Sta.
Surv. Sta.

EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

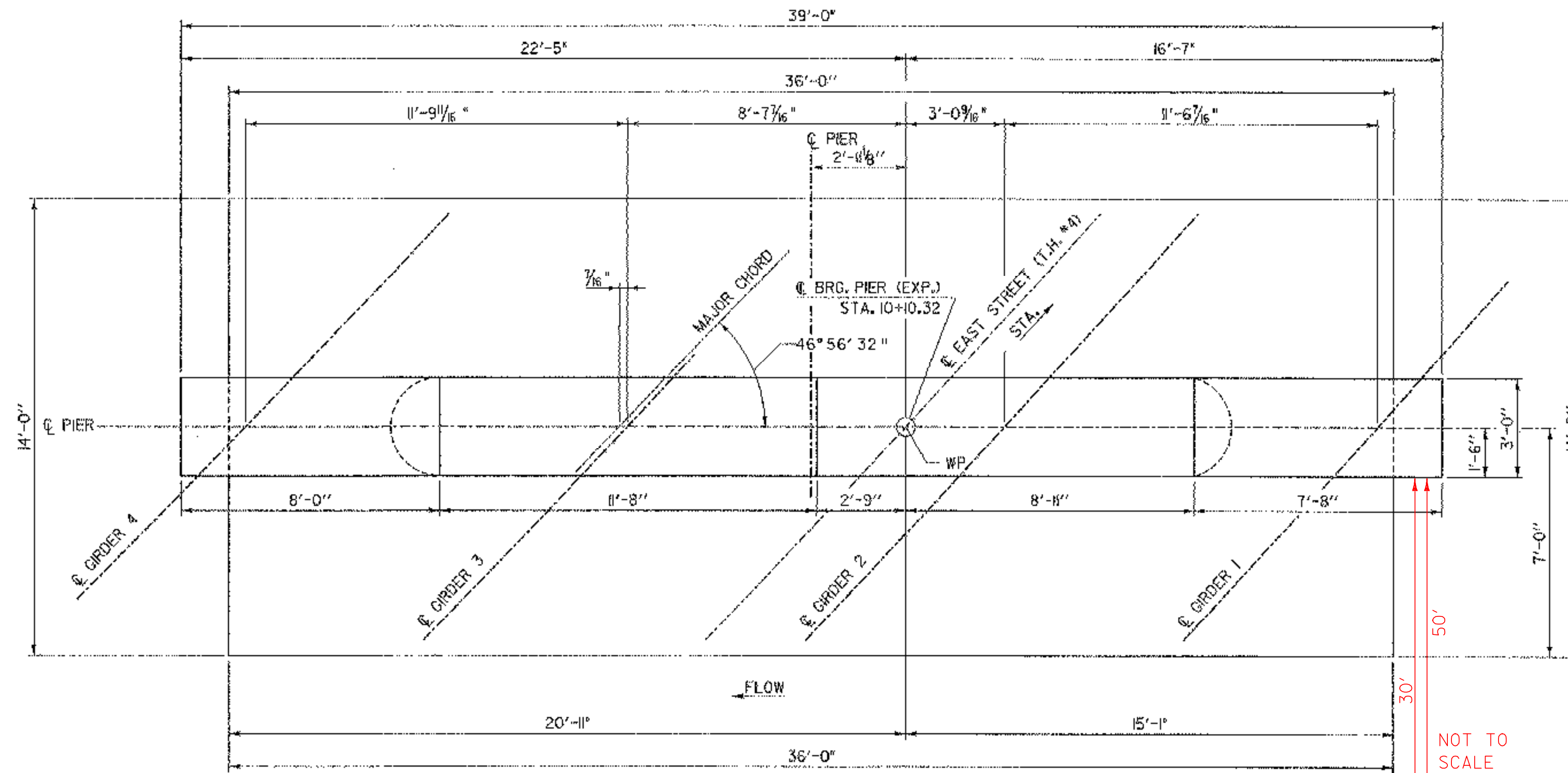
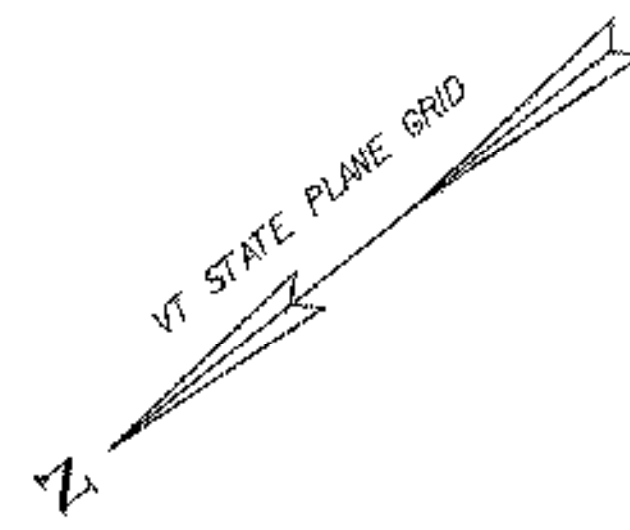
ABUTMENT NO. 2 WINGWALL DETAILS

Designed By T. KNIGHT Drawn By J. SOTER
Checked By Date Bridge Design Supervisor
S. BURBANK 06/05 M. CHENETTE Date 09/05

PROJECT HUNTINGTON PROJECT NO.
BRO 1445 (29)

DH Dgn: ...Cadd\Trans\z01\302abut2w.dgn Plot Date: 1/30/2006
Bridge Sheet No. Sheet 47 of 63

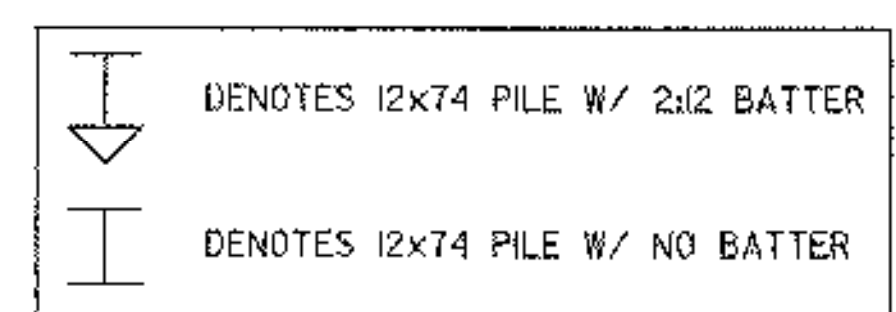
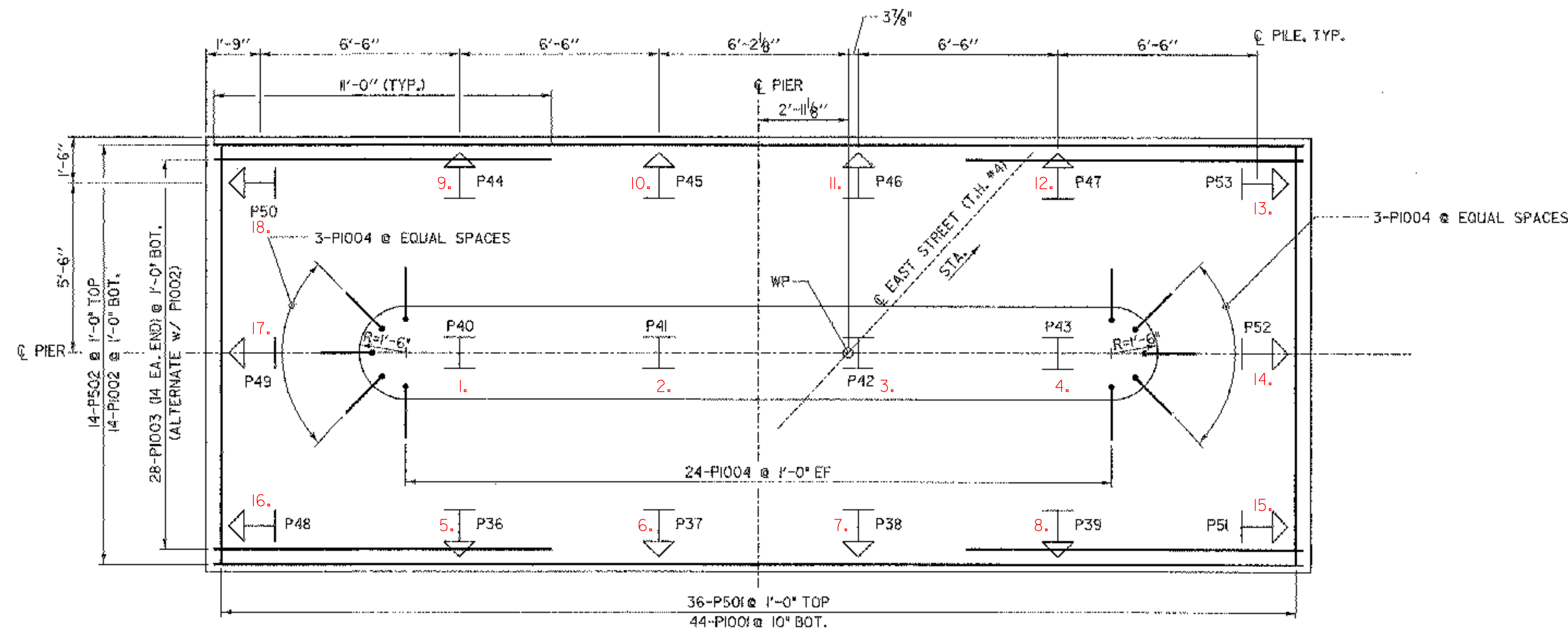
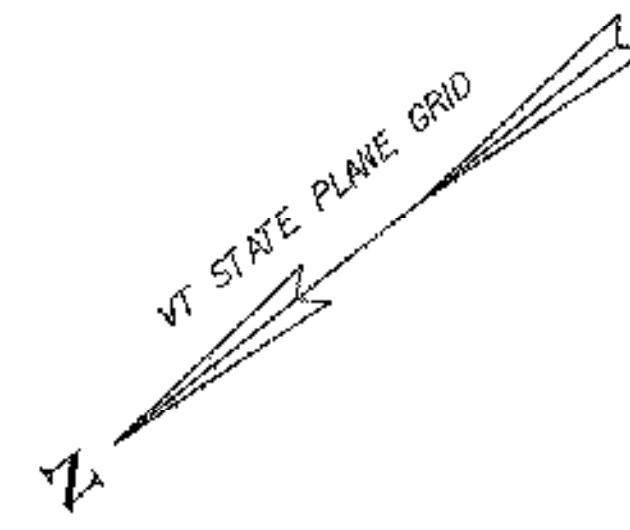




COL #	PD. FT.	ACTUAL DRIVEN
1. P. 40	TEST PILE	77'
2. P. 41	75'	74.6'
3. P. 42	76'	75.9'
4. P. 43	78'	77.5'
5. P. 36	78'	77.8'
6. P. 37	76'	75.7'
7. P. 38	76'	76.2'
8. P. 39	76'	76.4'
9. P. 44	78'	78.2'
10. P. 45	78'	78.2'
11. P. 46	80'	79.9'
12. P. 47	79'	78.5'
13. P. 53	77'	77.15'
14. P. 52	77'	76.85'
15. P. 51	76'	75.65'
16. P. 58	81'	81.1'
17. P. 49	80'	80.0'
18. P. 50	80'	79.6'

1321 TOTAL PAID
PILE DRIVEN TO REFUSAL
AVERAGE DEPTH TO LEDGE FROM 600.0'
IS 77.66'

NOT TO SCALE
50'
30'
20'

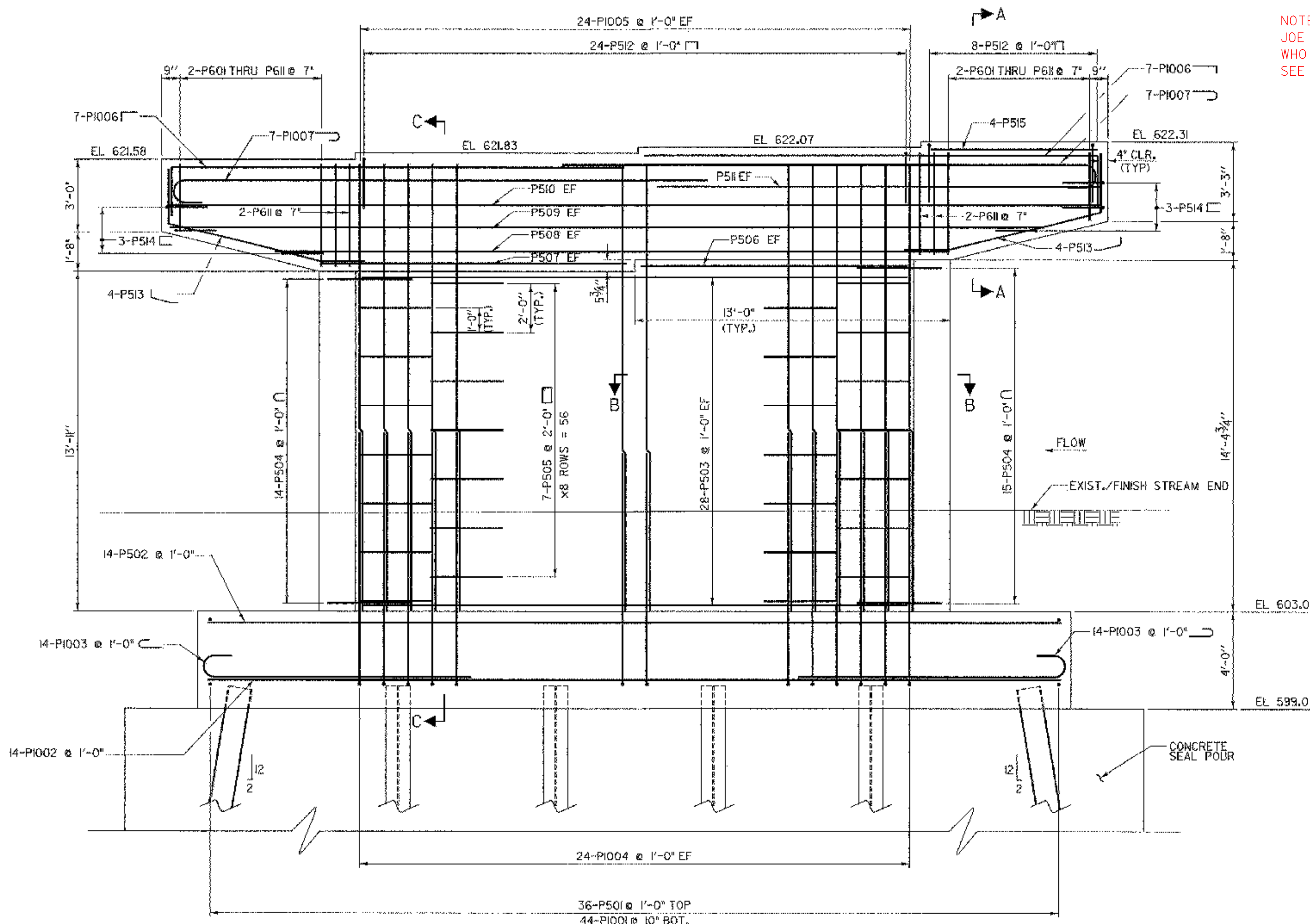


NOTE:
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FF = FAR FACE
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3" CLR. UNLESS OTHERWISE SPECIFIED ON THE PLANS.

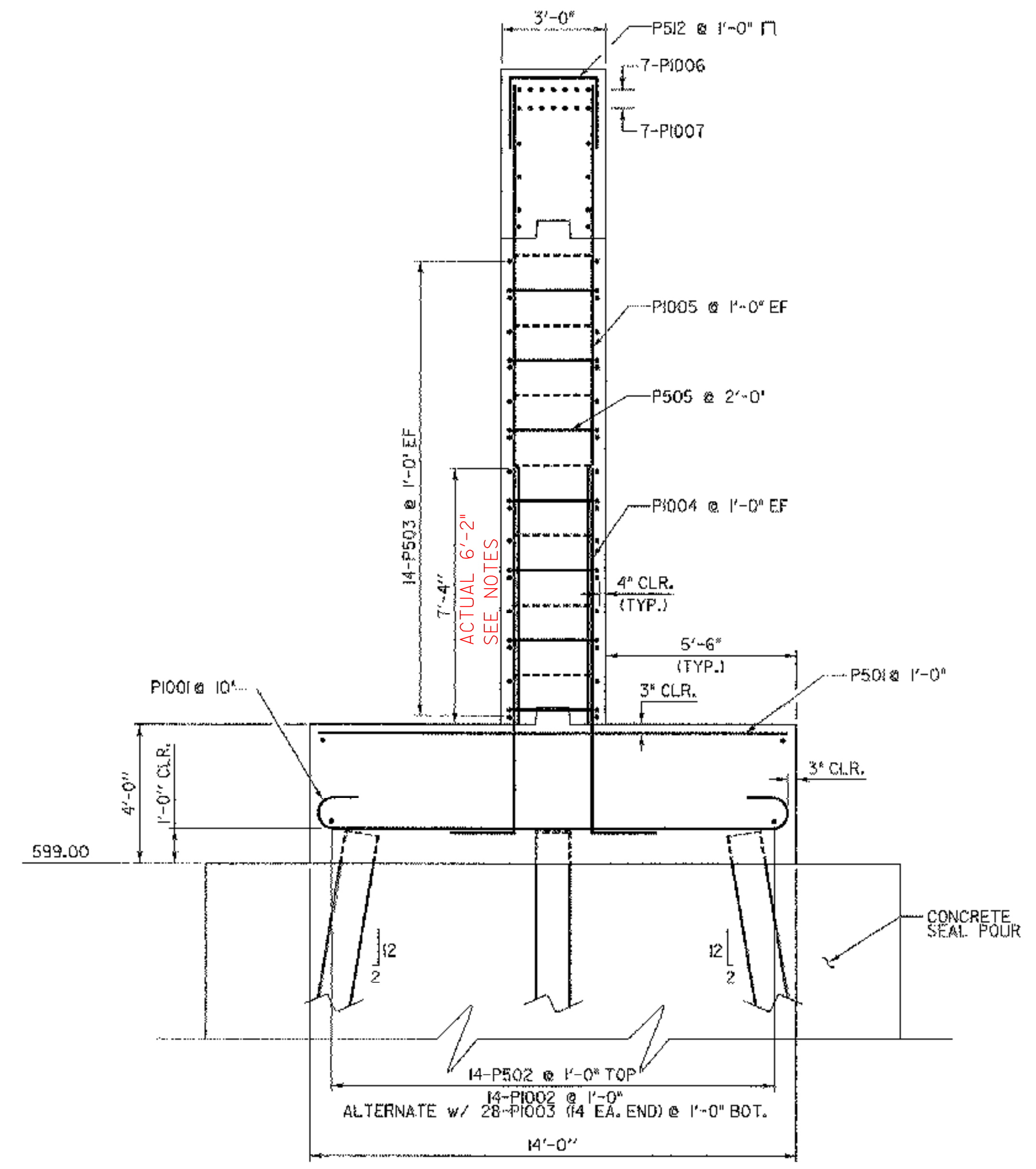
STATE OF VERMONT AGENCY OF TRANSPORTATION			
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
PIER DETAILS 1			
Designed By	M. CHENETTE	Drawn By	J. SOTER
Checked By	Date	Bridge Design Supervisor	
G. BOGUE	08/05	M. CHENETTE	Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgnr ... \Cadd\Trans\201302\pier1.dgn		Plot Date:	1/12/2005
Bridge Sheet No.		Sheet	48 of 63



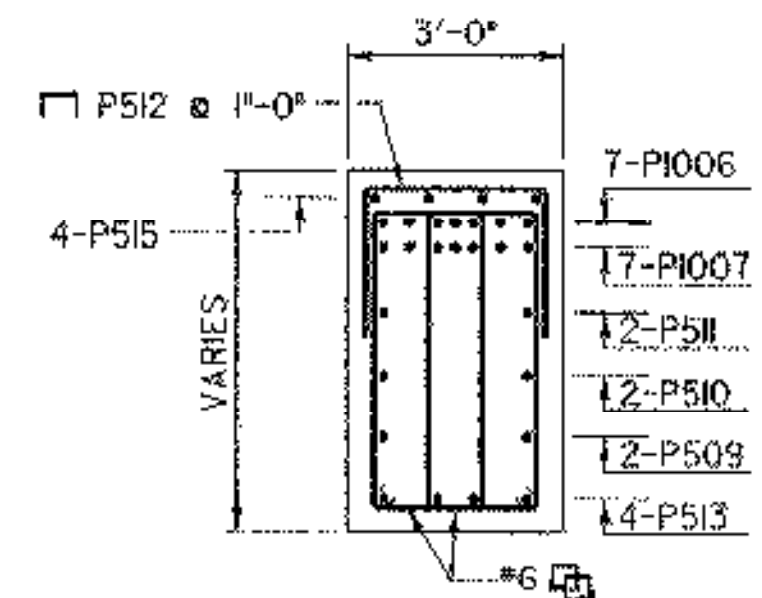
NOTES:
 JOE L. IDENTIFIED DISCREPANCY, CALLED MICKE CHEVETTE,
 WHO OK'D STARTER LENGTH DUE TO CLOSE DOWEL SPACING.
 SEE BRIDGE CORRESPONDANCE FILE.



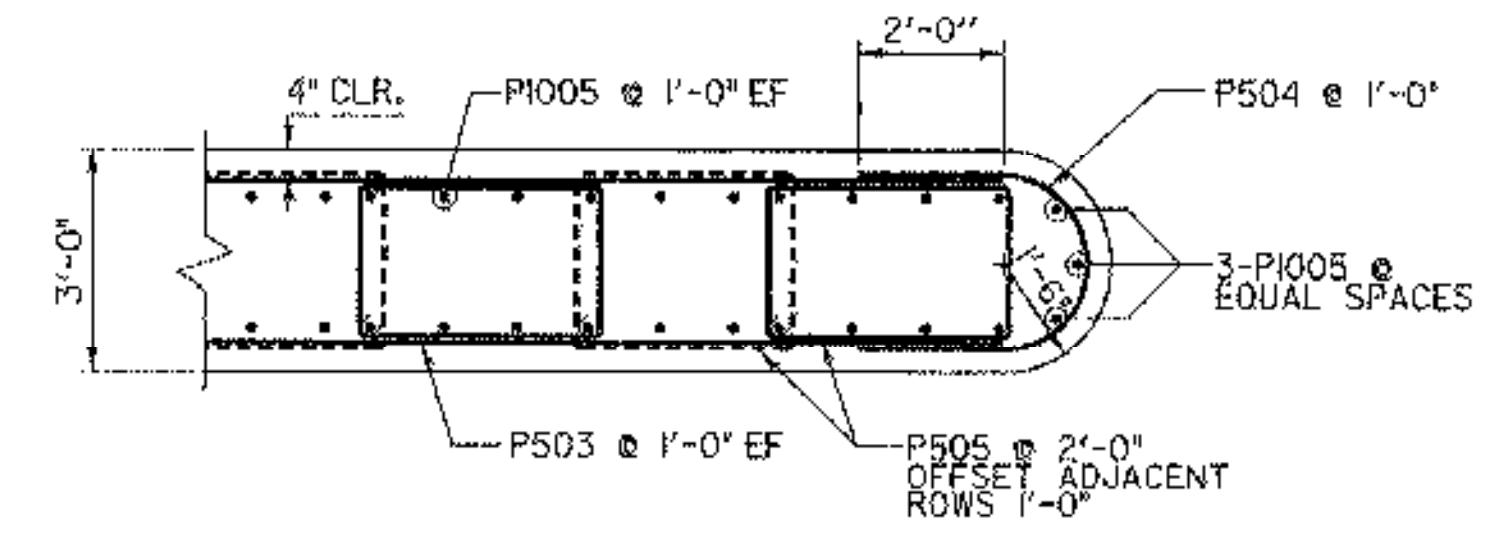
ELEVATION
 SCALE 3/8" = 1'-0"
 1 0 1 2 3 4



SECTION C-C
 SCALE 3/8" = 1'-0"
 1 0 1 2 3 4



SECTION A-A
 SCALE 3/8" = 1'-0"
 1 0 1 2 3 4



SECTION B-B
 SCALE 3/8" = 1'-0"
 1 0 1 2 3 4

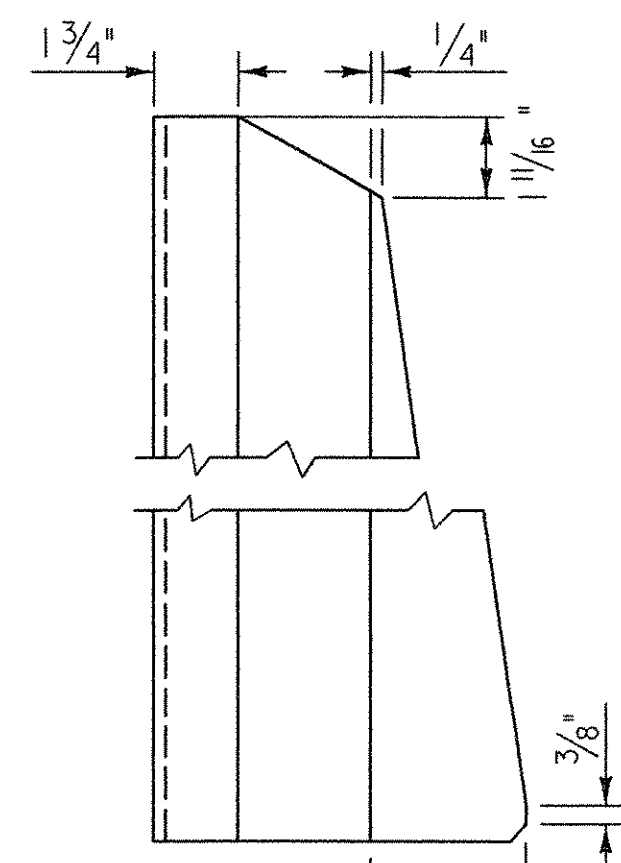
**STATE OF VERMONT
 AGENCY OF TRANSPORTATION**

Town Of **HUNTINGTON** Bridge No. **42**
 Highway No. **T.H. 4** Log Sta. _____
 Surv. Sta. _____
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

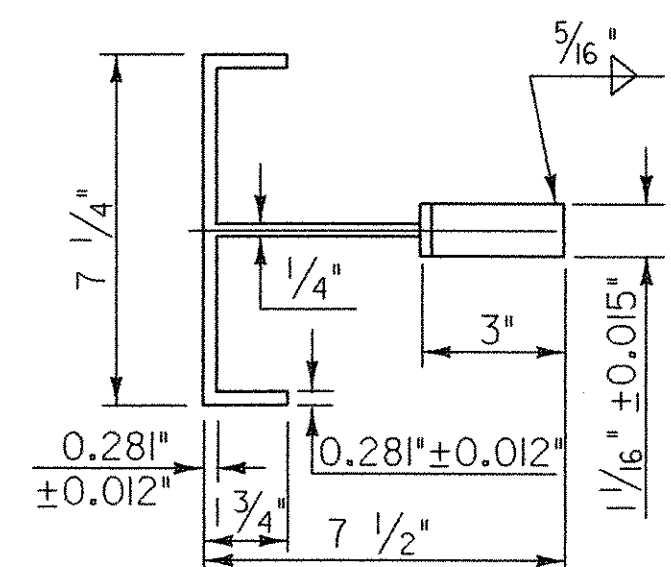
PIER DETAILS 2

Designed By **M. CHENETTE** Drawn By **J. SOTER**
 Checked By _____ Date _____ Bridge Design Supervisor _____
G. BOGUE 06/05 **M. CHENETTE** Date 09/05
 PROJECT **HUNTINGTON** PROJECT NO. **BRO 1445 (29)**
 DH Dgnr: ... \Cadd\Trans\201302pter2.dgn Plot Date: 1/12/2006
 Bridge Sheet No. _____ Sheet 49 of 63

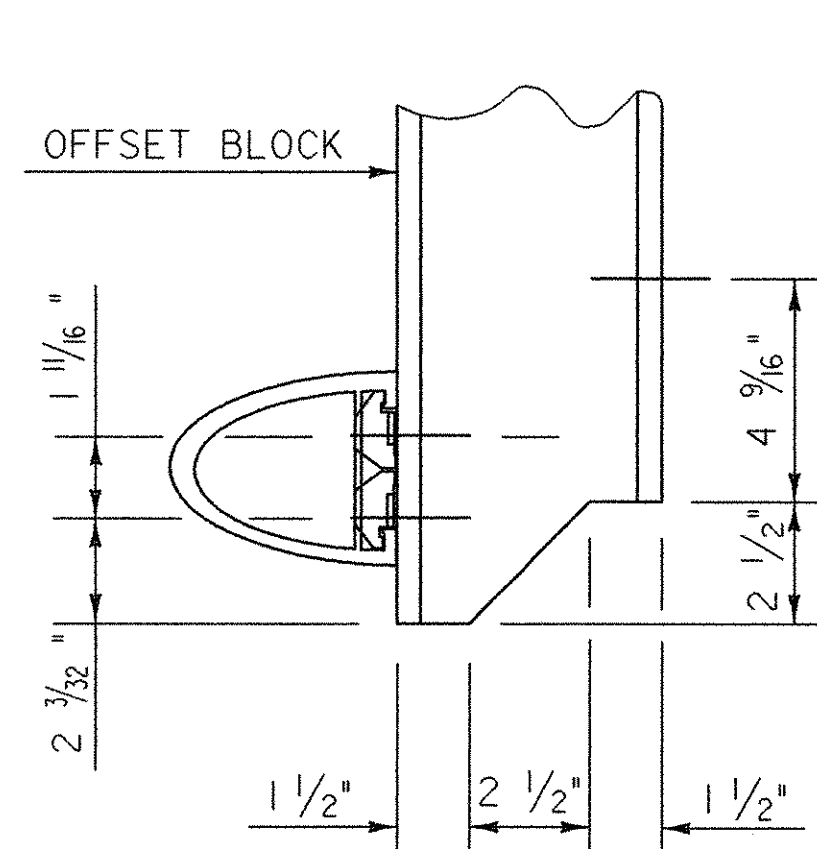




POST SIDE VIEW



POST PLAN VIEW



COPING DETAIL

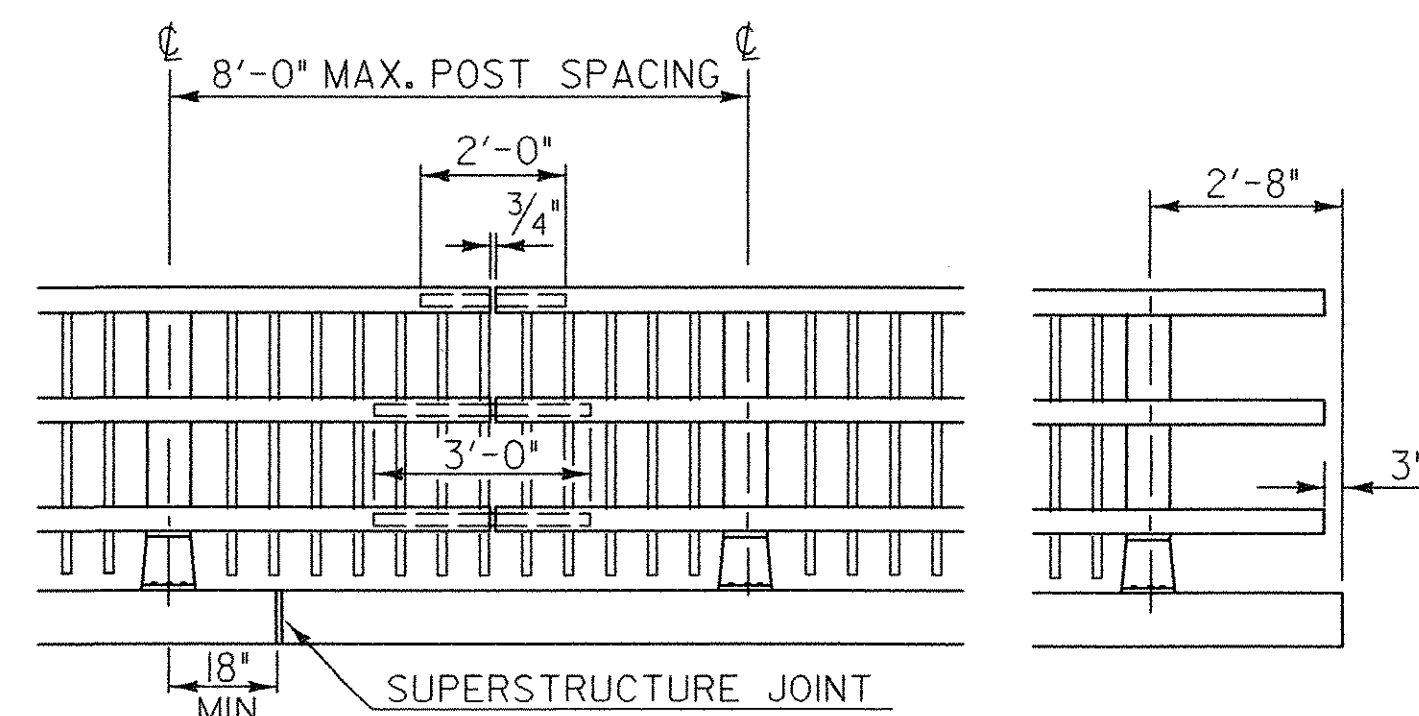
THIS REFLECTORIZED ALUMINUM DELINEATOR IS TO BE ERECTED EVERY 30 FEET (OR CLOSEST POST) WITH 2 NO. 8 X 3/4\"/>

DELINEATORS SHALL MEET SPECIFICATION REQUIREMENTS FOR ASTM B209 ALLOY 5052-H32.

REFLECTIVE MATERIAL SHALL MEET THE REQUIREMENTS OF SUBSECTION 750.08 AND SHALL BE OF ENCAPSULATED LENS SILVER OR AMBER. AMBER IS TO BE INSTALLED ON THE DRIVER'S LEFT AND SILVER ON THEIR RIGHT.

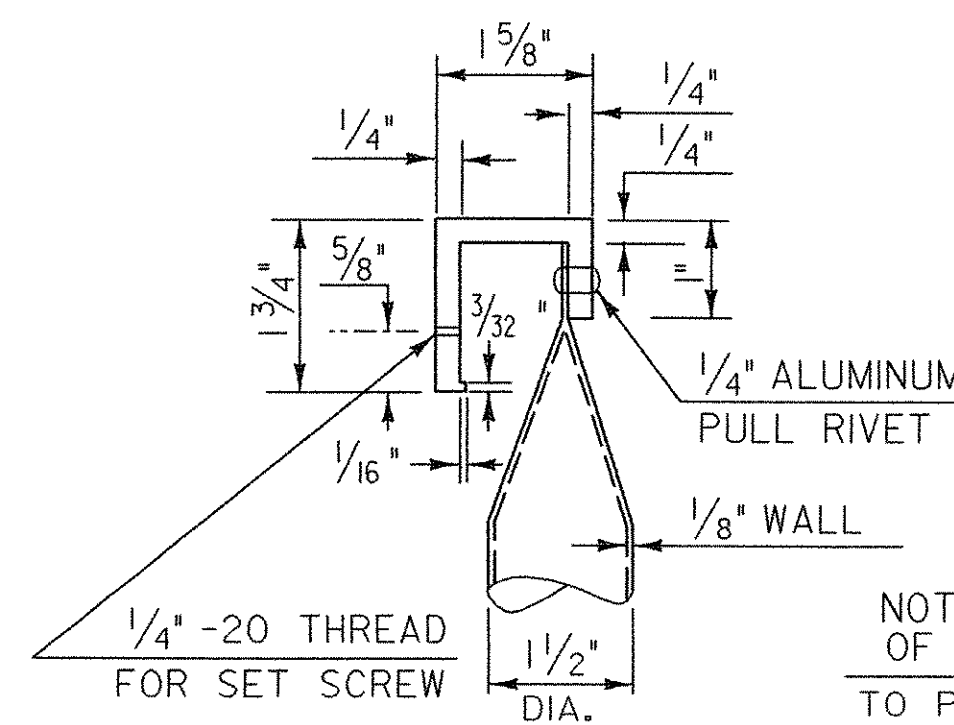
ON BRIDGES WITH A SIDEWALK, DELINEATORS ARE NOT TO BE INSTALLED ON THE SIDEWALK SIDE OF THE BRIDGE (I.E. DELINEATORS INSTALLED ONLY ON THE CURB SIDE AND ON THE APPROACH RAIL ON THE CURB SIDE)

PAYMENT SHALL BE INCIDENTAL TO ALL OTHER ITEMS.



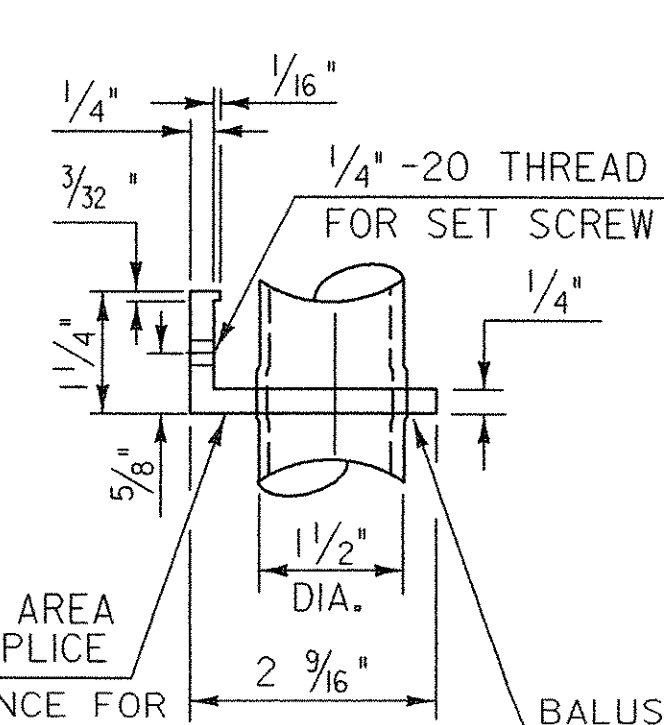
FRONT ELEVATION OF THREE RAIL WITH SPINDLES

NOTE : RAIL POSTS ARE TO BE SET NORMAL TO GRADE UNLESS OTHERWISE DESIGNATED ON BRIDGE PLANS. ALL DIMENSIONS ARE TYPICAL UNLESS OTHERWISE DESIGNATED ON BRIDGE PLANS.



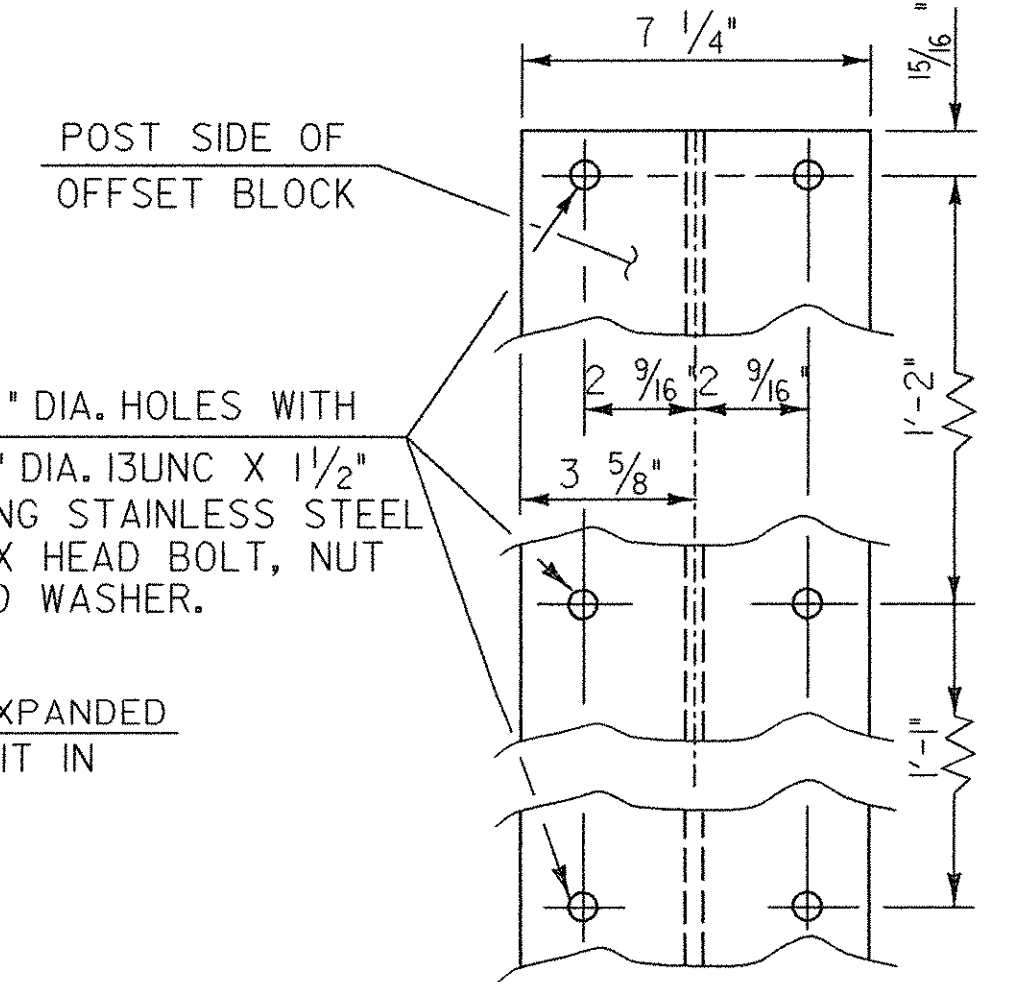
DETAIL A

NOTCH BRACKET IN AREA OF BARRIER RAIL SPLICE TO PROVIDE CLEARANCE FOR BOLT HEADS USED IN SPLICE (BRACKET TO BE NOTCHED IN FIELD)

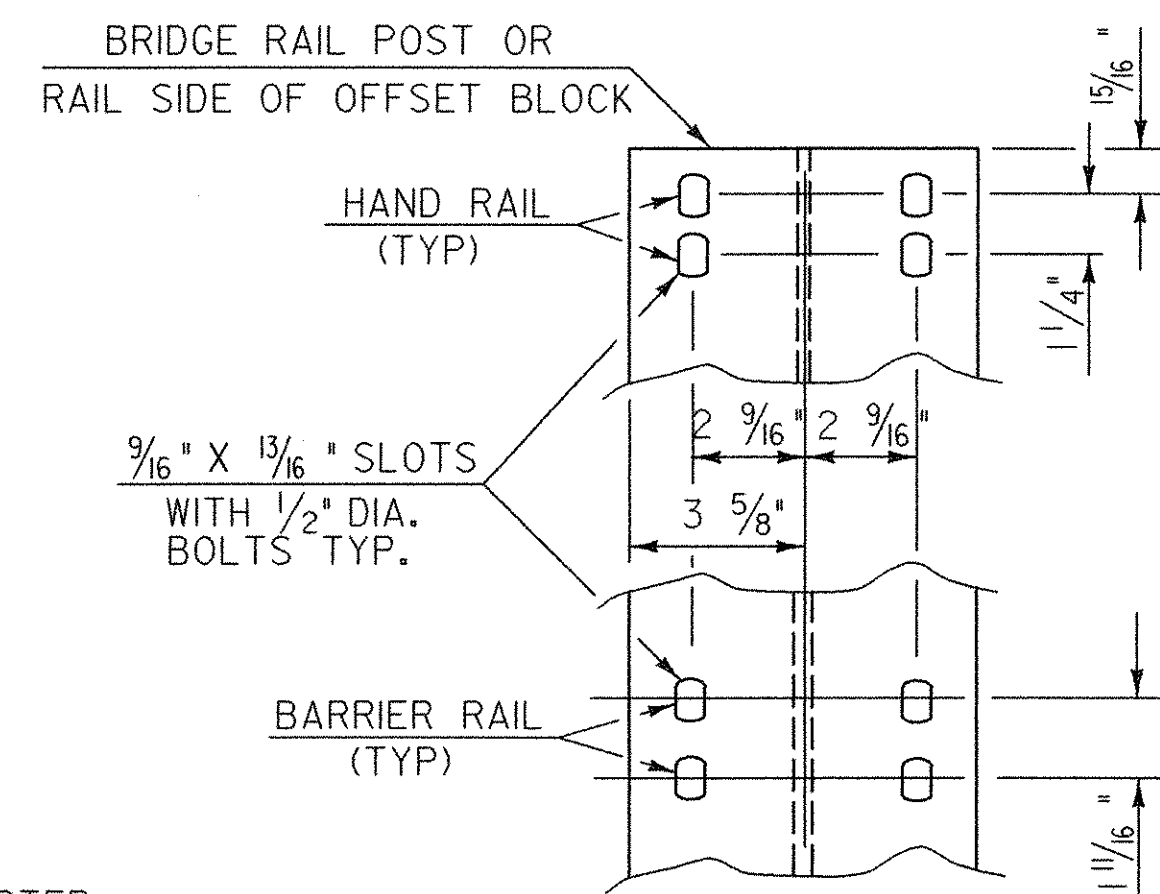


DETAIL B

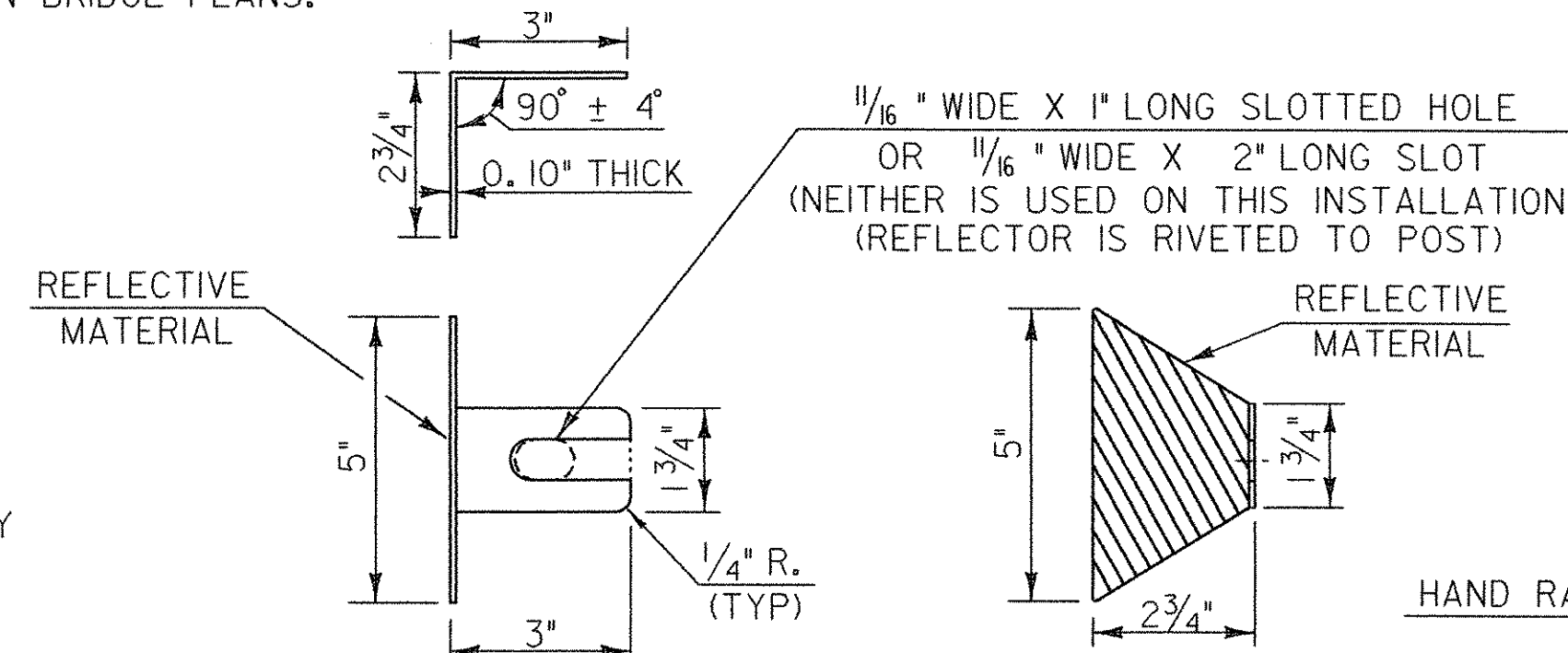
ALLOWABLE STRESSES:
RAILING : 21,000 PSI TENSION
22,000 PSI COMPRESSION
POSTS : 17,000 PSI TENSION
19,000 PSI COMPRESSION



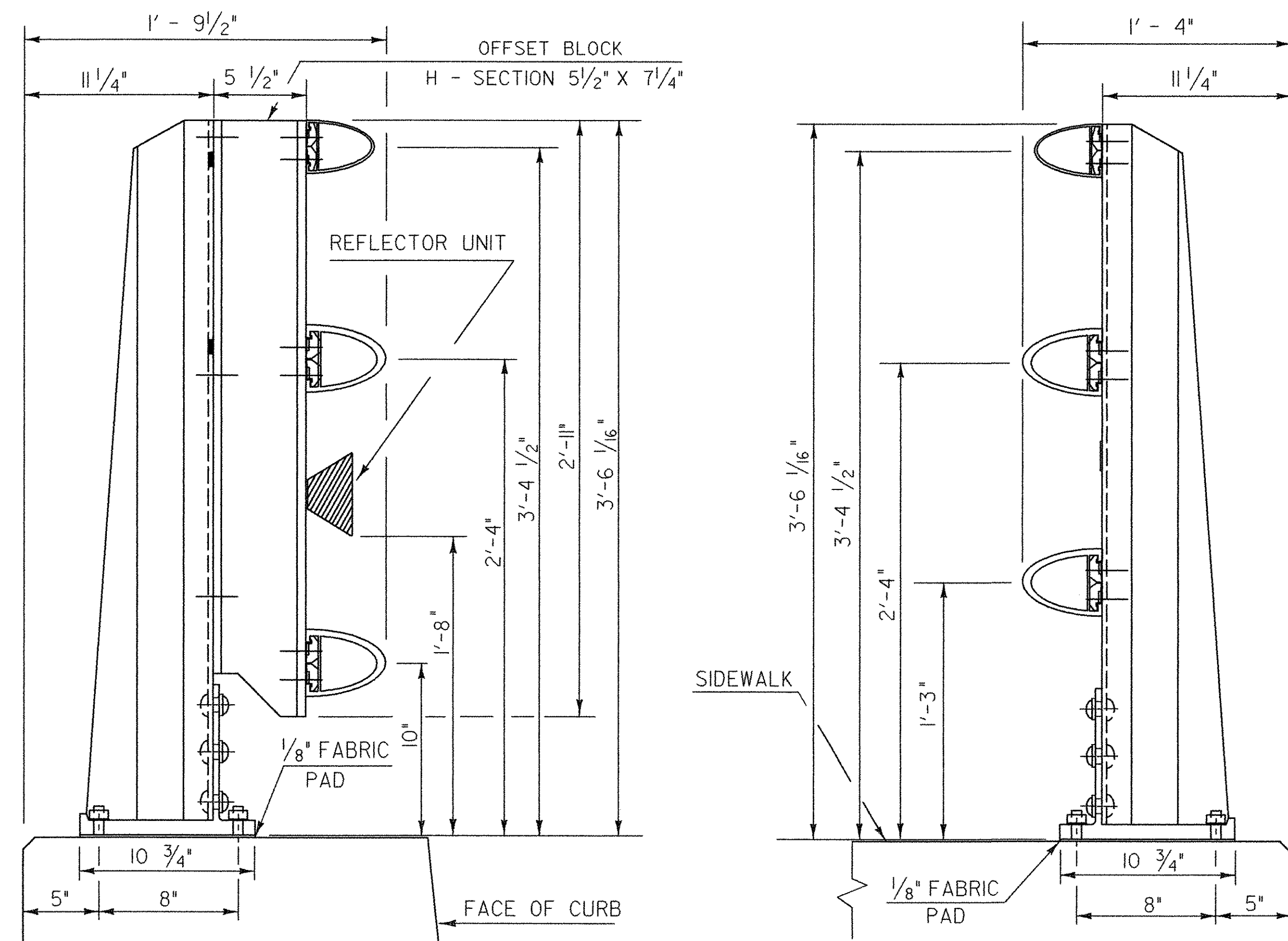
OFFSET BLOCK CONNECTION



RAIL CONNECTION



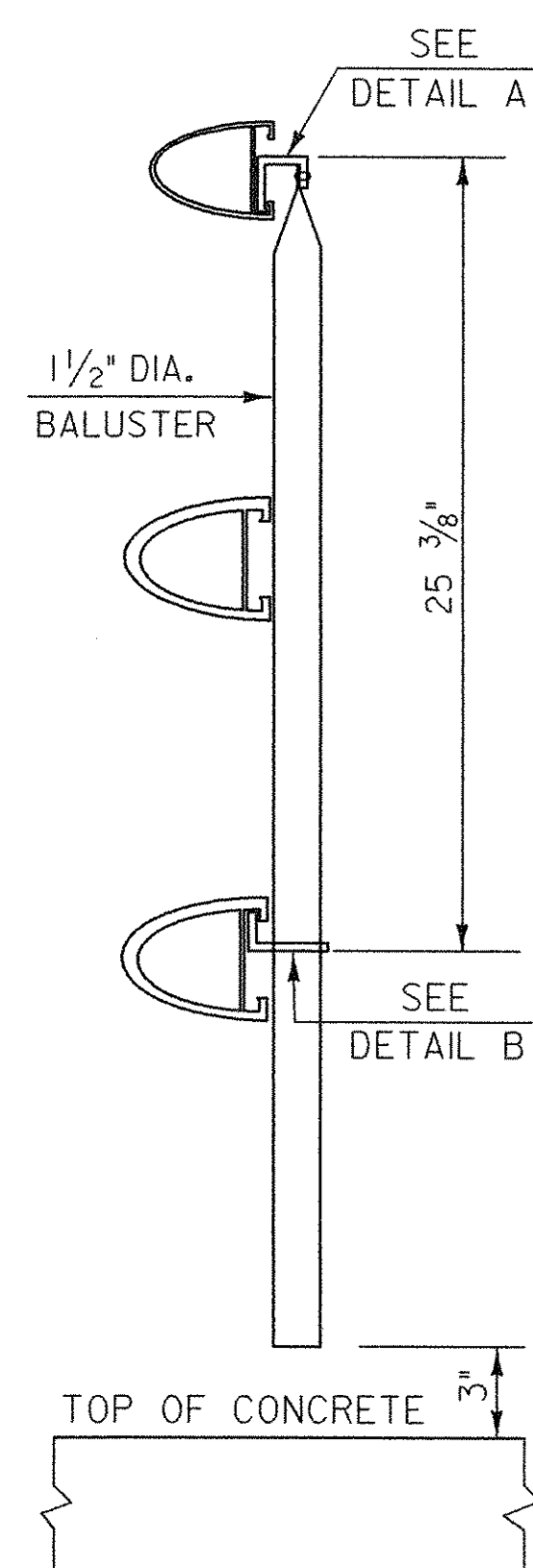
REFLECTOR DETAILS



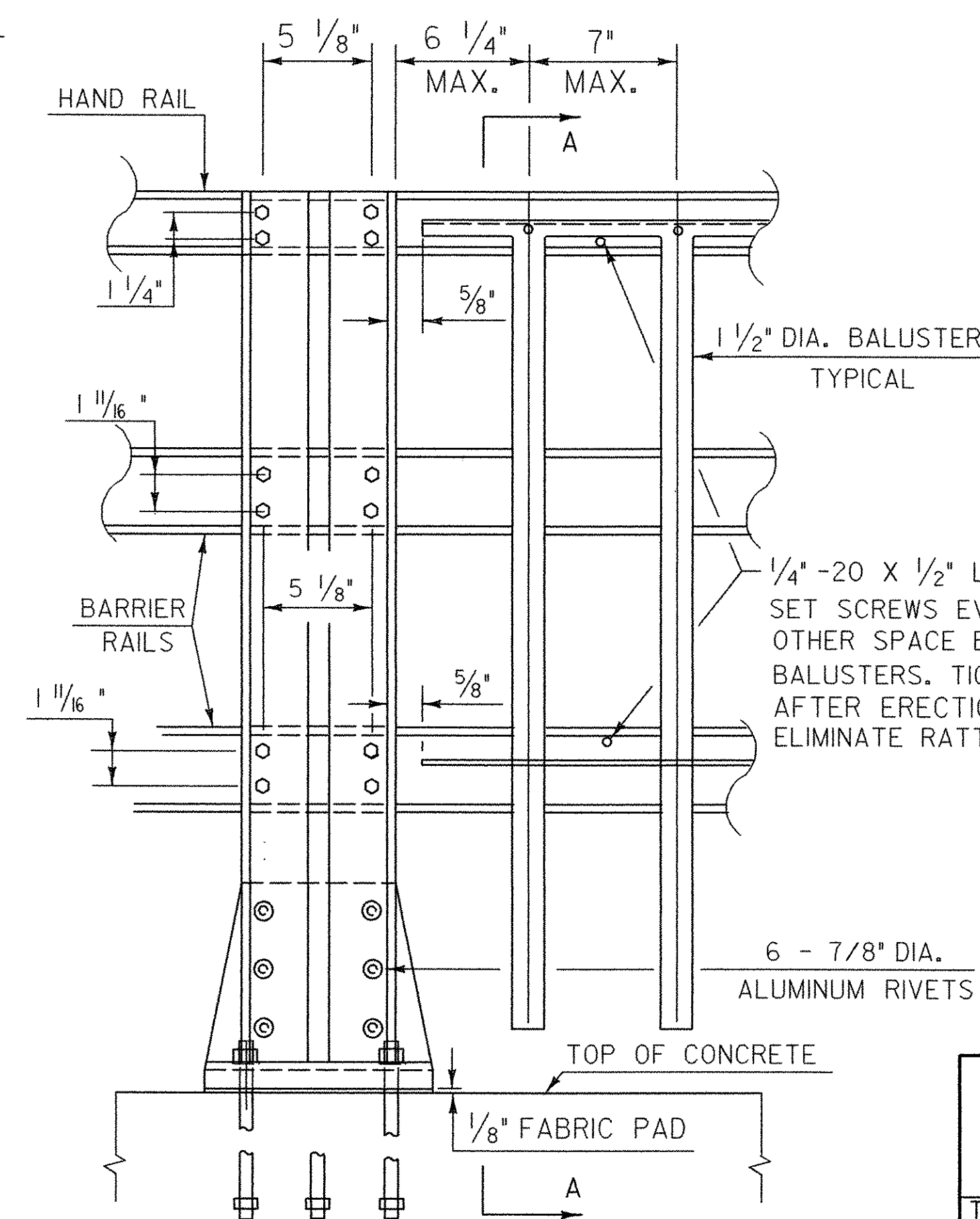
SIDE ELEVATION OF THREE RAIL TO BE USED ON CURB SIDE

SIDE ELEVATION OF THREE RAIL TO BE USED ON CURB SIDE

RAIL POST DETAILS ON SUPERSTRUCTURE

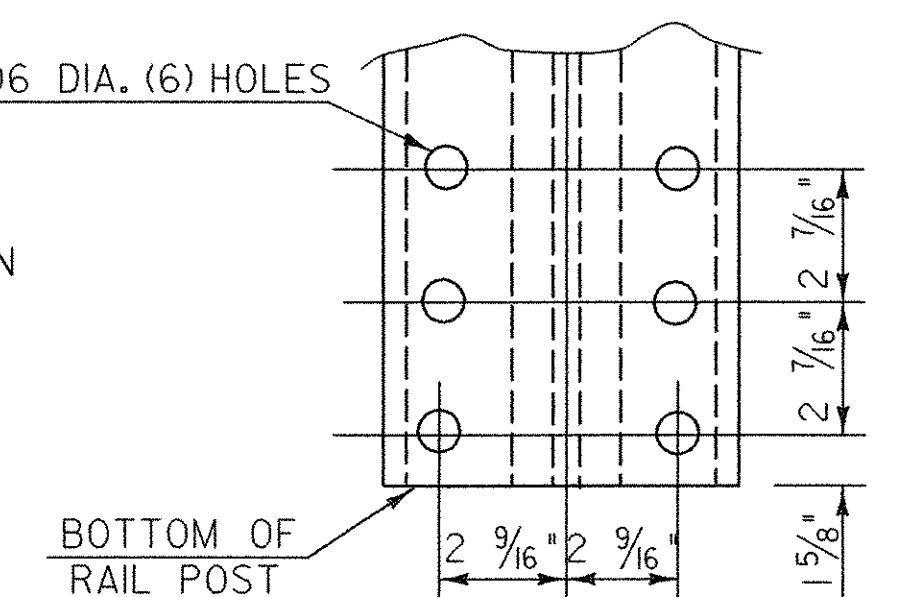


SECTION AA



OUTSIDE ELEVATION OF THREE RAIL POST & SPINDLES

DETAILS OF SPINDLES FOR ALUMINUM RAILING



POST BASE BOLT HOLE DETAILS

STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	

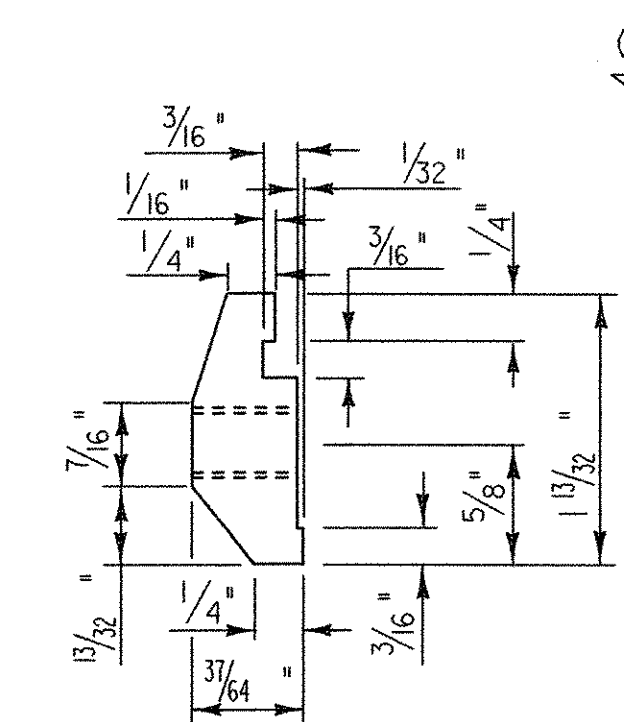
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

ALUMINUM BRIDGE RAIL DETAILS

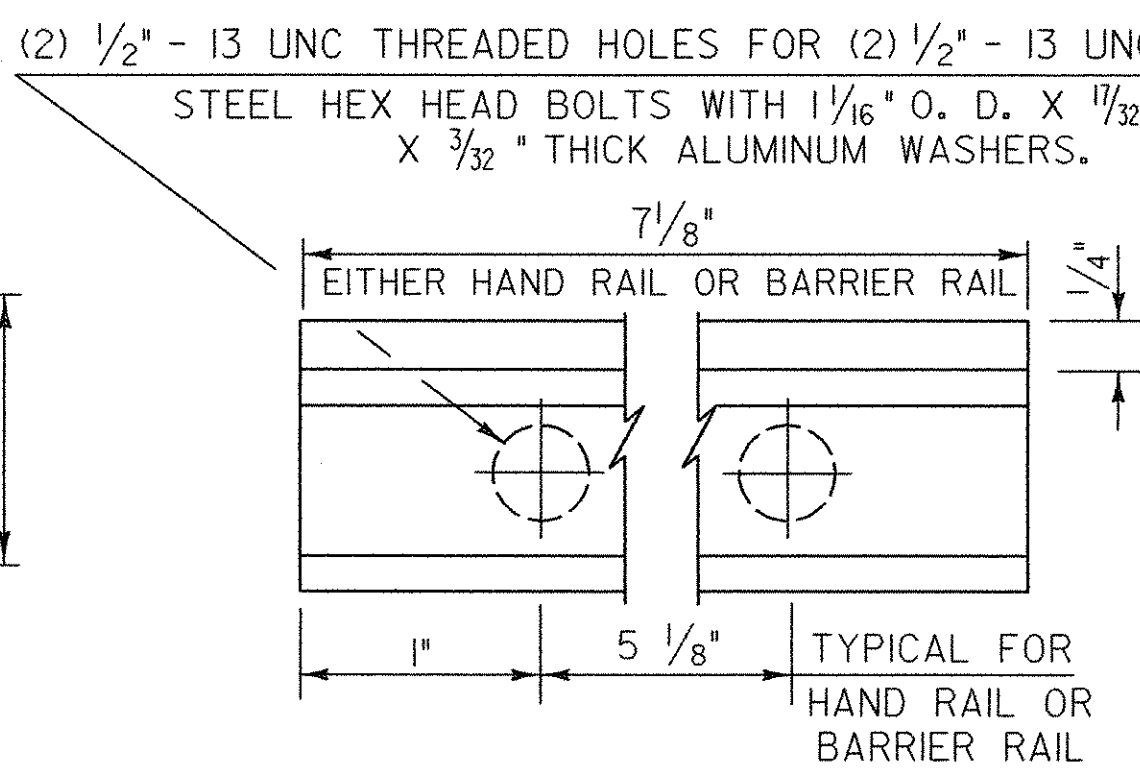
Designed By	VTrans	Drawn By	VTrans
Checked By	Date	Bridge Design Supervisor	Date
M. CHENETTE	08/05	M. CHENETTE	09/05

PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
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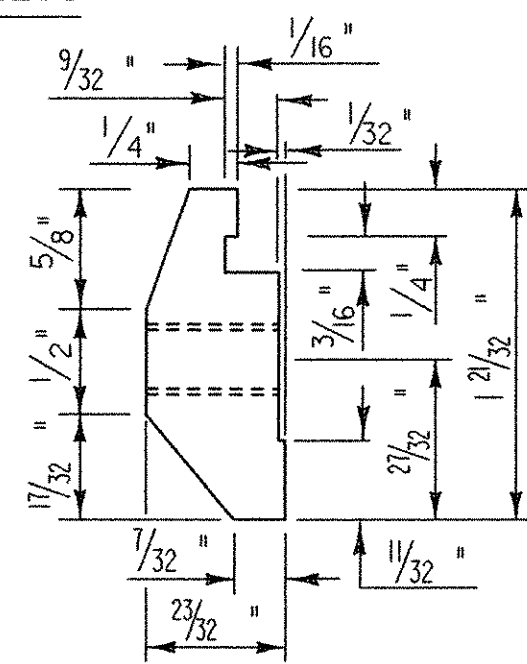
DH Dgn.: ...Cadd\Trans\z01j302airrail.dgn	Plot Date:	1/12/2006
Bridge Sheet No.	Sheet	50 of 63



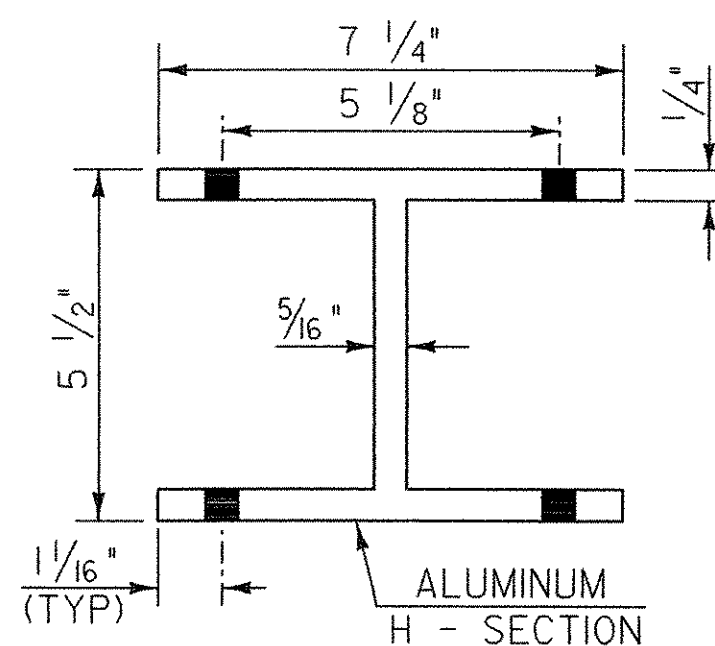
POST CONNECTION HANDRAIL SECTION



POST CONNECTION ELEVATION

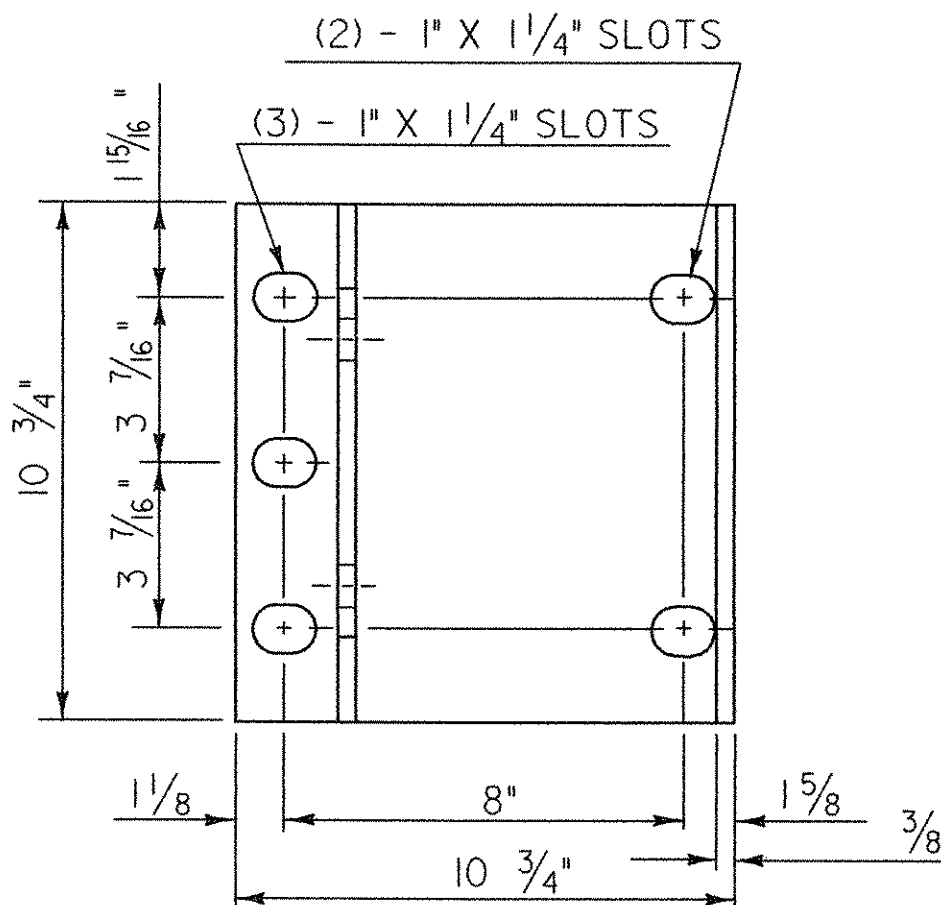


POST CONNECTION BARRIER RAIL SECTION

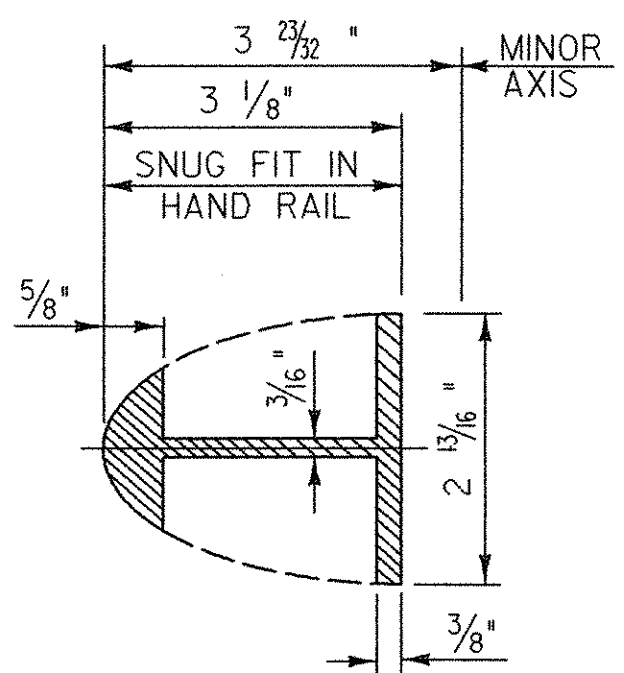


PLAN VIEW OF OFFSET BLOCK

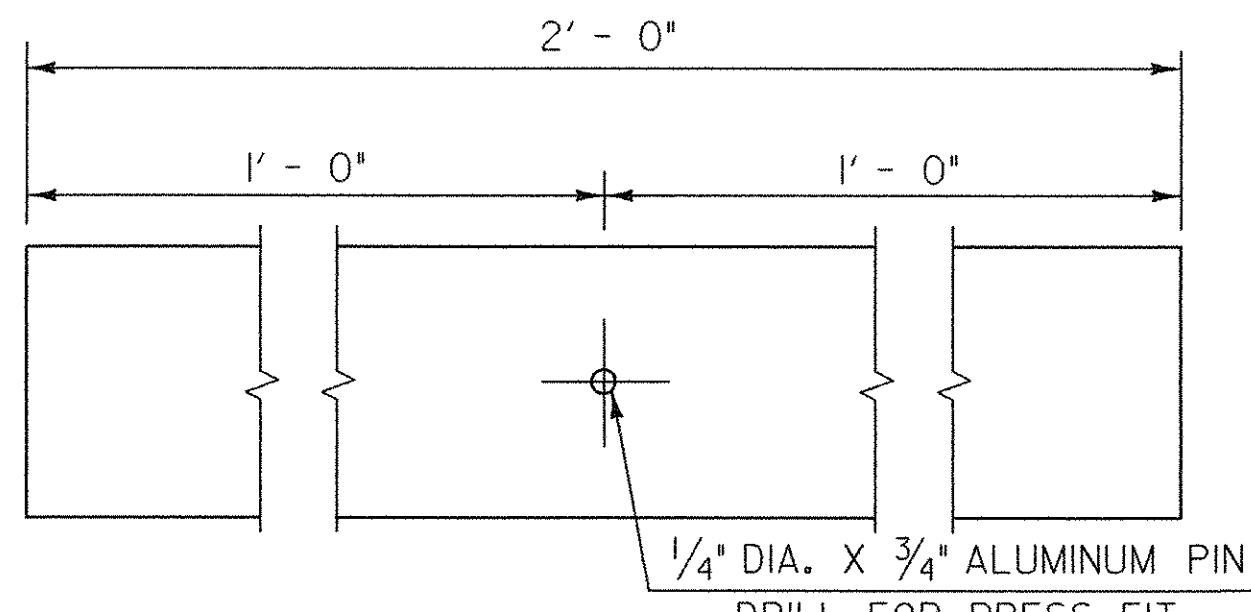
(TO BE USED ON SUPERSTRUCTURE ON CURB SIDE)



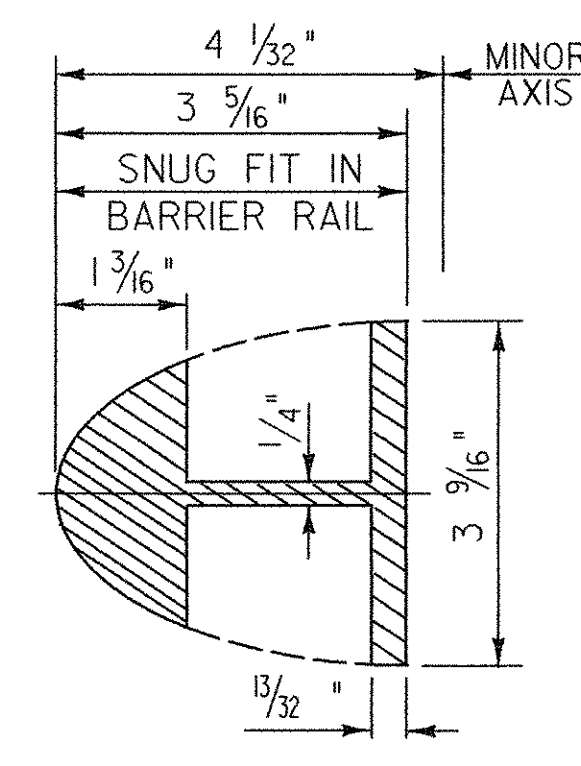
POST BASE PLAN



HAND RAIL SPLICE SECTION

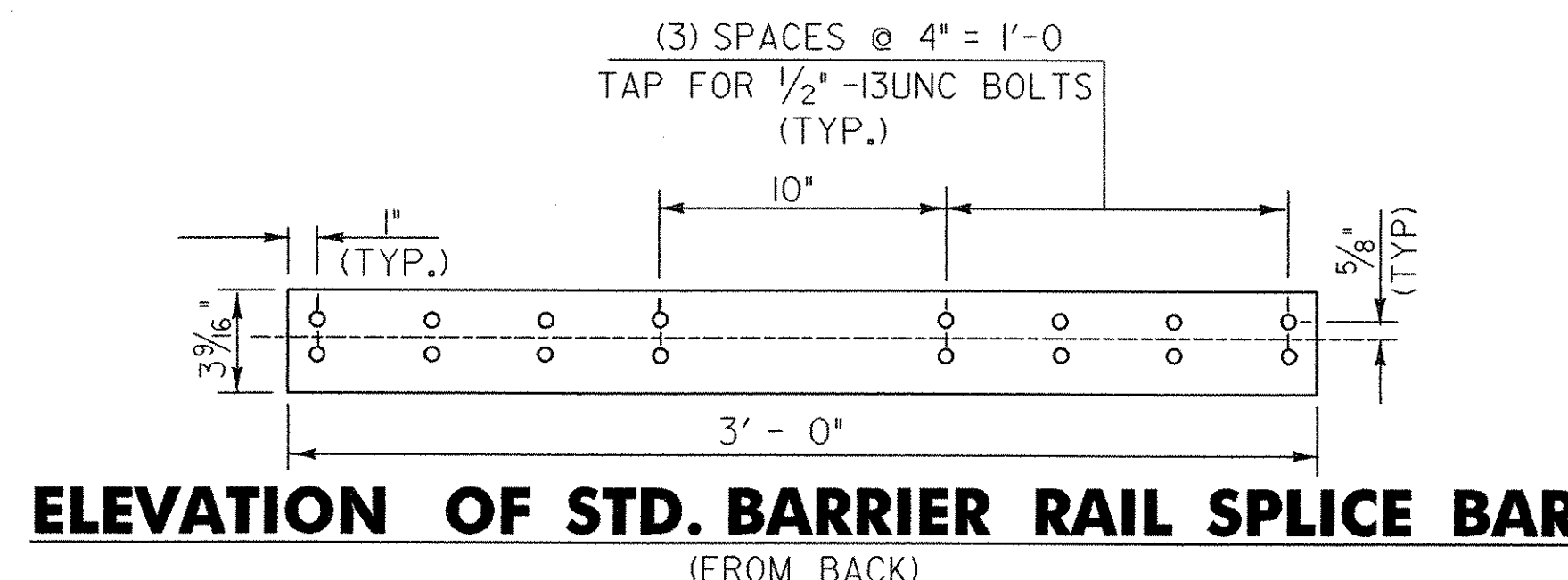


ELEVATION OF HANDRAIL SPLICE BAR



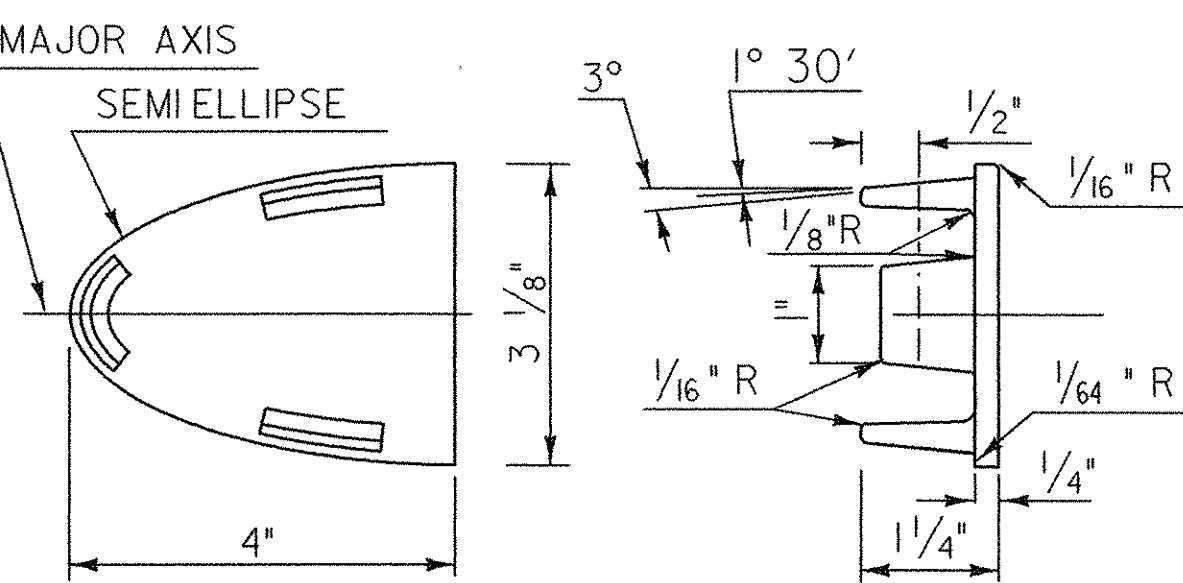
BARRIER RAIL SPLICE SECTION

(SEE SHEET xx FOR ELEVATION OF BARRIER RAIL SPLICE BAR)

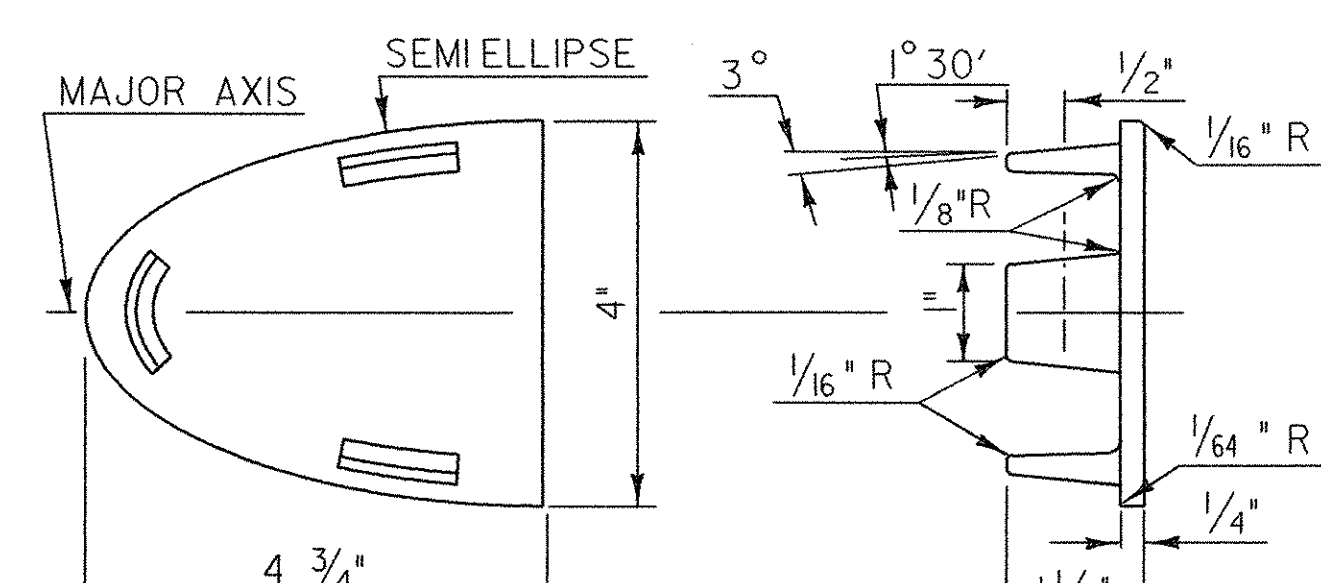


ELEVATION OF STD. BARRIER RAIL SPLICE BAR

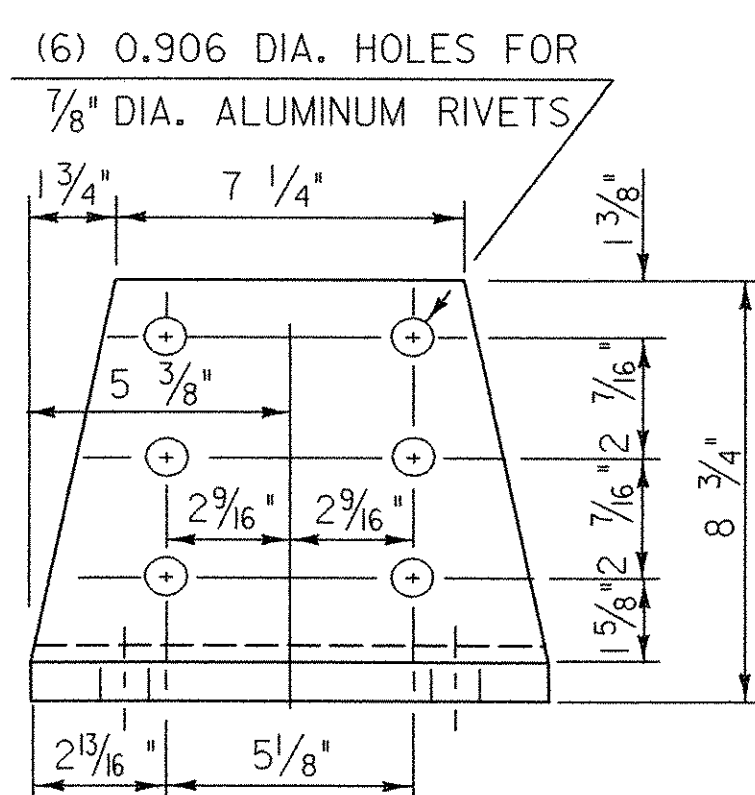
(FROM BACK)



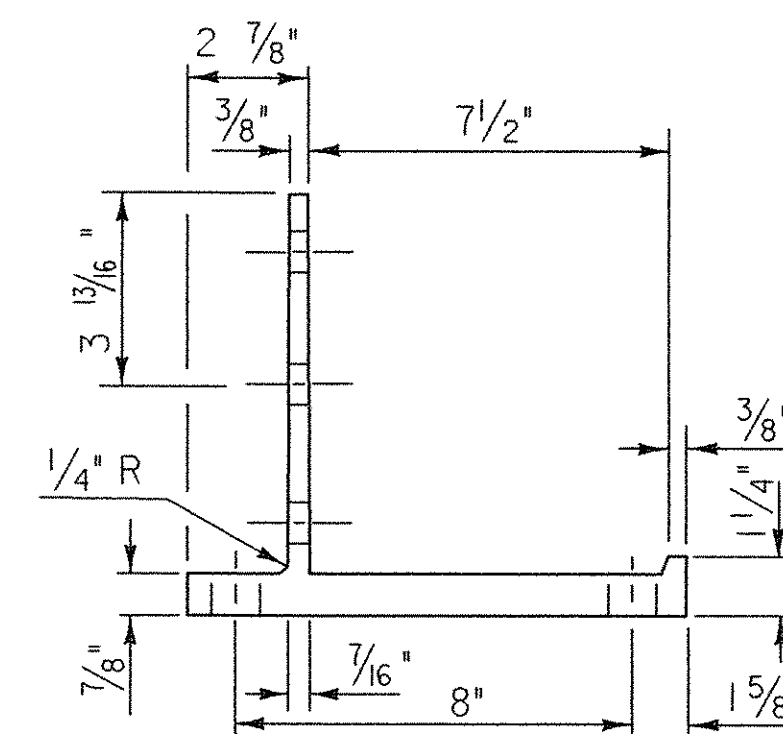
HANDRAIL END CAP



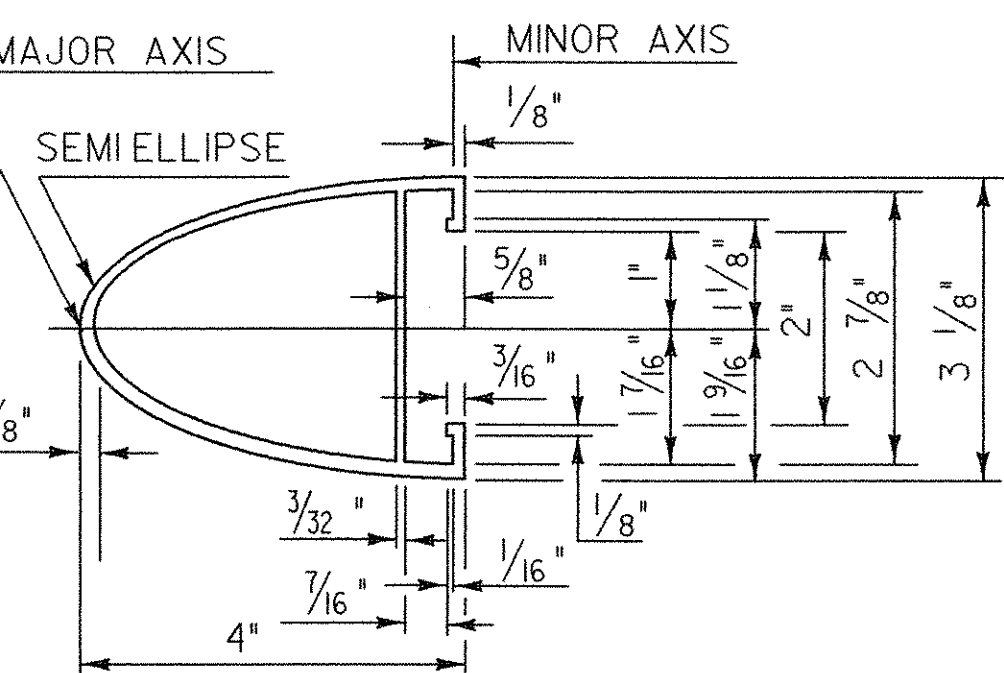
BARRIER RAIL END CAP



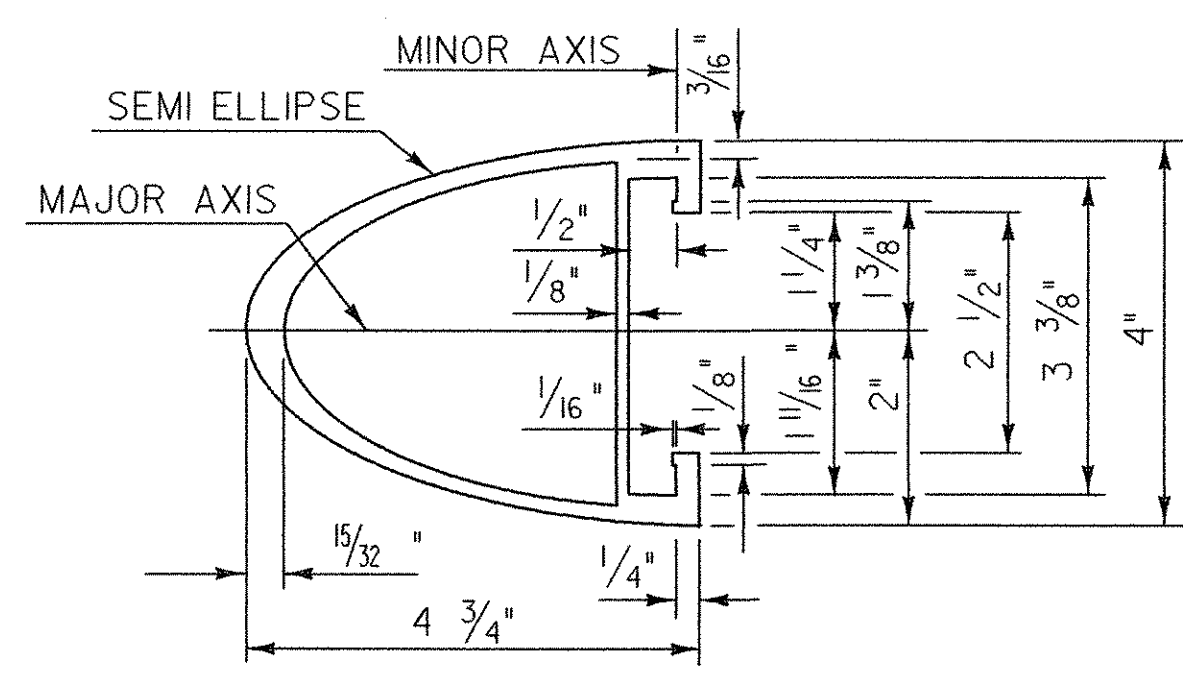
POST BASE FRONT ELEVATION



POST BASE SECTION

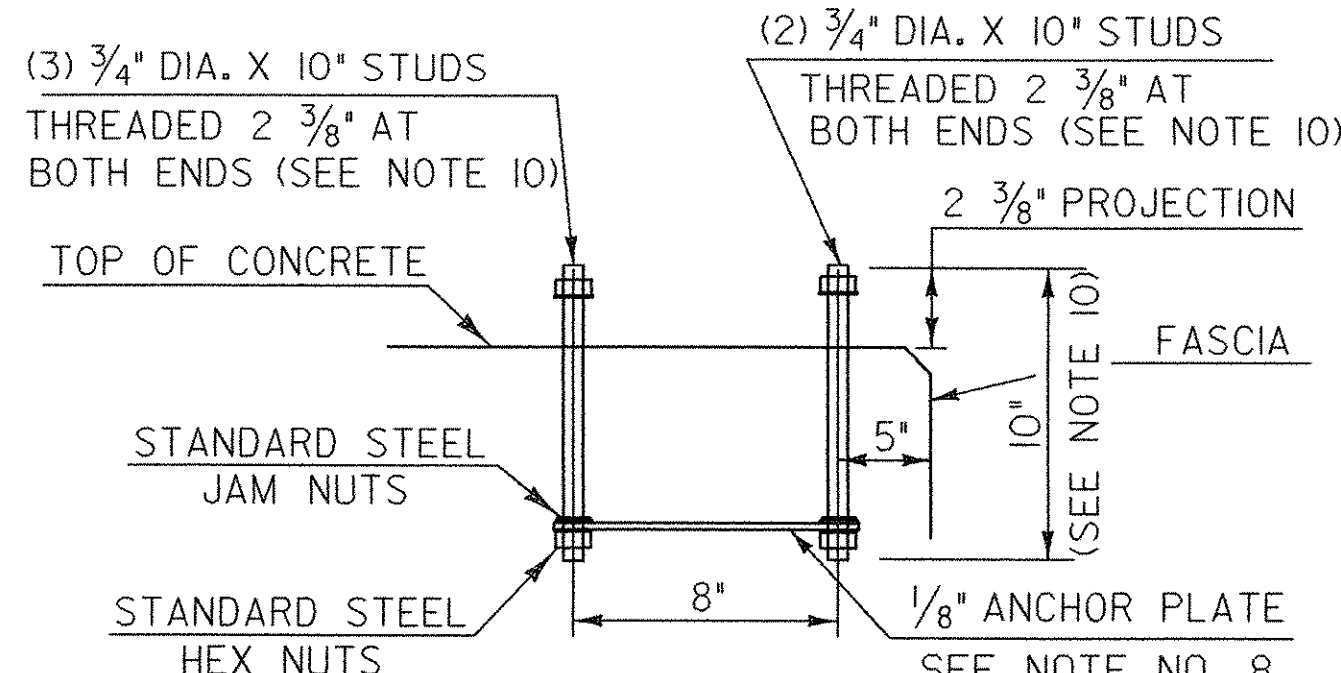


HANDRAIL SECTION

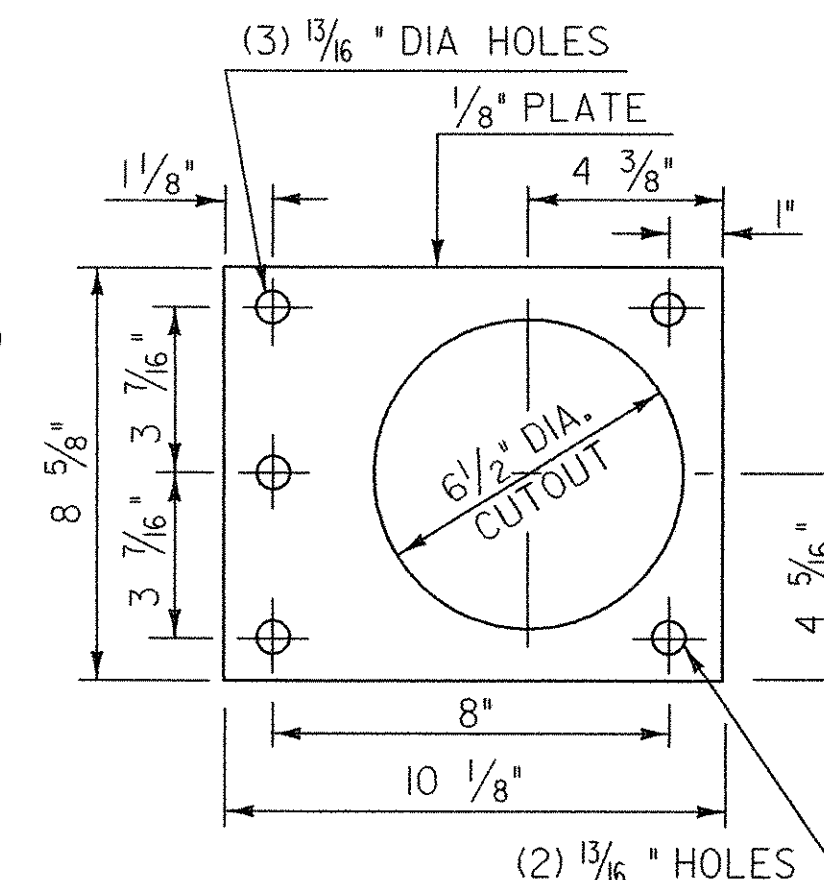


BARRIER RAIL SECTION

(SEE SHEET xx FOR ELEVATION OF BARRIER RAIL)



POST ANCHOR ASSEMBLY

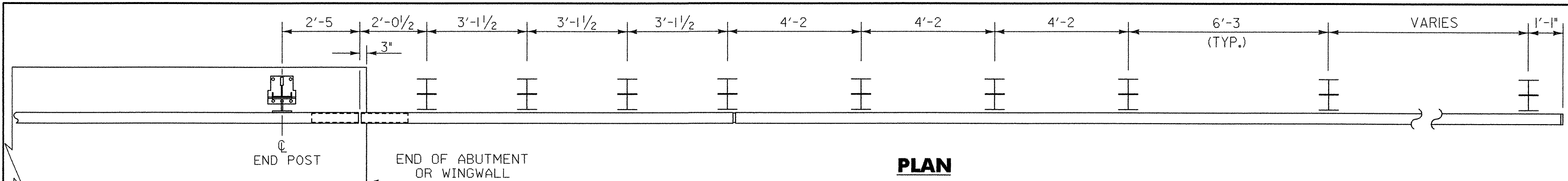


ANCHOR PLATE

NOTES

1. ANCHOR BOLTS, WASHERS AND HEAVY HEX NUTS MAY BE ANY OF THE FOLLOWING:
 - A. ASTM A449 GALVANIZED, OR
 - B. AASHTO M164 (ASTM A325) GALVANIZED
 - C. BOLTS AND WASHERS OF STAINLESS STEEL ASTM A276, TYPE 304 (MINIMUM ULTIMATE STRENGTH OF 100,000 PSI) WITH STAINLESS STEEL NUTS OF ASTM A194, GRADE 8NA.
2. ALUMINUM POSTS, POST BASES, SPLICE BARS, CONNECTION BARS, RAILS AND BALUSTER FRAMES SHALL CONFORM TO ASTM B221 ALLOY 6061-T6 OR ALLOY 6351-T5. MINIMUM ALLOWABLE STRESS $F_y = 35,000$ PSI.
3. ALUMINUM BALUSTER TUBES SHALL CONFORM TO ASTM B210 ALLOY 6061-T5 OR 6063-T5.
4. ALUMINUM RAIL END CAPS SHALL CONFORM TO ASTM B26 ALLOY 356-T6.
5. THE POST, RAIL, AND OFFSET BLOCK CONNECTION BOLTS SHALL BE EITHER ASTM A193 OR ASTM A320. EITHER ONE SHALL BE CLASS 1, B8 GRADE AISI 304 WITH AN ULTIMATE TENSILE STRENGTH OF 75,000 PSI. NUTS FOR EITHER OF THE ABOVE BOLTS SHALL BE ASTM A194, GRADE 8, STAINLESS STEEL WITH AN ULTIMATE TENSILE STRENGTH OF 75,000 PSI.
6. SET SCREWS FOR ATTACHING BALUSTERS TO RAILING SHALL BE ASTM F880, TYPE 303 MATERIAL.
7. RIVETS SHALL BE COLD DRIVEN HIGH BUTTON HEAD 'CONE POINT', CONFORMING TO ASTM B316 ALLOY 6061-T6.
8. THE ANCHOR PLATE FOR THE POST ANCHOR ASSEMBLY SHALL BE ASTM A36 STRUCTURAL STEEL.
9. WELDING SHALL CONFORM TO THE REQUIREMENTS OF SUBSECTION 506.10 USING THE GMAW-INERT GAS PROCESS AND AWS ER 5356 ELECTRODE WIRE.
10. UNLESS OTHERWISE SPECIFIED, ANCHOR BOLTS SHALL BE CAST INTO THE CONCRETE AS DETAILED.
11. WHENEVER FEASIBLE BARRIER RAIL AND HAND RAIL SECTIONS, SHALL BE FULL LENGTH SECTIONS ($40' \pm$) AND WHEN POSSIBLE SHALL BE ATTACHED TO THREE POSTS. RAILS SHALL BE SPLICED AT EACH DECK JOINT AND INTERMITTENTLY AS REQUIRED. SPLICES SHALL OCCUR WITHIN THE SAME PANEL.
12. ENDS OF RAILS SHALL BE CUT SQUARE AND GROUND FREE OF BURRS OR RAGGED EDGES. EXPOSED ENDS SHALL BE CAPPED.
13. THE CONCRETE CONTACT SURFACE AT THE POST BASE SHALL BE BUSH HAMMERED AND/OR SHIMMED AS REQUIRED FOR PROPER POST ALIGNMENT. POST HEIGHT ADJUSTMENTS LESS THAN 1/4" SHALL BE WITH 1/16" AND 1/8" SHIMS. CORRECTIONS EXCEEDING 1/4" SHALL BE WITH EPOXY MORTAR CONFORMING WITH SECTION 530. FABRIC BEARING PADS AND ANY REQUIRED SHIMS OR EPOXY MORTAR ARE INCIDENTAL TO THE UNIT PRICE BID FOR THE RAILING.
14. SHIMS AND 1/8" FABRIC BEARING PADS SHALL BE 10 3/4" SQUARE WITH SLOTTED HOLES SIZED AND LOCATED THE SAME AS THE POST BASE DETAIL. FABRIC BEARING PADS SHALL CONFORM TO SUBSECTION 731.01 OR 731.02. SHIM MATERIAL SHALL BE ASTM B 209 ALLOY 1100-O.
15. EXTRUDED SECTIONS ARE DETAILED TO COMPLY WITH CURRENT AASHTO-AGC-ARTBA STANDARDS. MINOR VARIATIONS OF THE DETAILS SHOWN MAY BE CONSIDERED PROVIDING THEY DO NOT REDUCE THE STRENGTH CAPACITY OF THE RAIL SYSTEM.
16. ALUMINUM WASHERS SHALL BE ASTM B209 ALLOY ACLAD 2024-T4.

STATE OF VERMONT AGENCY OF TRANSPORTATION			
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
ALUMINUM BRIDGE RAIL DETAILS			
Designed By	VTrans	Drawn By	VTrans
Checked By	Date	Bridge Design Supervisor	
M. CHENETTE	08/05	M. CHENETTE	Date 09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn.: ...Cadd\Trans\z01j302alrall.dgn	Plot Date: 1/12/2006	Bridge Sheet No.	Sheet 51 of 63

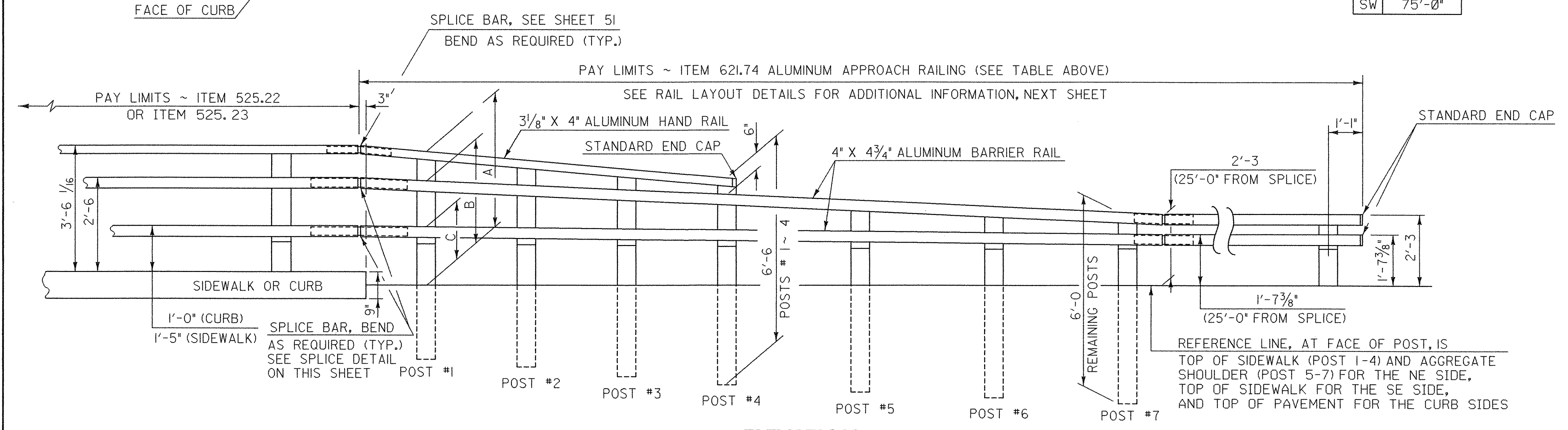


PLAN

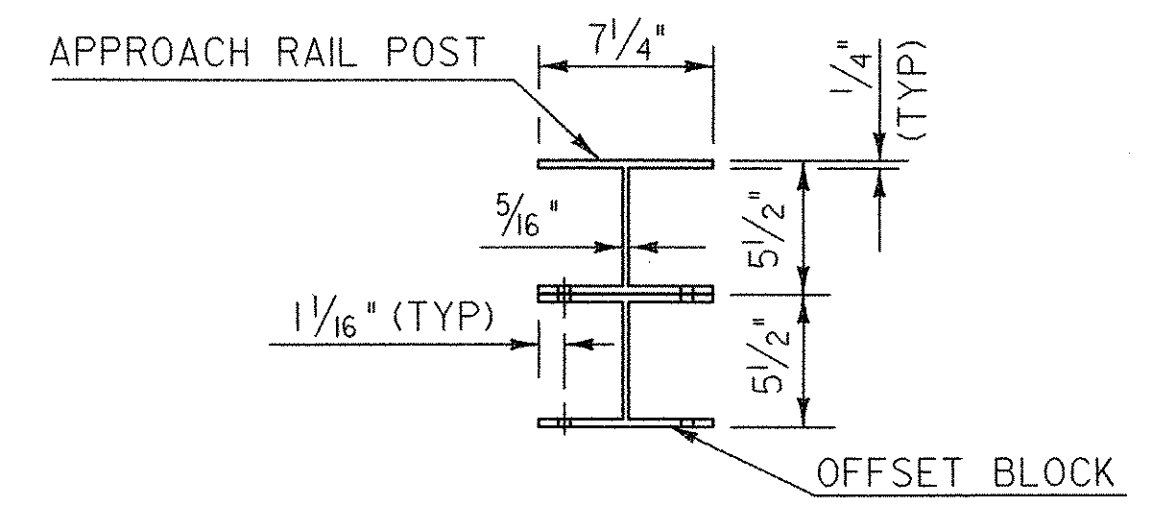
PAY LIMITS - ITEM 621.74
ALUMINUM APPROACH RAILING

NE	106'-3"
SE	87'-6"
NW	12'-6"
SW	75'-0"

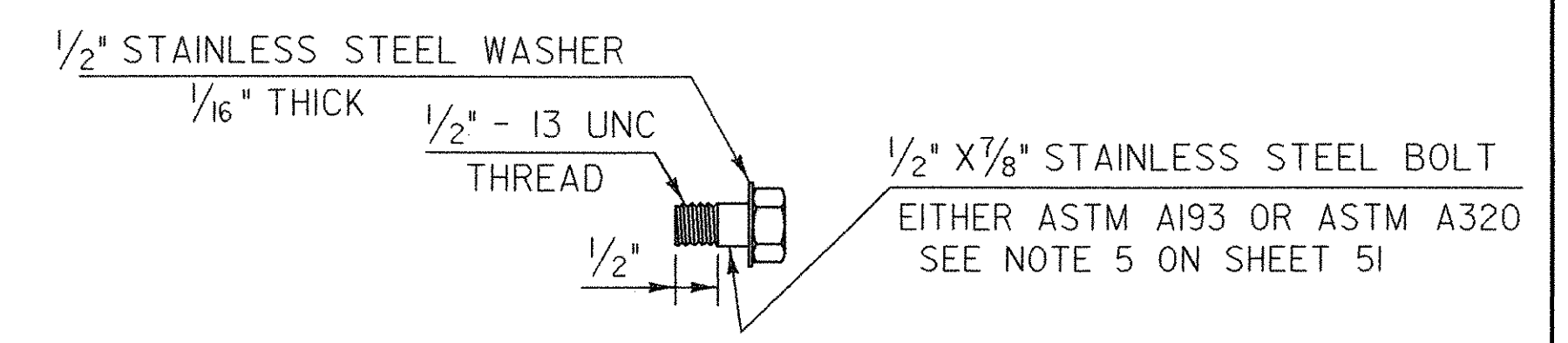
- NOTES**
1. ALL POSTS SHALL BE EXTRUDED ALUMINUM.
 2. ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 36 GALVANIZED AFTER FABRICATION.
 3. ALL ITEMS NOT OTHERWISE INDICATED SHALL MEET THE SPECIFICATION REQUIREMENTS OF THE STANDARD SHEETS ON WHICH THEY ARE DETAILED.
 4. SEE SHEETS 50 AND 51 FOR ALUMINUM BRIDGE RAILING DETAILS.
 5. THE COST OF ALL MATERIALS AND LABOR FOR THE SPLICE BETWEEN THE ALUMINUM APPROACH RAILING AND THE STEEL BEAM GUARD RAIL SHALL BE INCIDENTAL TO ITEM 621.74, ALUMINUM APPROACH RAILING.
 6. DETAILS ARE SHOWN FOR TRANSITION TO A 3 RAIL ALUMINUM BRIDGE RAILING.
 7. DIMENSIONS SHOWN ARE FROM A REFERENCE LINE AT THE FACE OF POST FOR A NORMAL CROWNED SECTION. APPROPRIATE CORRECTIONS SHALL BE MADE FOR CROSS SLOPES OTHER THAN A NORMAL SECTION.



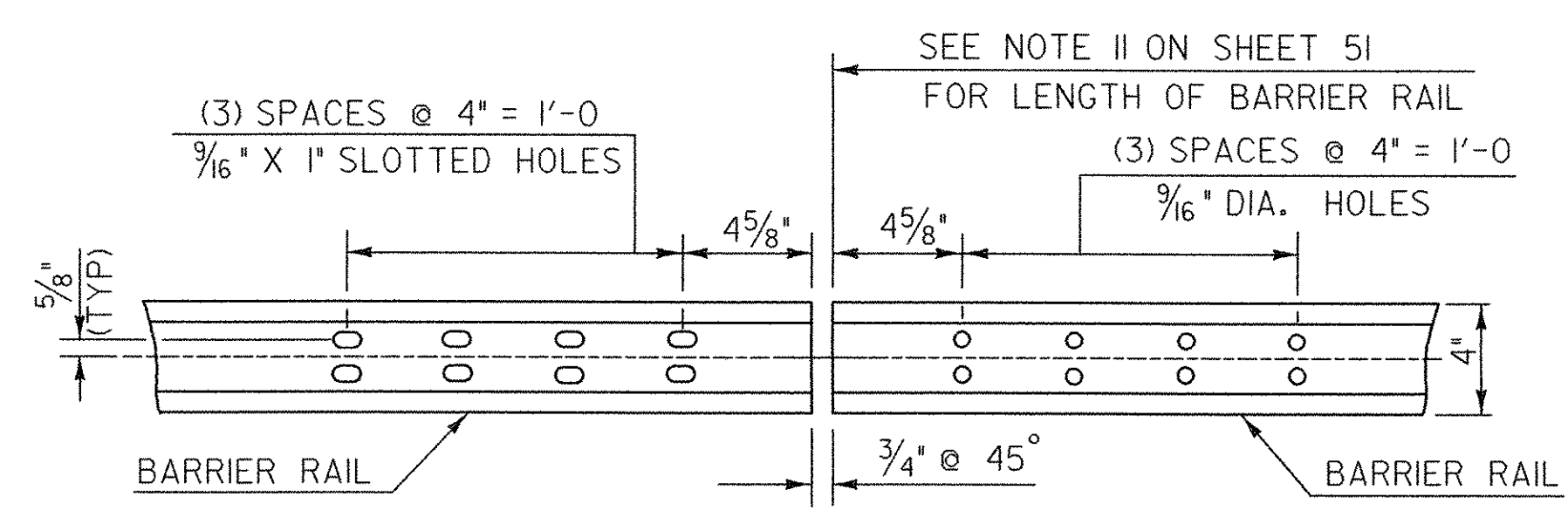
ELEVATION



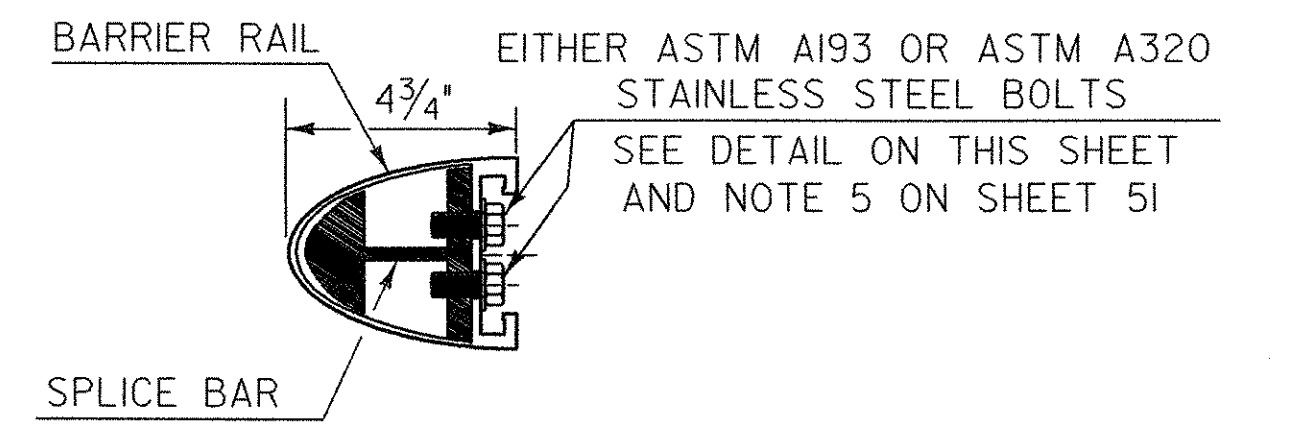
PLAN



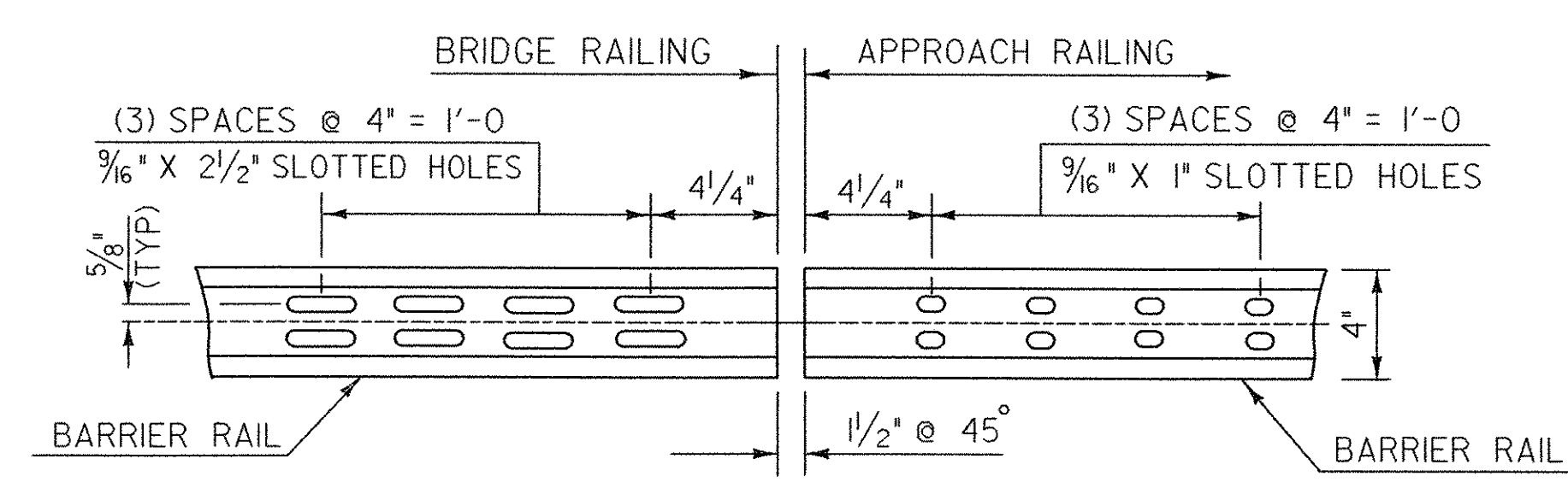
STAINLESS STEEL BOLT DETAILS



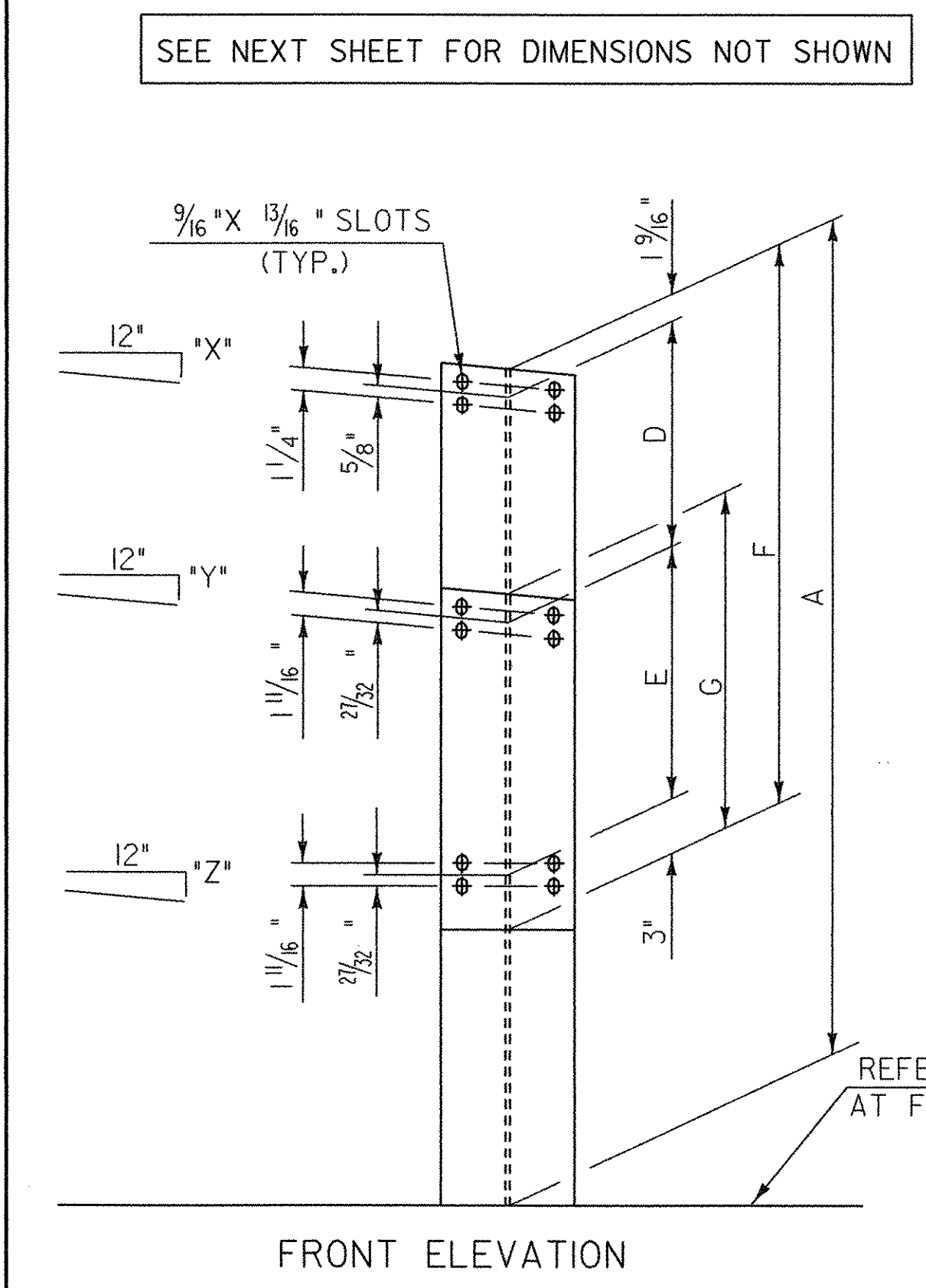
ELEVATION OF BARRIER RAIL (FROM BACK) AT ALL INTERMEDIATE RAIL SPLICES



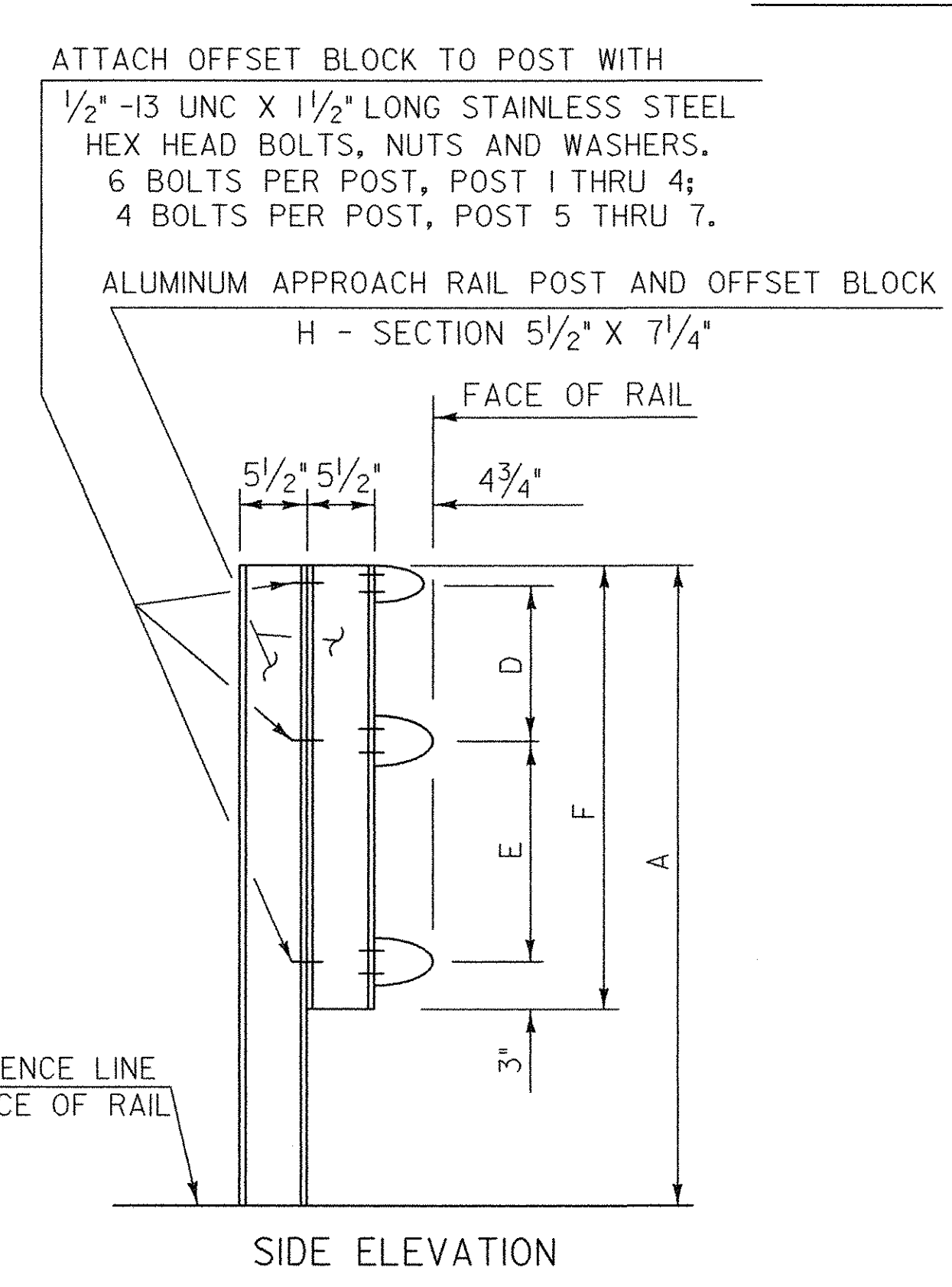
TYPICAL SECTION THROUGH BARRIER RAIL SPLICE



ELEVATION OF BARRIER RAIL (FROM BACK)



APPROACH RAIL DETAILS



SIDE ELEVATION

STATE OF VERMONT AGENCY OF TRANSPORTATION			
Town Of	HUNTINGTON	Bridge No.	42
Highway No.	T.H. 4	Log Sta.	
		Surv. Sta.	
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER			
ALUMINUM APPROACH RAILING DETAILS			
Designed By	VTrans/D-H	Drawn By	VTrans/D-H
Checked By	Date	Bridge Design Supervisor	Date
M. CHENETTE	08/05	M. CHENETTE	09/05
PROJECT	HUNTINGTON	PROJECT NO.	BRO 1445 (29)
DH Dgn. ... \Cadd\Trans\201302airrail.dgn		Plot Date:	1/12/2006
Bridge Sheet No.		Sheet	52 of 63



NE ALUMINUM RAIL DIMENSIONS

POST NO.	RAIL HEIGHT DIMENSIONS			OFFSET BLOCK DIMENSIONS			
	A	B	C	D	E	F	G
1	3'-4"	2'-5 ¹ / ₁₆ "	1'-4 ¹ / ₂ "	11 ⁷ / ₁₆ "	1'-0 ⁹ / ₁₆ "	2'-4 ⁹ / ₁₆ "	
2	3'-0 ⁵ / ₁₆ "	2'-3 ⁵ / ₈ "	1'-3 ³ / ₄ "	9 ³ / ₄ "	11 ⁷ / ₈ "	2'-2 ³ / ₁₆ "	
3	2'-9 ³ / ₄ "	2'-2 ¹ / ₈ "	1'-2 ⁷ / ₈ "	8 ¹ / ₈ "	11 ³ / ₁₆ "	1'-11 ⁷ / ₈ "	
4	2'-9 ⁷ / ₁₆ "	2'-3 ⁷ / ₁₆ "	1'-4 ¹⁵ / ₁₆ "	6 ⁷ / ₁₆ "	10 ⁹ / ₁₆ "	1'-9 ⁹ / ₁₆ "	
5		2'-5 ³ / ₁₆ "	1'-7 ¹⁵ / ₁₆ "		9 ⁵ / ₈ "		1'-2 ⁵ / ₈ "
6		2'-4 ⁹ / ₁₆ "	1'-7 ¹⁵ / ₁₆ "		8 ³ / ₄ "		1'-1 ³ / ₄ "
7		2'-3 ¹ / ₁₆ "	1'-7 ¹⁵ / ₁₆ "		7 ⁷ / ₈ "		1'-0 ⁷ / ₈ "

ALL REMAINING POSTS TO HAVE THE SAME DIMENSIONS AS POST NO. 7

SE ALUMINUM RAIL DIMENSIONS

POST NO.	RAIL HEIGHT DIMENSIONS			OFFSET BLOCK DIMENSIONS			
	A	B	C	D	E	F	G
1	3'-4 ¹³ / ₁₆ "	2'-5 ¹³ / ₁₆ "	1'-5 ¹ / ₄ "	11 ⁷ / ₁₆ "	1'-0 ⁹ / ₁₆ "	2'-4 ⁹ / ₁₆ "	
2	3'-3 ¹ / ₈ "	2'-5 ¹³ / ₁₆ "	1'-5 ¹⁵ / ₁₆ "	9 ³ / ₄ "	11 ⁷ / ₈ "	2'-2 ³ / ₁₆ "	
3	3'-1 ¹ / ₂ "	2'-5 ¹³ / ₁₆ "	1'-6 ⁵ / ₈ "	8 ¹ / ₈ "	11 ³ / ₁₆ "	1'-11 ⁷ / ₈ "	
4	2'-11 ¹¹ / ₁₆ "	2'-5 ¹¹ / ₁₆ "	1'-7 ¹ / ₈ "	6 ⁷ / ₁₆ "	10 ⁹ / ₁₆ "	1'-9 ⁹ / ₁₆ "	
5		2'-4 ¹³ / ₁₆ "	1'-7 ³ / ₁₆ "		9 ⁵ / ₈ "		1'-2 ⁵ / ₈ "
6		2'-4 ¹ / ₁₆ "	1'-7 ⁵ / ₁₆ "		8 ³ / ₄ "		1'-1 ³ / ₄ "
7		2'-3 ⁵ / ₁₆ "	1'-7 ³ / ₁₆ "		7 ⁷ / ₈ "		1'-0 ⁷ / ₈ "

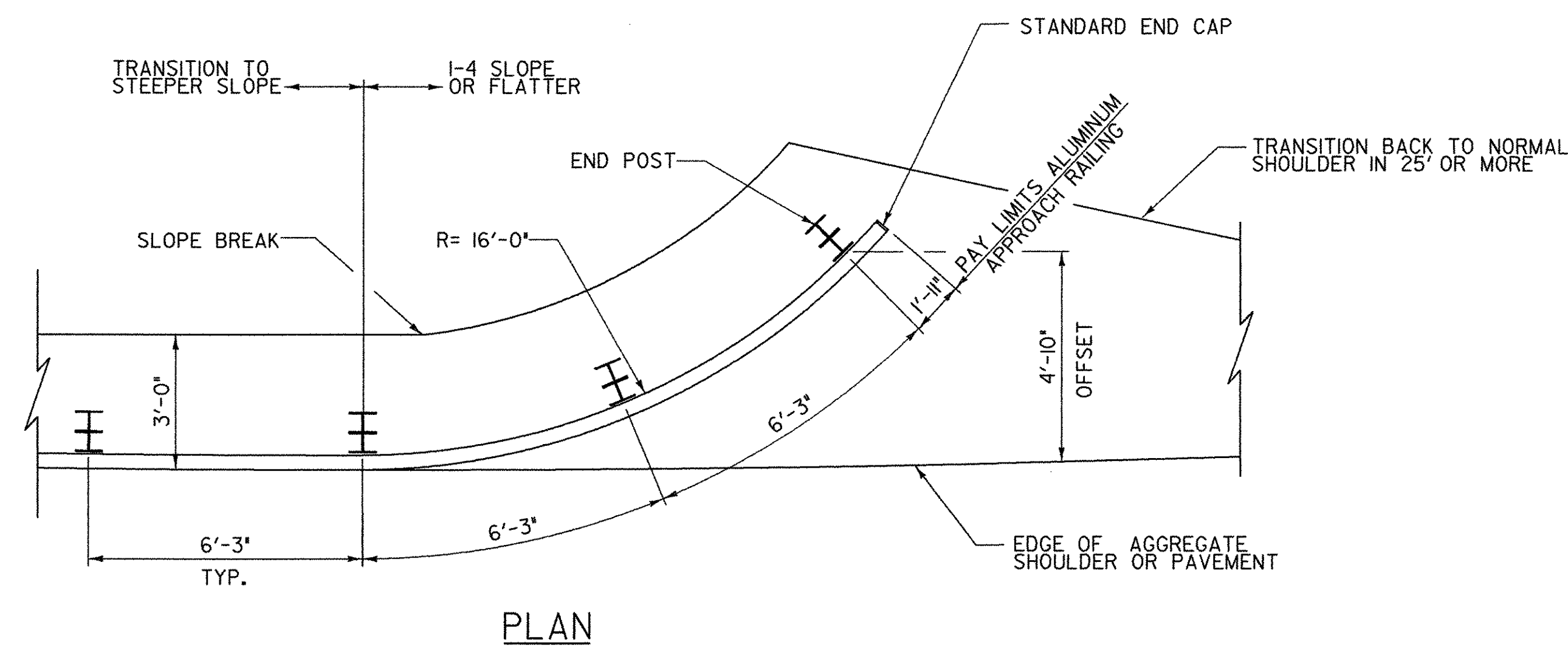
ALL REMAINING POSTS TO HAVE THE SAME DIMENSIONS AS POST NO. 7

SW ALUMINUM RAIL DIMENSIONS

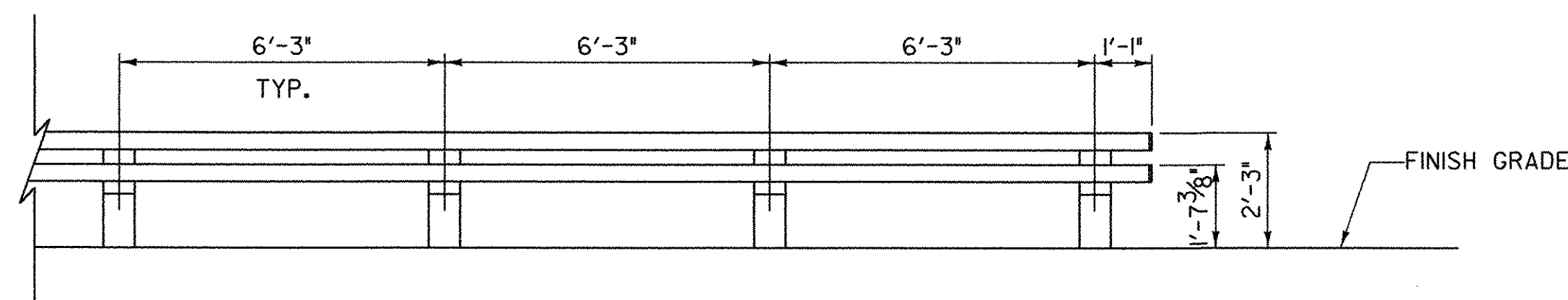
POST NO.	RAIL HEIGHT DIMENSIONS			OFFSET BLOCK DIMENSIONS			
	A	B	C	D	E	F	G
1	4'-0 ³ / ₄ "	3'-1 ¹ / ₁₆ "	1'-8 ⁵ / ₈ "	11 ⁷ / ₁₆ "	1'-5 ¹ / ₈ "	2'-9 ¹ / ₈ "	
2	3'-9 ⁵ / ₁₆ "	3'-0"	1'-8 ¹ / ₈ "	9 ³ / ₄ "	1'-3 ⁷ / ₈ "	2'-6 ³ / ₁₆ "	
3	3'-5 ⁷ / ₈ "	2'-10 ³ / ₁₆ "	1'-7 ⁵ / ₈ "	8 ¹ / ₈ "	1'-2 ⁹ / ₁₆ "	2'-3 ¹ / ₄ "	
4	3'-2 ⁵ / ₈ "	2'-8 ⁵ / ₈ "	1'-7 ³ / ₈ "	6 ⁷ / ₁₆ "	1'-1 ¹ / ₄ "	2'-0 ¹ / ₄ "	
5		2'-6 ⁷ / ₈ "	1'-7 ⁵ / ₁₆ "		11 ¹ / ₂ "		1'-4 ¹ / ₂ "
6		2'-5 ¹ / ₈ "	1'-7 ⁵ / ₁₆ "		9 ¹³ / ₁₆ "		1'-2 ¹³ / ₁₆ "
7		2'-3 ³ / ₈ "	1'-7 ⁵ / ₁₆ "		8 ¹ / ₁₆ "		1'-1 ¹ / ₁₆ "

ALL REMAINING POSTS TO HAVE THE SAME DIMENSIONS AS POST NO. 7

	NE SIDE	SE SIDE	NW SIDE	SW SIDE
X	1 ³ / ₁₆ "	1 ⁵ / ₁₆ "	1 ⁵ / ₁₆ "	1 ¹ / ₄ "
Y	5 ¹ / ₈ "	3 ³ / ₈ "	3 ¹ / ₄ "	5 ¹ / ₈ "
Z	1 ¹ / ₁₆ "	3 ¹ / ₁₆ "	3 ³ / ₈ "	1 ¹ / ₄ "



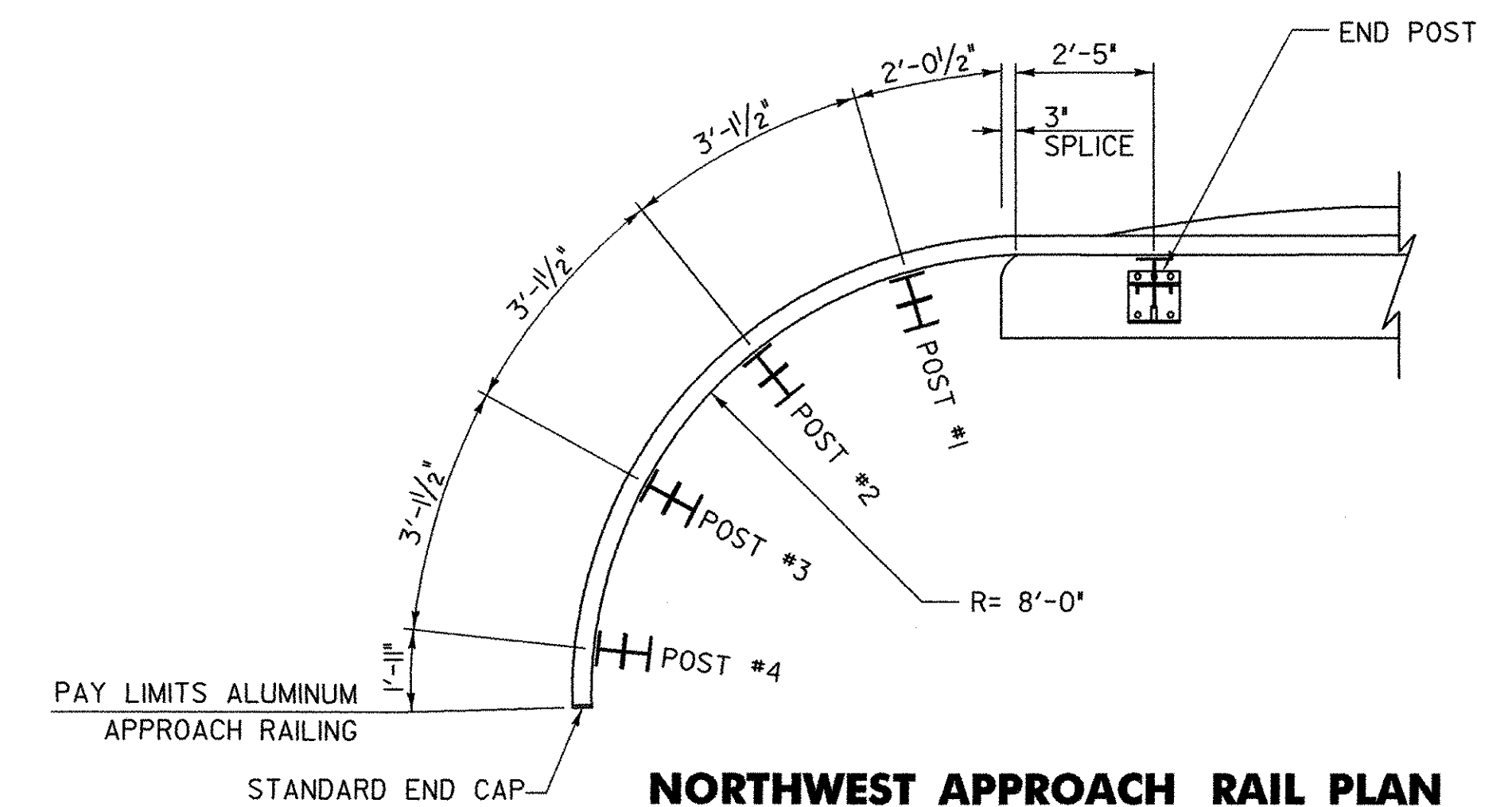
PLAN



ELEVATION

ALUMINUM APPROACH RAILING END DETAIL

SCALE 3/8" = 1'-0"
1 0 1 2 3 4



NORTHWEST APPROACH RAIL PLAN

SCALE 3/8" = 1'-0"
1 0 1 2 3 4

NW ALUMINUM RAIL DIMENSIONS

POST NO.	RAIL HEIGHT DIMENSIONS			OFFSET BLOCK DIMENSIONS			
	A	B	C	D	E	F	G
1	4'-1 ³ / ₁₆ "	3'-2 ³ / ₁₆ "	1'-9"	11 ⁷ / ₁₆ "	1'-5 ¹ / ₈ "	2'-9 ¹ / ₈ "	
2	3'-9 ¹³ / ₁₆ "	3'-0 ⁵ / ₈ "	1'-8 ³ / ₄ "	9 ³ / ₄ "	1'-3 ⁷ / ₈ "	2'-6 ³ / ₁₆ "	
3	3'-7 ³ / ₁₆ "	2'-11 ¹ / ₂ "	1'-8 ¹⁵ / ₁₆ "	8 ¹ / ₈ "	1'-2 ⁹ / ₁₆ "	2'-3 ¹ / ₄ "	
4	3'-4 ⁷ / ₈ "	2'-10 ⁷ / ₈ "	1'-9 ⁵ / ₈ "	6 ⁷ / ₁₆ "	1'-1 ¹ / ₄ "	2'-0 ¹ / ₄ "	

STATE OF VERMONT
AGENCY OF TRANSPORTATION

Town Of HUNTINGTON Bridge No. 42
Highway No. T.H. 4 Log Sta.
Surv. Sta.

EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

ALUMINUM APPROACH RAILING DETAILS

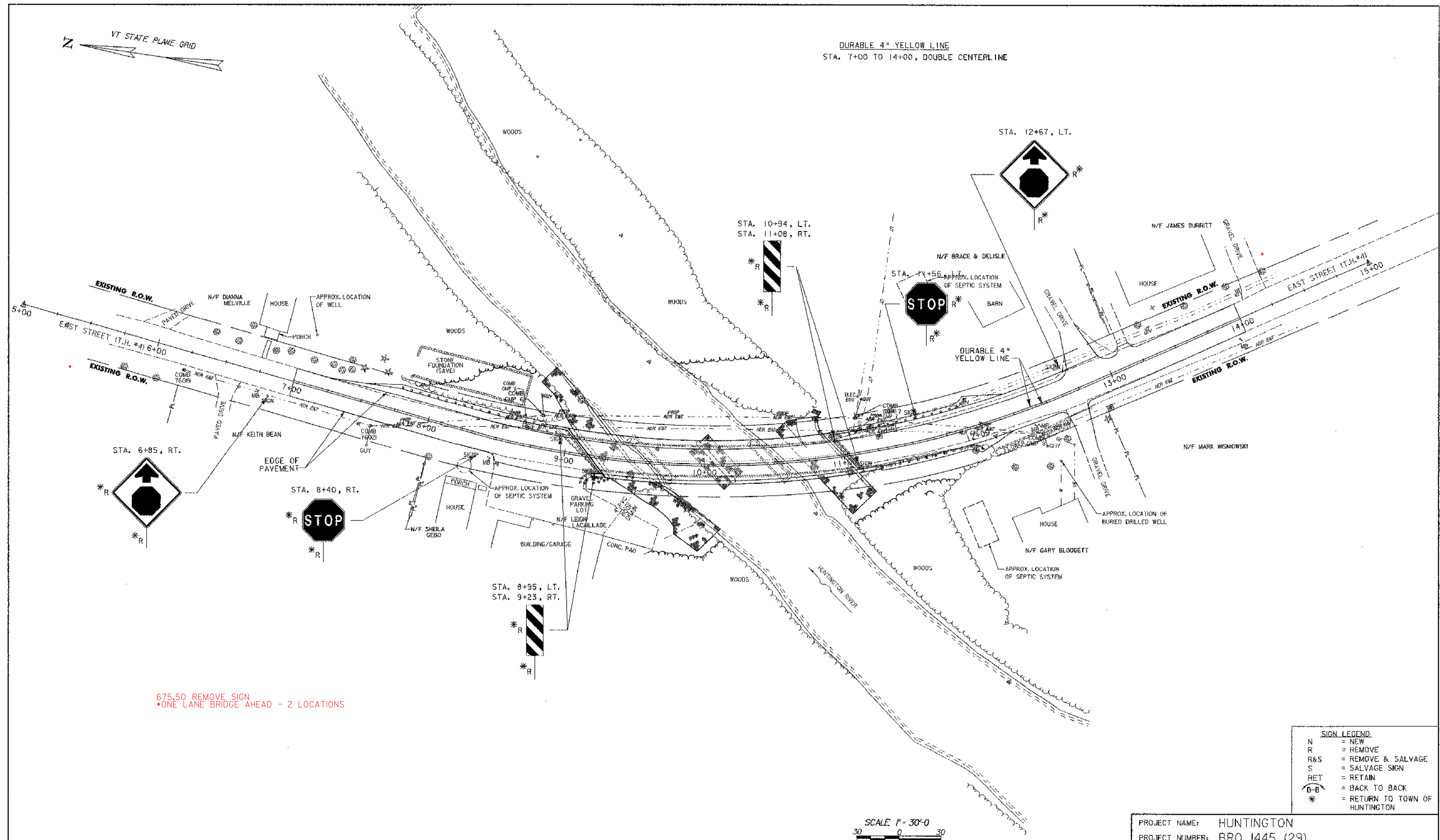
Designed By VTrans/D-H Drawn By J. SOTER
Checked By Date Bridge Design Supervisor
M. CHENETTE 08/05 M. CHENETTE Date 09/05

PROJECT HUNTINGTON PROJECT NO. BRO 1445 (29)

DH Dufresne-Henry Bridge Sheet No. Plot Date: 1/12/2006 Sheet 53 of 63



DURABLE 4" YELLOW LINE
STA. 7+00 TO 14+00, DOUBLE CENTERLINE



675.50 REMOVE SIGN
*ONE LANE BRIDGE AHEAD - 2 LOCATIONS

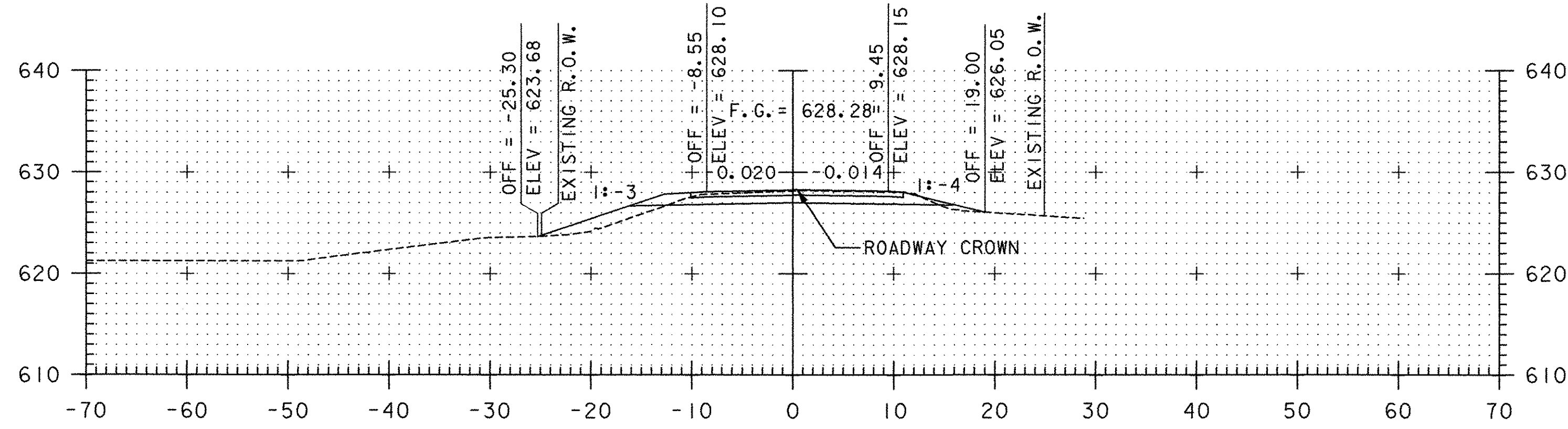
SIGN LEGEND	
N	= NEW
R	= REMOVE
R&S	= REMOVE & SALVAGE
S	= SALVAGE SIGN
RET	= RETAIN
B-B	= BACK TO BACK
*	= RETURN TO TOWN OF HUNTINGTON

SCALE 1" = 30'-0"
30 0 30

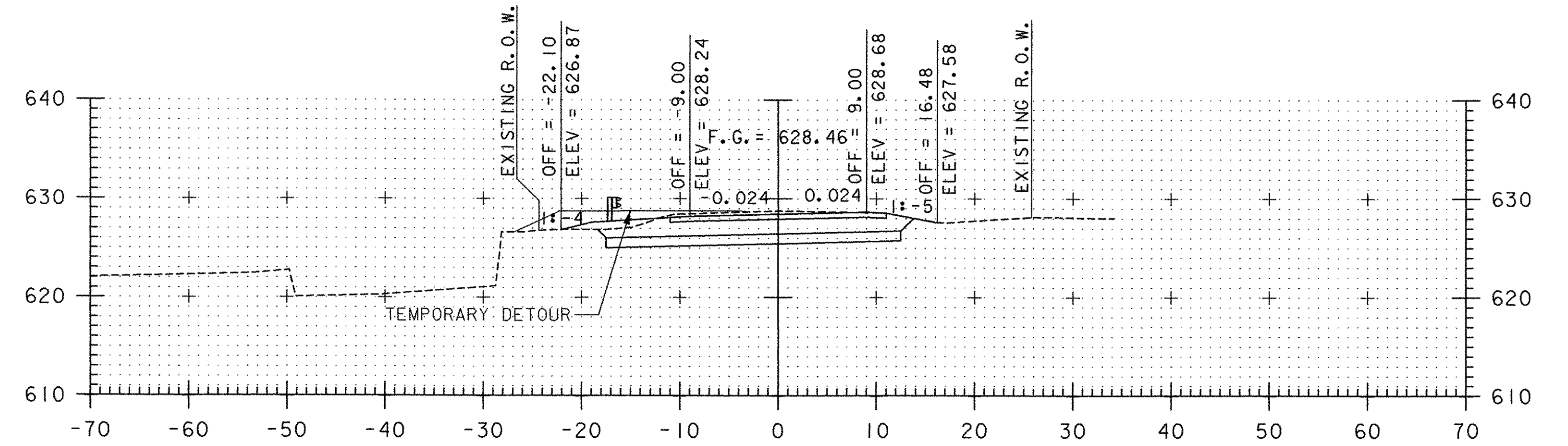
PROJECT NAME:	HUNTINGTON
PROJECT NUMBER:	BRO 1445 (29)
FILE NAME:	...\\Cadd\Trans\z01j302pvmrk.dgn
PROJECT LEADER:	M. CHENETTE
DESIGNED BY:	D. ALTERI
PAVEMENT MARKING PLAN	
PLOT DATE:	1/30/2006
DRAWN BY:	J. OAKMAN
CHECKED BY:	D. ALTERI
SHEET 54 OF 63	



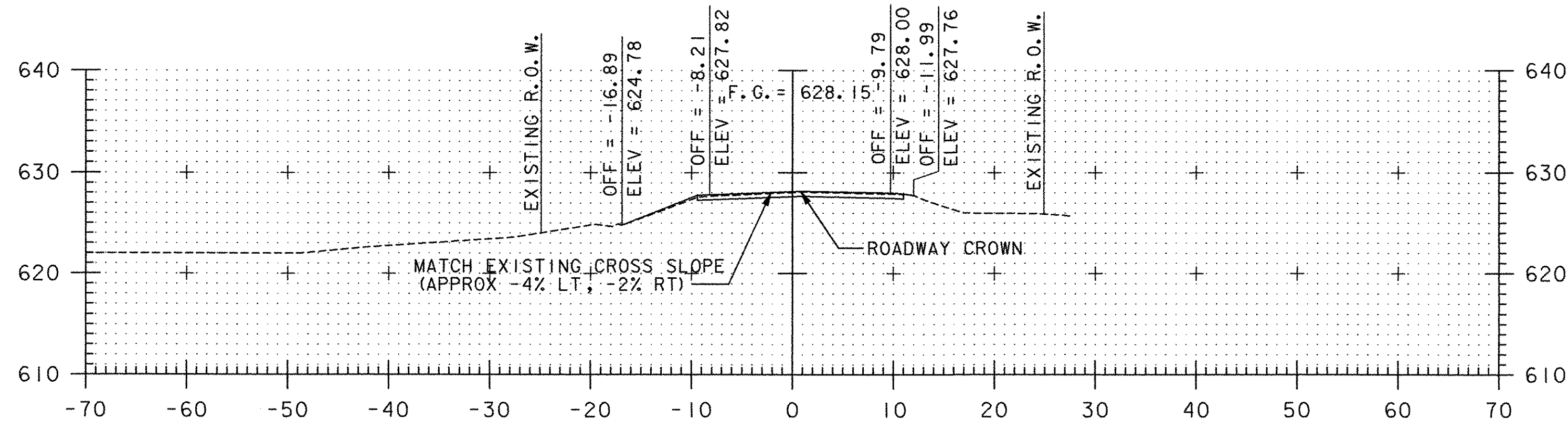
ITEM	NO. PIECES	SIZE	LENGTH	MARK	TYPE	A	B	C	D	E	F	G	H	J	K	R	O	ITEM	NO. PIECES	SIZE	LENGTH	MARK	TYPE	A	B	C	D	E	F	G	H	J	K	R	O
ABUTMENT NO. 1																		WINGWALL NO. 3 (CONTINUED)																	
75	5	4'-6"		IA501	IT		10'	3'-8"										101	5	6	10'-10"	3W601	STR.												
47	5	10'-7"		IA502	STR.													102	3	6	18'-5"	3W602	STR.												
47	5	6'-1"		IA503	STR.													103	7	6	10'-7"	3W603	STR.												
10	5	4'-8"		IA504	STR.													104	7	6	7'-3"	3W604	STR.												
40	5	7'-2"		IA505	STR.													WINGWALL NO. 4																	
7	5	4'-5"		IA506	STR.													106	4	5	6'-9"	4W501	STR.												
12	5	4'-0"		IA507	22		2'-0"	2'-0"										108	2	5	10'-7"	4W502	STR.												
12	5	4'-0"		IA508	22		2'-0"	2'-0"										109	6	5	12'-9"	4W503	STR.												
43	5	6'-4"		IA509	IT			2'-10"	3'-6"									110	7	5	8'-7"	4W504	STR.												
43	5	3'-9"		IA510	STR.													111	5	5	10'-1"	4W505	STR.												
3	5	12'-4"		IEA511	IT		5'-9"	10'	5'-9"									112	3	5	17'-4"	4W506	STR.												
31	5	10'-10"		IEA512	IT		5'-0"	10'	5'-0"									113	7	5	10'-0"	4W507	STR.												
10	5	4'-0"		IA513	STR.													114	7	5	6'-11"	4W508	STR.												
31	5	5'-10"		IA514	16	10'	1'-10"	7'	2'-7"									115	8	5	8'-10"	4W509	STR.												
4	5	38'-0"		IA515	STR.													116	7	5	8'-7"	4W510	STR.												
2	5	42'-11"		IEA516	STR.													117	13	5	5'-9 1/2"	4W511	IT		2'-0"	1'-9 1/2"	2'-0"								
7	5	17'-7"		IA517	1	7'	16'-5"											PIER																	
8	5	8'-4"		IA518	1	7'	7'-2"											124	37	5	13'-6"	P501	STR.												
ABUTMENT NO. 2																		126	14	5	35'-6"	P502	STR.												
64	5	4'-6"		2A501	IT		10'	3'-8"										127	28	5	23'-0"	P503	STR.												
40	5	10'-7"		2A502	STR.													128	29	5	7'-8"	P504	STR.												
41	5	6'-2"		2A503	STR.													129	56	5	12'-3"	P505	TI		5 1/2"	3'-4"	2'-4"	3'-4"	2'-4"		5 1/2"	3'-2"		2'-4"	
10	5	35'-9"		2A504	STR.													130	2	5	11'-6"	P506	STR.												
34	5	5'-1"		2A505	STR.													131	2	5	12'-7 1/2"	P507	STR.												
7	5	36'-10"		2A506	STR.													132	2	5	27'-9 1/2"	P508	STR.												
7	5	4'-0"		2A507	22		2'-0"	2'-0"										133	2	5	35'-6 1/2"	P509	STR.												
7	5	4'-0"		2A508	27		2'-0"	2'-0"										134	2	5	38'-4"	P510	STR.												
37	5	6'-0"		2A509	IT													135	2	5	18'-3"	P511	STR.												
38	5	3'-3"		2A510	STR.													136	32	5	6'-10"	P512	IT		2'-3"	2'-4"	2'-3"								
38	5	7'-7"		2A511	IT													137	8	5	10'-10 1/2"	P513	22		2'-5"	6'-5 1/2"	2'-0"					2'-4"	6"	7 1/4"	1'-11 1/4"
10	5	36'-8"		2A512	STR.													138	6	5	6'-2 1/2"	P514	IT												
33	5	5'-2"		2A513	16	10'	1'-7"	7'	2'-2"									139	4	5	7'-0"	P515	STR.												
4	5	32'-3"		2A514	STR.													141	4	6	9'-4"	P601	TI		6'	2'-6"	1'-8"	2'-6"	1'-8"					6'	
7	5	7'-9"		2A515	1	7'	6'-7"											142	4	6	9'-7 1/2"	P602	TI		6'	2'-7 3/4"	1'-8"	2'-7 3/4"	1'-8"					6'	
7	5	15'-4"		2A516	1	7'	14'-2"											143	4	6	9'-11"	P603	TI		6'	2'-9 1/2"	1'-8"	2'-9 1/2"	1'-8"					6'	
34	6	7'-8"		2A601	IT		1'-0"	6'-8"										144	4	6	10'-3"	P604	TI		6'	2'-10 1/2"	1'-8"	2'-10 1/2"	1'-8"					6'	
77	7	12'-11"		2A701	IT		1'-2"	10'-7"	1'-2"									145	4	6	10'-6 1/2"	P605	TI		6'	3'-1 1/4"	1'-8"	3'-1 1/4"	1'-8"					6'	
25	7	39'-0"		2A702	STR.													146	4	6	10'-10"	P606	TI		6'	3'-3"	1'-8"	3'-3"	1'-8"					6'	
16	8	9'-4"		2A801	IT		1'-4"	8'-0"										147	4	6	11'-1 1/2"	P607	TI		6'	3'-4 3/4"	1'-8"	3'-4 3/4"	1'-8"					6'	
17	8	4'-0"		2EA802	22		2'-0"	2'-0"										148	4	6	11'-5"	P608	TI		6'	3'-6 1/2"	1'-8"	3'-6 1/2"	1'-8"					6'	
WINGWALL NO. 1																		149	4	6	11'-9"	P609	TI		6'	3'-8 1/2"	1'-8"	3'-8 1/2"	1'-8"					6'	
5	5	6'-0"		1W501	STR.													150	4	6	12'-0 1/2"	P610	TI		6'	3'-10 1/4"	1'-8"	3'-10 1/4"	1'-8"					6'	
2	5	10'-5"		1W502	STR.													151	4	6	12'-4"	P611	TI		6'	4'-0"	1'-8"	4'-0"	1'-8"					6'	
8	5	12'-6"		1W503	STR.													152																	
4	5	12'-8"		1W504	STR.													153	44	10	16'-4"	P1001	I		1'-5"	13'-6"									
3	5	17'-10"		1W506	STR.													154	14	10	35'-6"	P1002	STR.												
7	5	11'-10"		1W507	STR.													155	28	10	12'-5"	P1003	I		1'-5"	11'-0"									
7	5	7'-0"		1W508	STR.													156	54	10	11'-0"	P1004	IT		1'-8"	9'-4"									
10	5	8'-7"		1W509	STR.													157	55	10	18'-6"	P1005	STR.												
7	5	8'-5"		1W510	STR.													158	14	10	20'-8"	P1006	IT			2'-0"	18'-8"								
15	5	5'-1"		1W511	IT		2'-0"	1'-1"	2'-0"									159	14	10	23'-5"	P1007	I		1'-5"	22'-0"									
WINGWALL NO. 2																		DECK																	
4	5	5'-10"		2W501	STR.													163	471	5	39'-0"	ES501	STR.												
2	5	11'-5"		2W502	STR.													163	183	5	10'-2"	ES502	S5		10'	1'-4"	5'-10"	1'-4"						10'	
9	5	13'-1"		2W503	STR.													164	186	5	6'-1"	ES503	S5		10'	1'-4"	1'-9"	1'-4"						10'	
8	5	12'-6"		2W504	STR.													165	37	5	8'-6"	ES504	S5		10'	1'-4"	4'-3"	1'-4"						10'	
7	5	9'-5"		2W505	STR.													166	4	5	36'-11"	ES505	STR.												
7	5	9'-4"		2W506	STR.													167	43	5	7'-1"	ES506	S5		10'	1'-4"	2'-9"	1'-4"						10'	
10	5	5'-10 1/2"		2W507	IT		2'-0"	1'-9 1/2"	2'-0"									168	5	5	42'-10"	ES507	STR.												
WINGWALL NO. 3																		169	1	5	9'-1"	ES508	S6		10'	1'-4"	4'-9"	1'-4"							
4	5	6'-1"		3W501	STR.													170	1	5	8'-2"	ES509	S6		10'	1'-4"	3'-10"	1'-4"							
1	5	9'-9"		3W502	STR.													171	1	5	7'-2"	ES510	S6		10'	1'-4"	2'-10"	1'-4"							
8	5	12'-3"		3W503	STR.													172	1	5	6'-3"	ES511	S6		10'	1'-4"	1'-11"	1'-4"							
8	5	8'-8"		3W50																															



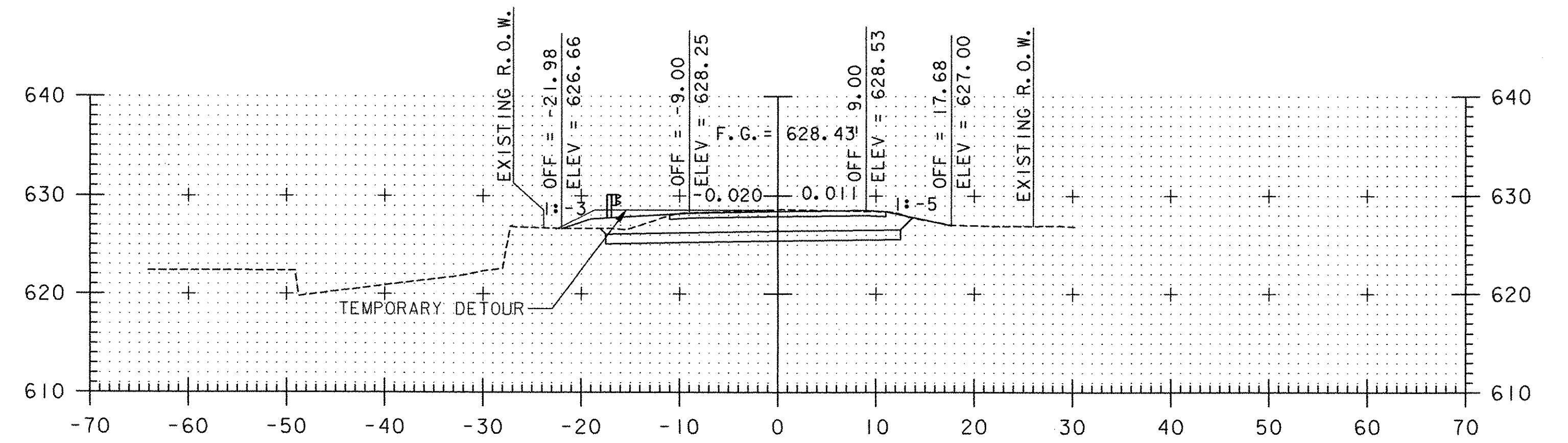
7+50



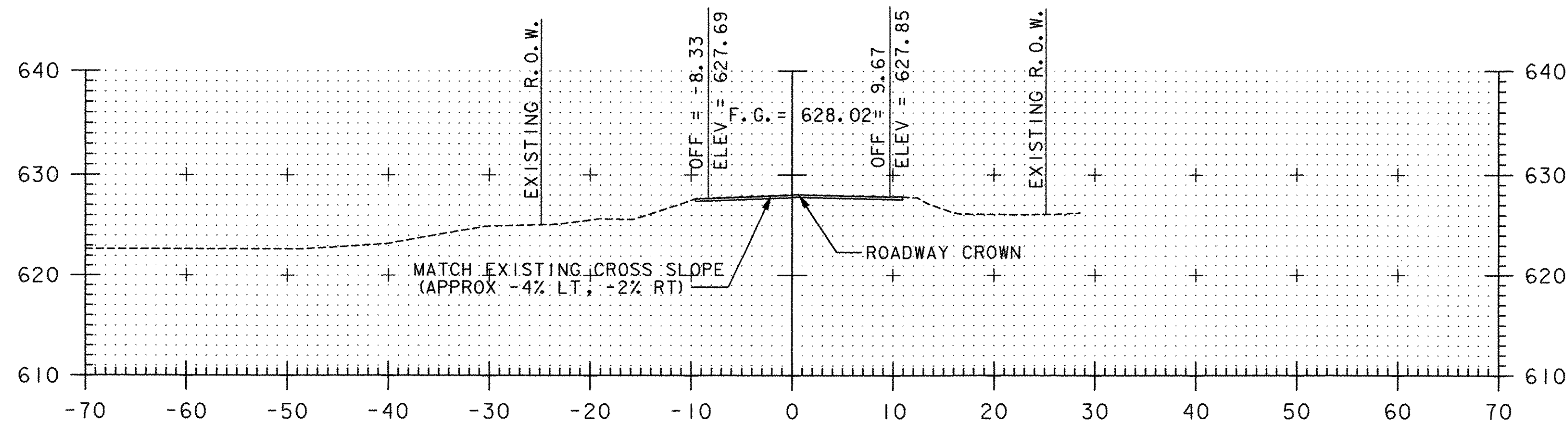
8+25



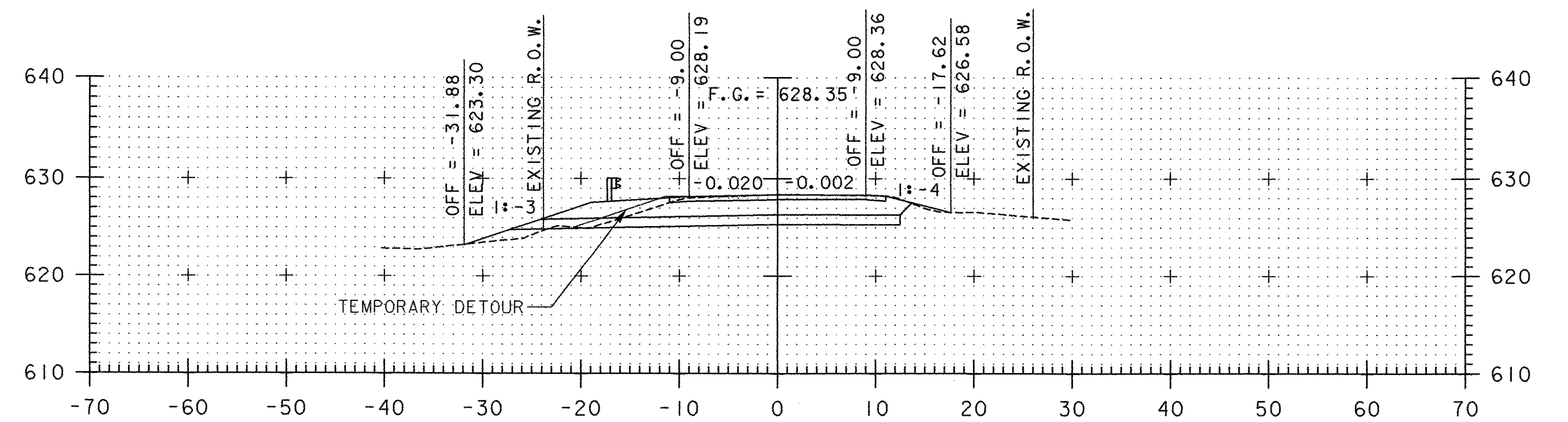
7+25



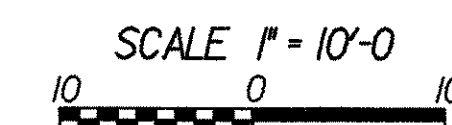
8+00



7+00



7+75

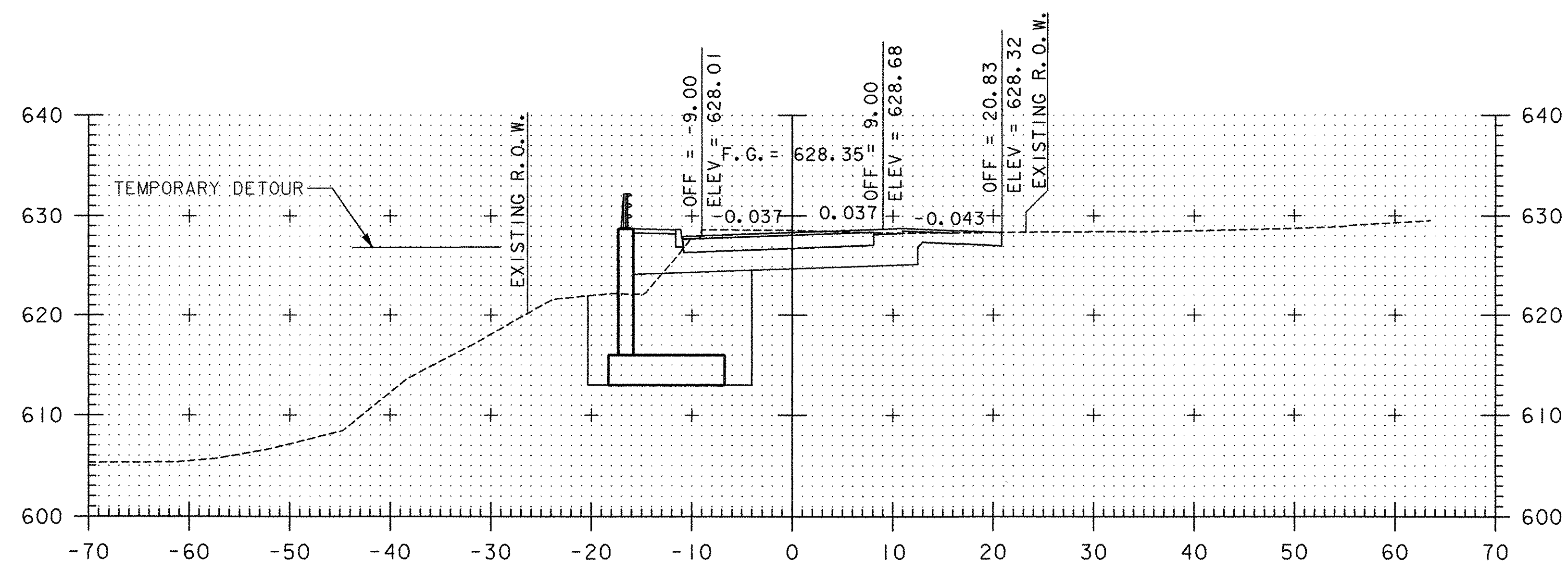


STA. 7+00 TO STA. 8+25

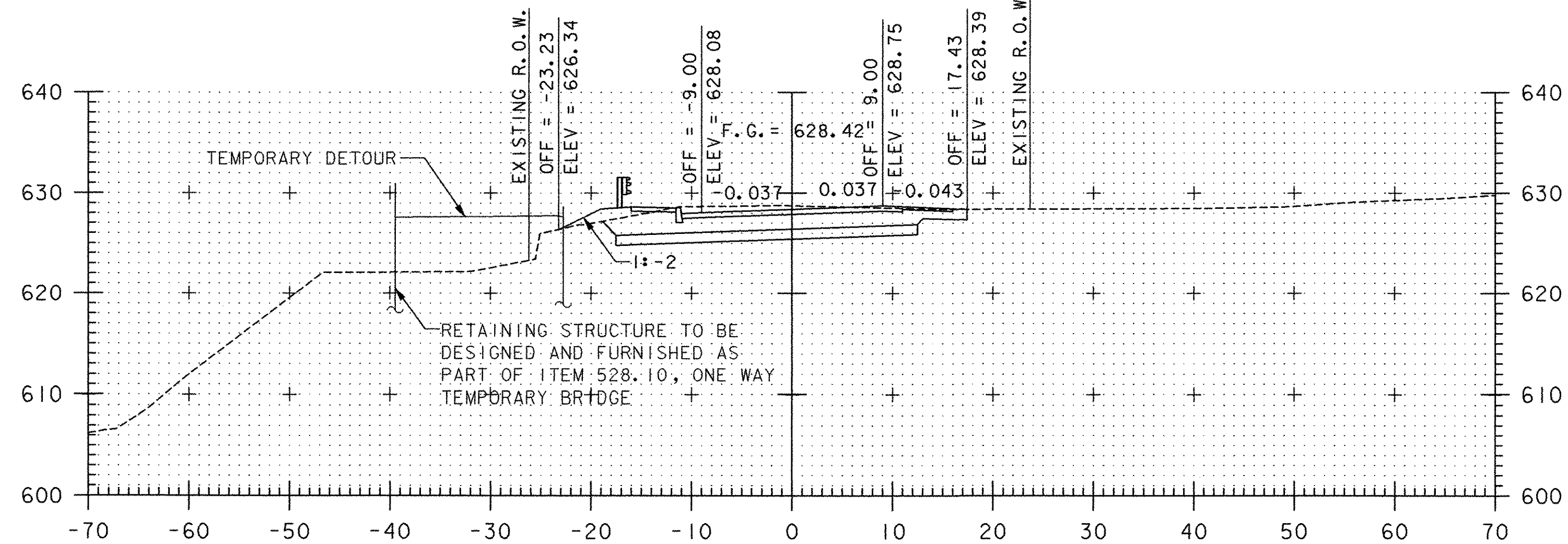


PROJECT NAME: HUNTINGTON
PROJECT NUMBER: BRO 1445 (29)

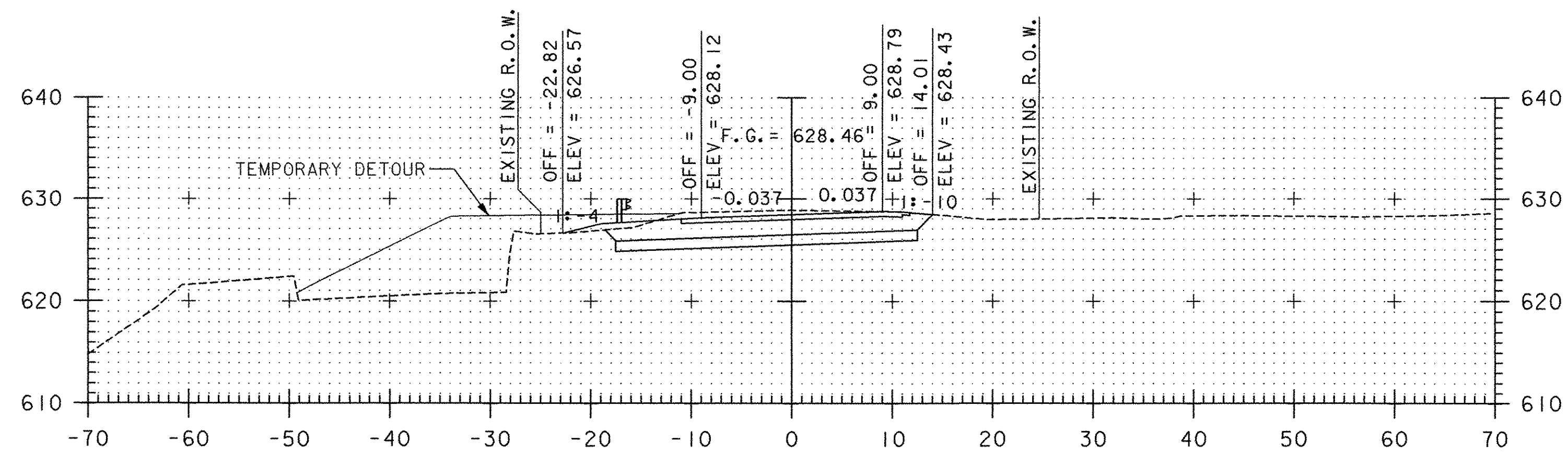
FILE NAME: ...Cadd\Trans\z01j302xsl.dgn PLOT DATE: 1/12/2006
PROJECT LEADER: M. CHENETTE DRAWN BY: J. OAKMAN
DESIGNED BY: D. ALTERI CHECKED BY: D. ALTERI
ROADWAY CROSS SECTION SHEET 56 OF 63



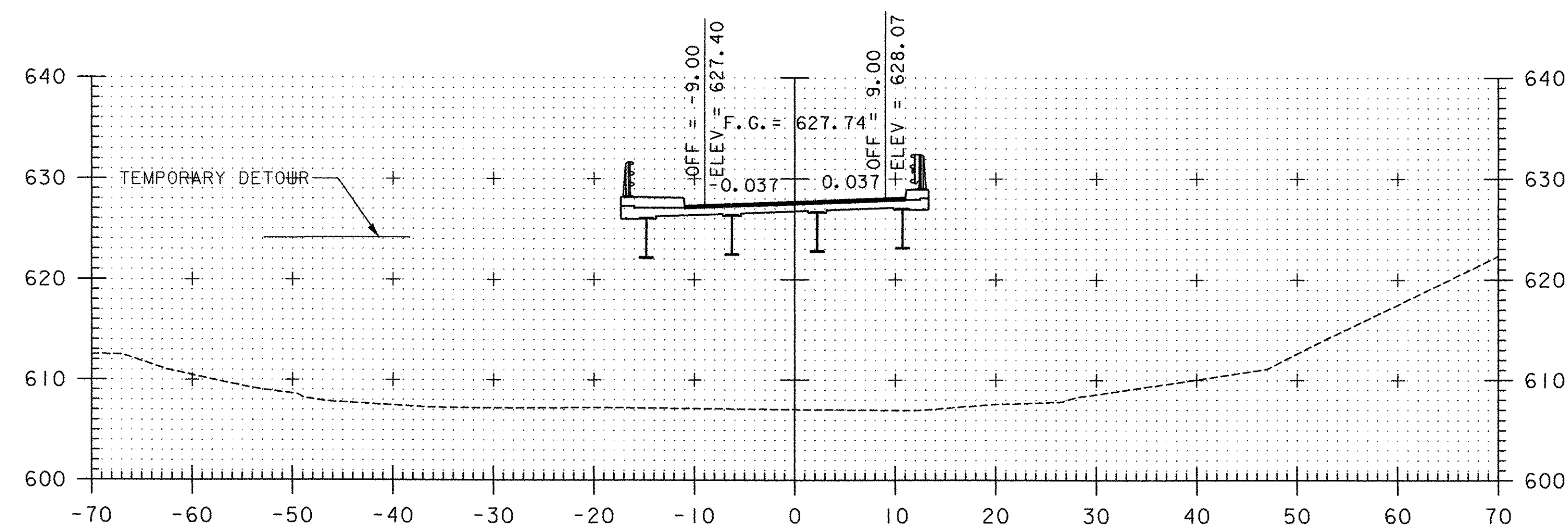
9+00



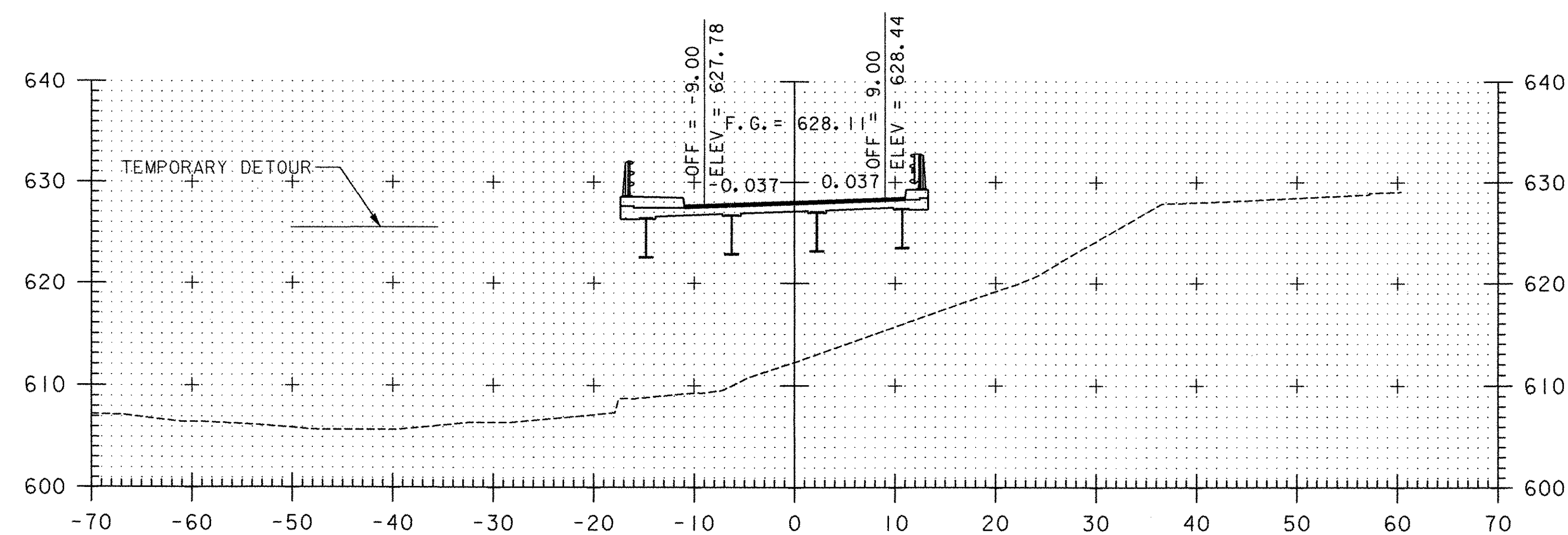
8+75



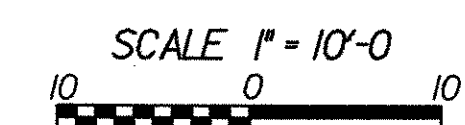
8+50



10+00



9+50



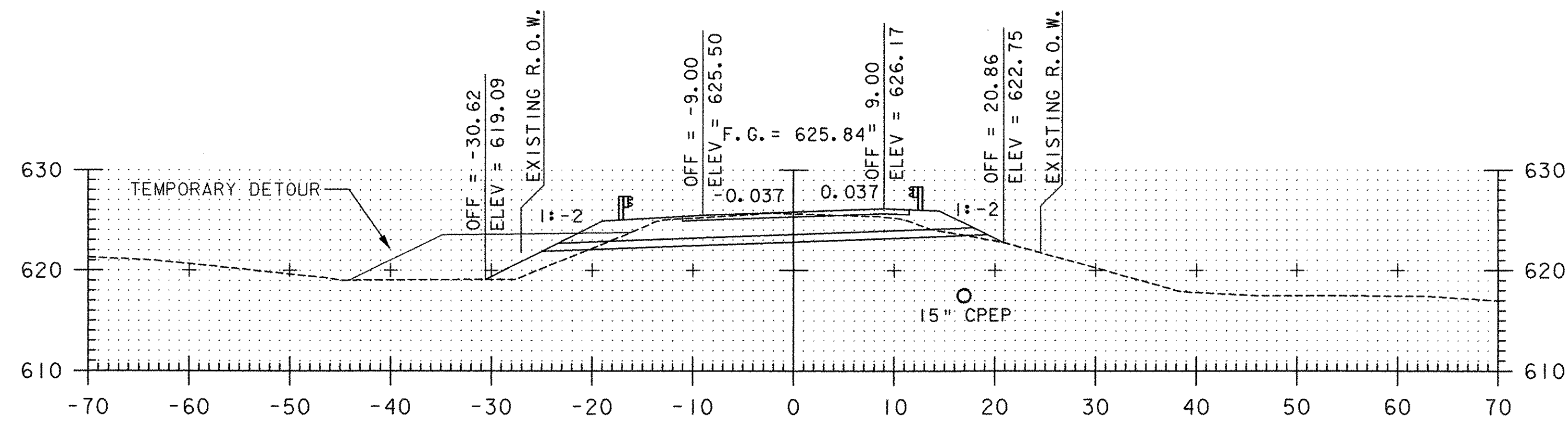
STA. 8+50 TO STA. 10+00



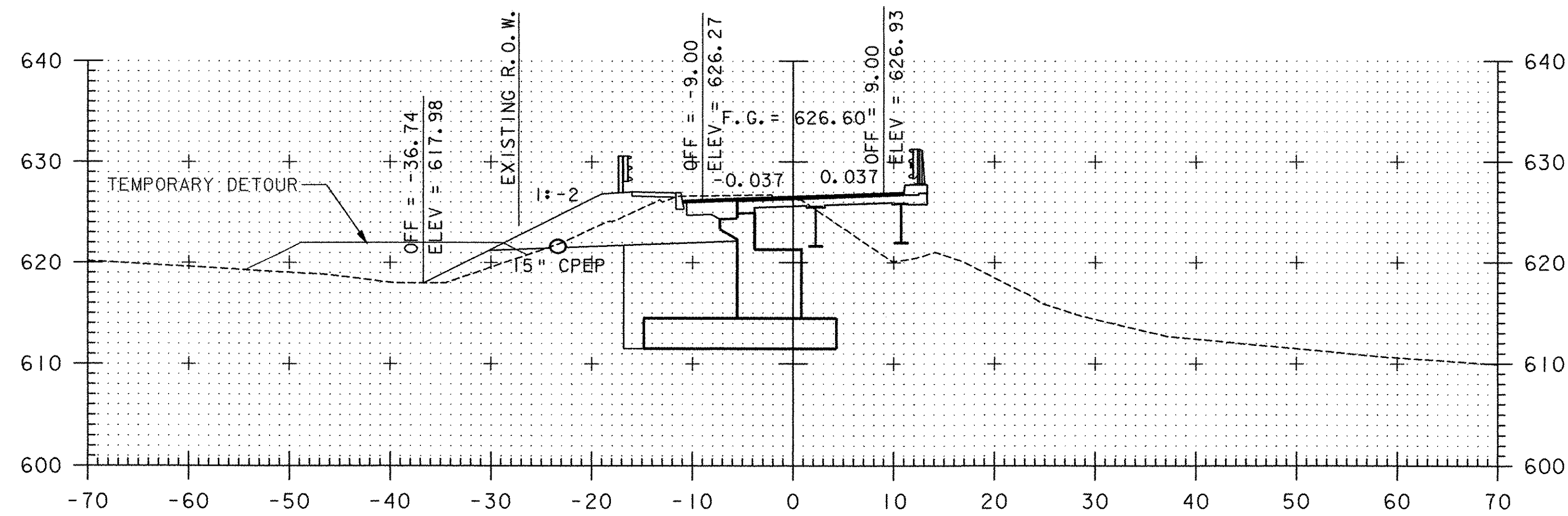
PROJECT NAME: HUNTINGTON
PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...Cadd\Trans\z01j302xsl.dgn
PROJECT LEADER: M. CHENETTE
DESIGNED BY: D. ALTERI
ROADWAY CROSS SECTION

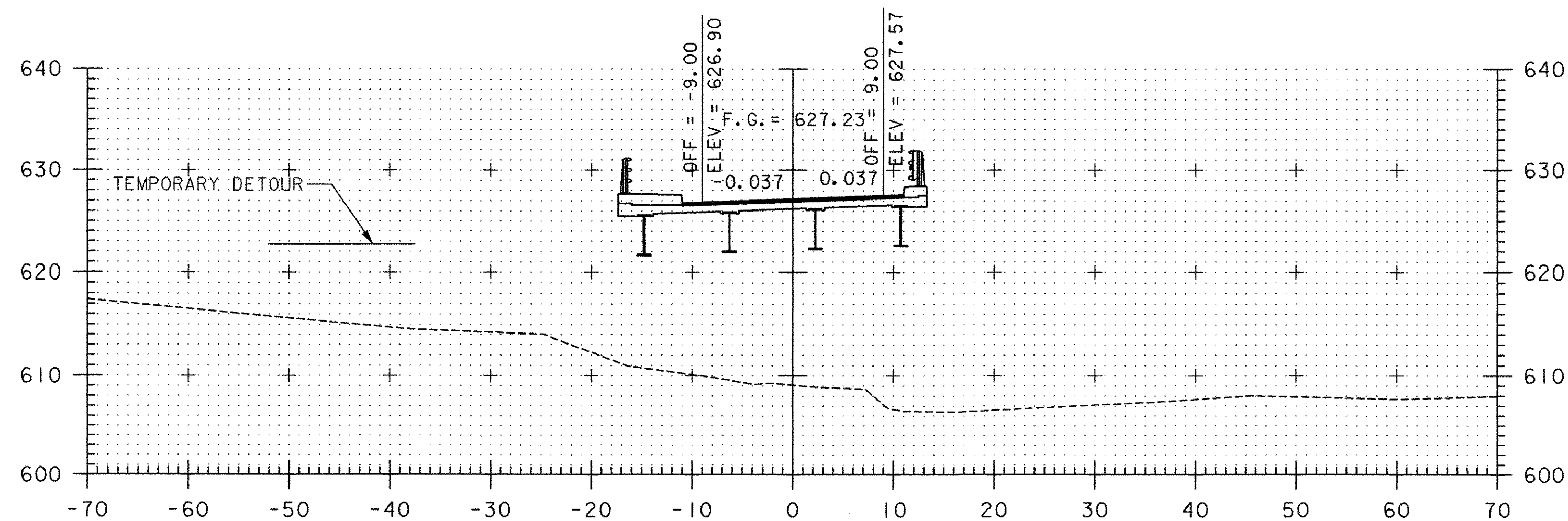
PLOT DATE: 1/12/2006
DRAWN BY: J. OAKMAN
CHECKED BY: D. ALTERI
SHEET 57 OF 63



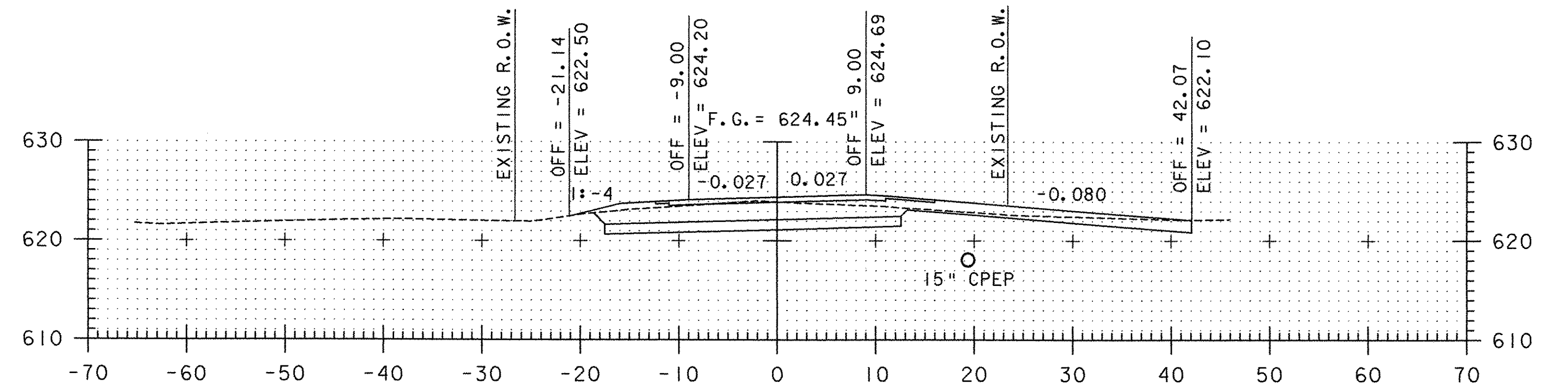
11+50



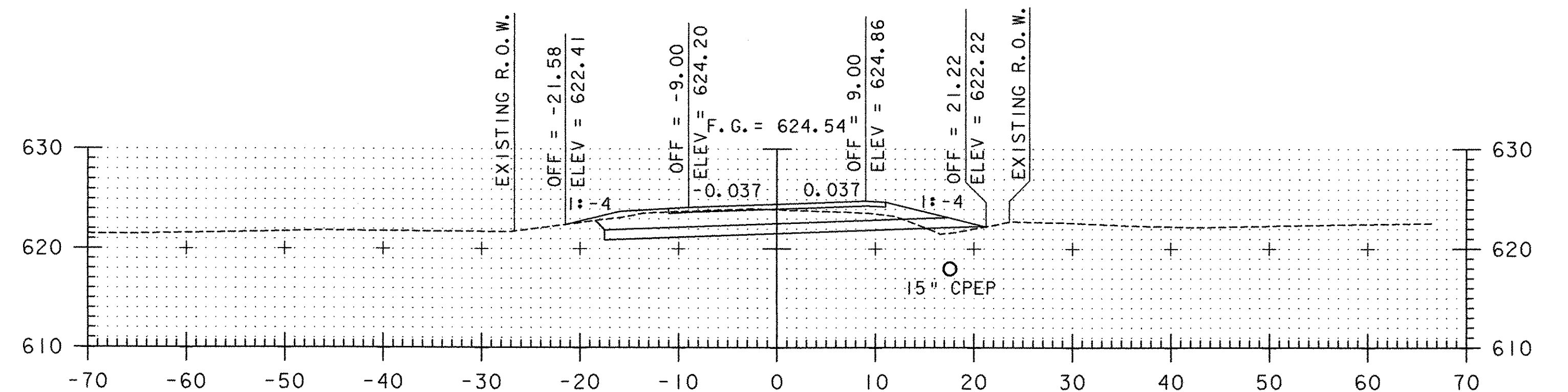
11+00



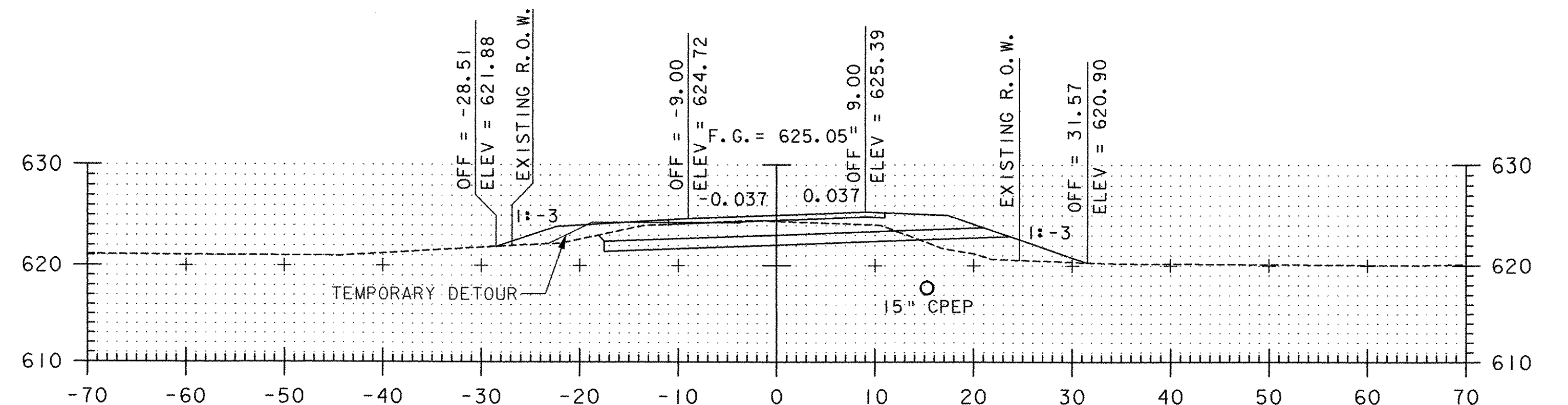
10+50



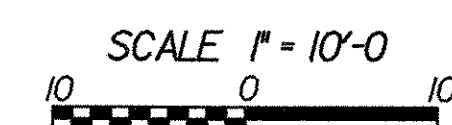
12+68



12+50



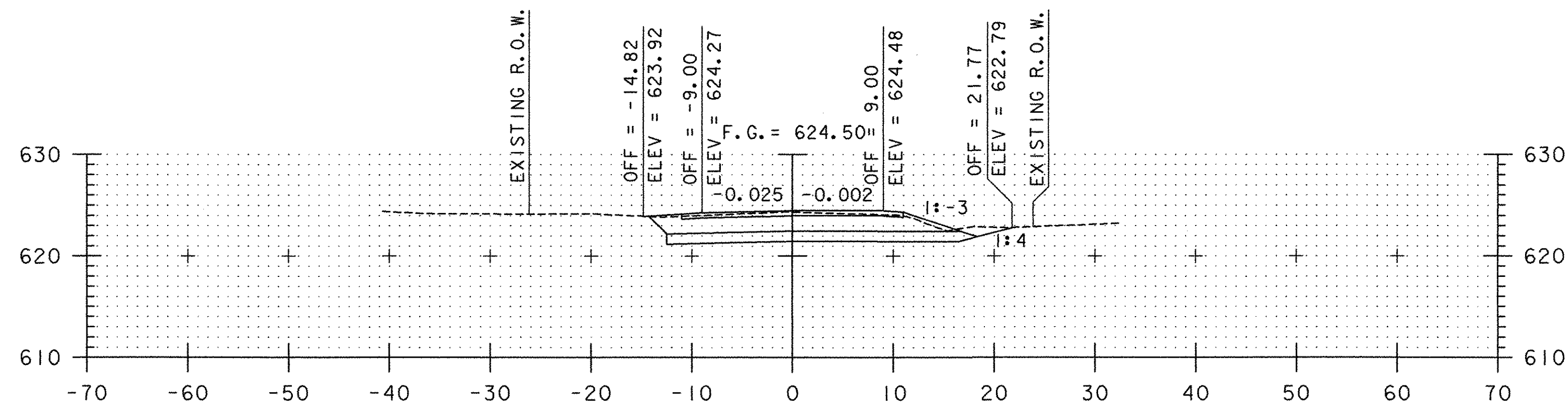
12+00



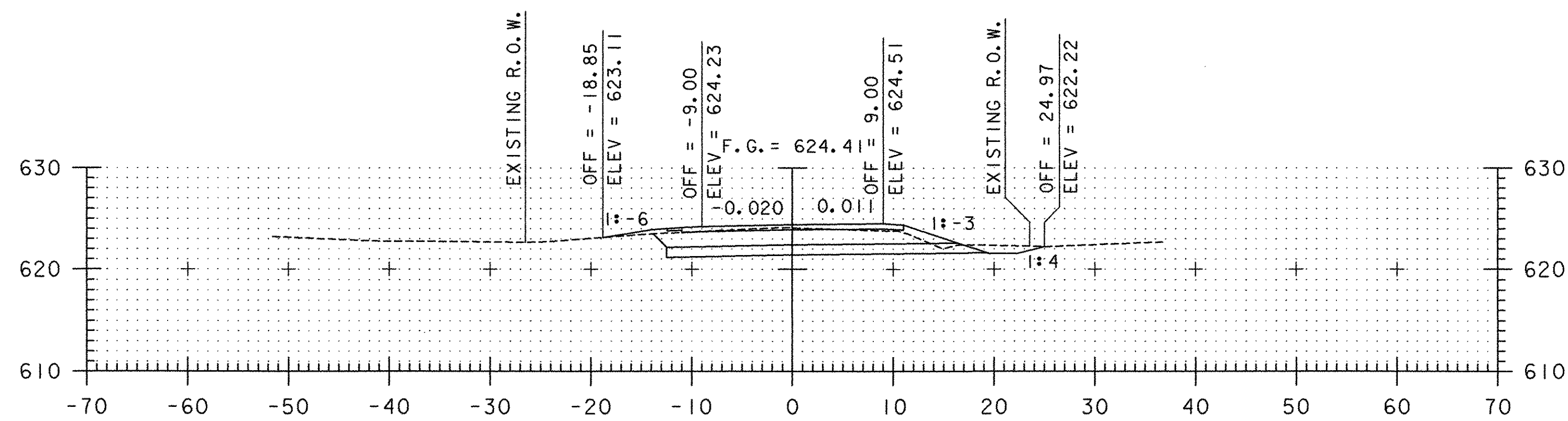
STA. 10+50 TO STA. 12+68



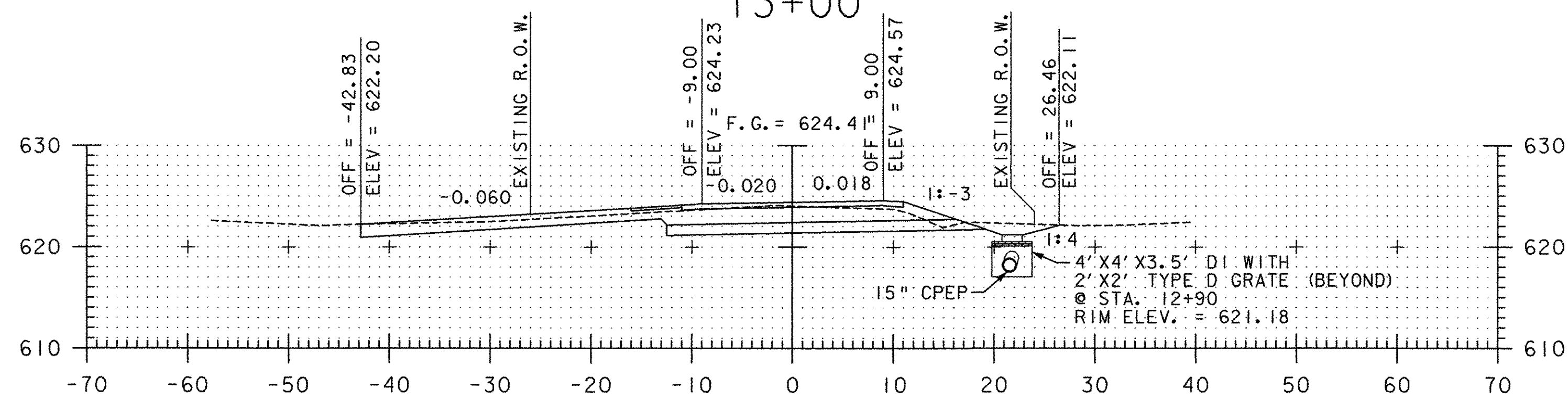
PROJECT NAME:	HUNTINGTON	PLOT DATE:	1/12/2006
PROJECT NUMBER:	BRO 1445 (29)	DRAWN BY:	J. OAKMAN
FILE NAME:	...Cadd\Trans\z0j302xsl.dgn	CHECKED BY:	D. ALTERI
PROJECT LEADER:	M. CHENETTE	SHEET	58 OF 63
DESIGNED BY:	D. ALTERI	ROADWAY CROSS SECTION	



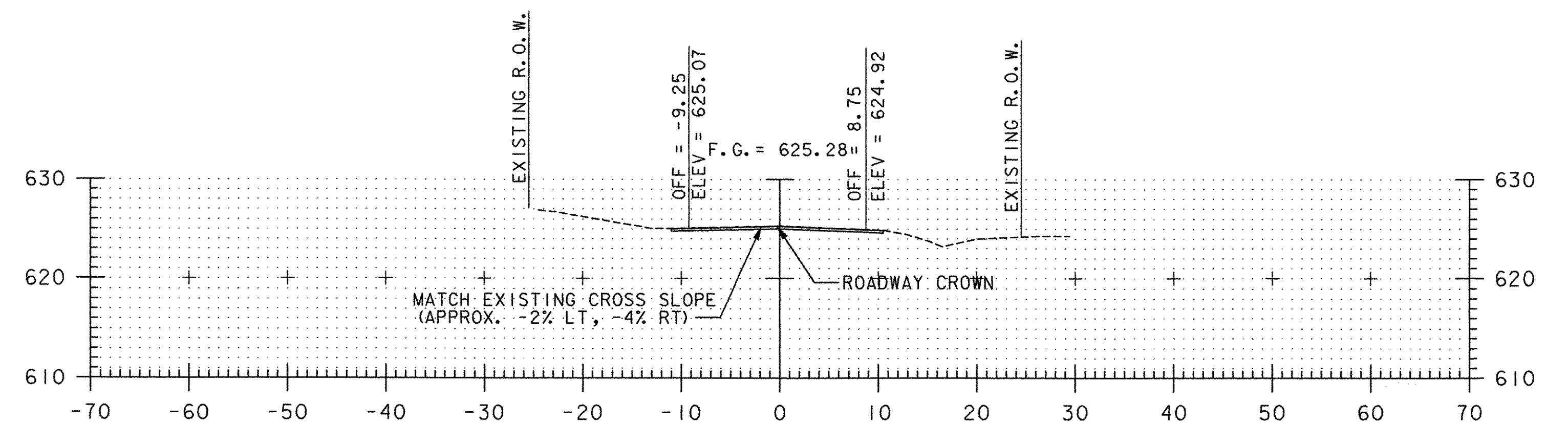
13+25



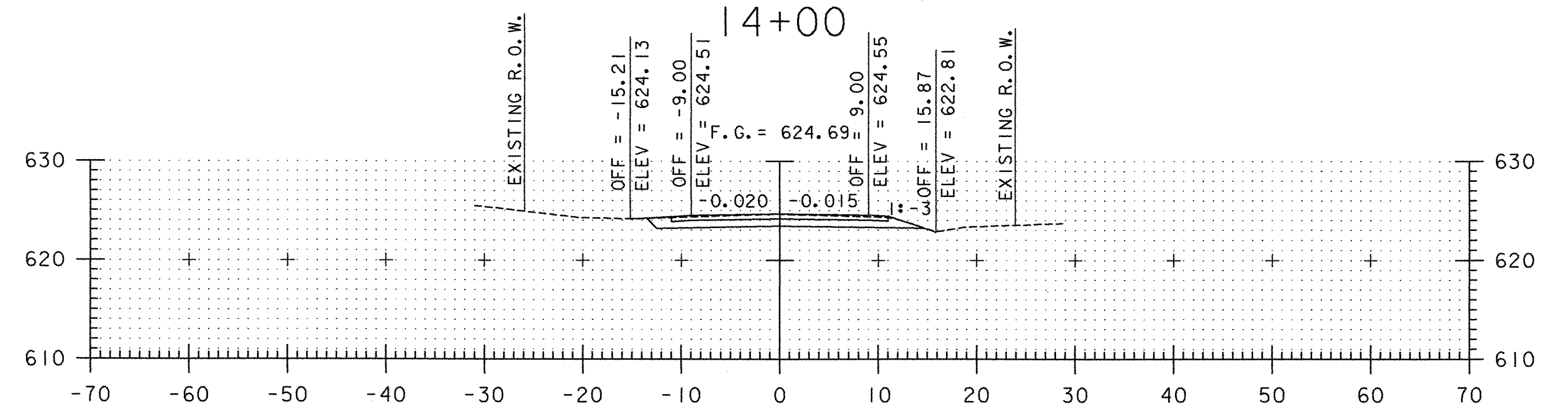
13+00



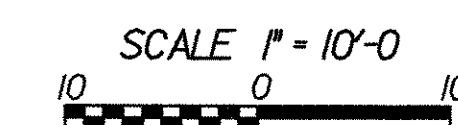
12+86



14+00



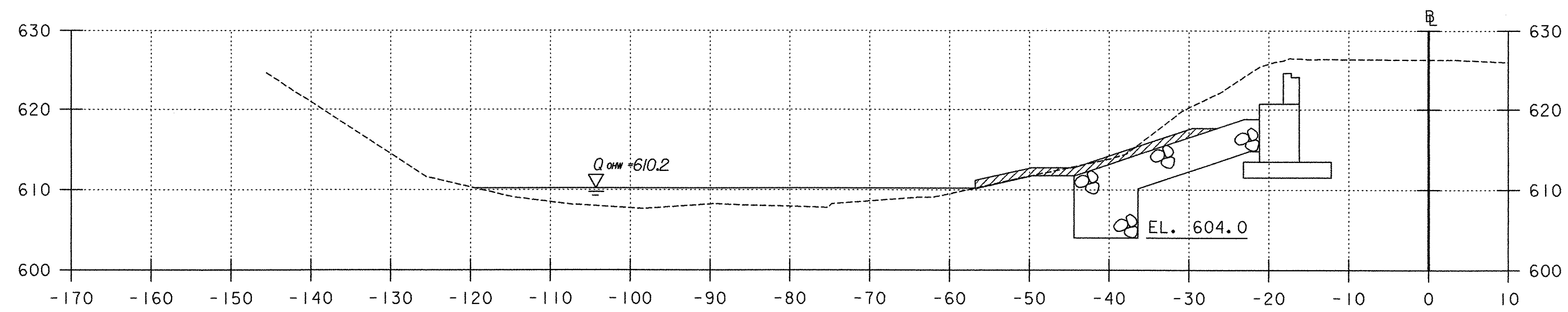
13+50



STA. 12+86 TO STA. 14+00

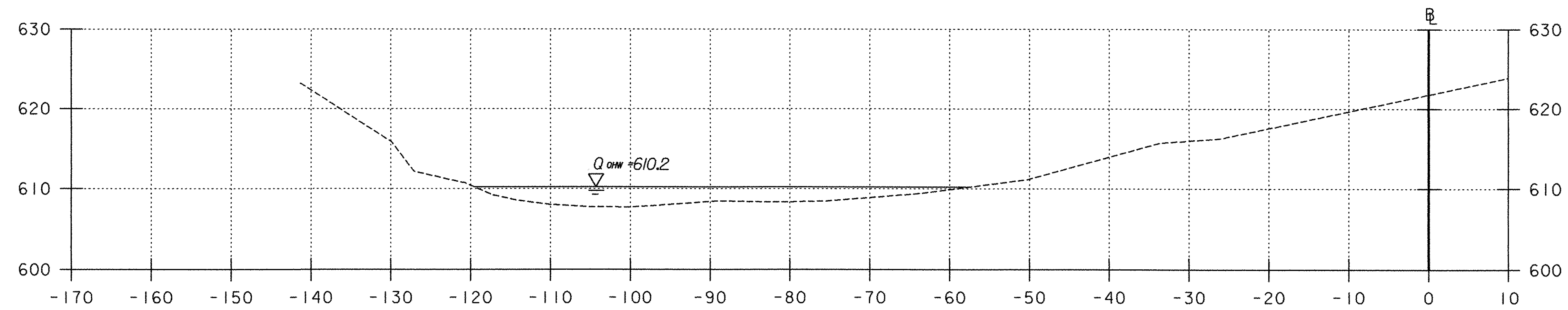


PROJECT NAME:	HUNTINGTON	PROJECT NUMBER:	BRO 1445 (29)
FILE NAME:	...Cadd\Trans\z01j302xsl.dgn	PLOT DATE:	1/12/2006
PROJECT LEADER:	M. CHENETTE	DRAWN BY:	J. OAKMAN
DESIGNED BY:	D. ALTERI	CHECKED BY:	D. ALTERI
ROADWAY CROSS SECTION		SHEET 59 OF 63	

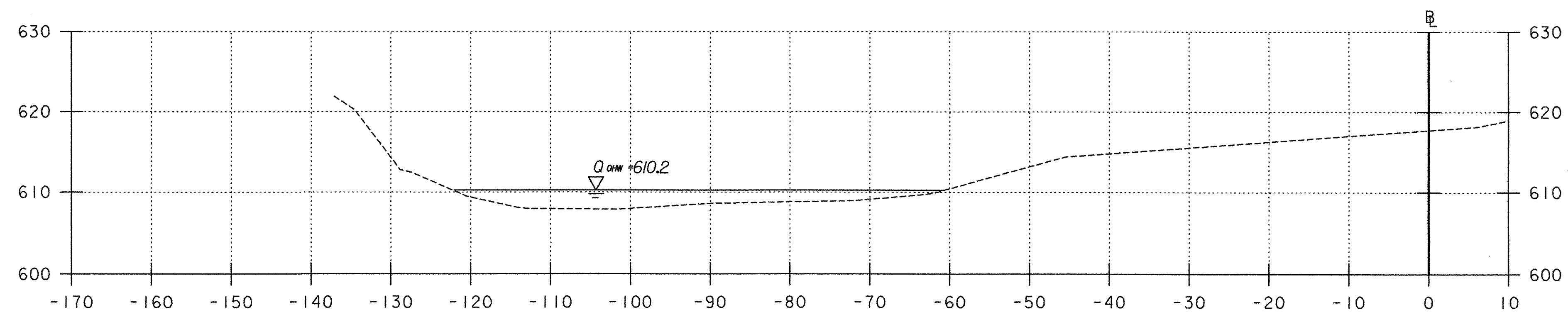


50+50

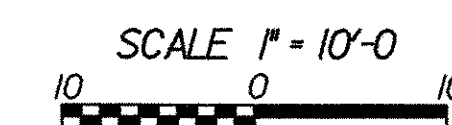
STA. 50+30 (ABUTMENT NO. 2)
 BEGIN UNCLASSIFIED CHANNEL EXCAVATION
 GEOTEXTILE UNDER STONE FILL
 STONE FILL, TYPE IV



50+25



50+00

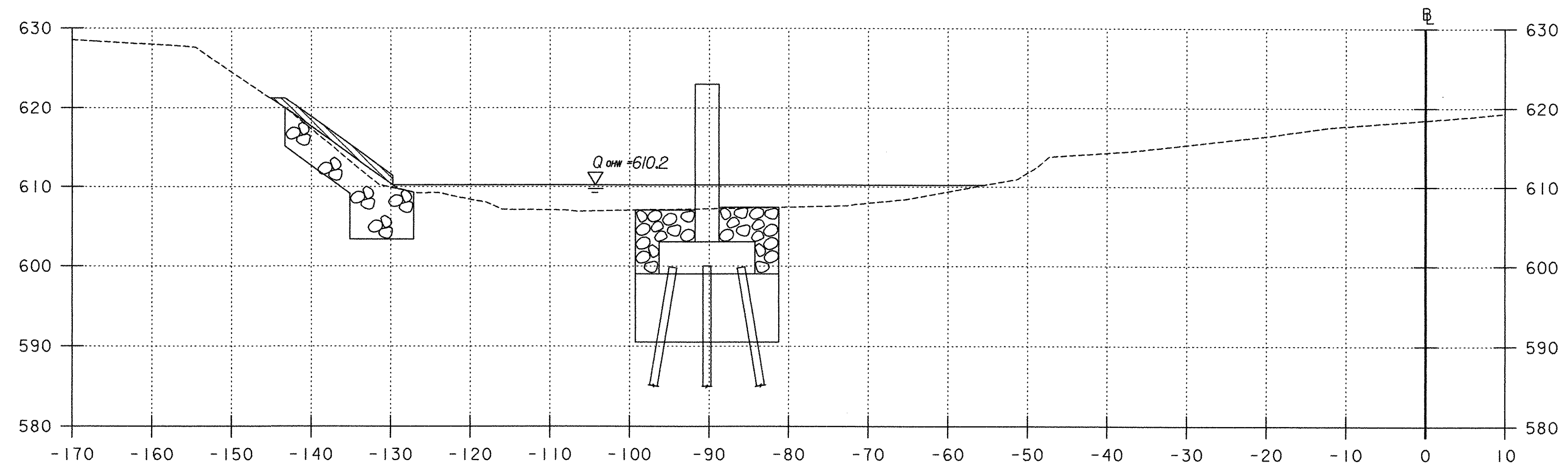


STA. 50+00 TO STA. 50+50



PROJECT NAME: HUNTINGTON	FILE NAME: ...\\Cadd\Trans\z0j302xsl.dgn	PLOT DATE: 1/12/2006
PROJECT NUMBER: BRO 1445 (29)	PROJECT LEADER: M. CHENETTE	DRAWN BY: D. HARRINGTON
	DESIGNED BY: T. KNIGHT	CHECKED BY: M. CHENETTE
		SHEET 60 OF 63

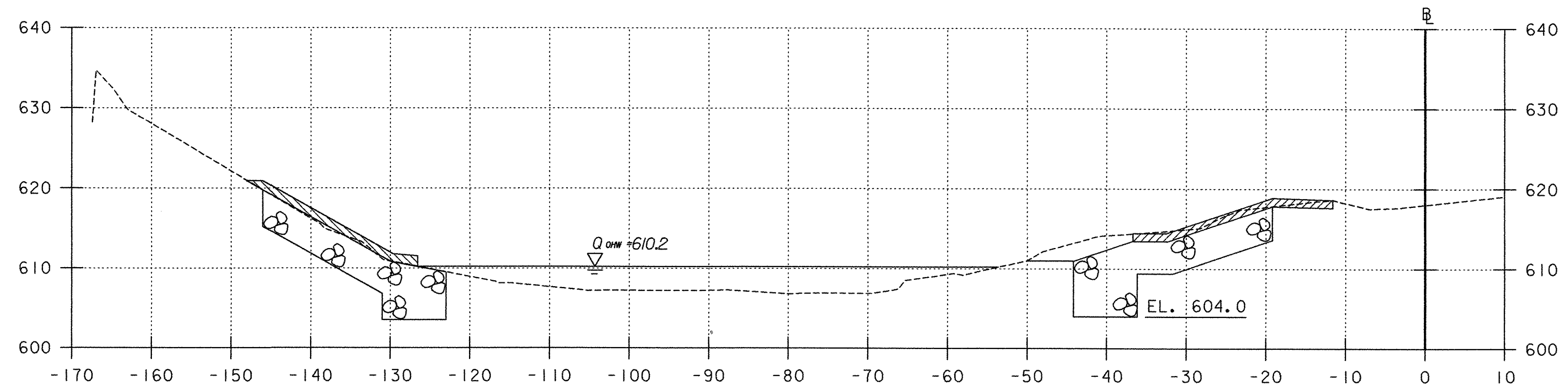
CHANNEL CROSS SECTIONS



51+25

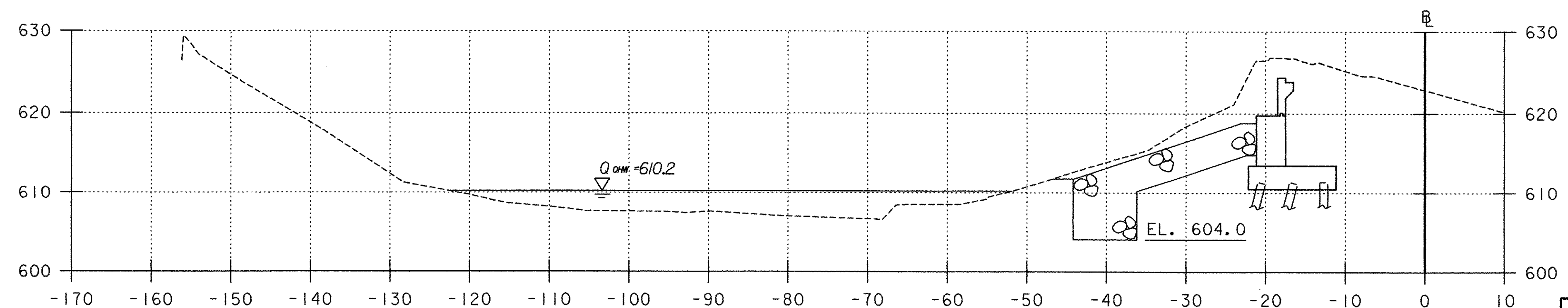
STA. 51+03 (PIER)
 BEGIN COFFERDAM
 COFFERDAM EXCAVATION, EARTH
 STONE FILL, TYPE IV

STA. 51+10 (ABUTMENT NO. 2)
 END UNCLASSIFIED CHANNEL EXCAVATION
 GEOTEXTILE UNDER STONE FILL
 STONE FILL, TYPE IV

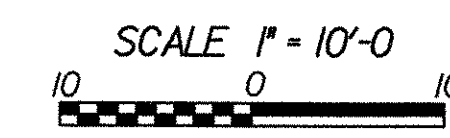


51+00

STA. 50+87 (ABUTMENT NO. 1)
 BEGIN UNCLASSIFIED CHANNEL EXCAVATION
 GEOTEXTILE UNDER STONE FILL
 STONE FILL, TYPE IV



50+75

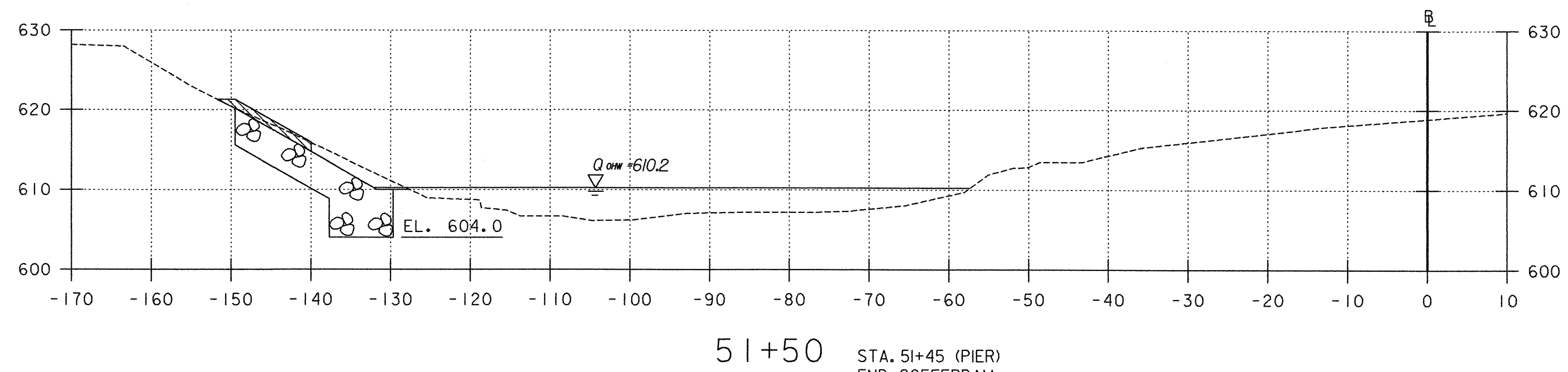
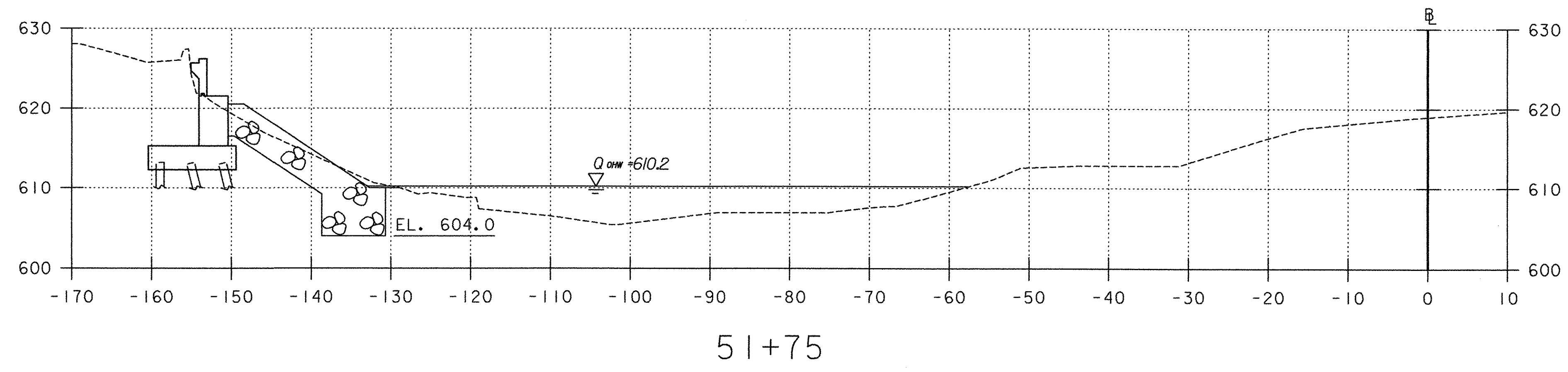
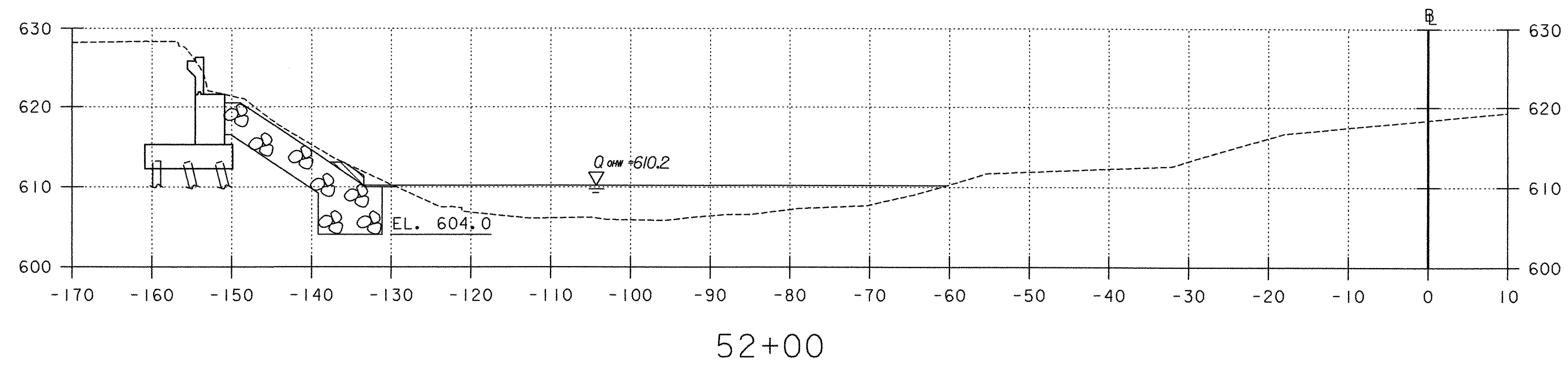


STA. 50+75 TO STA. 51+25

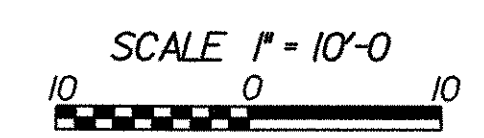


PROJECT NAME:	HUNTINGTON	FILE NAME:	...\\cadd\Trans\z01j302xsl.dgn	PLOT DATE:	1/12/2006
PROJECT NUMBER:	BRO 1445 (29)	PROJECT LEADER:	M. CHENETTE	DRAWN BY:	D. HARRINGTON
		DESIGNED BY:	T. KNIGHT	CHECKED BY:	M. CHENETTE
				SHEET	61 OF 63

CHANNEL CROSS SECTIONS



STA. 51+45 (PIER)
 END COFFERDAM
 COFFERDAM EXCAVATION, EARTH
 STONE FILL, TYPE IV

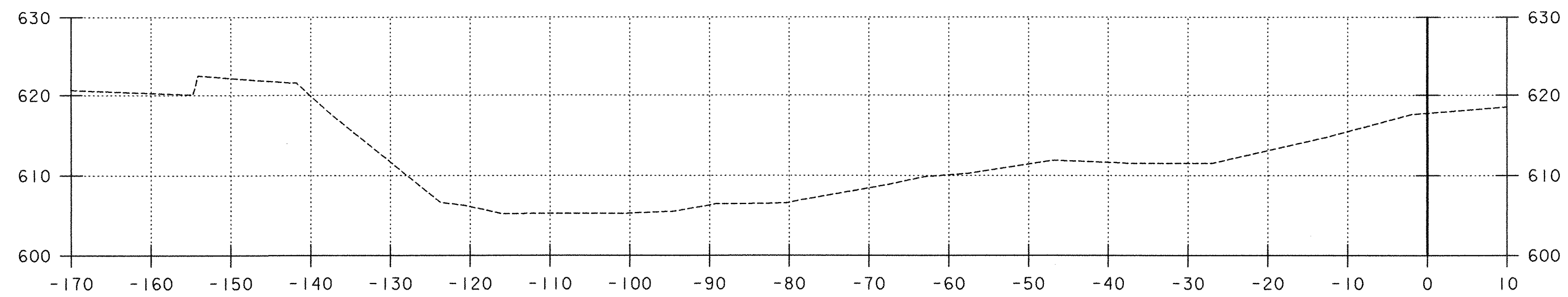


STA. 51+50 TO STA. 52+00

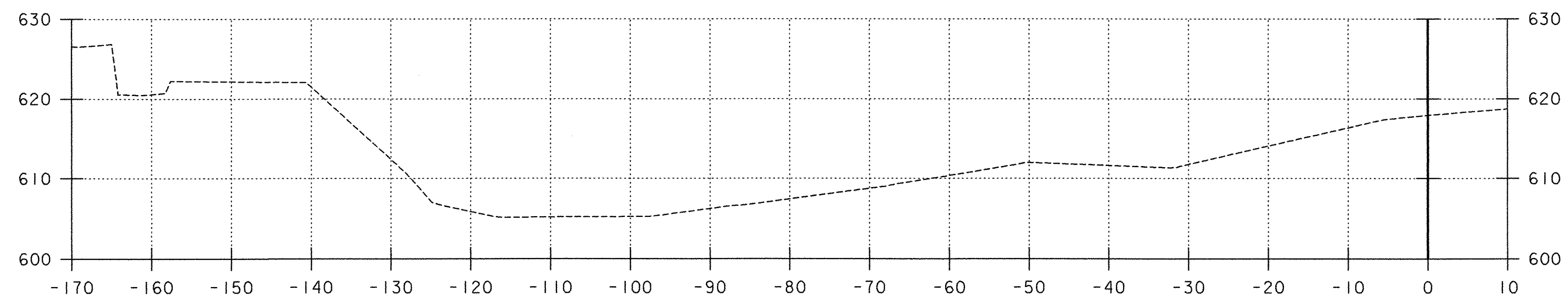


PROJECT NAME: HUNTINGTON	PLOT DATE: 1/12/2006
PROJECT NUMBER: BRO 1445 (29)	DRAWN BY: D.HARRINGTON
FILE NAME: ...Cadd\Trans\z01j302xsl.dgn	CHECKED BY: M.CHENETTE
PROJECT LEADER: M. CHENETTE	SHEET 62 OF 63
DESIGNED BY: T.KNIGHT	

CHANNEL CROSS SECTIONS

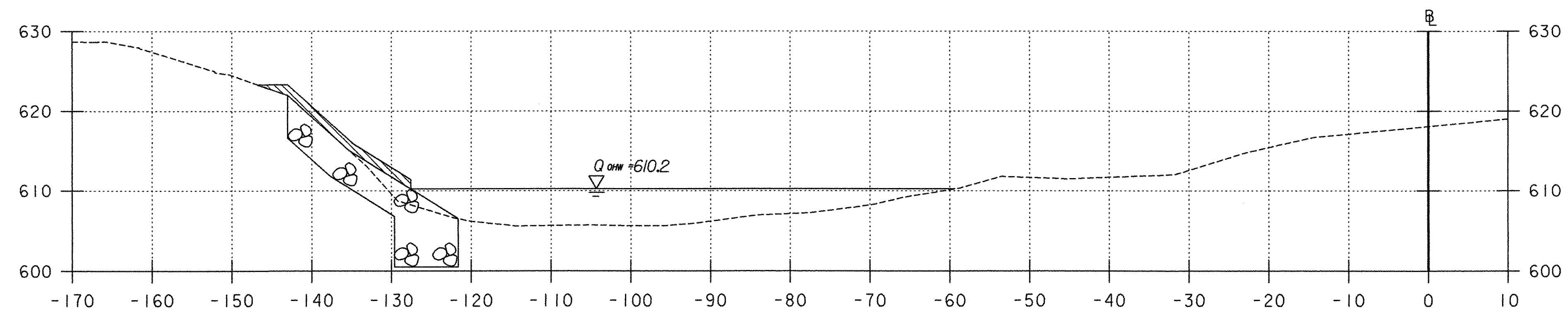


52+75

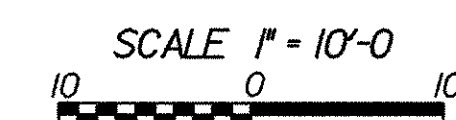


52+50

STA. 52+50 (ABUTMENT NO. 1)
 END UNCLASSIFIED CHANNEL EXCAVATION
 GEOTEXTILE UNDER STONE FILL
 STONE FILL, TYPE IV



52+25



STA. 52+25 TO STA. 52+75



PROJECT NAME: HUNTINGTON
 PROJECT NUMBER: BRO 1445 (29)

FILE NAME: ...Cadd\Trans\z01j302xsl.dgn
 PROJECT LEADER: M. CHENETTE
 DESIGNED BY: T. KNIGHT

PLOT DATE: 1/12/2006
 DRAWN BY: D. HARRINGTON
 CHECKED BY: M. CHENETTE
 SHEET 63 OF 63

CHANNEL CROSS SECTIONS



State of Vermont
PDD/Structures Design Section
National Life Building - Drawer 33
Montpelier, VT 05633-5001
www.aot.state.vt.us

[phone] 802-828-2621
[fax] 802-828-3566
[toll] 800-953-0191

Agency of Transportation

*Rec'd @ home
Dec 27th, 2006
DKN*

DATE: December 27, 2006

Cosmec, Inc.
70 South Street
Walpole, MA 02081

Project Name: **Huntington** Project #: **BRO 1445(29)**

Structure Identification: East Street Over Huntington River

The following bearing device details [Item # 531.10, Name: Bearing Device Assembly] for the above project (Vendor's Job # 60630) transmitted with a letter from Parent Construction, Inc. dated 12/11/06 have been reviewed and are being returned herewith.

Plan Sheets Number 5057, 5058, and 5059, Welding Procedures FCAW-FILLET, GTAW-WF, and Bonding Procedure EMS-QC-110 **are approved**. If any changes are made to any of these sheets, please submit white prints for our use in record plans for this project. Otherwise, our file copy will become the record set.

You must provide written notice to this office as to the date fabrication represented by these drawings will begin. That notice must be received at least seven days prior to that date, as per Specification 506.03. Any material fabricated prior to the notification date is subject to rejection without further cause.

Sincerely,

Warren Tripp

Attachments

cc: Resident Engineer Dale Norton, w/prints
 Shop Inspector Jeff Clark, w/prints
 Contractor Parent Construction, Inc., w/prints
 Construction Division - letter only
 Materials & Research Section (C&IA Unit) - letter only
 Files (Structures & Central)

064 BB



VERMONT
AGENCY OF TRANSPORTATION
MONTPELIER
PROJECT BRO 1445 (29)

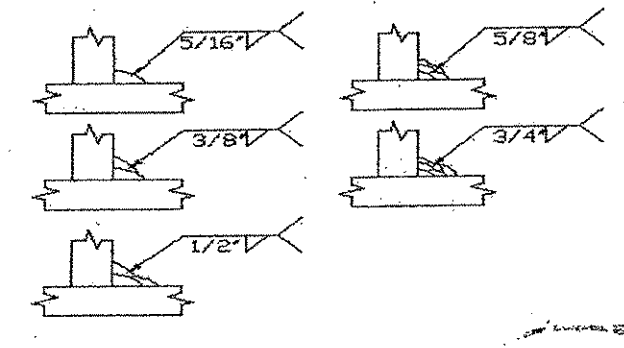
COSMEC INC.
WELDING PROCEDURE SPECIFICATION

SPECIFICATIONS AND CODE: AASHTO-AWS D1.5
MATERIAL SPECIFICATION ASTM A709 GR 50W, GR 50, GR 38
WELDING PROCESS FCAW-G
MANUAL OR MACHINE SEMI-AUTOMATIC
POSITION OF WELDING 1F & 2F
FILLER METAL SPECIFICATION AWS 5.20 CLASSIFICATION: E71T
MANUFACTURER: LINCOLN ELECTRIC TRADENAME: E71T OUTERSHIELD
FLUX INTERNAL FLOW RATE 45 CFH
SHIELDING GAS CO2
SINGLE OR MULTIPLE PASSES MULTIPLE
SINGLE OR MULTIPLE ARC SINGLE
WELDING CURRENT DC
POLARITY: REVERSE (EP)
WELDING PROGRESSION
ROOT TREATMENT CLEANED & PREPARED BRIGHT METAL
PREHEAT AND INTERPASS TEMPERATURE SEE BELOW
POSTHEAT TEMPERATURE N/A
HEAT INPUT MIN. MAX.

WELDING PROCEDURE

PASS NO.	ELECTRODE SIZE	WELDING CURRENT AMPERES	WELDING VOLTAGE VOLTS	TRAVEL SPEED	JOINT DETAIL
ALL	0.045	190-230	27-29	8-9	

PREHEAT TEMPS.
THICKNESS T TEMP.
UP TO 3/4" 50 DEG. F
OVER 3/4" TO 1 1/2" 70 DEG. F
OVER 1 1/2" TO 2 1/2" 150 DEG. F
OVER 2 1/2" 225 DEG. F
INTERPASS TEMP: 400 DEG. F



THIS PROCEDURE MAY VARY DUE TO FABRICATION SEQUENCE, FIT-UP, PASS SIZE, ETC. WITHIN THE LIMITATION OF VARIABLES GIVEN IN SECTION 5.

PROCEDURE NO. FCAW-FILLET
SUPPORTING PQR: FCAW-1.0-05
REVISION NO.

CONTRACTOR: COSMEC INC.
AUTHORIZED BY: DONALD VOSE
DATE: 1/31/2005



TRANS RECEIVED
OK'D BY: JWC
DEC 14 2006
RESUBMIT APPROVED
BY: DATE: 12-18-06

065 BB

VERMONT
AGENCY OF TRANSPORTATION
JUNTINGTON
PROJECT BRO 1445(29)

COSMEC INC.

WELDING PROCEDURE SPECIFICATION

SPECIFICATIONS AND CODE: D1.9
MATERIAL SPECIFICATION ASTM A240 TYPE 304 TO ASTM A709 GR 50W
WELDING PROCESS GTAW
MANUAL OR MACHINE MANUAL
POSITION OF WELDING 1F & 2F
FILLER METAL SPECIFICATION ER309L CLASSIFICATION: A5.9
MANUFACTURER: HARRIS WELCO TRADENAME:
FLUX INTERNAL FLOW RATE 45 CFH
SHIELDING GAS ARGON
SINGLE OR MULTIPLE PASSES SINGLE
SINGLE OR MULTIPLE ARC SINGLE
WELDING CURRENT DC
POLARITY: REVERSE (EN)
WELDING PROGRESSION
ROOT TREATMENT CLEANED & PREPARED BRIGHT METAL
PREHEAT AND INTERPASS TEMPERATURE SEE BELOW
POSTHEAT TEMPERATURE N/A
HEAT INPUT MIN. MAX.

WELDING PROCEDURE

PASS NO.	ELECTRODE SIZE	WELDING CURRENT AMPERES	TRAVEL SPEED	JOINT DETAIL
ALL	3/32"	130-155 14-17	6.75-8.5	LAP JOINT

18 GA THRU 10 GA
BM THICKNESS

PREHEAT TEMPS. ***PREHEAT UNTIL NO MOSITURE PRESENT

THICKNESS	TEMP.
UP TO 3/4"	100 DEG.
OVER 3/4" TO 1 1/2"	100 DEG.
OVER 1 1/2" TO 2 1/2"	100 DEG.
OVER 2 1/2"	100 DEG.

THIS PROCEDURE MAY VARY DUE TO FABRICATION SEQUENCE, FIT-UP, PASS SIZE, ETC.
WITHIN THE LIMITATION OF VARIABLES GIVEN IN SECTION 5.
SUPPORTING PQR: GTAW-WF-05
PROCEDURE NO. GTAW-WF

CONTRACTOR: COSMEC INC.
AUTHORIZED BY: DONALD VOSE
CW# 01100651 *Donald Vose*

REVISION NO. 1

DATE: 2/1/2005



TRANS
RECEIVED
OK'D BY *JWC*
DEC 14 2006
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BY DATE 11-18-06

066 BB

VERMONT
AGENCY OF TRANSPORTATION
MONTPELIER
PROJECT BRO. 1445 (29)

EMS-QC-110

COSMEC, INC.
70 SOUTH STREET
WALPOLE, MA 02081
PH# 508-668-6600
FAX# 508-660-1022

RECEIVED
VTRANS
OK'D BY _____ OK'D BY JUC
DEC 14 2006
RESUBMIT _____ APPROVED _____
DATE 12-18-06

ENGINEERING AND MANUFACTURING STANDARD
PTFE FACING AND STEEL
OR PREFORMED FABRIC SUBSTRATE
SURFACE PREPARATION AND ADHESIVE PROCEDURE

The PTFE facing shall be prepared for bonding to a substrate material by chemically etching the face to be bonded using the sodium ammonia process.

The mating surface of the substrate shall be prepared for bonding using a three-step process as follows:

1. preliminary degrease using methyl ethyl ketone.
2. mechanically roughen to approx. 125 RMS and thoroughly brush and clean for final degreasing.
3. final degrease using methyl ethyl ketone

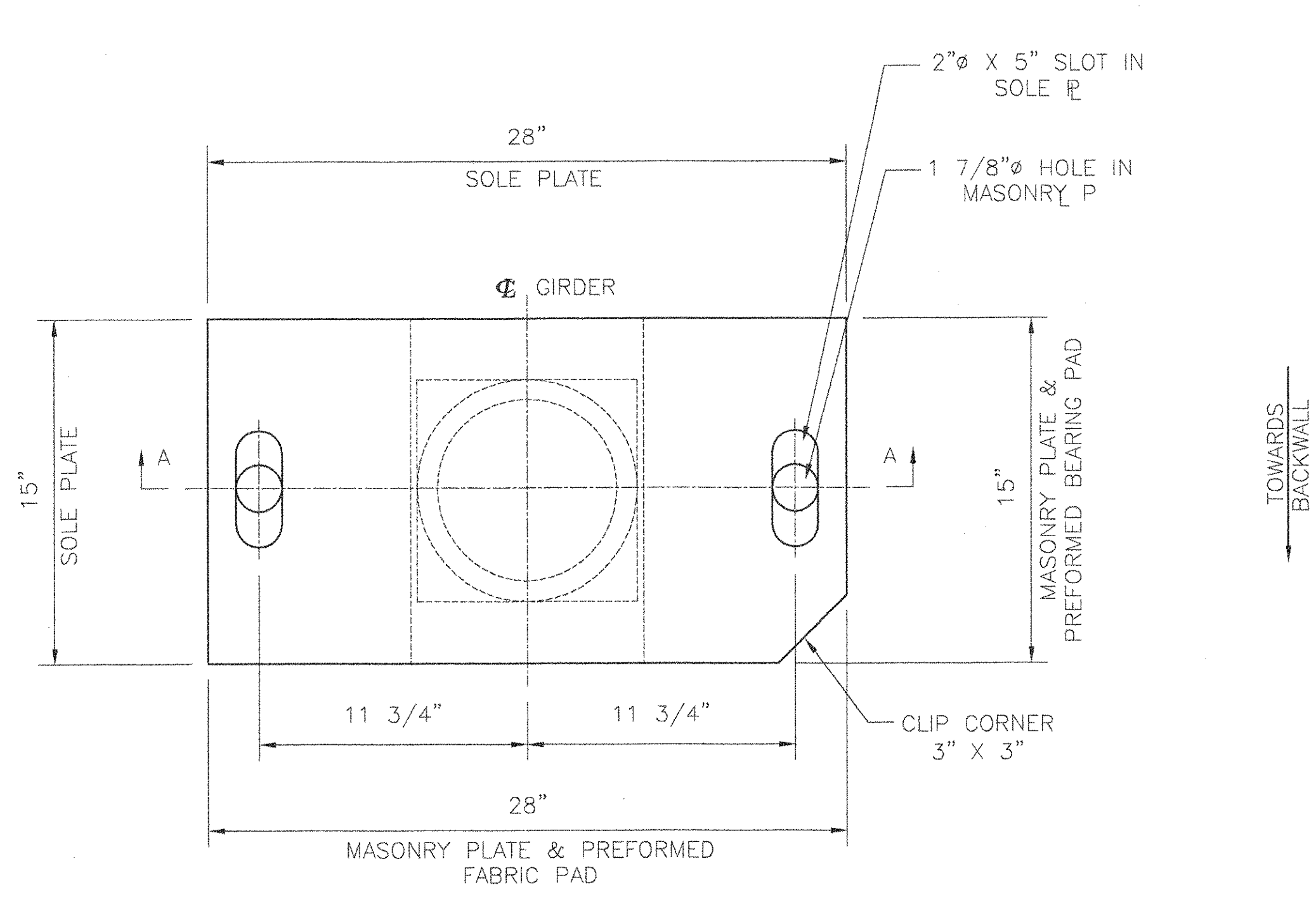
The PTFE and substrate mating surfaces shall be clean and dry with final degreasing performed within 30 minutes of bonding. Adhesive shall consist of a two-part epoxy adhesive system conforming to Military Specification MMM-A-134

The adhesive shall be applied to the full area of the contact surface in an even manner so as to establish a glue line not less than .002 inch nor more than .010 inch thick. Surfaces being bonded are to be assembled immediately with open assembly time not to exceed 20 minutes.

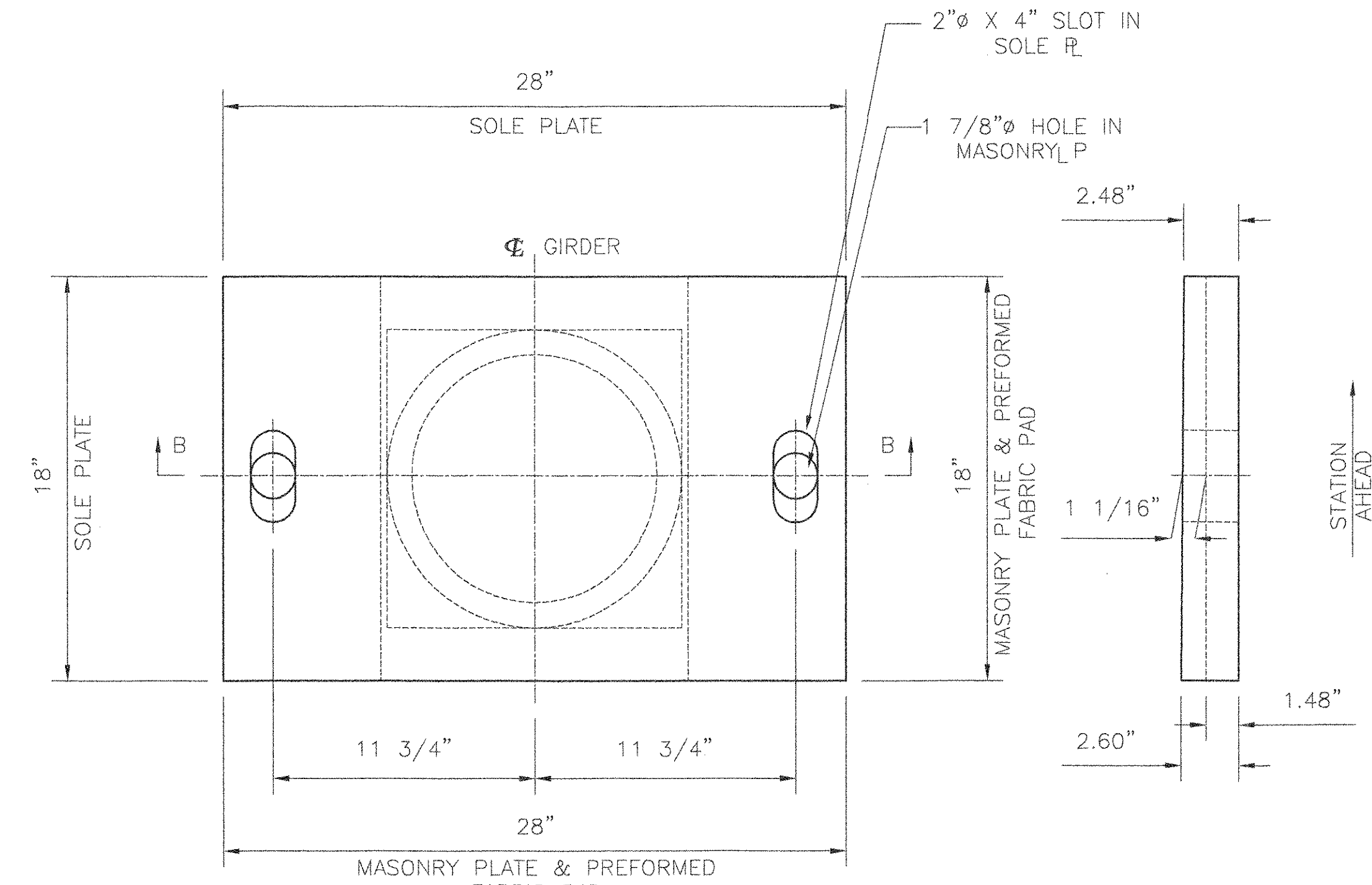
The PTFE material shall be greater in width and length than the substrate material by at least 1/4 inch when bonded. The PTFE shall be applied with contact starting at one edge and with contact progressing across entire bonded surface to eliminate air entrapment. The PTFE is to be in full contact with the steel or preformed fabric substrate. Curing of the bond shall be done under pressure of approx. 2-100 psi for 10-12 hours at approx. 70°F or other schedules as established by the manufacturer of the adhesive.

The PTFE shall be carefully trimmed to the same size as the substrate after bond curing and all bonds shall be visually inspected for bond retention.

067 EB

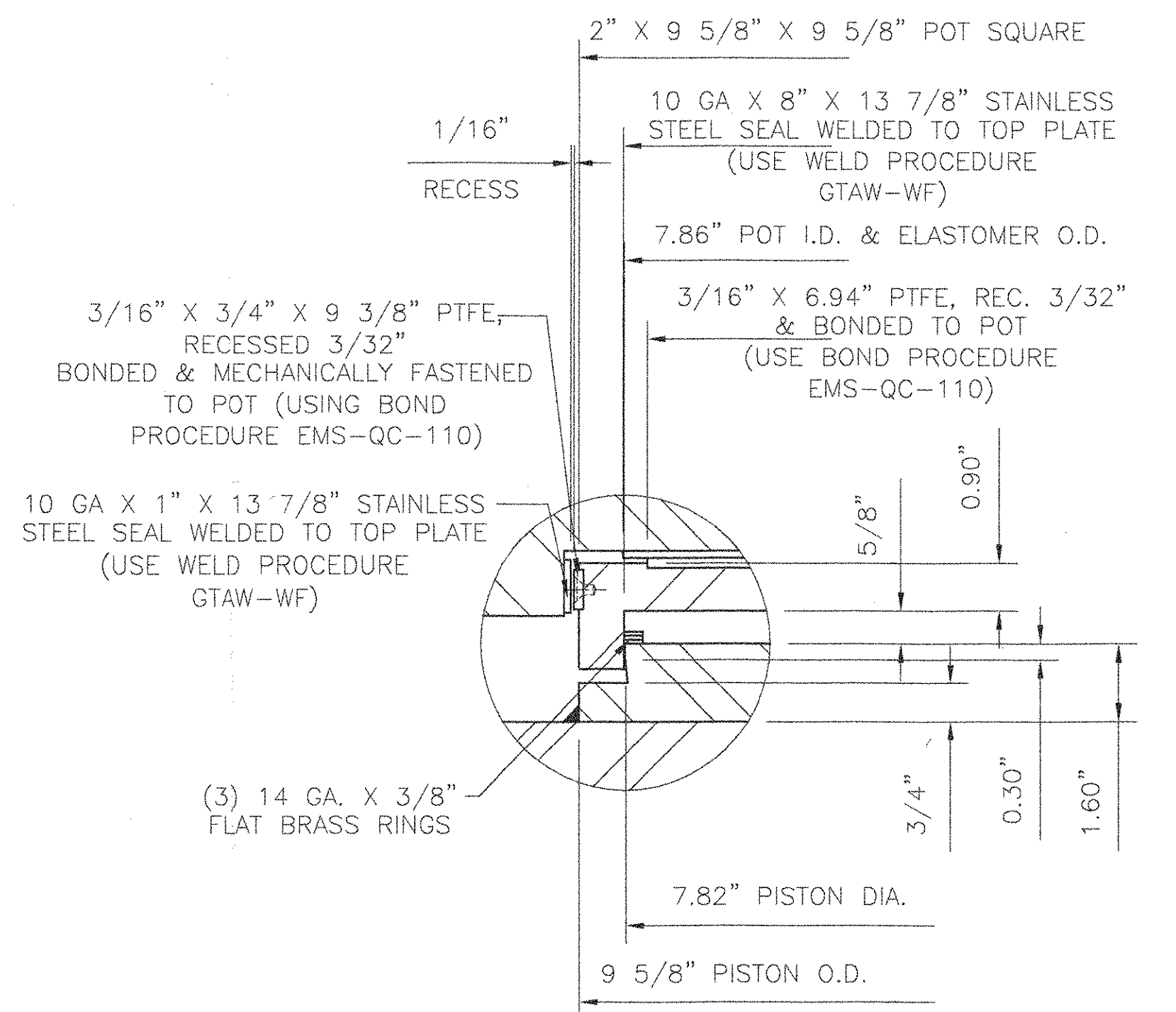


PLAN

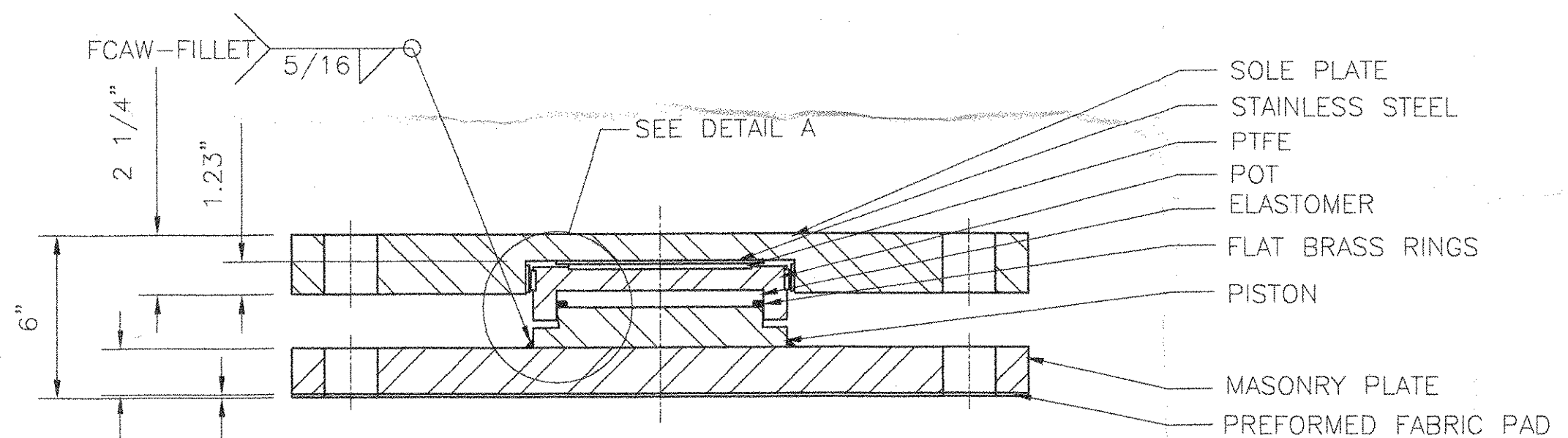


PLAN

SIDE ELEVATION



DETAIL A
N.T.S.



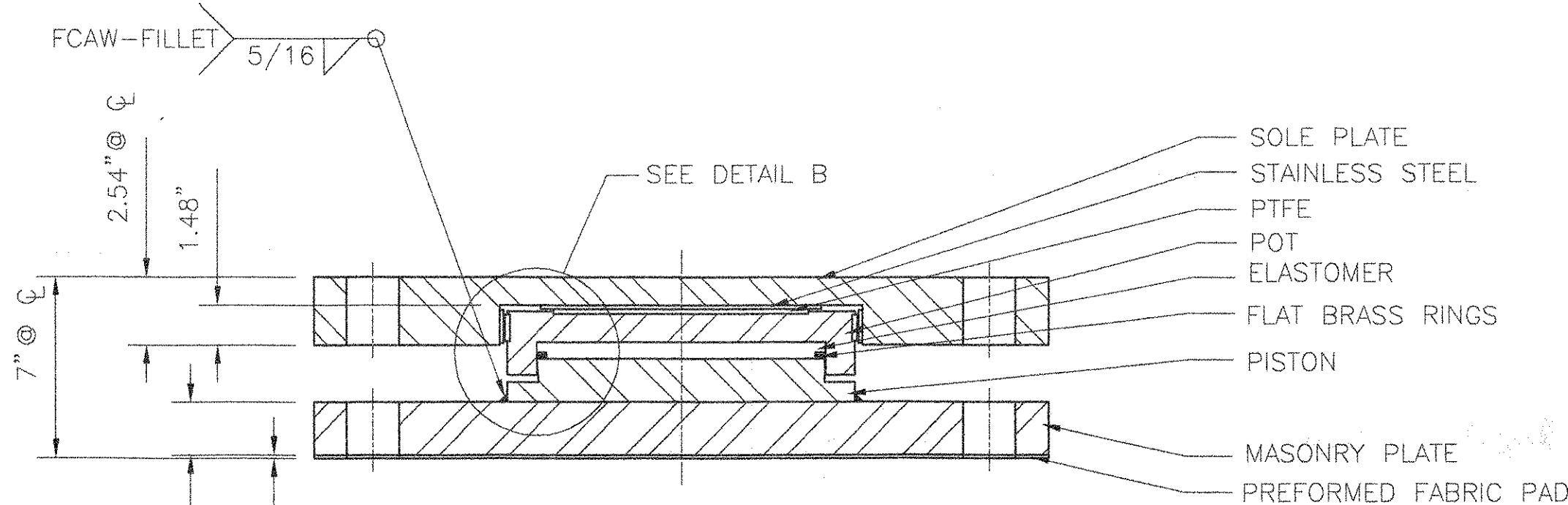
SECTION A - A

COSMEC GUIDED EXPANSION POT BEARING

QUANTITY: 2
 LOCATION: ABUTMENT 1, GIRDERS 2 & 3
 VERTICAL LOAD CAPACITY: 170 KIPS
 TRANSVERSE LOAD CAPACITY: 34 KIPS
 LONGITUDINAL LOAD CAPACITY: N/A
 MOVEMENT CAPACITY: 2 1/2"
 ROTATION CAPACITY: .020 RADIAN

BEARING NOTES

1. ALL MATERIALS USED IN THE FABRICATION OF THESE BEARINGS SHALL BE MADE IN THE U.S.A.
2. BEARINGS ARE TO BE SHIPPED AS COMPLETE UNITS, STEEL BANDED, AND SHALL BE WRAPPED TO PROTECT FROM MOISTURE AND DIRT DURING TRANSIT AND STORAGE.
3. LOCATION OF FABRICATION PLANT - 70 SOUTH STREET WALPOLE, MA 02081
4. COSMEC, INC. REPRESENTATIVE - MR. MATT McANDREWS (508) 668-6600
5. BEARINGS SHALL BE STORED IN A CLEAN, DRY, LEVEL UPRIGHT POSITION.
6. BEARINGS MUST NOT BE DISASSEMBLED
7. PROTECT PTFE & S.S. SURFACES FROM DIRT, WELD SPATTER, & ANY OTHER FOREIGN SUBSTANCES.



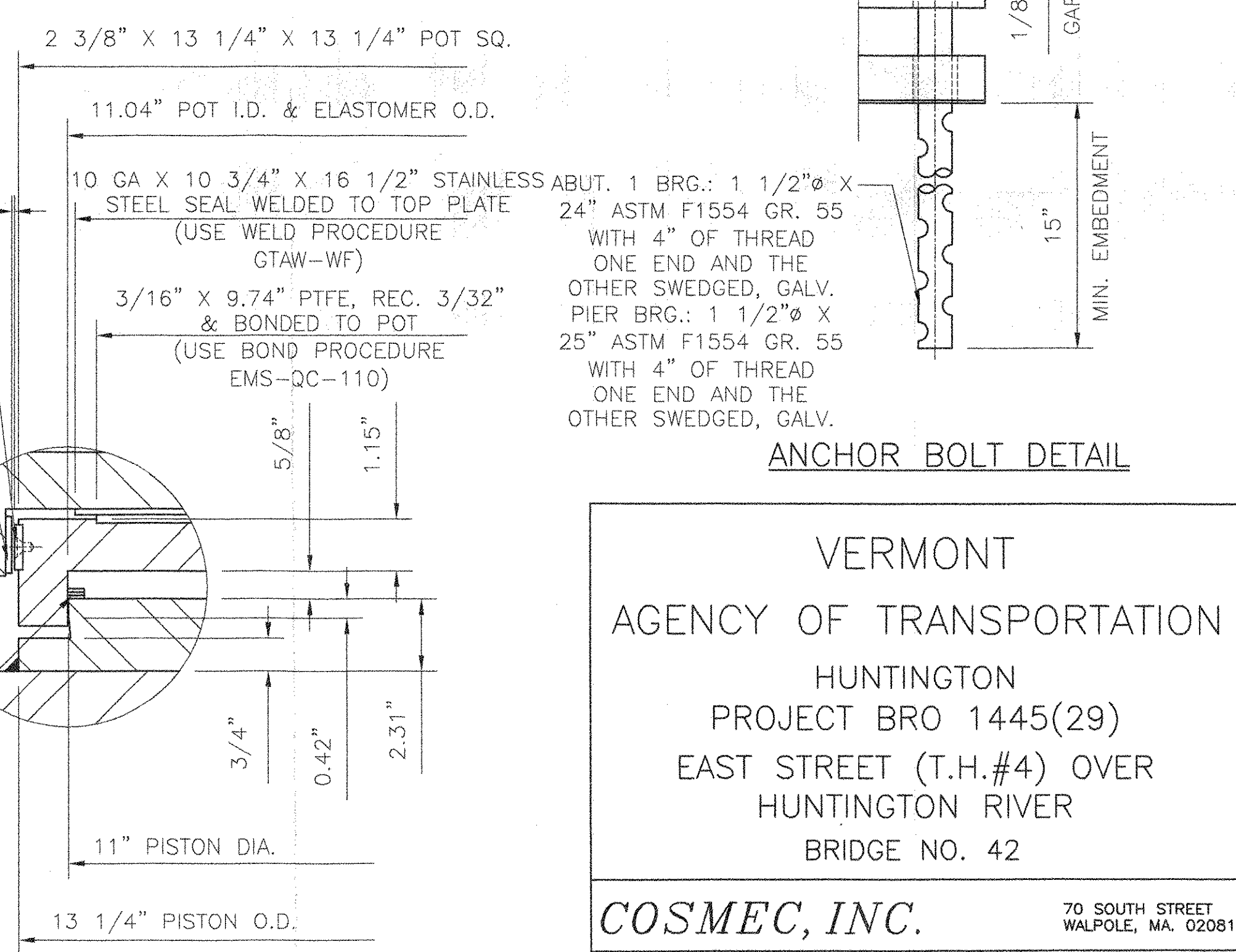
SECTION B - B

COSMEC GUIDED EXPANSION POT BEARING

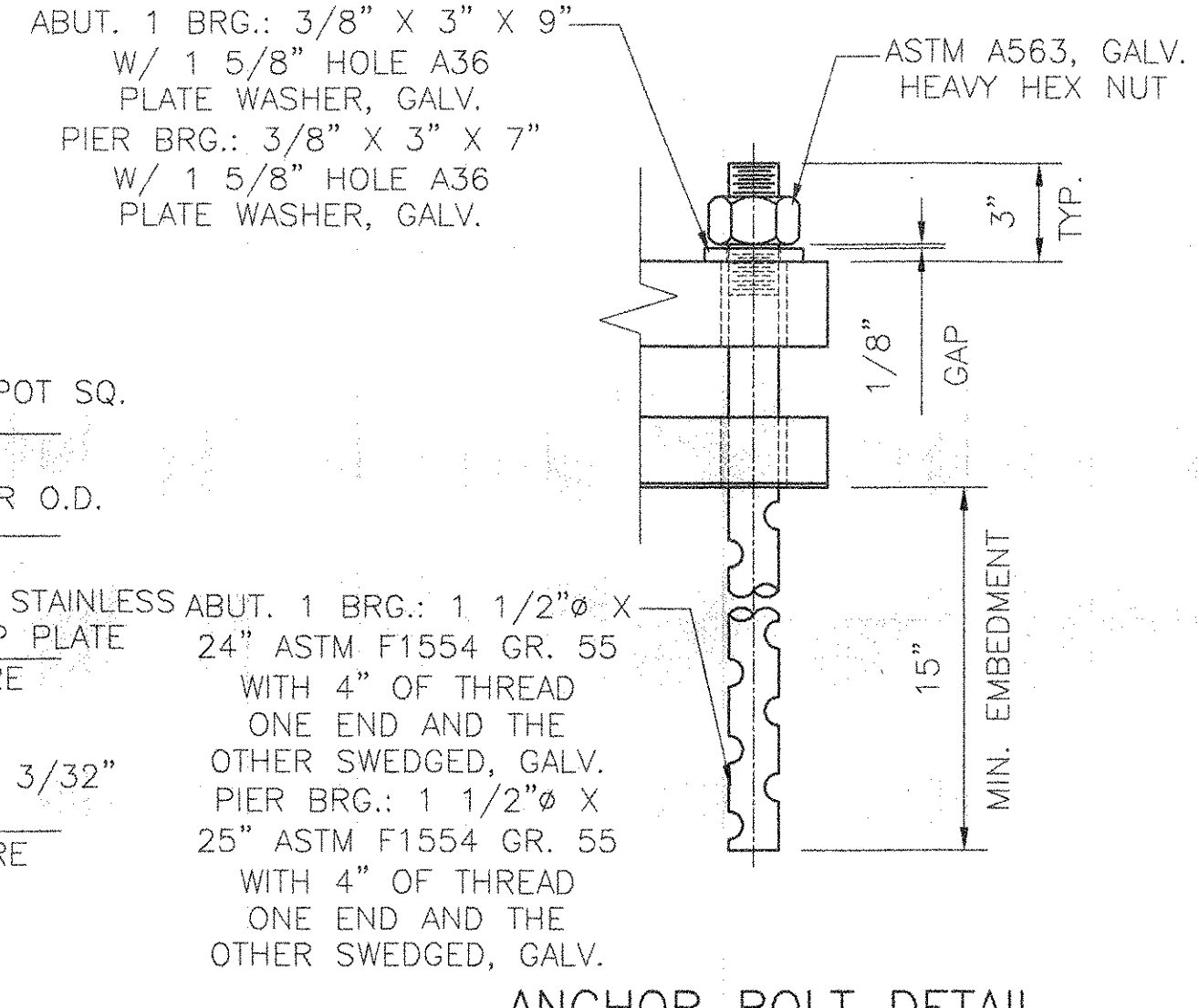
QUANTITY: 2
 LOCATION: PIER, GIRDERS 2 & 3
 VERTICAL LOAD CAPACITY: 335 KIPS
 TRANSVERSE LOAD CAPACITY: 67 KIPS
 LONGITUDINAL LOAD CAPACITY: N/A
 MOVEMENT CAPACITY: 1 1/2"
 ROTATION CAPACITY: .015 RADIAN

MATERIALS

- STEEL - ASTM A709 GRADE 36, ZINC METALLIZED
- STAINLESS STEEL - ASTM A240 TYPE 304, WITH 10 RMS FINISH OR LESS
- PREFORMED FABRIC PAD - AASHTO DIV. II SPEC. 18.4.9.1
- ANCHOR BOLTS - SEE ANCHOR BOLT DETAIL
- PTFE - ASTM D.4894 VIRGIN UNFILLED
- ELASTOMER - AASHTO SHORE A DUROMETER 50 ± 5 DUROMETER
- BRASS SEALING RINGS - ASTM B.36 HALF HARD



DETAIL B
N.T.S.



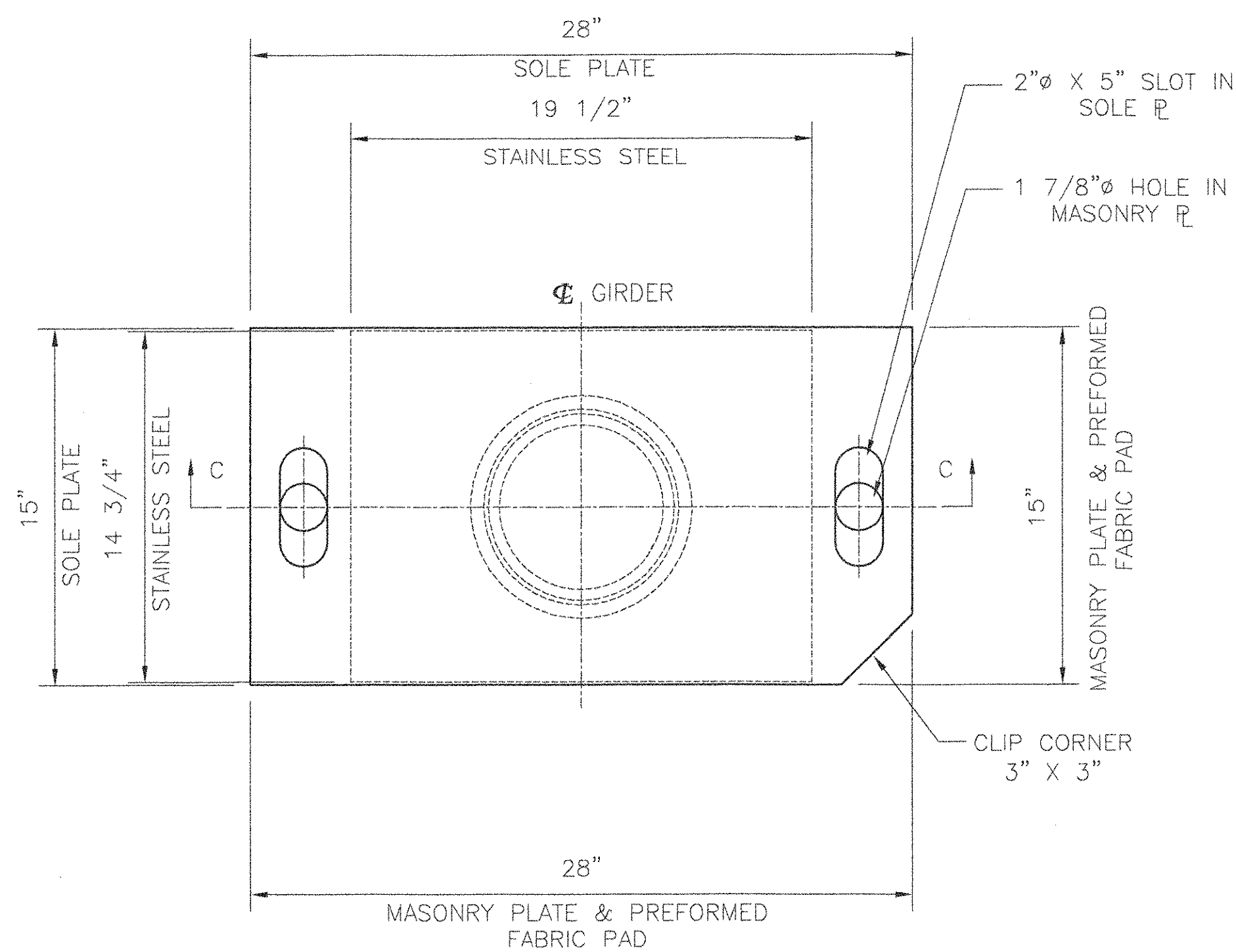
ANCHOR BOLT DETAIL

VERMONT
 AGENCY OF TRANSPORTATION
 HUNTINGTON
 PROJECT BRO 1445(29)
 EAST STREET (T.H.#4) OVER
 HUNTINGTON RIVER
 BRIDGE NO. 42

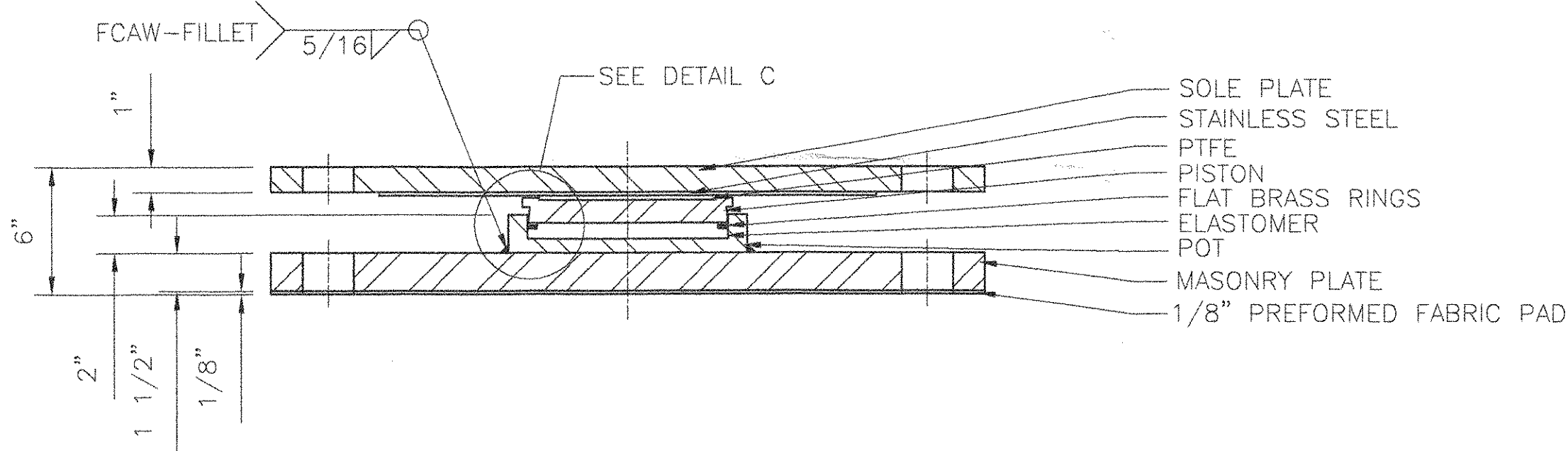
COSMEC, INC.		70 SOUTH STREET WALPOLE, MA. 02081	
SCALE: 3/16"=1"	DRAWN BY: MRR	CHECKED BY: MCM	
DATE: 11/16/06	DATE: 12/8/06	DATE: 12/8/06	
COSMEC POT BEARINGS			
CUSTOMER PARENT CONSTRUCTION INC.	S.O. NUMBER 60630	DRAWING NUMBER 5057	REV.

RECEIVED
 CK'D BY: *[Signature]*
 DEC 14 2006
 RESUBMIT: _____ APPROVED: *[Signature]*
 BY: *[Signature]* DATE: 12/27/06

REV.	BY	DATE	CK'D	DATE



PLAN



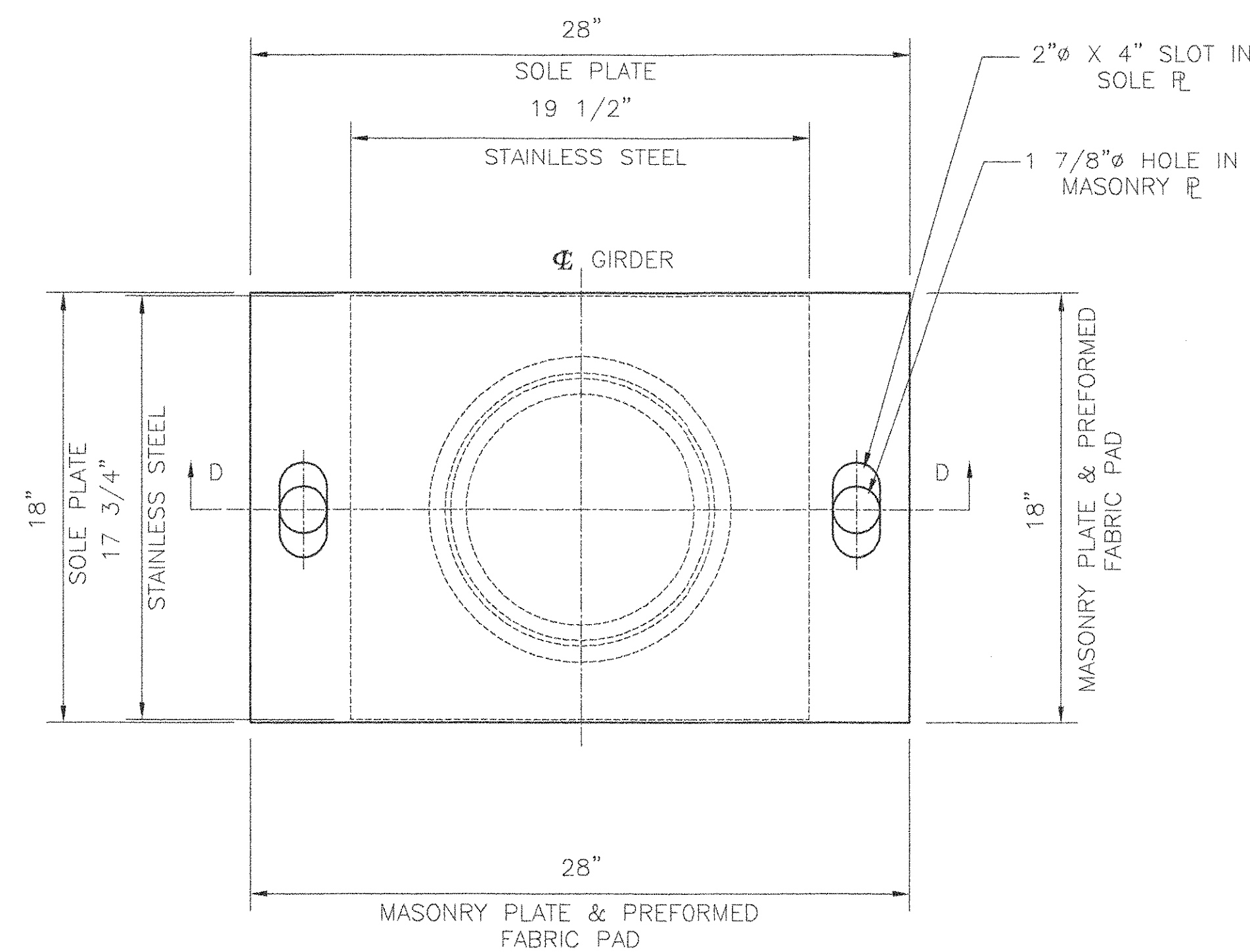
SECTION C - C

COSMEC NON-GUIDED EXPANSION POT BEARING

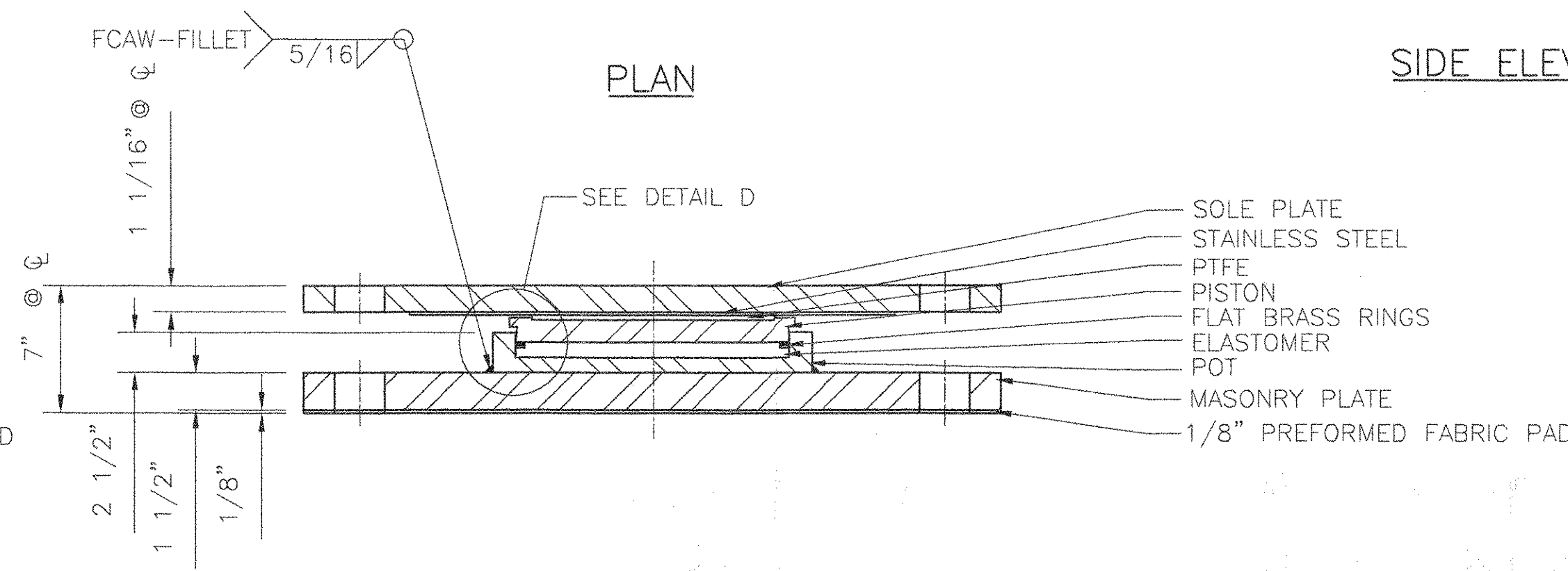
QUANTITY: 2
 LOCATION: ABUTMENT 1, GIRDERS 1 & 4
 VERTICAL LOAD CAPACITY: 170 KIPS
 TRANSVERSE LOAD CAPACITY: N/A
 LONGITUDINAL LOAD CAPACITY: N/A
 MOVEMENT CAPACITY: 2 1/2"
 ROTATION CAPACITY: .020 RADANS

BEARING NOTES

1. ALL MATERIALS USED IN THE FABRICATION OF THESE BEARINGS SHALL BE MADE IN THE U.S.A.
2. BEARINGS ARE TO BE SHIPPED AS COMPLETE UNITS, STEEL BANDED, AND SHALL BE WRAPPED TO PROTECT FROM MOISTURE AND DIRT DURING TRANSIT AND STORAGE.
3. LOCATION OF FABRICATION PLANT - 70 SOUTH STREET WALPOLE, MA 02081
4. COSMEC, INC. REPRESENTATIVE - MR. MATT McANDREWS (508) 668-6600
5. BEARINGS SHALL BE STORED IN A CLEAN, DRY, LEVEL UPRIGHT POSITION.
6. BEARINGS MUST NOT BE DISASSEMBLED
7. PROTECT PTFE & S.S. SURFACES FROM DIRT, WELD SPATTER, & ANY OTHER FOREIGN SUBSTANCES.



PLAN



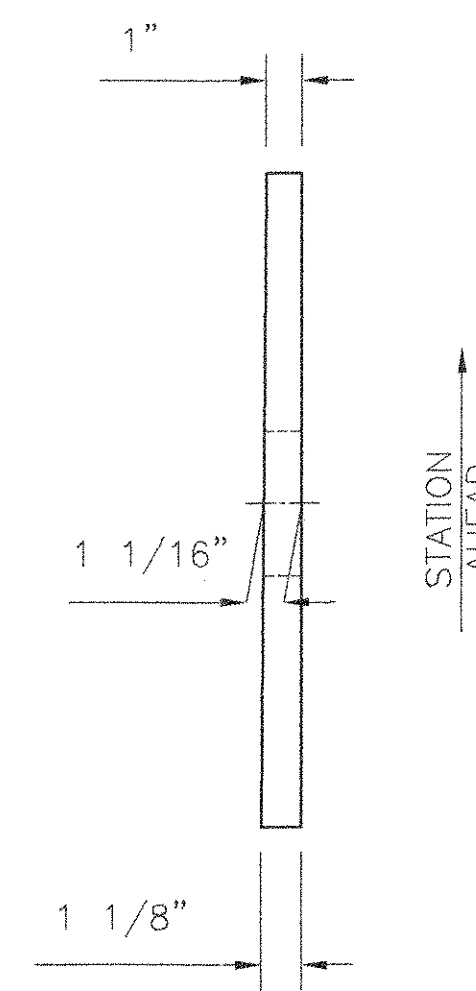
SECTION D - D

COSMEC NON-GUIDED EXPANSION POT BEARING

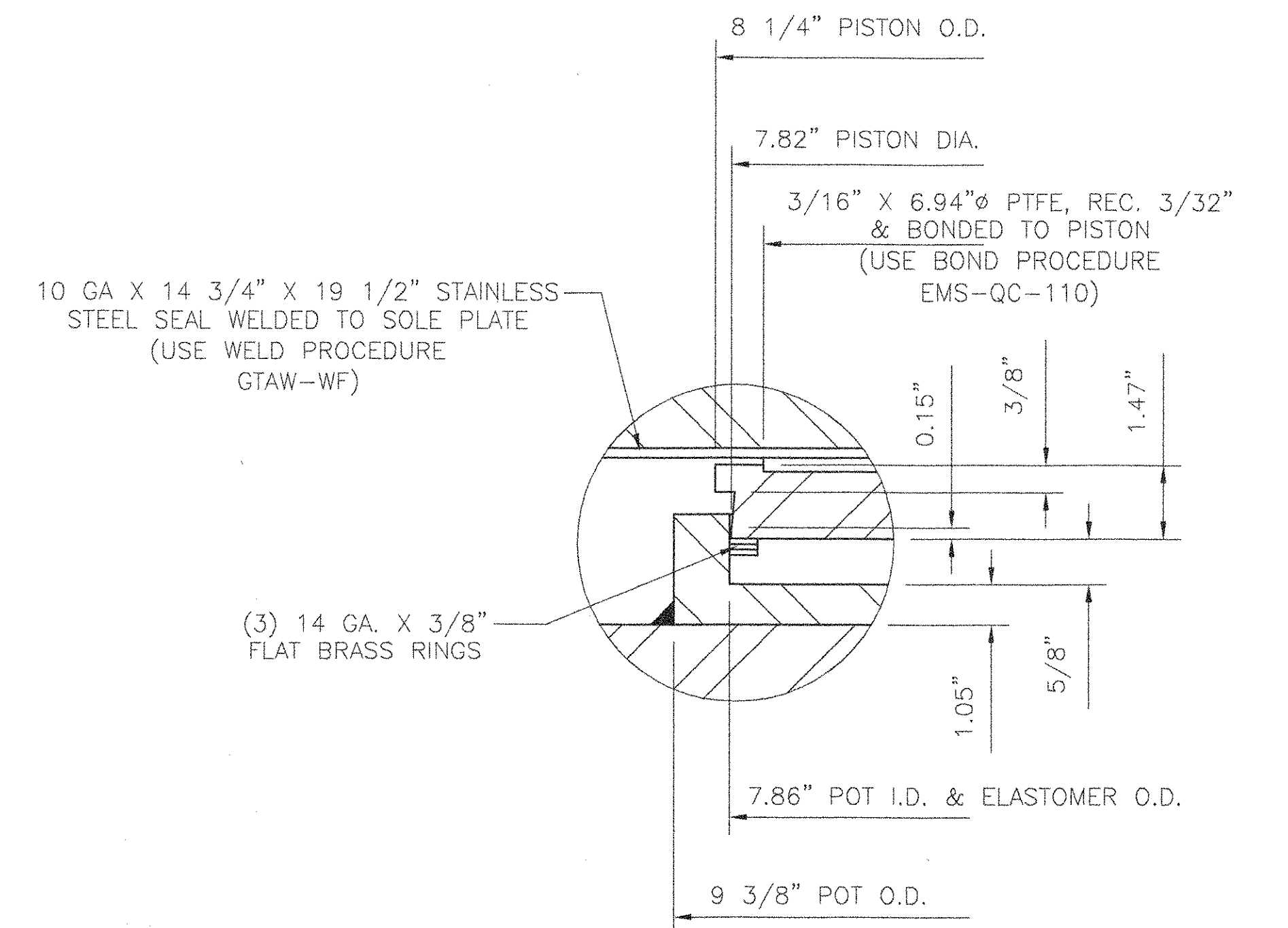
QUANTITY: 2
 LOCATION: PIER, GIRDERS 1 & 4
 VERTICAL LOAD CAPACITY: 335 KIPS
 TRANSVERSE LOAD CAPACITY: N/A
 LONGITUDINAL LOAD CAPACITY: N/A
 MOVEMENT CAPACITY: 1 1/2"
 ROTATION CAPACITY: .015 RADANS

MATERIALS

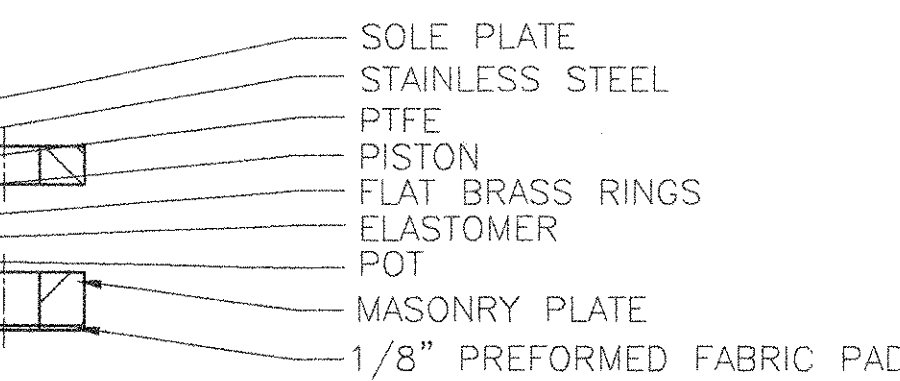
- STEEL - ASTM A709 GRADE 36, ZINC METALLIZED
- STAINLESS STEEL - ASTM A240 TYPE 304, WITH 20 RMS FINISH OR LESS
- PREFORMED FABRIC PAD - AASHTO DIV. II SPEC. 18.4.9.1
- ANCHOR BOLTS - SEE ANCHOR BOLT DETAIL
- PTFE - ASTM D.4894 VIRGIN UNFILLED
- ELASTOMER - AASHTO SHORE A DUROMETER 50 ± 5 DUROMETER
- BRASS SEALING RINGS - ASTM B.36 HALF HARD



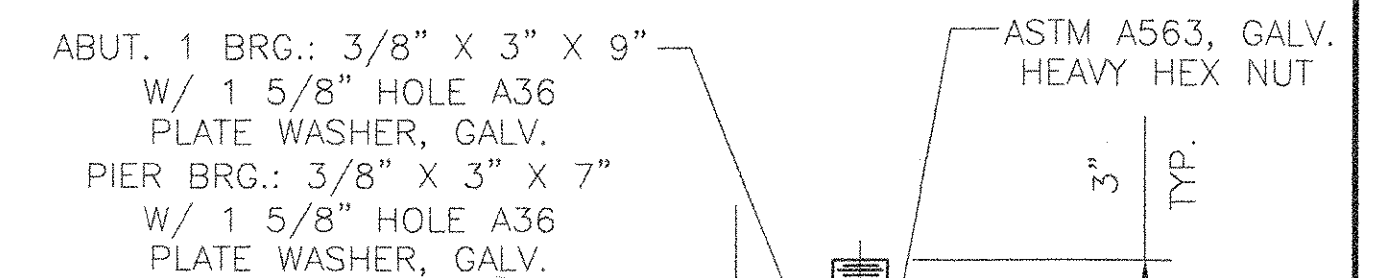
SIDE ELEVATION



DETAIL C
N.T.S.



DETAIL D
N.T.S.



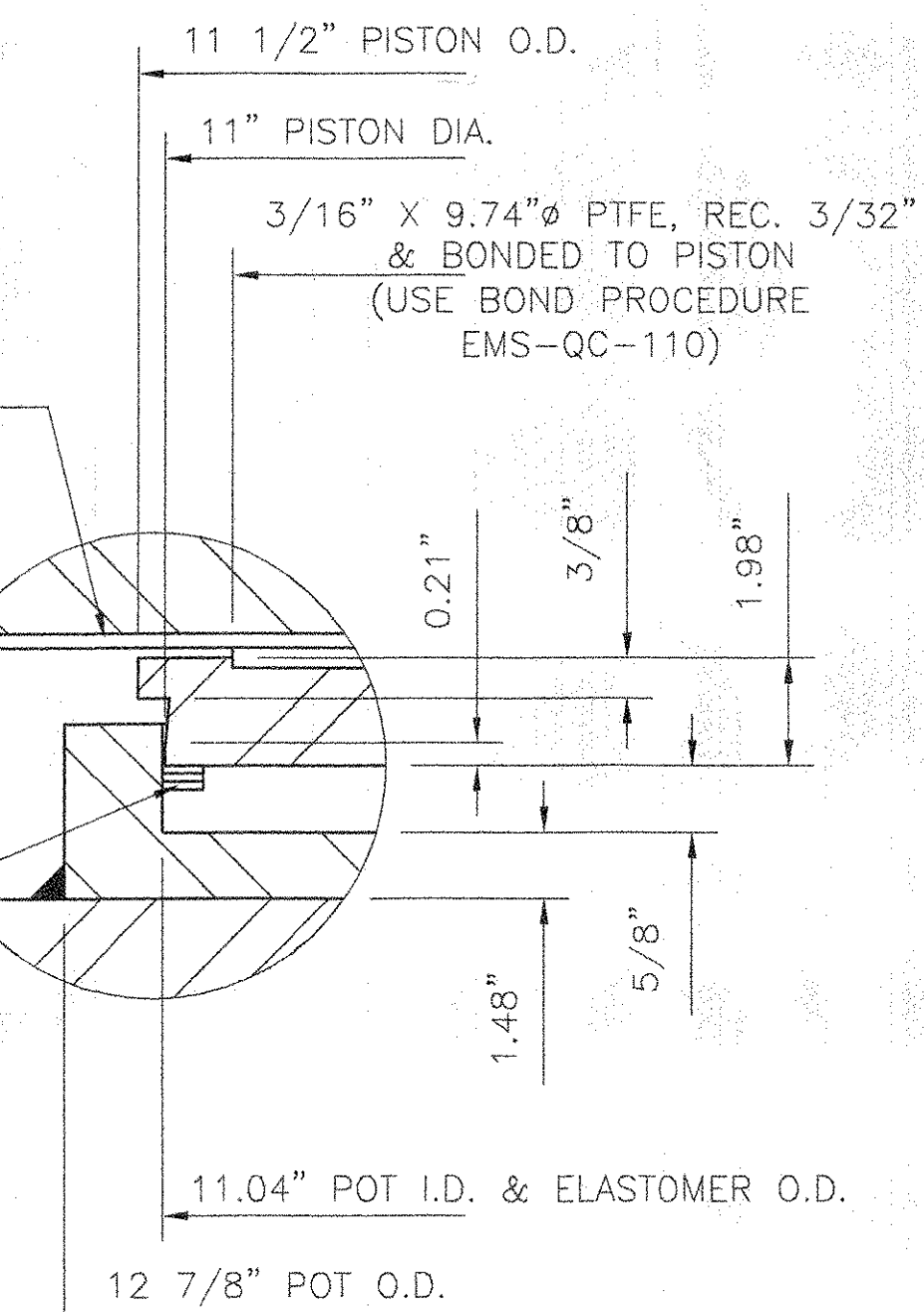
ANCHOR BOLT DETAIL

ABUT. 1 BRG.: 3/8" X 3" X 9"
 W/ 1 5/8" HOLE A36
 PLATE WASHER, GALV.
 PIER BRG.: 3/8" X 3" X 7"
 W/ 1 5/8" HOLE A36
 PLATE WASHER, GALV.

VERMONT
 AGENCY OF TRANSPORTATION
 HUNTINGTON
 PROJECT BRO 1445(29)
 EAST STREET (T.H.#4) OVER
 HUNTINGTON RIVER
 BRIDGE NO. 42

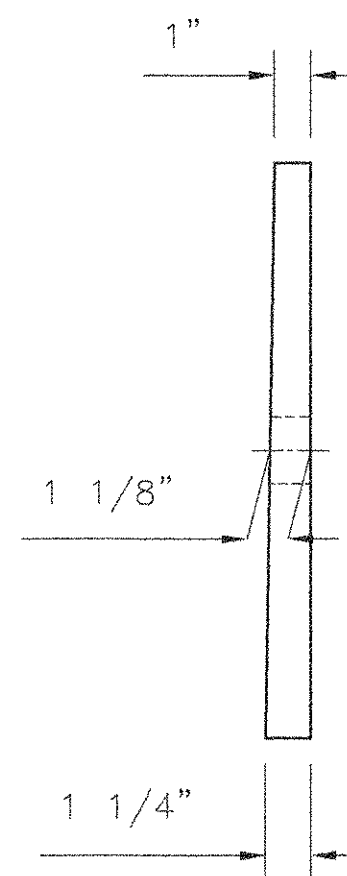
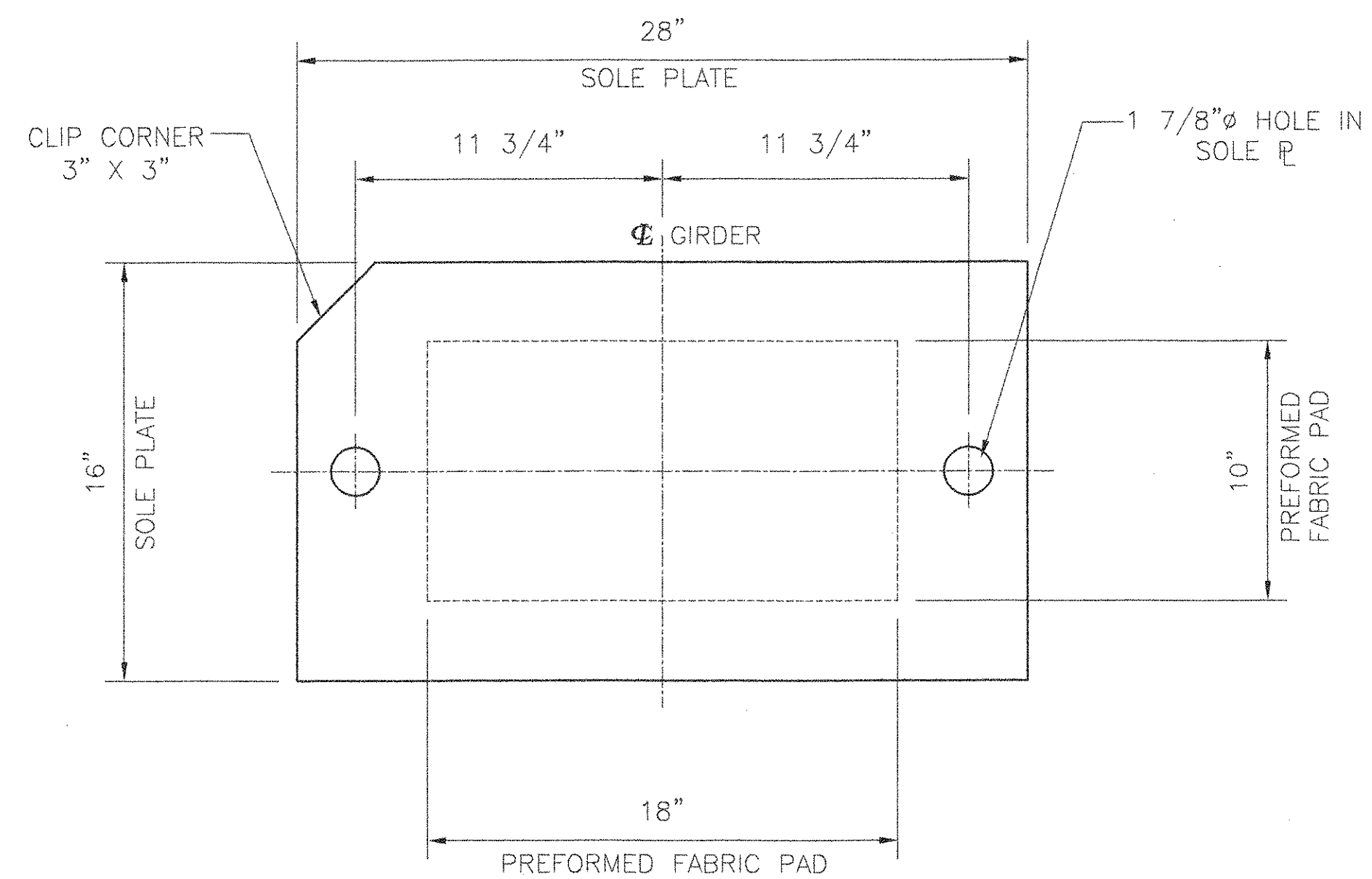
COSMEC, INC.		70 SOUTH STREET WALPOLE, MA 02081	
SCALE: 3/16"=1"	DRAWN BY: MRR	CHECKED BY: MCM	
DATE: 11/16/06	DATE: 11/16/06	DATE: 12/8/06	
COSMEC POT BEARINGS			
CUSTOMER PARENT CONSTRUCTION INC.	S.O. NUMBER 60630	DRAWING NUMBER 5058	REV.

RECEIVED
 CK'D BY: [Signature]
 DEC 14 2006
 RESUBMIT APPROVED: [Signature]
 BY: [Signature] DATE: 12/27/06

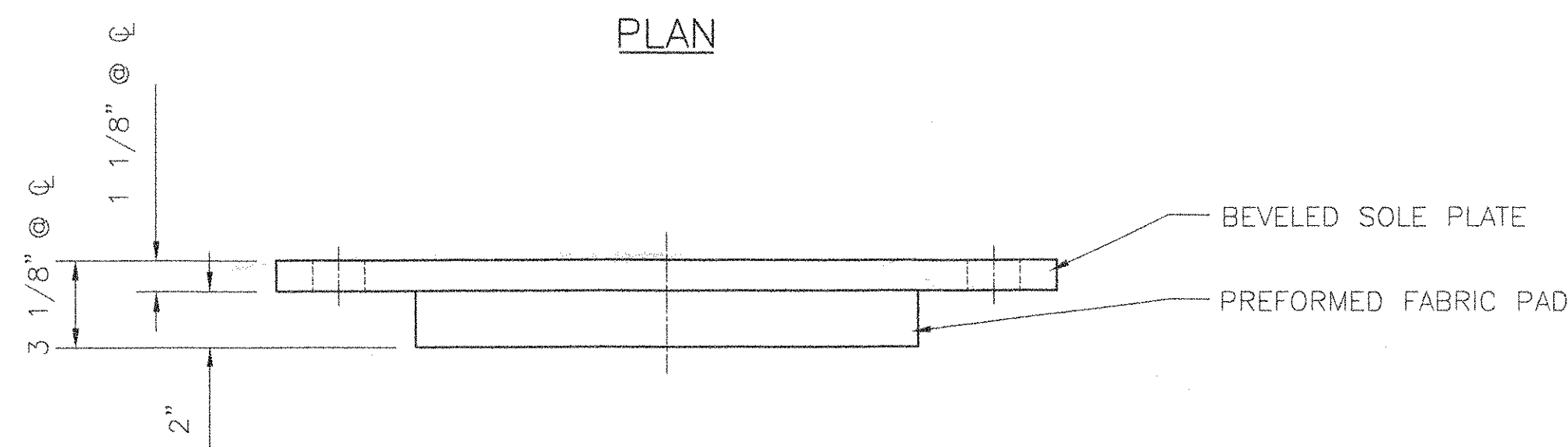


DETAIL D
N.T.S.

REV.	BY	DATE	CK'D	DATE



SIDE ELEVATION



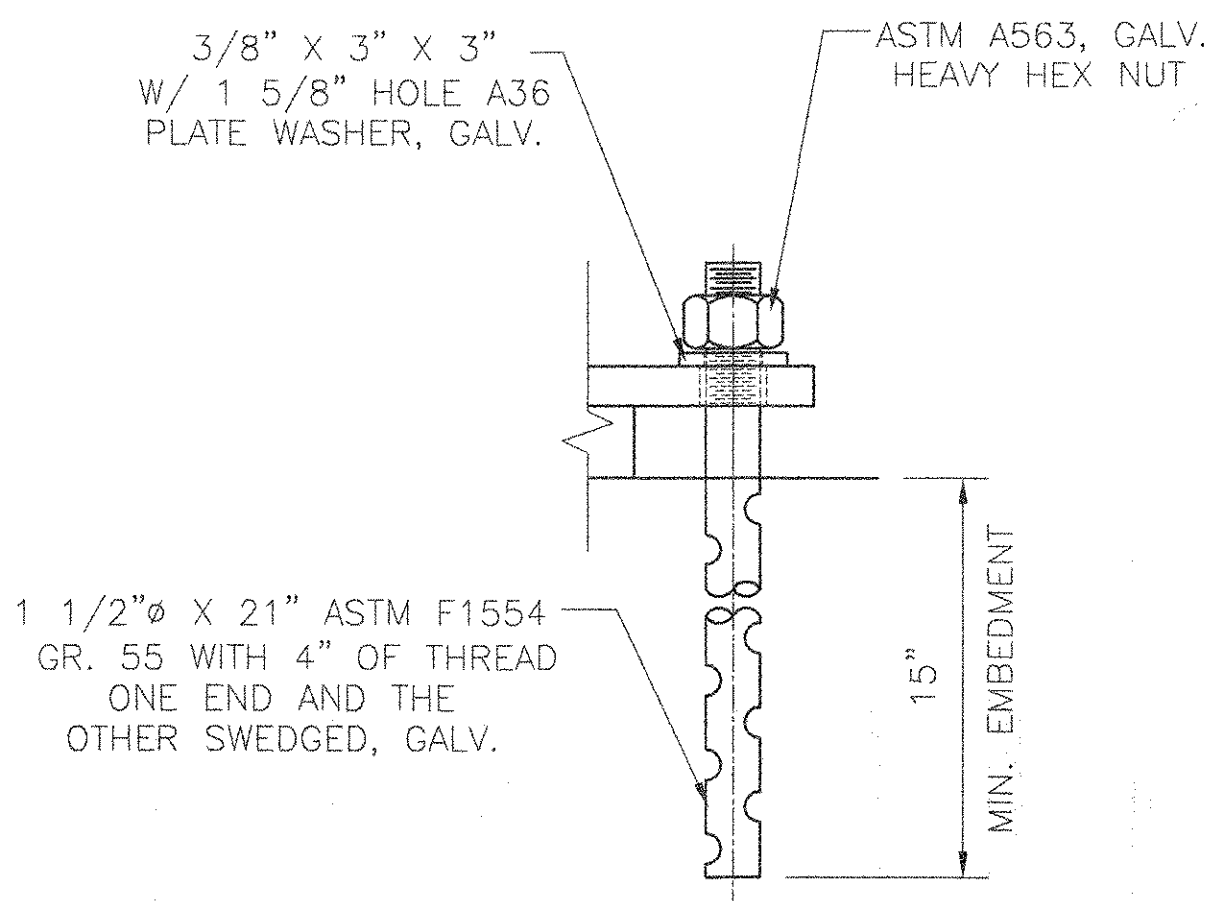
FRONT ELEVATION

COSMEC FIXED FABRIC PAD BEARING

QUANTITY: 4
 LOCATION: ABUTMENT 2, GIRDERS 1-4
 VERTICAL LOAD CAPACITY: 150 KIPS
 TRANSVERSE LOAD CAPACITY: 15 KIPS
 LONGITUDINAL LOAD CAPACITY: 15 KIPS
 MOVEMENT CAPACITY: 0"
 ROTATION CAPACITY: .016 RADIAN

BEARING NOTES

- ALL MATERIALS USED IN THE FABRICATION OF THESE BEARINGS SHALL BE MADE IN THE U.S.A.
- BEARINGS SHALL BE STORED IN A CLEAN, DRY, LEVEL UPRIGHT POSITION.
- LOCATION OF FABRICATION PLANT - 70 SOUTH STREET WALPOLE, MA 02081
- COSMEC, INC. REPRESENTATIVE - MR. MATT McANDREWS (508) 668-6600



MATERIALS

STEEL - ASTM A709 GRADE 36, ZINC METALLIZED
 PREFORMED FABRIC PAD - AASHTO DIV. II SPEC. 18.4.9.1
 ANCHOR BOLTS - SEE ANCHOR BOLT DETAIL

RECEIVED
 DEC 14 2006
 SUBMITTED BY: [Signature] DATE: 12/13/06

VERMONT
 AGENCY OF TRANSPORTATION
 HUNTINGTON
 PROJECT BRO 1445(29)
 EAST STREET (T.H.#4) OVER
 HUNTINGTON RIVER
 BRIDGE NO. 42

COSMEC, INC.		70 SOUTH STREET WALPOLE, MA 02081	
SCALE: 3/16"=1"	DRAWN BY: MRR	CHECKED BY: MCM	
DATE: 11/16/06	DATE: 11/16/06	DATE: 12/8/06	
COSMEC FABRIC PAD BEARING			
CUSTOMER PARENT CONSTRUCTION INC.	S.O. NUMBER 60630	DRAWING NUMBER 5059	REV.

REV.	BY	DATE	CHK'D	DATE



State of Vermont
PDD/Structures Design Section
National Life Building - Drawer 33
Montpelier, VT 05633-5001
www.aot.state.vt.us

[phone] 802-828-2621
[fax] 802-828-3566
[tdd] 800-253-0191

Agency of Transportation

October 30, 2006

Carolina Steel Corporation
P. O. Box 20888
Greensboro, NC 27420

Attention: Mike Williams, Manager, Bridge Drafting

Project Name: **Huntington** Project #: **BRO 1445(29)**
Structure Identification: **TH # 4, Bridge #42, East Street over Huntington River**

The following structural steel details and welding procedures [Item 506.55, Structural Steel (Plate Girder),] for the above project (**Vendor's Job # 18362H25**) transmitted with your letters dated August 30, 2006 and October 10, 2006, have been reviewed and are being returned herewith.

Sheets: HN1, HE1-HE3, HF1A/B-HF8A/B, HF9-HF19 and Welding Procedures 3N47, 3N48, 3N43, 3N34, and 3N6-10 **are approved or approved "as noted"**.

Upon submitting extended weights for our approval, please make appropriate changes as indicated on these "as noted" drawings and submit white prints for out use in the record plans for this project.

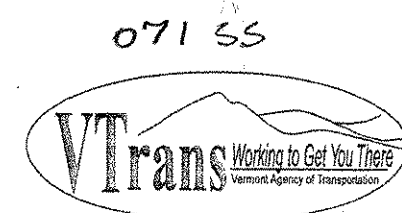
You must provide written notice to this office as to the date fabrication represented by these drawings will begin. That notice must be received at least seven days prior to that date, as per Specification 506.03. Any material fabricated prior to the notification date is subject to rejection without further cause.

Sincerely,

Warren Tripp, Project Manager

Attachments

cc: Resident Engineer Dale Norton w/prints
 Shop Inspector Jeff Clark w/prints
 Parent Construction, Inc. w/prints
 Subcontractor - letter only
 Construction Division - letter only
 Materials & Research Section (C&IA Unit) - letter only
 Files (Structures & Central)



CAROLINA STEEL FABRICATION INC.
NASH COUNTY PLANT

VERMONT PROJECT: VT # BR01445 (29) CSC JOB NO.: 18362H25
 BRIDGE: #42 East Street (TH #4)
 REFERENCE SPECS: AWS D1.5-02, VT-DOT STD SPECS - 2001
 STRUCTURE: BRIDGE IN HUNTINGTON, VT.

**JOINT WELDING
PROCEDURE SPECIFICATION**

MATERIAL SPECIFICATION: M270-50W
 WELDING PROCESS: SUBMERGED ARC WELDING
 MANUAL OR MACHINE: MACHINE (STIFFENER WELDER)
 POSITION OF WELDING: 2F-HORIZONTAL
 FILLER METAL SPECIFICATION: AWS A5.23
 FILLER METAL CLASSIFICATION: F7A4 EM12K-H8 LINCOLN L-61
 ELECTRODE STICKOUT: 3/4" SHIELDING GAS: NA
 FLUX: F7A4-LINCOLN AXXX10 SINGLE OR MULTIPLE PASS: SEE BELOW
 SINGLE OR MULTIPLE ARC: SINGLE
 WELDING CURRENT: DC-(STRAIGHT)
 WELDING PROGRESSION: NA
 ROOT TREATMENT: PER AWS D1.5 PARAGRAPH 3.2.1
 PREHEAT & INTERPASS TEMPERATURE: PER AASHTO TABLE 4.4
 HEAT INPUT MAXIMUM: 72 MINIMUM: 43.2
 MAXIMUM INTERPASS TEMPERATURE: 316°C / 600 F

PASS NO.	ELECTRODE SIZE	WELDING CURRENT		TRAVEL SPEED IPM	JOINT DETAIL FILLETS
		AMPERES	VOLTS		
1	3/32"	400 - 450	28 - 32	12 - 15	1/4"
1	3/32"	400 - 450	28 - 32	12 - 15	5/16"

*THE MAXIMUM SINGLE PASS FILLET WELD SHALL NOT EXCEED 5/16**
 NON-FCM PROCEDURE

THIS PROCEDURE MAY VARY DUE TO FABRICATION SEQUENCE, FIT-UP, PASS SIZE ECT., WITHIN THE LIMITATION OF VARIABLES GIVEN IN SECTION 5- AWS D1.5.

PROCEDURE NO. 3N34 REV. 0 APPROVED BY: *[Signature]*
 WPCR REFERENCE NO.: 213-FC & CSC 142 B QUALITY CONTROL SUPERVISOR

THICKNESS	TEMP.
UP TO 3/4"	50F
OVER 3/4" TO 1-1/2"	70F
OVER 1-1/2" TO 2-1/2"	150F
OVER 2-1/2"	225F

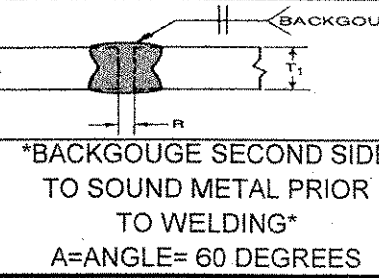

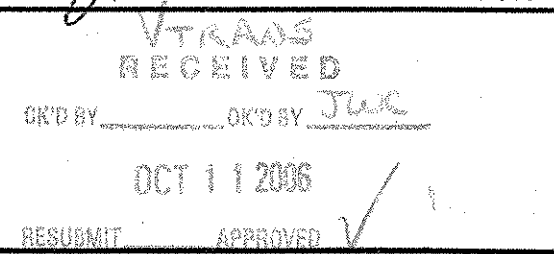
DATE: 10/30/06

CAROLINA STEEL FABRICATION INC.
NASH COUNTY PLANT

VERMONT PROJECT: VT# BRO 1445 (29)		CSC JOB NO.: 18362H25			
BRIDGE: #42 East Street (TH #4)					
REFERENCE SPECS: AWS D1.5-02, VT-DOT STD SPECS - 2001					
STRUCTURE: BRIDGE IN HUNTINGTON VT					
JOINT WELDING PROCEDURE SPECIFICATION					
MATERIAL SPECIFICATION: M270-50W					
WELDING PROCESS: SUBMERGED ARC WELDING					
MANUAL OR MACHINE: MACHINE					
POSITION OF WELDING: 1F-FLAT					
FILLER METAL SPECIFICATION: AWS A5.23					
FILLER METAL CLASSIFICATION: F7A4 EM12K - H8 LINCOLN L-81					
ELECTRODE STICKOUT: 1"		SHIELDING GAS: NA			
FLUX: LINCOLN F7A4 AXXX10		SINGLE OR MULTIPLE PASS: SINGLE			
SINGLE OR MULTIPLE ARC: MULTIPLE					
WELDING CURRENT: DC- (REVERSE)					
WELDING CURRENT: DC-(LEAD), AC-(TRAIL)					
ROOT TREATMENT: PER AWS D1.5 PARAGRAPH 3.2.1					
PREHEAT & INTERPASS TEMPERATURE: PER AASHTO TABLE 4.4					
HEAT INPUT: MAXIMUM 112.2 MINIMUM: 87.3					
MAXIMUM INTERPASS TEMPERATURE: 316°C / 600 F					
PASS NO.	ELECTRODE SIZE	WELDING CURRENT AMPERES	VOLTS	TRAVEL SPEED IPH	JOINT DETAIL FILLETS
1	3/16"	DC 750 - 850	30 - 32	30 - 40	1/4"
	3/16"	AC 725 - 825	32 - 35		
1	3/16"	DC 750 - 850	30 - 32	30 - 40	5/16"
	3/16"	AC 725 - 825	32 - 35		
STICKOUT		ARC SPACING		TRACTOR WELDER	
DC 1" TO 1-1/2"		5/8"			
AC 1" TO 1-1/2"		5/8" WITH 10 DEGREE TILT			
THIS PROCEDURE MAY VARY DUE TO FABRICATION SEQUENCE, FIT-UP, PASS SIZE ECT., WITHIN THE LIMITATION OF VARIABLES GIVEN IN SECTION 5- AWS D1.5.					
PROCEDURE NO.: 3N43 REV. 0		APPROVED BY: <i>[Signature]</i>			
WPQR REFERENCE NO.: CSC 208 - FC & CSC 143 A		QUALITY CONTROL SUPERVISOR			
PREHEAT		VTRANS RECEIVED OK'D BY: <i>[Signature]</i> OK'D BY: <i>[Signature]</i> OCT 11 2006 SUBMIT APPROVED <i>[Signature]</i> BY: DATE 10-30-06			
THICKNESS	TEMP.				
UP TO 3/4"	50F				
OVER 3/4" TO 1-1/2"	70F				
OVER 1-1/2" TO 2-1/2"	150F				
OVER 2-1/2"	225F				

07355

CAROLINA STEEL FABRICATION INC.
NASH COUNTY PLANT

VERMONT PROJECT: VT# BR0 1445 (29)		CSC JOB NO.: 18362H25			
BRIDGE: #42 East Street (TH #4)					
REFERENCE SPECS: AWS D1.5-02, VT-DOT STD SPECS - 2001					
STRUCTURE: BRIDGE IN HUNTINGTON VT					
JOINT WELDING PROCEDURE SPECIFICATION					
MATERIAL SPECIFICATION: M270-50W					
WELDING PROCESS: SUBMERGED ARC WELDING					
MANDUAL OR MACHINE: MACHINE AND SEMI-AUTO					
POSITION OF WELDING: TG (FLAT GROOVE)					
FILLER METAL SPECIFICATION: AWS A5.23					
FILLER METAL CLASSIFICATION: F7A4-EM12K-H8 (LINCOLN L-61)					
FLUX: F7A4-LINCOLN AXXX10		SHIELDING GAS: NA			
ELECTRODE STICKOUT: 3/4"		SINGLE OR MULTIPLE PASS: MULTIPLE			
SINGLE OR MULTIPLE ARC: SINGLE					
WELDING CURRENT: DC-REVERSE					
WELDING PROGRESSION: NA					
ROOT TREATMENT: PER AWS D1.5 PARAGRAPH 3.2.1					
PREHEAT & INTERPASS TEMPERATURE: PER AASHTO TABLE 4.4(SEE BELOW)					
HEAT INPUT MAXIMUM & MINIMUM: 57.6					
MAXIMUM INTERPASS TEMPERATURE: 318°C / 600 F					
PASS NO.	ELECTRODE SIZE	WELDING CURRENT AMPERES	VOLTS	TRAVEL SPEED IPM	JOINT DETAIL 5/8" MAX B-L-T-A-S
ALL	3/32"	430-500	29-32	10 - 13	
* POSITION FLAT GROOVE *					
T1= BASE METAL	R= ROOT	R= ROOT FACE		*BACKGOUGE SECOND SIDE TO SOUND METAL PRIOR TO WELDING*	
OVER 1/2" TO 1"	0	1/4" MINIMUM			
OVER 1" TO 1-1/2"	0	3/8" MINIMUM			
OVER 1-1/2" TO 2"	0	1/2" MINIMUM			
NON-FCM PROCEDURE					
THIS PROCEDURE MAY VARY DUE TO FABRICATION SEQUENCE, FIT-UP, PASS SIZE ECT., WITHIN THE LIMITATION OF VARIABLES GIVEN IN SECTION 5- AWS D1.5.					
PROCEDURE NO.: 3N47 REV. 0		APPROVED BY: 			
WPQR REFERENCE NO.: CSC 207 - FC		QUALITY CONTROL SUPERVISOR			
PREHEAT					
THICKNESS	TEMP.				
UP TO 3/4"	50F				
OVER 3/4" TO 1-1/2"	70F				
OVER 1-1/2" TO 2-1/2"	150F				
OVER 2-1/2"	225F				

BY _____ DATE 10-30-06

CAROLINA STEEL FABRICATION INC.
NASH COUNTY PLANT

VERMONT PROJECT: VT# BRO 1445 (29)		CSC JOB NO.: 18362H25	
BRIDGE: #42 East Street (TH #4)			
REFERENCE SPECS: AWS D1.5-02, VT-DOT STD SPECS - 2001			
STRUCTURE: BRIDGE IN HUNTINGTON VT			
JOINT WELDING PROCEDURE SPECIFICATION			
MATERIAL SPECIFICATION: M270-50W			
WELDING PROCESS: SUBMERGED ARC WELDING			
MANUAL OR MACHINE: MACHINE			
POSITION OF WELDING: 1G (FLAT GROOVE)			
FILLER METAL SPECIFICATION: AWS A5.23			
FILLER METAL CLASSIFICATION: F7A4-EM12K - H8 (LINCOLN L-81)			
FLUX: F7A4- LINCOLN AXXX10		SHIELDING GAS: NA	
ELECTRODE STICKOUT: 3/4"		SINGLE OR MULTIPLE PASS: MULTIPLE	
SINGLE OR MULTIPLE ARC: SINGLE			
WELDING CURRENT: DC-REVERSE			
WELDING PROGRESSION: NA			
ROOT TREATMENT: PER AWS D1.5 PARAGRAPH 3.2.1			
PREHEAT & INTERPASS TEMPERATURE: PER AASHTO TABLE 4.4			
HEAT INPUT MAXIMUM 95 MINIMUM: 57.6			
MAXIMUM INTERPASS TEMPERATURE: 316°C / 600 F			
PASS NO.	ELECTRODE SIZE	WELDING CURRENT AMPERES	TRAVEL SPEED IPM
		VOLTS	JOINT DETAIL
ALL	3/32"	430-500	28-32 10T013
"POSITION- 1G FLAT"			
T1= BASE METAL	R= ROOT	R= ROOT FACE	
OVER 1/2" TO 1"	0	1/4" MINIMUM	
OVER 1" TO 1-1/2"	0	3/8" MINIMUM	
OVER 1-1/2" TO 2"	0	1/2" MINIMUM	
"NON-FCM PROCEDURE"			"BACKGOUGE SECOND SIDE TO SOUND METAL PRIOR TO WELDING" A=ANGLE= 60 DEGREES
THIS PROCEDURE MAY VARY DUE TO FABRICATION SEQUENCE, FIT-UP, PASS SIZE ECT., WITHIN THE LIMITATION OF VARIABLES GIVEN IN SECTION 5- AWS D1.5.			
PROCEDURE NO.: 3N48 REV. 0		APPROVED BY: <i>[Signature]</i>	
WPO# REFERENCE NO.: CSC 207 - FC		QUALITY CONTROL SUPERVISOR	
PREHEAT			
THICKNESS	TEMP.		
UP TO 3/4"	50F		
OVER 3/4" TO 1-1/2"	70F		
OVER 1-1/2" TO 2-1/2"	150F		
OVER 2-1/2"	225F		
BY _____		DATE 10-30-06	

CAROLINA STEEL FABRICATION INC.
NASH COUNTY PLANT

VERMONT PROJECT: VT# BRO 1445 (29)		CSC JOB NO.: 18382H25			
BRIDGE: #42 East Street (TH #4)					
REFERENCE SPECS: AWS D1.5-02, VT-DOT STD SPECS - 2001					
STRUCTURE: BRIDGE IN HUNTINGTON VT					
JOINT WELDING PROCEDURE SPECIFICATION					
MATERIAL SPECIFICATION: M270-50W					
WELDING PROCESS: SHIELDED METAL ARC WELDING (SMAW)					
MANUAL OR MACHINE: MANUAL					
POSITION OF WELDING: 1F-FLAT & 2F-HORIZONTAL					
FILLER METAL SPECIFICATION: AWS A5.5					
FILLER METAL CLASSIFICATION: E8018-C3					
FLUX: NA		SHIELDING GAS: NA			
FLOW RATE: NA		SINGLE OR MULTIPLE PASS: (SEE BELOW)			
SINGLE OR MULTIPLE ARC: SINGLE					
WELDING CURRENT: AC					
WELDING PROGRESSION: NA					
ROOT TREATMENT: PER AWS D1.5 PARAGRAPH 3.2.1					
PREHEAT & INTERPASS TEMPERATURE: PER AASHTO TABLE 4.4					
MAXIMUM INTERPASS TEMPERATURE: 300 F					
PASS NO.	ELECTRODE SIZE	WELDING CURRENT AMPERES	VOLTS	TRAVEL SPEED IPM	JOINT DETAIL FILLETS
ALL	5/32"	140-225	23-27	NA	3/16" TO 3/8"
ALL	3/16"	230-305	23-27	NA	3/16" TO 3/8"
ALL	7/32"	270-365	23-27	NA	3/16" TO 3/8"
ALL	1/4"	315-400	23-27	NA	3/16" TO 3/8"
* THE MAXIMUM SIZE SINGLE PASS FILLET WELD SHALL BE 5/16" FOR THE HORIZONTAL POSITION AND 3/8" FOR THE FLAT POSITION. *					
NON-FCM PROCEDURE					
THIS PROCEDURE MAY VARY DUE TO FABRICATION SEQUENCE, FIT-UP, PASS SIZE ECT., WITHIN THE LIMITATION OF VARIABLES GIVEN IN SECTION 5- AWS D1.5.					
PROCEDURE NO.: 3N6-10 REV. 0.		APPROVED BY: <i>[Signature]</i>			
WPQR REFERENCE NO.: NA		QUALITY CONTROL SUPERVISOR			
PREHEAT		RECEIVED			
THICKNESS	TEMP.	OK'D BY: <i>[Signature]</i> OK'D BY: <i>[Signature]</i>			
UP TO 3/4"	50F	OCT 11 2006			
OVER 3/4" TO 1-1/2"	70F	RESUBMIT: _____ APPROVED: <i>[Signature]</i>			
OVER 1-1/2" TO 2-1/2"	150F	DATE 10-30-06			
OVER 2-1/2"	225F				

SPECIFICATIONS

CONSTRUCTION - VERMONT AGENCY OF TRANSPORTATION 2001 STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION & LATEST REVISIONS.
 AASHTO STANDARD BRIDGE SECTION SPECIFICATIONS FOR HIGHWAY BRIDGES, 2002 EDITION, WITH LATEST REVISIONS.
 GENERAL SPECIAL PROVISIONS & SUPPLEMENTAL SPECIFICATIONS DATED 12-27-05
 WELDING - AWS/AASHTO/AWS BRIDGE WELDING CODE D1.5-02

MATERIALS

ALL STRUCTURAL STEEL SHALL COMPLY WITH AASHTO M270, GRADE 50W.
 UNPAINTED WEATHERING REQUIREMENTS UNLESS NOTED OTHERWISE.
 ALL MATERIAL SHALL BE DOMESTIC.

FABRICATION NOTES

V - INDICATES CHAMFY V-NOTCH TESTED PER SPECIAL PROVISIONS (SUBSECTION 714.01) DATED 12/6/05
 HEAT NUMBERS SHALL BE PRESERVED AND VISIBLE TO THE INSPECTOR.
 THE BOTTOM FLANGE OF ORDERS AT BEARINGS SHALL BE SHOP STRAIGHTENED AS REQUIRED TO PROMOTE UNIFORM CONTACT BETWEEN THE ORDER FLANGE AND THE BEARINGS AT THE BRIDGE SEAT.
 THERE SHALL BE NO PAINT, CHALK, OR CHALK MARKS ON THE STEEL EXCEPT WHERE NOTED "PAINTED".
 FIELD SPLICES SHALL BE SHOP ASSEMBLED, AND THE HOLES IN THE WEB AND FLANGES SHALL BE DRILLED FROM THE SOLID USING THE OUTSIDE SPICE PLATES AS TEMPLATES.
 (C) INDICATE NONDESTRUCTIVE INSPECTION REQ-D-ULTRASONIC INSPECTION MAY BE USED PER STANDARD SPEC SECTION 306.06(A)
 (D) INDICATE WELDS SUBJECTED TO TENSION.
 ORDERS LESS THAN 250' AND SHALL BE FABRICATED BY CUTTING THE FLANGE PLATE TO THE REQUIRED CURVATURE. EACH PL SHALL BE FLAME CUT SIMULTANEOUSLY ON BOTH EDGES TO REDUCE UNBALANCED SHRINKAGE.

IDENTIFICATION MARKING STEEL MEMBERS

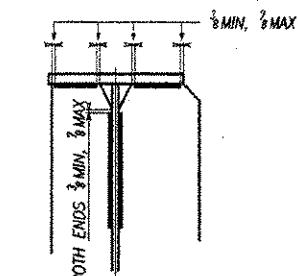
ALL STEEL MILL AND FABRICATOR IDENTIFICATION MARKINGS FOR STEEL PLATES, SHAPES, OR FABRICATED MEMBERS SHALL BE BY METAL TAGS, COMPASSION OF SOME OTHER READILY REMOVABLE MATERIAL, OR SHALL BE MARKED IN AN AREA OF THE COMPLETED MEMBER WHICH WILL BE ENCLOSED OR COVERED WITH CONCRETE. MARKING METHODS AND LOCATIONS ARE SUBJECT TO APPROVAL OF THE ENGINEER.
 DO NOT USE PAINT OR INK-BASED CRAYONS FOR MARKINGS

RECEIVED

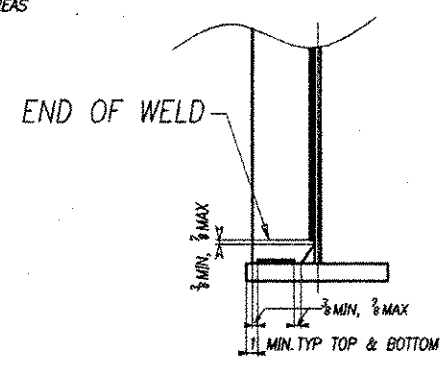
OK'D BY _____ OK'D BY _____
 FEB 7 - 2007
 RESUBMIT _____ APPROVED _____
 BY _____ DATE _____

PAINT NOTES

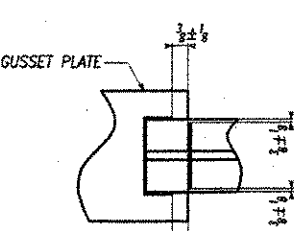
SURFACE PREPARATION - SSPC-SP10 (NEAR WHITE BLAST CLEANING).
 PRIOR TO BLAST CLEANING, ALL OIL, GREASE OR OTHER FOREIGN MATTER SHALL BE REMOVED BY SOLVENT CLEANING (SSPC-SP11).
 ORDERS WILL BE COATED IN PAINTED AREA WITH:
 A 3-COAT PAINT SYSTEM PER MANUFACTURING RECOMMENDATIONS
 FINAL PAINT COAT TO BE DARK BROWN, FEDERAL COLOR CHIP NO.20059
 SHOP PAINT - NO PAINT, UNLESS NOTED TO BE PAINTED ON DETAILS.
 PAINT AREAS ARE PER SECTION 513 OF AUG. 2000 SUPPLEMENTAL SPECIFICATIONS
 PRIMER - EPOXY PRIMER CARBOLINE 850R (SHOP APPLIED)
 INTERMEDIATE - EPOXY CARBOLINE 850R (SHOP APPLIED)
 TOP COAT - 13LB ALIPHATIC POLYURETHANE (SHOP APPLIED).
 COLOR TO BE FED STANDARD 3005, AND THE GLOSS AT AN ANGLE OF 60 DEGREES SHALL NOT EXCEED 25.
 PAINT MANUFACTURER: CARBOLINE
 SEE DETAIL BELOW AND INDIVIDUAL ORDER DETAILS FOR "NO PAINT" AREAS WITHIN THE AREAS DESIGNATED TO BE PAINTED.
 NOTE B - WEST COAT PRIMER ONLY (1.5 MILS DFT)
 NOTE C - INDICATE PRIMER ONLY INTERMEDIATE AND TOP COATS TO BE STAGERED A MINIMUM OF 1" FROM FINISH SURFACES AND EACH OTHER.



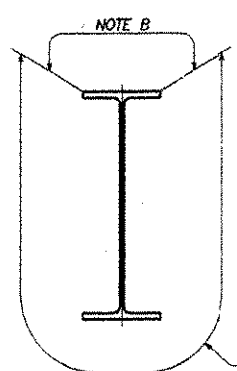
TYPICAL STEENER OR CONNECTOR PLATE CONNECTIONS



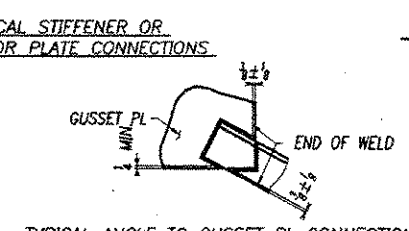
END OF WELD



TYPICAL WT TO GUSSET WELD DETAIL



AREA TO BE PAINTED. SEE ORDER DETAILS FOR LENGTH



TYPICAL ANGLE TO GUSSET PL CONNECTION

WELD TERMINATION DETAILS

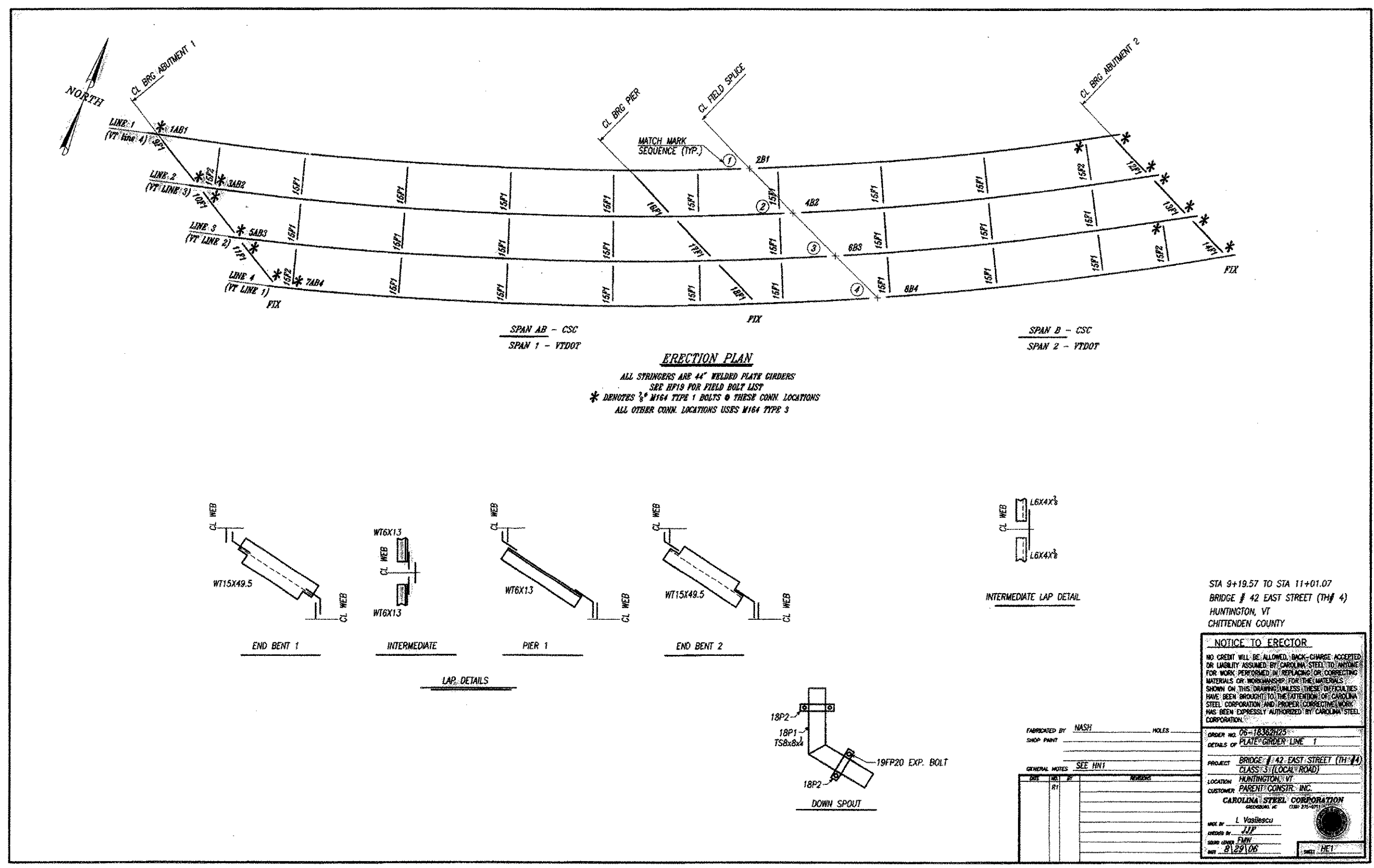
ERECTION NOTES

1" HEAVY HEX HIGH STRENGTH BOLTS (M16 TYPE 3) WITH ONE HARDENED WASHER ARE PROVIDED FOR THIS PROJECT AT UNPAINTED AREAS.
 1" HEAVY HEX HIGH STRENGTH BOLTS (M16 TYPE 1) WITH ONE HARDENED WASHER ARE PROVIDED FOR THIS PROJECT AT PAINTED AREAS.
 PLACE WASHER UNDER THE TURNED ELEMENT.
 FIELD WELDS ARE NOT BY CAROLINA STEEL CORPORATION

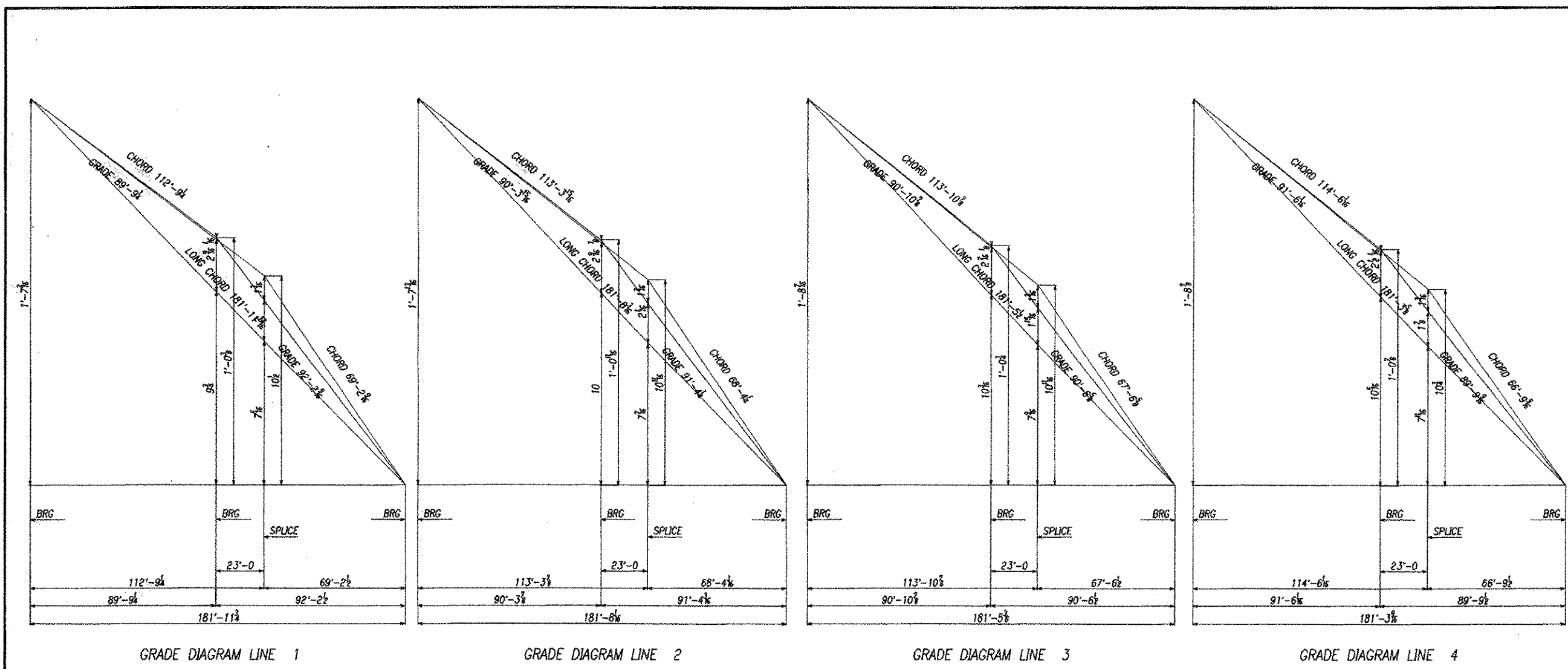
FABRICATED BY	WELD	DATE	BY
SHOP PAINT			
GENERAL NOTES			
ON	OFF		

DESIGNED BY: _____
 CHECKED BY: _____
 PROJECT: BRIDGE # 42 EAST STREET (THE 4)
 LOCATION: HUNTINGTON, VT
 CONTRACTOR: CAROLINA STEEL CORPORATION
 DATE: FEB 7 2007
 DRAWN BY: _____
 SCALE: AS SHOWN

*Revised sheets
 re: shop paint at
 diaphragm connections*



079 35



DEFINITION OF TERMS
 ALL DIMENSIONS IN THE GRADE AND CAMBER DIAGRAMS, EXCEPT THOSE IDENTIFIED OTHERWISE, ARE ALONG THE LONG CHORD. THOSE IDENTIFIED BELOW ARE ALONG THE PLANES THROUGH THE ACTUAL ELEVATIONS.
 LONG CHORD (LNC)-FROM CL END BRG TO CL END BRG
 GRADE & CAMBER DIAGRAMS (MCS)
 GRADE LINE-FROM CL BRG TO CL BRG
 CHORD LINE-FROM CL END BRG TO CL NEAREST FIELD SPACE OR FROM CL FIELD SPACE TO CL FIELD SPACE

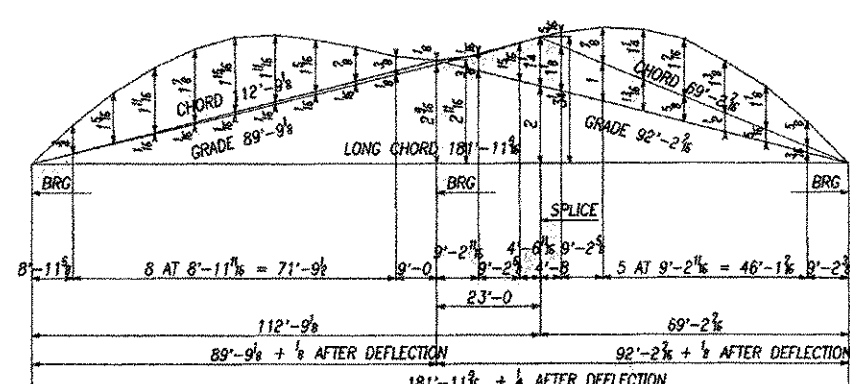
BLOCKING NOTE
 GIRDERS SHALL BE SHOP ASSEMBLED IN THE CAMBERED POSITION WITH THE BEARINGS AND SPLICES BLOCKED TO THE DIMENSIONS SHOWN, WITHOUT INDUCING STRESS INTO THE MEMBERS. FULLY ASSEMBLE AND CHECK ALL BLOCKING AND CAMBER ORDINATES IN THE SHOP, MAKING ADJUSTMENTS WHERE NECESSARY. BOLTS SHALL BE OILED FROM THE SOLE USING THE OUTSIDE SPLICE PLATE AS A TEMPLATE AND MATCH MARKED WITH SPLICE MARKING ASSIGNED TO THE GIRDERS.
 FOR FIELD ERECTION, GIRDERS SHALL BE BENT AND BOLTED, AND SPLICE ELEVATIONS SHALL BE CHECKED BEFORE BOLTS ARE TIGHTENED.
 SHIPPING BOLTS SHALL BE MACHINE BOLTS.
 ALL SHIPPING BOLTS SHALL BE REMOVED AND RECORDED.

STA 9+19.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (THRU 4)
 WINDSOR, VT
 CHITTENDEN COUNTY

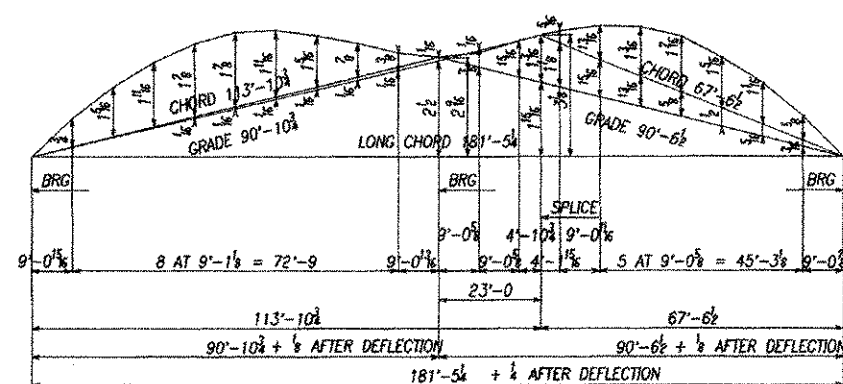
DESIGNED BY	DATE	SCALE	PROJECT
DRAWN BY	DATE	SCALE	PROJECT
CHECKED BY	DATE	SCALE	PROJECT
APPROVED BY	DATE	SCALE	PROJECT

CONTRACT NO. 11-1000
 CONTRACT DATE 11/10/00
 CONTRACT VALUE \$1,000,000
 CONTRACT TYPE BRIDGE # 42 EAST STREET (THRU 4)
 CLASSIFICATION CLASS 1 (LOCAL ROAD)
 CONTRACTOR
 CONTRACTOR ADDRESS
 CONTRACTOR PHONE
 CONTRACTOR FAX
 CONTRACTOR E-MAIL

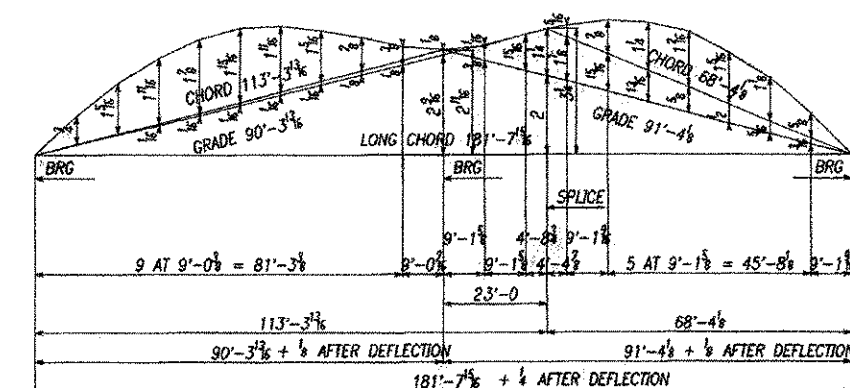
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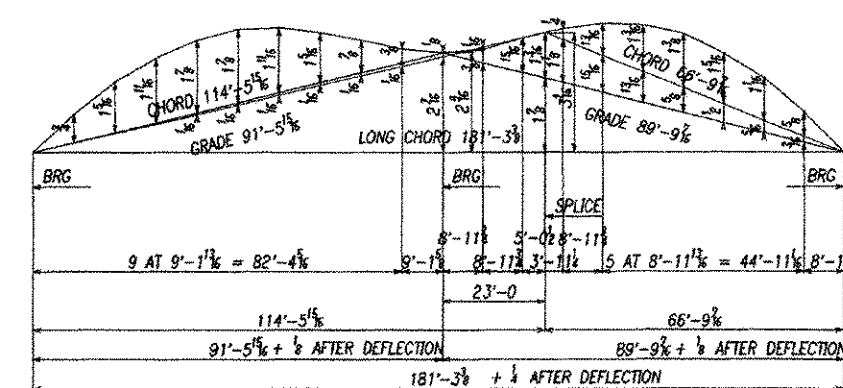
CAMBER DIAGRAM LINE 1



CAMBER DIAGRAM LINE 3



CAMBER DIAGRAM LINE 2



CAMBER DIAGRAM LINE 4

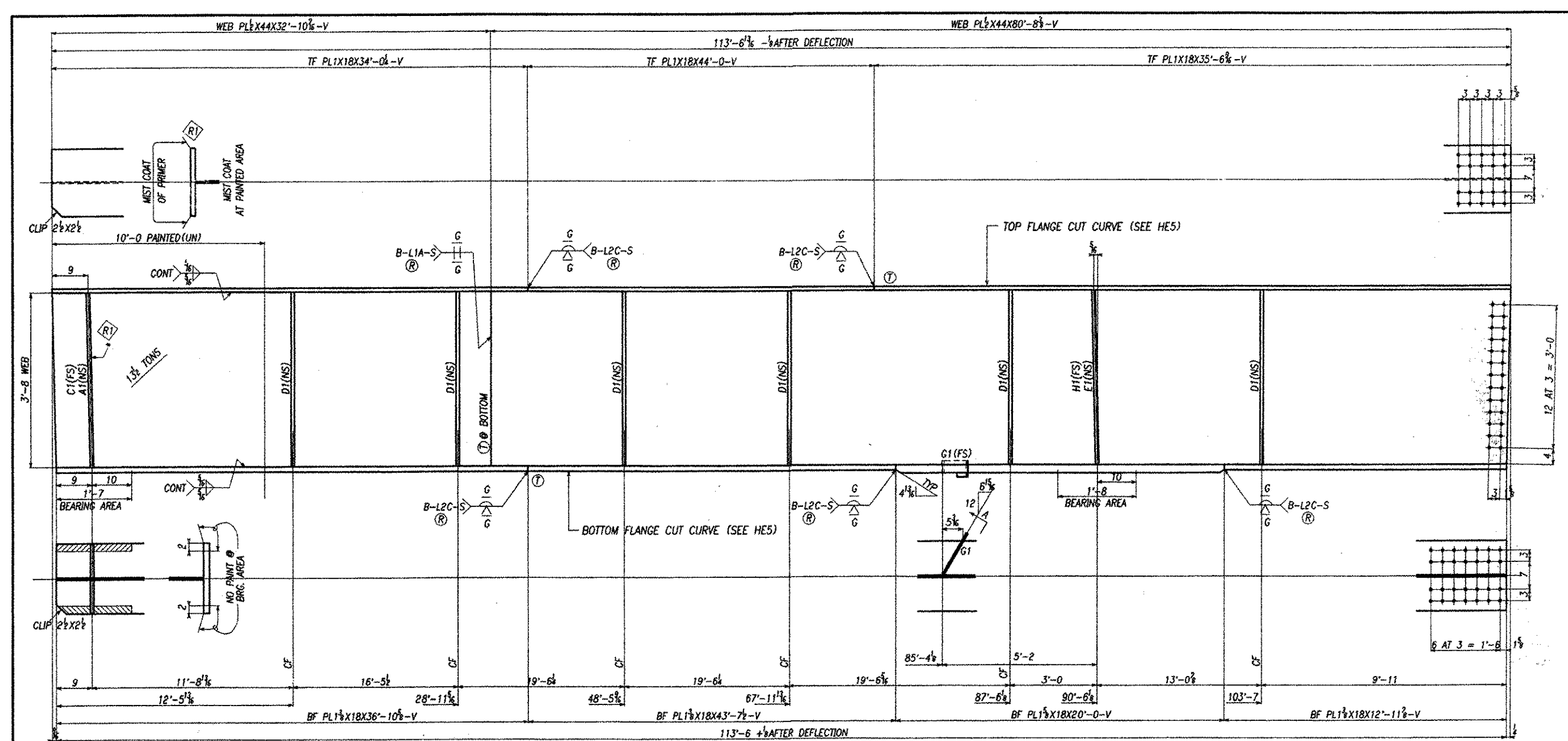
DEFINITION OF TERMS
 ALL DIMENSIONS IN THE GRADE AND CAMBER DIAGRAMS, EXCEPT THOSE IDENTIFIED OTHERWISE, ARE ALONG THE LONG CHORD. THOSE IDENTIFIED BELOW ARE ALONG THE PLANES THROUGH THE ACTUAL ELEVATIONS.
 LONG CHORD (ARC)-FROM CL END BRG TO CL END BRG
 GRADE & CAMBER DIAGRAMS (ARCS)
 GRADE LINE-FROM CL BRG TO CL BRG
 CHORD LINE-FROM CL END BRG TO CL NEAREST FIELD SPICE, OR FROM CL FIELD SPICE TO CL FIELD SPICE

BLOCKING NOTE
 GIRDERS SHALL BE SHIP ASSEMBLED IN THE CAMBERED POSITION WITH THE BEAMS AND SPICES BLOCKED TO THE DIMENSIONS SHOWN WITHOUT INCLUDING STRESS INTO THE MEMBERS. FULLY ASSEMBLE AND CHECK ALL BLOCKING AND CAMBER ORIGINATES IN THE SHOP. MAKE ADJUSTMENTS WHERE NECESSARY. HOLES SHALL BE DRILLED FROM THE SOLE USING THE OUTSIDE SPLICE PLATE AS A TEMPLATE AND MATCH MARKED WITH SPLICE MATERIAL ASSEMBLED TO THE GIRDERS.
 FOR FIELD ELEVATION GIRDERS SHALL BE PINNED AND BOLTED, AND SPLICE ELEVATIONS SHALL BE CHECKED BEFORE BOLTS ARE TIGHTENED.
 SHIPPING BOLTS SHALL BE MACHINE BOLTS.
 ALL SHIPPING BOLTS SHALL BE REMOVED AND DISCARDED.

STA 9+19.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (THRU 4)
 FARMINGTON, VT
 CANTONMENT COUNTY

APPROVED BY: MSH	DATE: 10/1/07
DESIGNED BY: SEE RIT	DATE: 10/1/07
CHECKED BY: SEE RIT	DATE: 10/1/07
CONTRACT NO. 07-00000000	PROJECT: BRIDGE # 42 EAST STREET (THRU 4) CLASS II (LOCAL ROAD)
LOCATION: FARMINGTON, VT	CONTRACTOR: CARROLLA ENTERPRISE CORPORATION
DATE: 10/1/07	SCALE: 1/4" = 1'-0"

08155



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 OK'D BY: *[Signature]*
 FEB 7 - 2007
 RESUBMIT: _____ APPROVED:
 BY: *[Signature]* DATE: 2/2/07

SECTION A

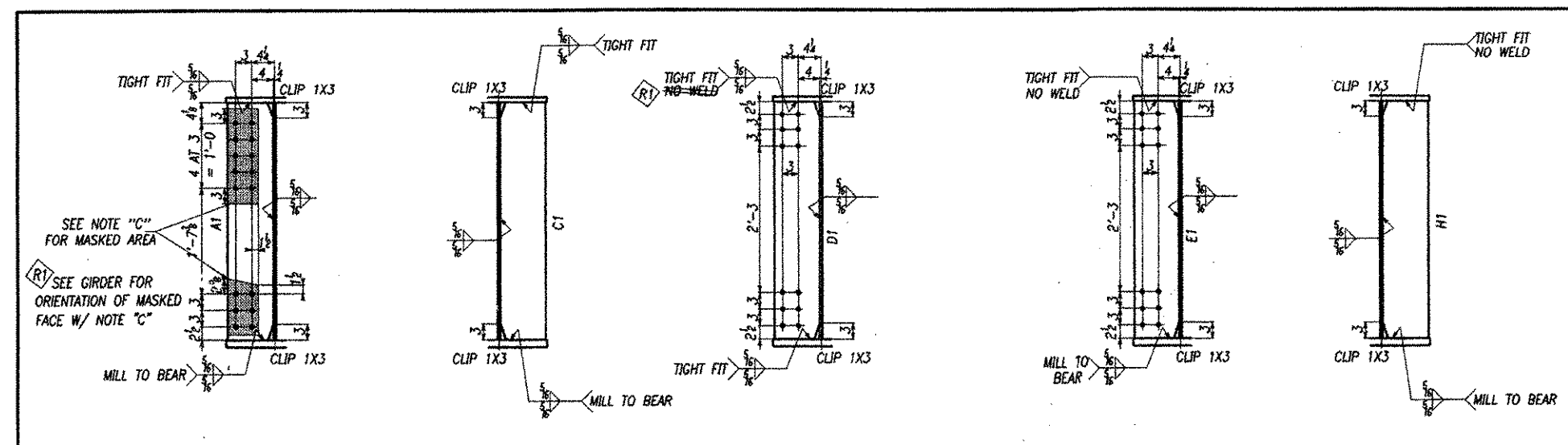
* ASTERISK * = DENOTES CONTACT SURFACE W/ PRIMER ONLY - * NOTE C *

STA 9+18.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (THRU 4)
 HUNTINGTON, KY
 CHITTENDEN COUNTY

DESIGNED BY	DATE	SCALE
CHECKED BY		
INVESTIGATED BY		
APPROVED BY		
DATE		

DRAWN BY: *[Signature]*
 CHECKED BY: *[Signature]*
 INVESTIGATED BY: *[Signature]*
 APPROVED BY: *[Signature]*
 DATE: 2/2/07

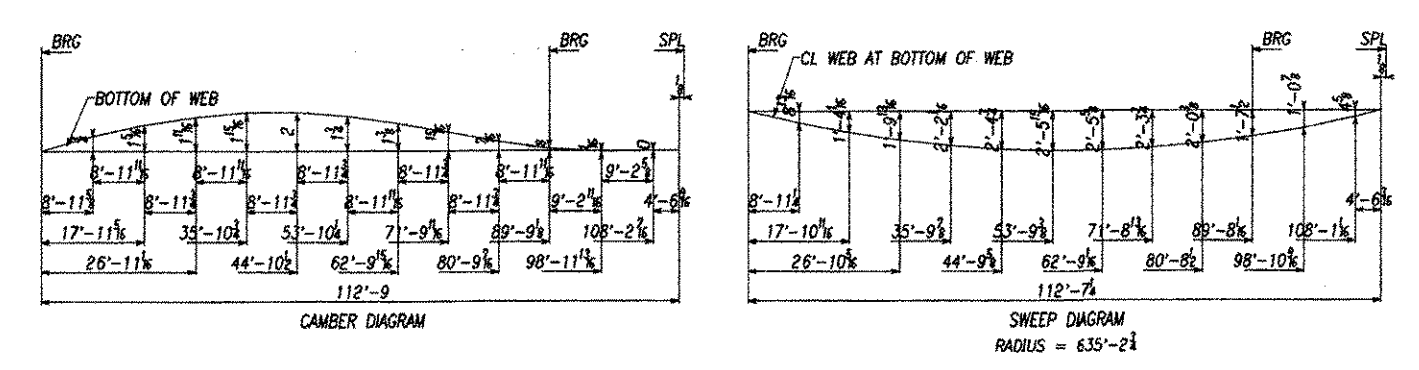
08255



SHOP BILL

NO.	QUANTITY	DESCRIPTION	UNIT	REMARKS	DATE	BY
1	1	PLATE ORDER				
1	PL 1X18	34	CL 17/168	Y	M270-S0W	26830W13
1	PL 1X18	44	CL 17/348	Y	M270-S0W	26830W17
1	PL 1X18	35	CL 17/348	Y	M270-S0W	26830W18
1	PL 1X218	36	CL 17/168	Y	M270-S0W	26830W14
1	PL 1X218	43	CL 17/348	Y	M270-S0W	26830W15
1	PL 1X218	20	CL 17/348	Y	M270-S0W	26830W17
1	PL 1X218	12	CL 17/348	Y	M270-S0W	26830W15
1	PL 5X84	32	CL 10/168	Y	M270-S0W	26830W14
1	PL 5X84	80	CL 10/348	Y	M270-S0W	26830W11
1	PL 3X85	3	CL 17/168	Y	M270-S0W	SON
1	PL 3X85	3	CL 17/348	Y	M270-S0W	SON
6	PL 3X85	3	CL 17/168	Y	M270-S0W	26830W15
1	PL 3X85	3	CL 17/348	Y	M270-S0W	SON
1	PL 3X35	1	CL 17/168	Y	M270-S0W	SON
1	PL 3X85	3	CL 17/348	Y	M270-S0W	SON

DOMESTIC MATERIAL



RECEIVED
 OK'D BY WMS OK'D BY _____
FEB 7 - 2007
 RESUBMIT _____ APPROVED _____
 BY W. Jupp DATE 2/2/07

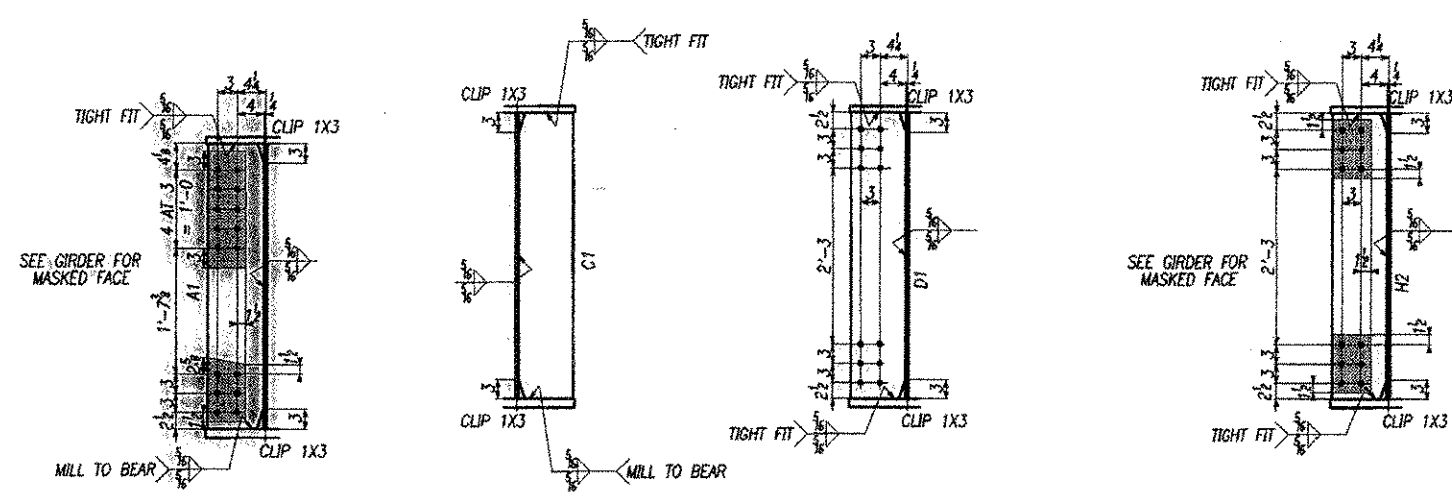
STA 94+19.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (TH 4)
 HARRINGTON, VT
 CHITTENDEN COUNTY

DESIGNED BY: WMS CHECKED BY: _____
 DRAWN BY: WMS DATE: _____
 SCALE: _____

PROJECT: BRIDGE # 42 EAST STREET (TH 4)
 DRAWING NO.: 112-2
 SHEET NO.: 1 OF 1

CONTRACT NO.: _____
 CONTRACT DESCRIPTION: _____
 CONTRACT VALUE: _____

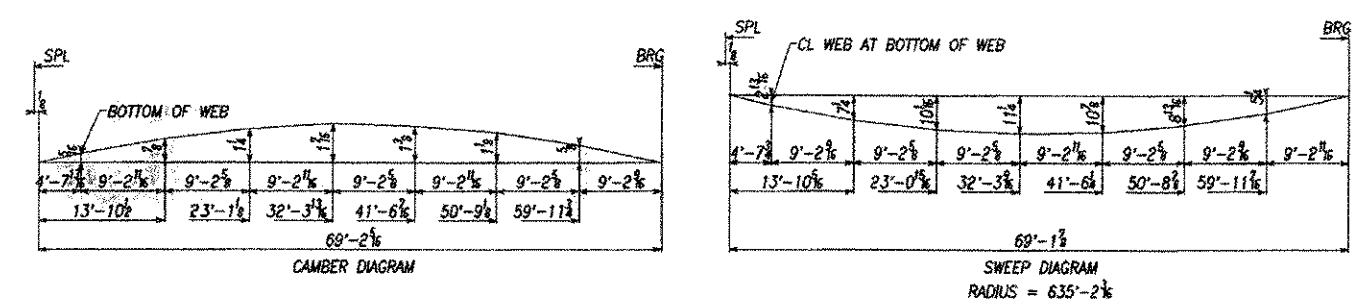
DATE: _____
 TIME: _____



SHOP BILL

QTY	UNIT	DESCRIPTION	SECTION	DATE	REVISION	BY	DATE	APPROVED
1	PL	PLATE GIRDER	69 11	1718	V	M270-50W		
1	PL	PLATE GIRDER	69 11 1/2	1718	V	M270-50W		
1	PL	PLATE GIRDER	69 11 1/4	1718	V	M270-50W		
2	PL	PLATE GIRDER	3 1/2	1718	V	M270-50W		
1	PL	PLATE GIRDER	3 1/2	1718	V	M270-50W		
1	PL	PLATE GIRDER	3 1/2	1718	V	M270-50W		
2	PL	PLATE GIRDER	2 1/2	1718	V	M270-50W		
2	PL	PLATE GIRDER	1 1/2	1718	V	M270-50W		
1	PL	PLATE GIRDER	3 1/2	1718	V	M270-50W		
1	PL	PLATE GIRDER	3 1/2	1718	V	M270-50W		
3	PL	PLATE GIRDER	3 1/2	1718	V	M270-50W		
1	PL	PLATE GIRDER	3 1/2	1718	V	M270-50W		
2	F BOLT					A307		
4	F BOLT					A307		
4	F BOLT					A307		
1	PL	TEMP				M270-50W		

DOMESTIC MATERIAL



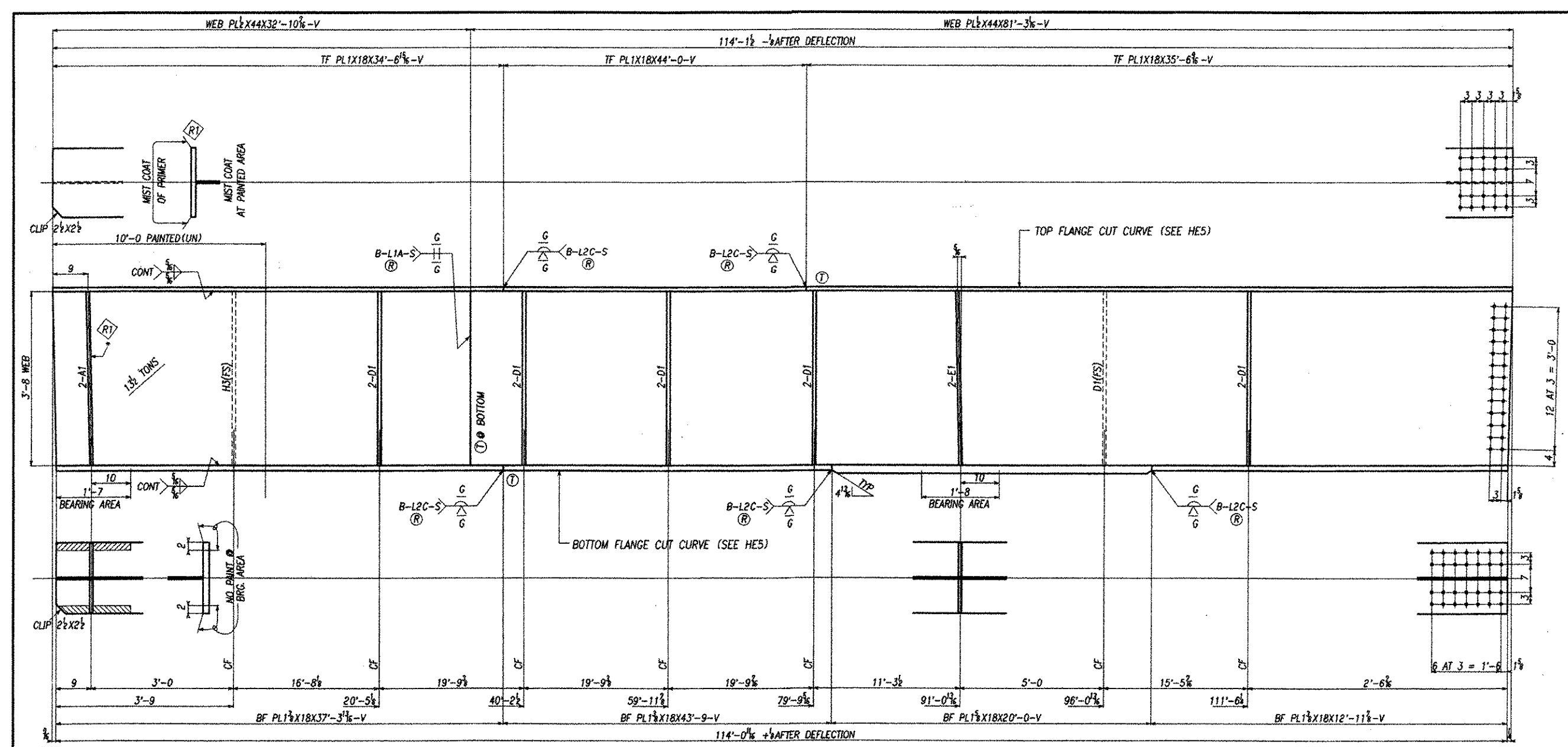
STA 24+19.57 TO STA 11+01.07
BRIDGE # 42 EAST STREET (7th St)
HENNINGTON, VT
CHITTENDEN COUNTY

DESIGNED BY: []
CHECKED BY: []
DATE: []

PROJECT: BRIDGE # 42 EAST STREET (7th St)
CLASS: []
DRAWN BY: []
DATE: []

SCALE: []
SHEET NO: []

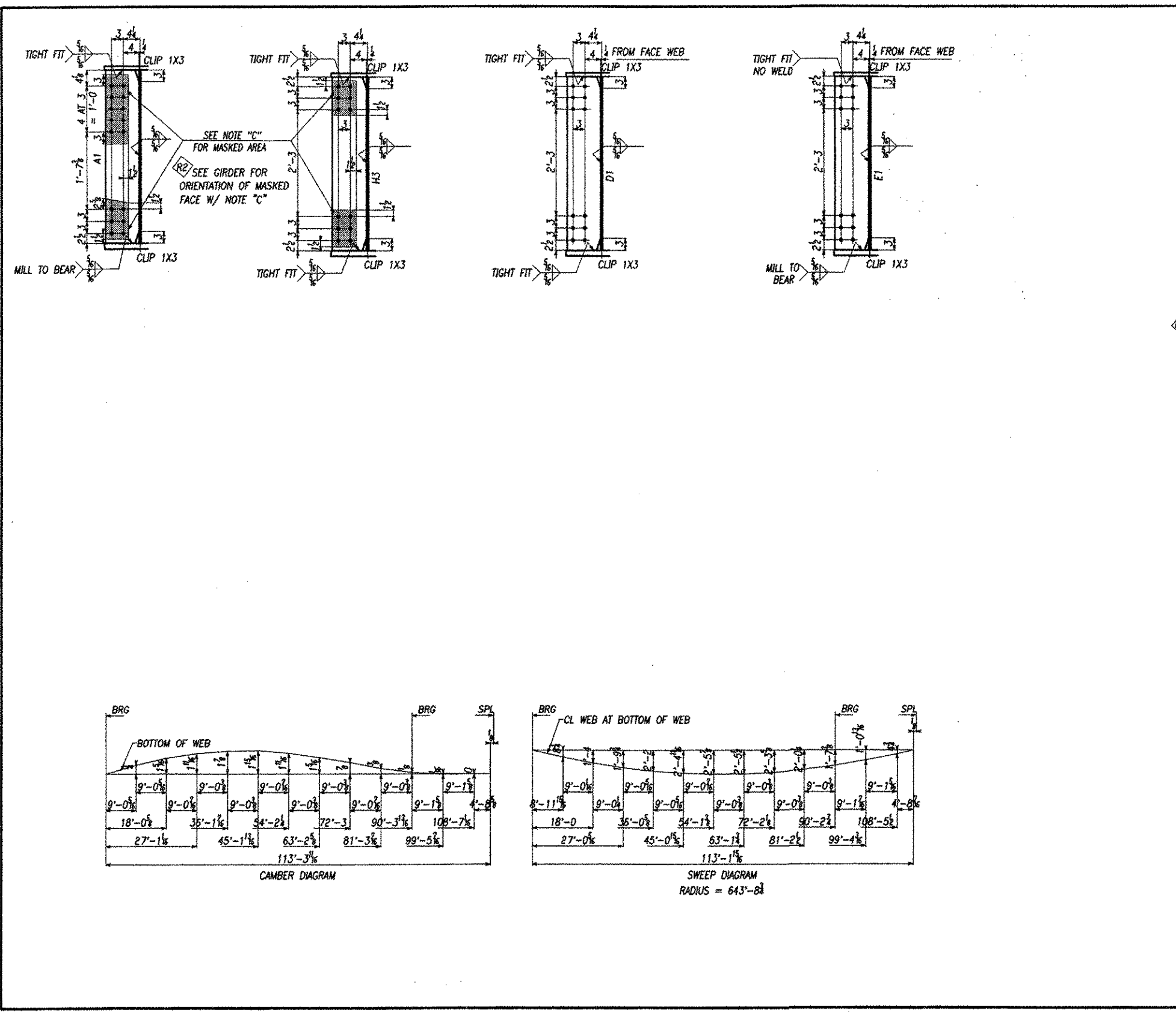
APPROVED BY: []
DATE: []



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 CK'D BY _____ OK'D BY *EWB*
 FEB 7 - 2007
 RESUBMIT _____ APPROVED _____
 BY *WJS* DATE 2/22/07

STA 9+10.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (TH 4)
 HUNTINGTON, VT
 CHITTENDEN COUNTY

DESIGNED BY	WJS	CHECKED BY	WJS
DATE	1/20/07	DATE	1/20/07
PROJECT	BRIDGE # 42 EAST STREET (TH 4)	LOCATION	HUNTINGTON, VT
CONTRACT NO.	SEE 203	CONTRACTOR	CAROLINA STEEL CORPORATION
SCALE	1" = 1'-0"	DATE	1/20/07



SHOP BILL

NO.	QUANTITY	DESCRIPTION	UNIT	REMARKS	WELD SPEC.	WELD NO.
1	1	NO WELD				
2	1	PLATE CONNECTION				
3	1	PL 1/2x18	34	0% TP1A2	V	M270-S0M P8830M1 6
4	1	PL 1/2x18	44	0% TP2A2	V	M270-S0M P8830M1 7
5	1	PL 1/2x18	35	6% TP3A2	V	M270-S0M P8830M1 8
6	1	PL 1/2x18	37	2% TP1A2	V	M270-S0M P8830M1 4
7	1	PL 1/2x18	43	9% TP2A2	V	M270-S0M P8830M1 5
8	1	PL 1/2x18	29	0% TP3A2	V	M270-S0M P8830M1 1
9	1	PL 1/2x18	12	11% TP1A2	V	M270-S0M P8830M1 3
10	1	PL 5/8x4	32	10% W1A2	V	M270-S0M P8830M1 14
11	1	PL 5/8x4	81	3% W2A2	V	M270-S0M P8830M1 15
12	2	PL 3/8x6	3	A1	M270-S0M	SHY
13	11	PL 3/8x6	3	D1	M270-S0M	P8830M1 15
14	2	PL 3/8x6	3	E1	M270-S0M	SHY
15	1	PL 3/8x6	3	H2	M270-S0M	P8830M1 15
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OK'D BY _____ OK'D BY MB

FEB 7 - 2007

RESUBMIT _____ APPROVED ✓

BY MB DATE 2/2/07

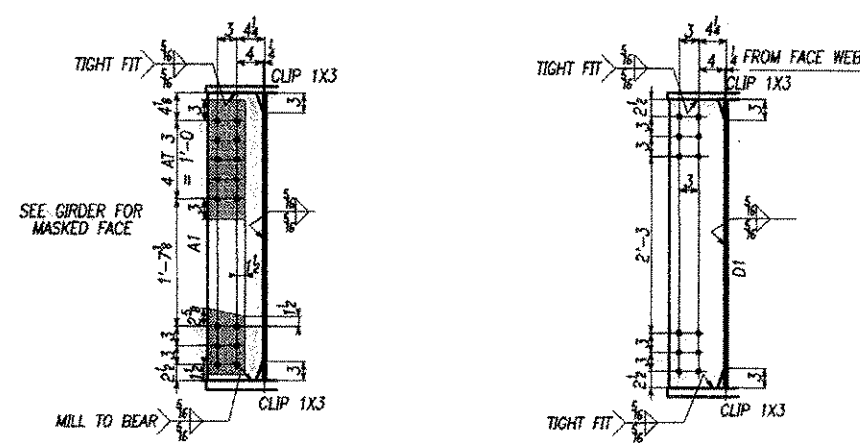
STA 8+19.57 TO STA 11+01.07
BRIDGE # 42 EAST STREET (TH# 4)
HARRINGTON, VT
CHITTENDEN COUNTY

DESIGNED BY	DATE	SCALE	NO. OF SHEETS
CHECKED BY	DATE	SCALE	NO. OF SHEETS
APPROVED BY	DATE	SCALE	NO. OF SHEETS
DATE	SCALE	NO. OF SHEETS	

WORK WITH SHEET # 100
SEE PLAN
11-12-07 (1) OF 100
11-12-07 (1) OF 100

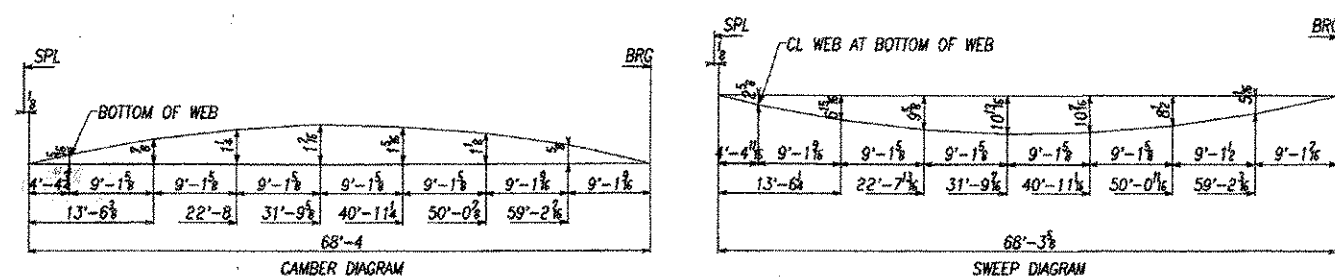
CONTRACTOR: CAROLINA STEEL CORPORATION
CUSTOMER: BARNES CONCRETE INC.

DATE: 2/2/07
SCALE: AS SHOWN
NO. OF SHEETS: 100



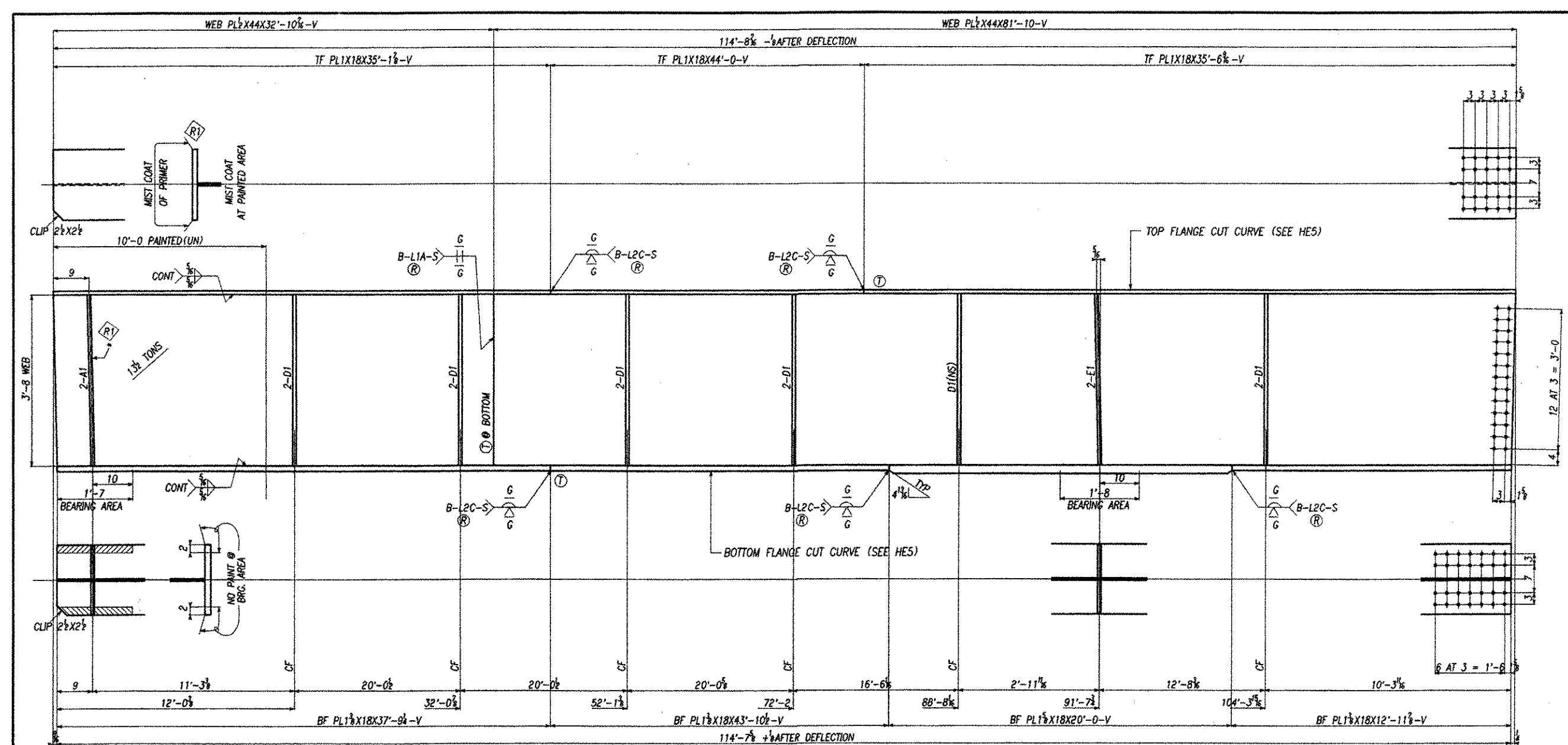
SHOP BILL

NO.	QTY	DESCRIPTION	UNIT	REMARKS	DATE
1	1	PLATE GIRDER			
1	1	PL 1x18	59	7718	M270-50W
1	1	PL 1x18	60	1 8718	M270-50W
1	1	PL 5x44	60	1 8718	M270-50W
2	2	PL 3x8	3	65	E2 V M270-50W
1	1	PL 3x18	3	65	E2 V M270-50W
1	1	PL 3x18	2	65	A2 V M270-50W
2	2	PL 3x8	2	65	C2 V M270-50W
2	2	PL 3x8	1	65	G2 V M270-50W
2	2	PL 3x8	3	8	A1 7/8\"/>



STA 9+19.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (HWY 4)
 CHITTENDEN VT
 CHITTENDEN COUNTY

DESIGNED BY: [Signature]
 DRAWN BY: [Signature]
 CHECKED BY: [Signature]
 DATE: 11/11/07



⊗ ASTERISK * - DENOTES CONTACT SURFACE W/ PRIMER ONLY - * NOTE C *

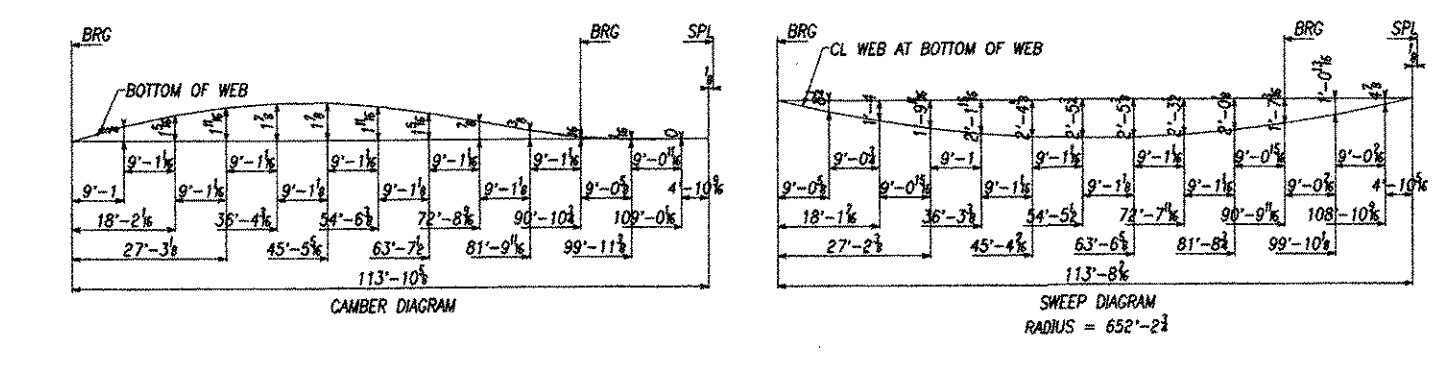
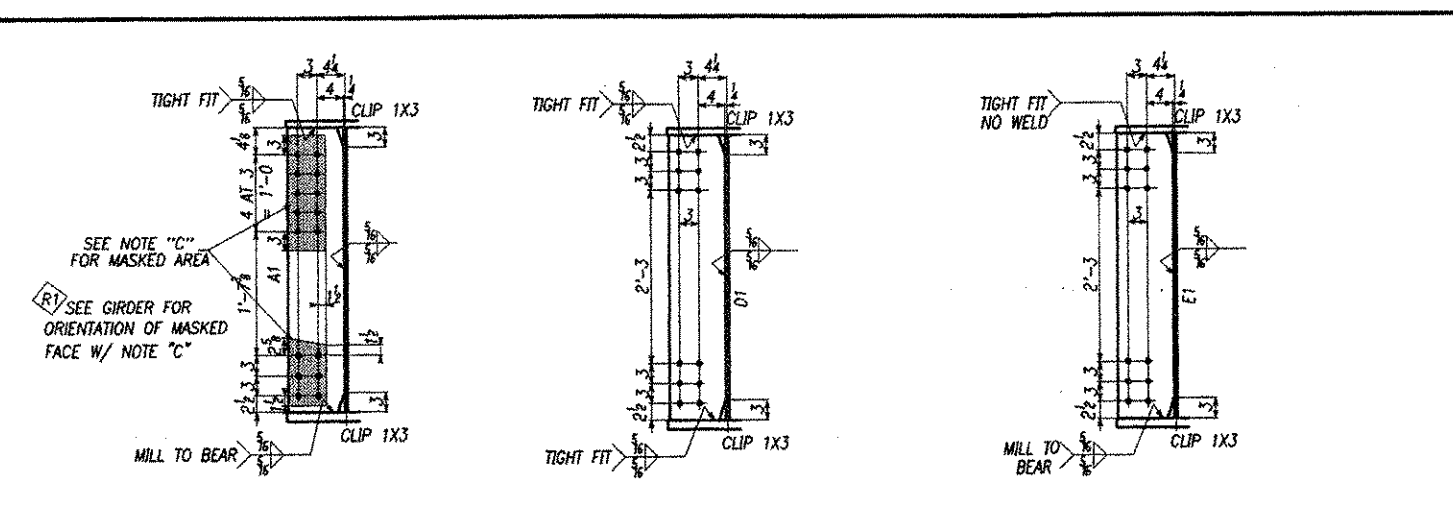
RECEIVED
 OK'D BY *[Signature]*
 FEB 7 - 2007
 RESUBMIT APPROVED *[Signature]*
 BY *[Signature]* DATE 2/23/07

STA 9+19.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (THRU)
 HUNTINGTON, VA
 COTTLENS COUNTY

DESIGNED BY	DATE	SCALE	PROJECT
DRAWN BY	DATE	SCALE	PROJECT
CHECKED BY	DATE	SCALE	PROJECT
APPROVED BY	DATE	SCALE	PROJECT

CAROLINA STEEL CORPORATION
 1234 EAST STREET
 HUNTINGTON, VA 24045
 TEL: 800-368-7263
 FAX: 540-885-1100

090 55



SHOP BILL

NO.	QUANTITY	DESCRIPTION	UNIT	REMARKS	EST. QTY.	EST. PRICE
1	1	PLATE GIRDER				
2	1	PL 1218	35 15	72x148 V	M270-S0M	2683001.2
3	1	PL 1218	44 0	72x148 V	M270-S0M	2683001.7
4	1	PL 1218	35 68	72x148 V	M270-S0M	2683001.8
5	1	PL 1218	37 64	84x148 V	M270-S0M	2683001.4
6	1	PL 1218	43 108	84x148 V	M270-S0M	2683001.3
7	1	PL 1218	20 0	84x148 V	M270-S0M	2683001.1
8	1	PL 1218	12 13	84x148 V	M270-S0M	2683001.5
9	1	PL 1264	32 108	114x148 V	M270-S0M	2683001.14
10	1	PL 1264	81 10	114x148 V	M270-S0M	2683001.15
11	2	PL 1268	3 8	AT 114x148 V	M270-S0M	SN
12	11	PL 1268	3 8	DT 114x148 V	M270-S0M	2683001.16
13	2	PL 1268	3 8	ET 114x148 V	M270-S0M	SN
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OK'D BY _____ OK'D BY CLB

FEB 7 - 2007

RESUBMIT _____ APPROVED ✓

BY MS/Supp DATE 2/2/07

STA 94+19.57 TO STA 11+01.07
BRIDGE # 42 EAST STREET (TH 4)
HUNTINGTON, VT
CHITTENDEN COUNTY

APPROVED BY _____ DATE _____
SCALE _____

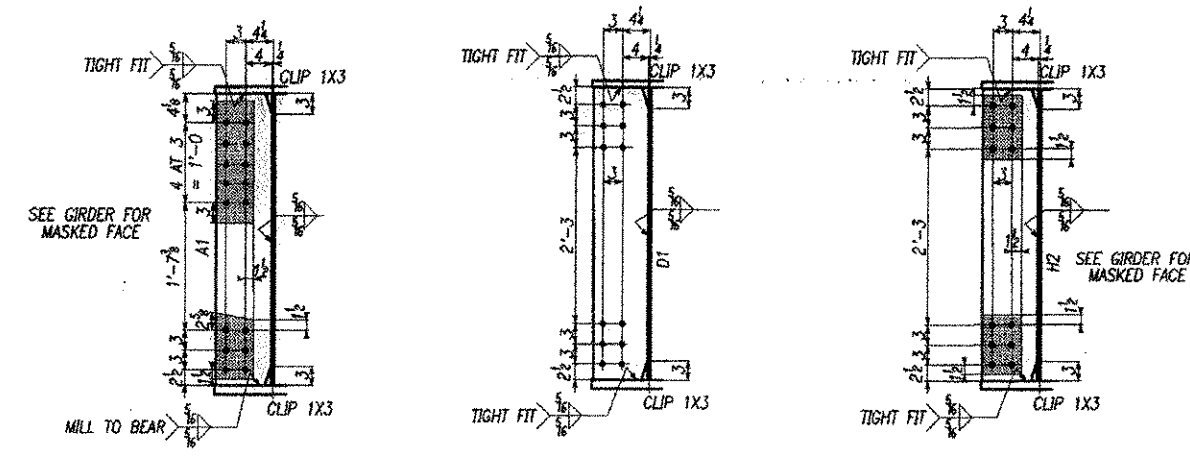
DESIGNED BY _____ DATE _____
SCALE _____

CHECKED BY _____ DATE _____
SCALE _____

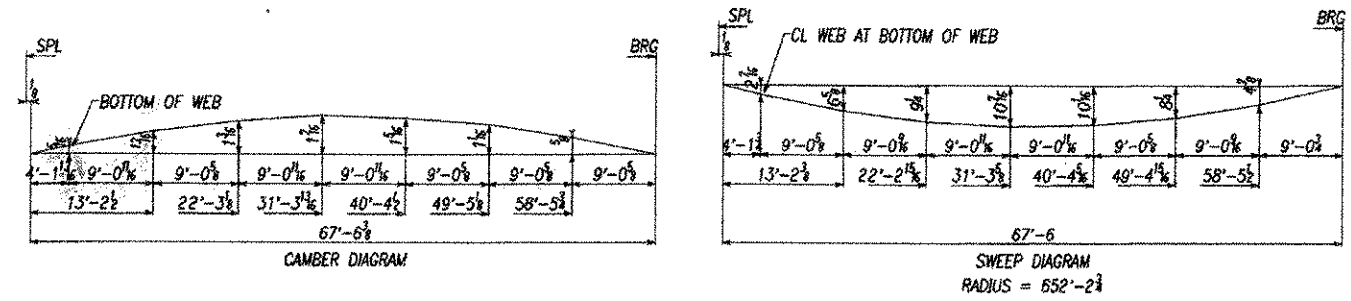
CONTRACT NO. _____
PROJECT NO. _____
SHEET NO. _____ OF _____

CLARENCE J. HARRIS
REGISTERED PROFESSIONAL ENGINEER
NO. 11512
EXPIRES 12/31/08

091 55



SHOP BILL										
LINE NO.	QTY	DESCRIPTION	SIZE	GRADE	MARK	PLACEMENT	WELD SPEC.	WELD APP.	WELD NO.	
1	1	PLATE GROSS								
2	1	PL 1x18	68	3/4	FF18	V	M270-50W			
3	1	PL 1x18	68	3/4	FF18	V	M270-50W			
4	1	PL 1x24	68	3/4	FF18	V	M270-50W			
5	2	PL 1x8	3	68	C2	V	M270-50W			
6	1	PL 1x18	3	68	C2	V	M270-50W			
7	1	PL 1x18	2	68	A2	V	M270-50W			
8	2	PL 1x8	2	68	C2	V	M270-50W			
9	2	PL 1x18	1	68	C2	V	M270-50W			
10	2	PL 1x8	3	8	A1	FF	M270-50W			
11	6	PL 1x8	3	8	D1	FF-2-E	M270-50W			
12	1	PL 1x8	3	8	H2	FF-2-E	M270-50W			
13	2	F BOLT	3/4	0	2 1/2		A307			
14	4	F BOLT	3/4	0	3 1/2		A307			
15	4	F BOLT	3/4	0	4		A307			
16	DOMESTIC MATERIAL									



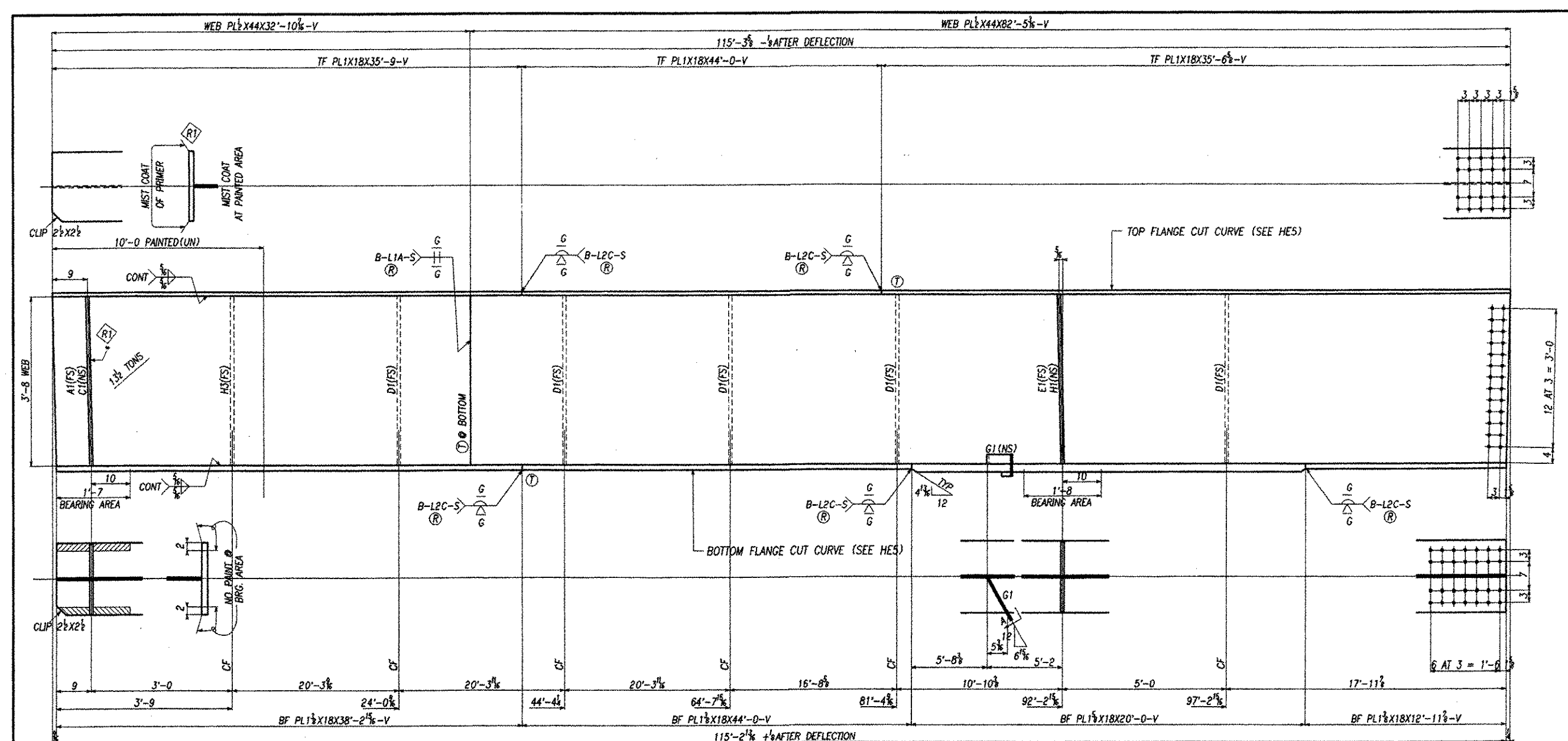
STA 9+19.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (HWY 4)
 HUNTINGTON, VT
 CHITTENDEN COUNTY

DESIGNED BY: HOSCH
 DRAWN BY: HOSCH
 CHECKED BY: HOSCH
 DATE: 11/11/07

PROJECT: BRIDGE # 42 EAST STREET (HWY 4)
 CLASS: FLOOD DAMAGE
 LOCATION: HUNTINGTON, VT
 CONTRACTOR: CAROLINA UTILITY CORPORATION

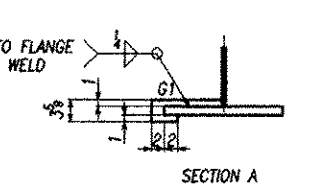
SCALE: 1/4" = 1'-0"

09355



ASTERSK * = DENOTES CONTACT SURFACE W/ PRIMER ONLY - * NOTE C *

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 FEB 7 - 2007
 RESUBMIT APPROVED
 BY DATE 2/22/07

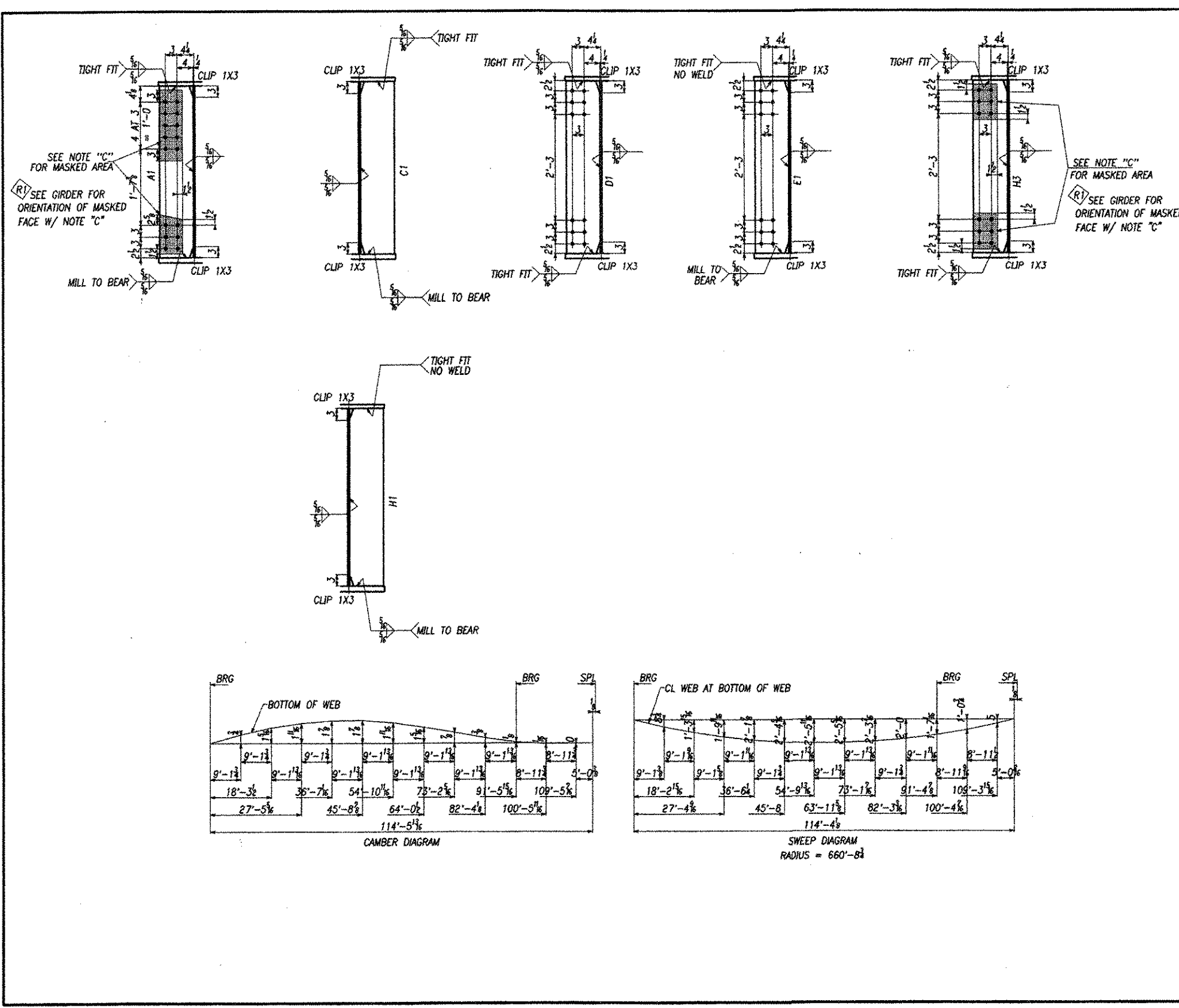


STA 9+18.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (HWY 4)
 HUNTINGTON, VT
 CANTONMENT COUNTY

DESIGNED BY	DATE	SCALE
CHECKED BY	DATE	SCALE
APPROVED BY	DATE	SCALE

PROJECT: BRIDGE # 42 EAST STREET (HWY 4)
 LOCATION: HUNTINGTON, VT
 CONTRACTOR: CARROLLA STEEL CORPORATION

09455



SHOP BILL

QTY	DESCRIPTION	UNIT	REMARKS	QTY	UNIT	QTY	UNIT
1	PLATE GIRDER						
1	PL 1818	35	B 17248	V	M270-SOM	268301	3
1	PL 1818	44	D 17248	V	M270-SOM	268301	7
1	PL 1818	35	68 17248	V	M270-SOM	268301	8
1	PL 1818	30	74 17248	V	M270-SOM	268301	4
1	PL 1818	44	D 17248	V	M270-SOM	268301	3
1	PL 1818	20	D 17248	V	M270-SOM	268301	7
1	PL 1818	22	118 17248	V	M270-SOM	268301	5
1	PL 1844	30	106 17248	V	M270-SOM	268301	14
1	PL 1844	82	28 17248	V	M270-SOM	268301	15
1	PL 1885	3	A1 17248	V	M270-SOM	268301	5
1	PL 1885	3	B 17248	V	M270-SOM	268301	5
1	PL 1885	3	D1 17248	V	M270-SOM	268301	5
1	PL 1885	3	E1 17248	V	M270-SOM	268301	5
1	PL 1885	3	H3 17248	V	M270-SOM	268301	5
1	PL 1885	3	H1 17248	V	M270-SOM	268301	5
1	PL 1825	1	08 01 17248	V	M270-SOM	268301	5

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OK'D BY: OK'D BY:

FEB 7 - 2007

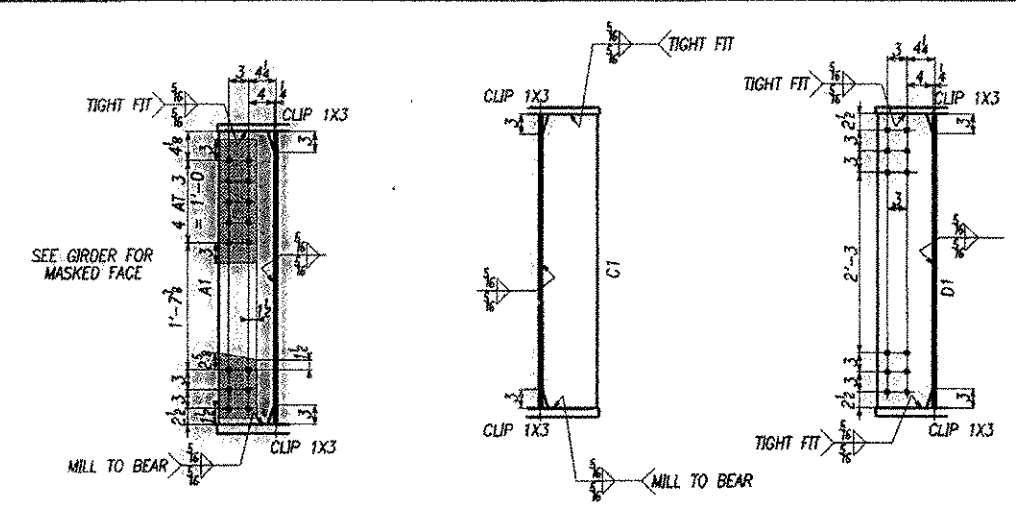
RESUBMIT: APPROVED:

BY: DATE:

STA 9+18.57 TO STA 11+01.07
BRIDGE # 42 EAST STREET (TH 4)
HARTINGTON, VT
CANTONMENT COUNTY

DESIGNED BY: <u> </u>	CHECKED BY: <u> </u>
DRAWN BY: <u> </u>	APPROVED BY: <u> </u>
SCALE: <u> </u>	DATE: <u> </u>

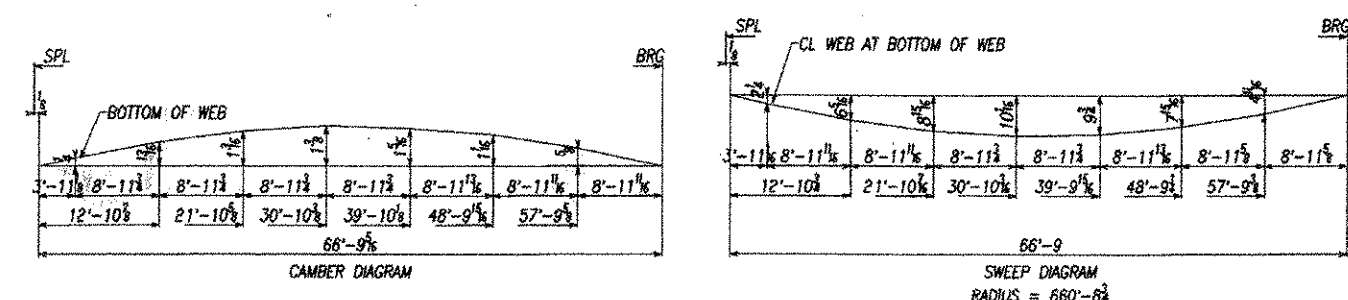
09555



SHOP BILL

QTY	UNIT	DESCRIPTION	QTY	UNIT	DESCRIPTION	QTY	UNIT	DESCRIPTION	QTY	UNIT	DESCRIPTION	QTY	UNIT	DESCRIPTION	QTY	UNIT	DESCRIPTION
1	PL	6	7718	V	M270-50W												
1	PL	6	8718	V	M270-50W												
1	PL	6	8718	V	M270-50W												
2	PL	3	65	E2	V	M270-50W											
1	PL	3	65	O2	V	M270-50W											
1	PL	2	65	A2	V	M270-50W											
2	PL	2	65	C2	V	M270-50W											
2	PL	1	65	G2	V	M270-50W											
1	PL	3	8	A1	V	M270-50W											
1	PL	3	8	C1	V	M270-50W											
4	PL	3	8	D1	V	M270-50W											
2	F	1	0	P2		A307											
4	F	1	0	S2		A307											
4	F	1	0	T		A307											
1	PL	1	05	G1	V	M270-50W											

DOMESTIC MATERIAL



STA 94+19.57 TO STA 11+01.07
 BRIDGE # 42 LAST STREET (THRU)
 HENNINGTON, VT
 CHITTENDEN COUNTY

DESIGNED BY: [Signature]
 CHECKED BY: [Signature]
 DATE: [Date]

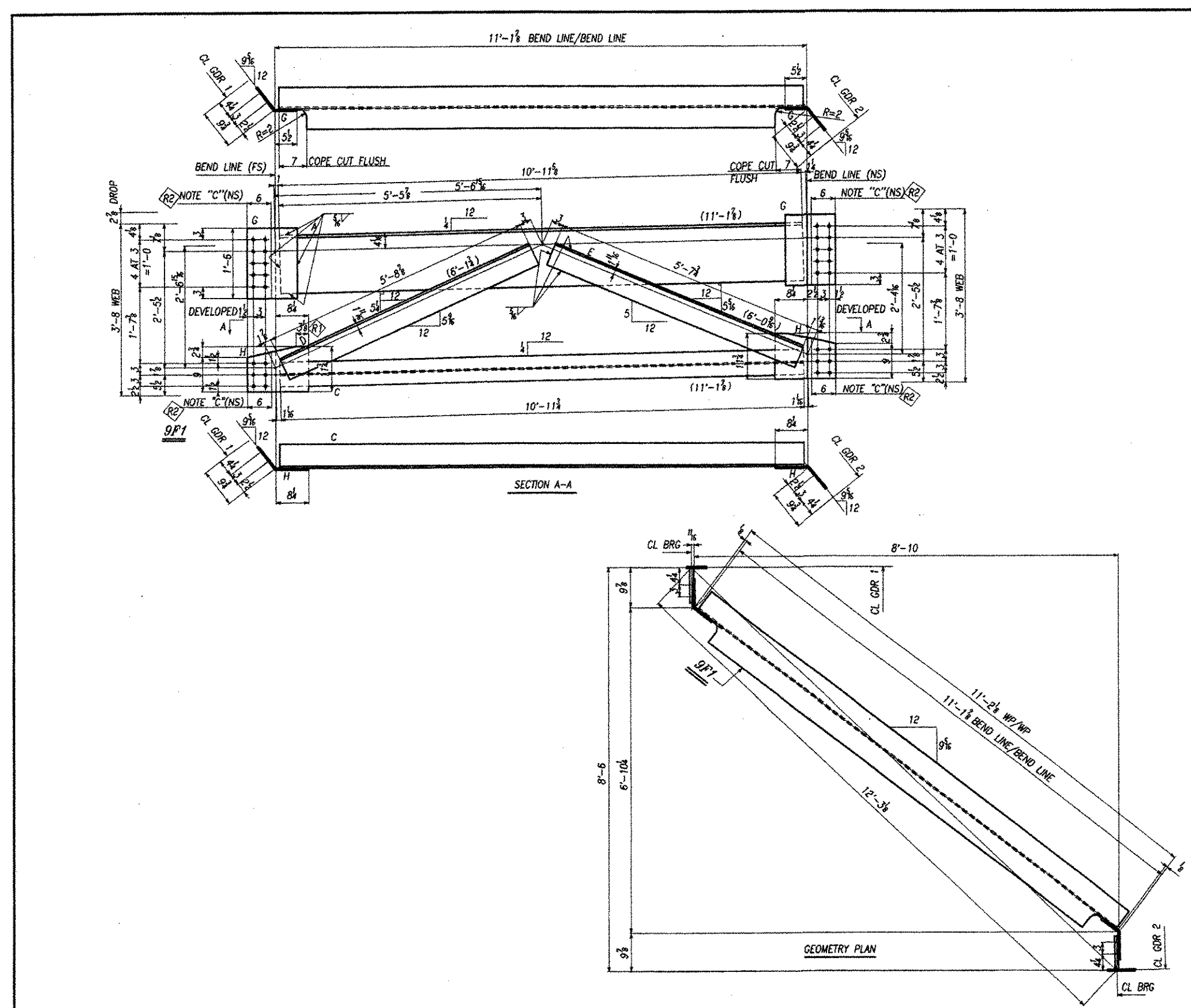
PROJECT: [Project Name]
 DRAWING NO.: [Drawing No.]

SCALE: [Scale]

DATE: [Date]

BY: [Signature]

097 ss



SHOP BILL

ITEM NO.	QUANTITY	DESCRIPTION	UNIT	PRICE	TOTAL	REMARKS	DATE	BY
1	1	CROSSFRAME						
2	1	WT 15648 S 10 I 15	A V			42 70'-SOM 270116 1		
3	1	WT 6313 10 I 11	C V			42 70'-SOM 270116 1		
4	1	L 60874 5 R 2	D V			42 70'-SOM 263007 1		
5	1	L 60874 5 R 2	C V			42 70'-SOM 263007 1		
6	2	PL 14124 1 6	D V			42 70'-SOM 263007 1		
7	2	PL 14124 1 6	H V			42 70'-SOM 263007 1		

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CK'D BY CK'D BY

FEB 7 - 2007

RESUBMIT APPROVED

BY DATE 2/22/07

STA 9+18.57 TO STA 11+01.07
BRIDGE # 42 EAST STREET (HW 4)
HUNTINGTON, VT
CHITTENDEN COUNTY

DESIGNED BY: PROJECT: BRIDGE # 42 EAST STREET (HW 4)

DRAWN BY: SHEET NO.:

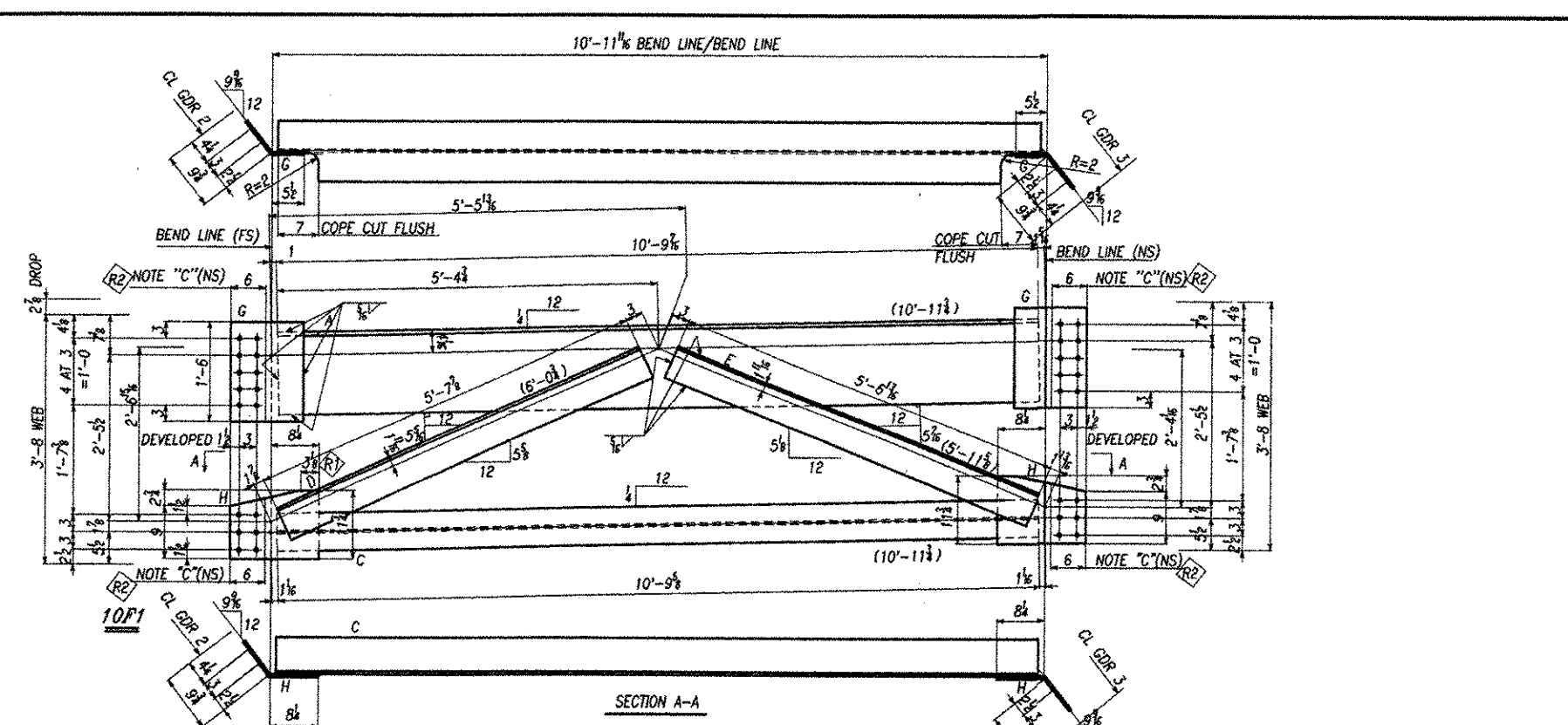
CHECKED BY: DATE:

SCALE:

APPROVED BY: TITLE:

DATE:

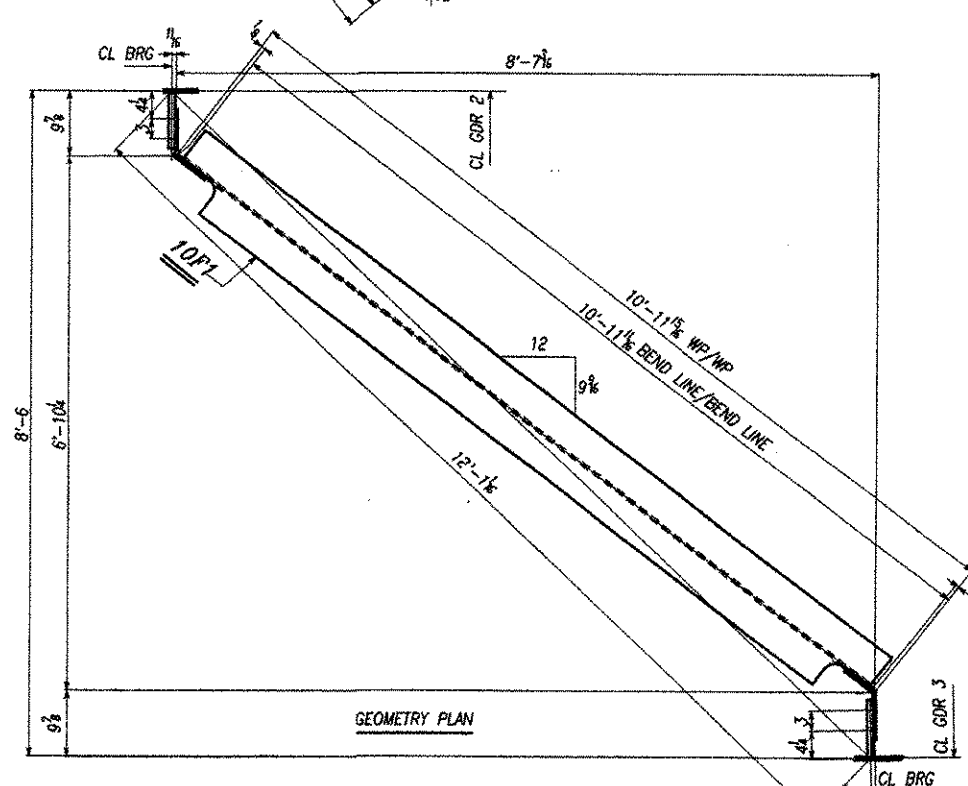
09855



SHOP BILL

NO.	QTY	DESCRIPTION	LENGTH	REMARKS	UNIT	PRICE	TOTAL
1	1	CROSSFRAMES					
2	1	WT 130x6.5 10 PK		A V	MT 70-SW	270.11	
3	1	WT 8x12 10 PK		C V	MT 70-SW	270.11	
4	1	L 8x8x5 5 PK		D V	MT 70-SW	270.11	
5	1	L 8x8x5 5 PK		E V	MT 70-SW	270.11	
6	2	PK 3x12 1 PK		G V	BEND MT 70-SW	270.11	
7	2	PK 3x12 1 PK		H V	BEND MT 70-SW	270.11	

DOMESTIC MATERIAL



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CK'D BY _____ CK'D BY LSB

FEB 7 - 2007

RESUBMIT _____ APPROVED _____

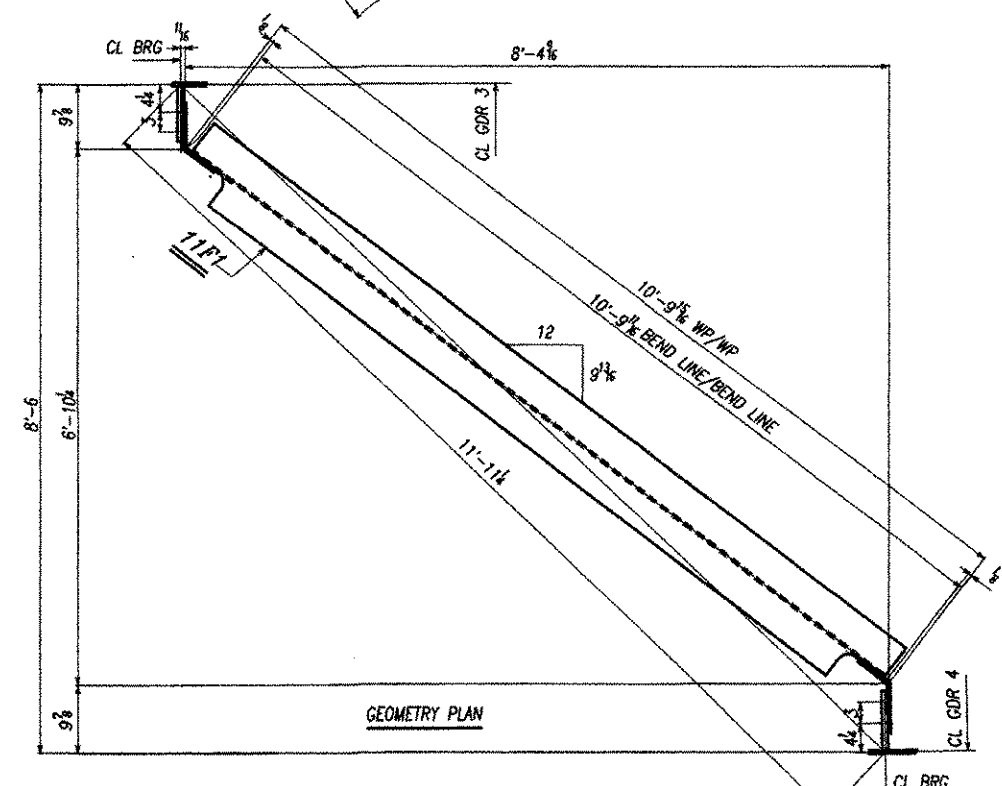
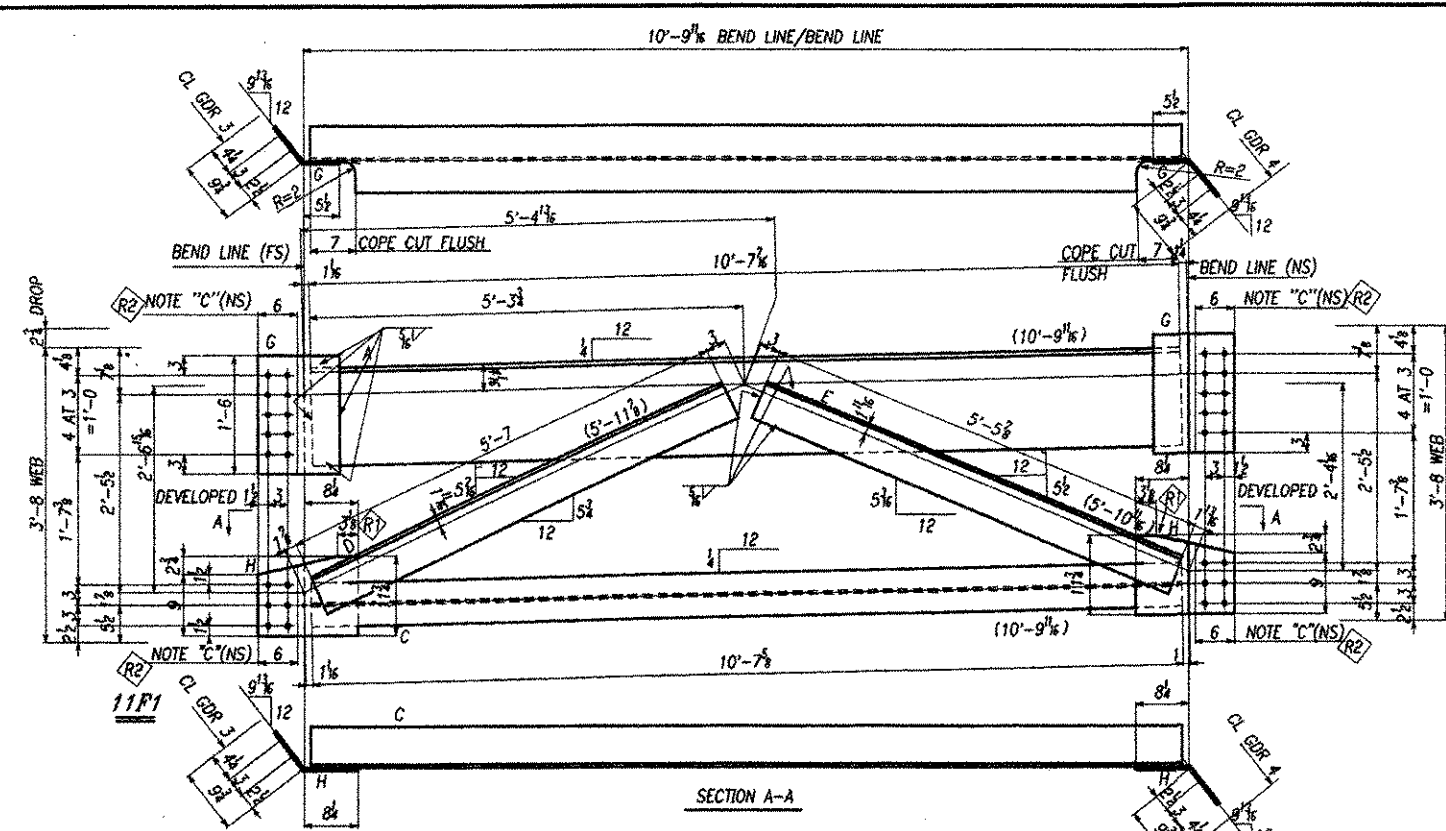
BY MSJ DATE 2/22/07

STA 94+0.57 TO STA 114+01.07
BRIDGE # 42 EAST STREET (TH 4)
HUNTINGTON, VT
CHITTENDEN COUNTY

DESIGNED BY: MSJ CHECKED BY: LSB

PROJECT: BRIDGE # 42 EAST STREET (TH 4)
CLASS: STANDARD
LOCATION: HUNTINGTON, VT
CONTRACT: PROJECT CONSTRUCTION
CONTRACTOR: CARDINAL STEEL CORPORATION

DATE: 2/22/07



SHOP BILL

ITEM NO.	QTY	DESCRIPTION	UNIT	REMARKS	DATE	BY
1	1	CHASSIS BRIMS				
2	1	WT 10x10x10	A	V	10-20-SW	27012
3	1	WT 8x12x10	C	V	10-20-SW	27012
4	1	WT 8x12x10	D	V	10-20-SW	26307
5	1	WT 8x12x10	E	V	10-20-SW	26307
6	2	PL 5x12x1/8	G	V	10-20-SW	26307
7	2	PL 5x12x1/8	H	V	10-20-SW	26307

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OK'D BY: [Signature]

FEB 7 - 2007

RESUBMIT: _____ APPROVED: [Signature]

BY: [Signature] DATE: 2/22/07

STA 9+19.57 TO STA 11+01.07
BRIDGE # 42 EAST STREET (HWY 4)
HARRINGTON, VT
CHITTENDEN COUNTY

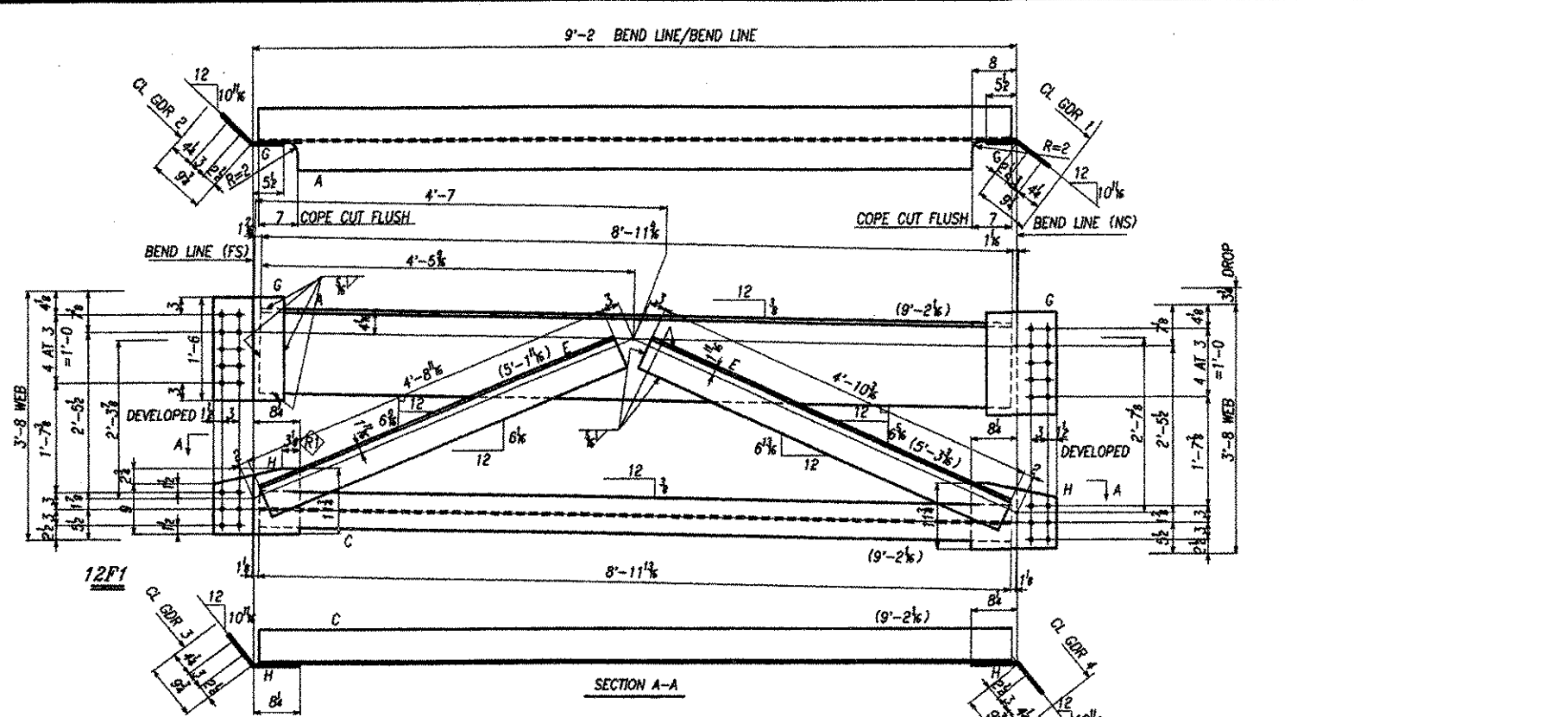
DESIGNED BY: [Blank] CHECKED BY: [Blank]

DATE: [Blank] SCALE: [Blank]

PROJECT: BRIDGE # 42 EAST STREET (HWY 4)
CLASS: 3 (LOCAL ROAD)

CONTRACTOR: PARENT CONCRETE, INC.
CAROLINA STEEL CORPORATION

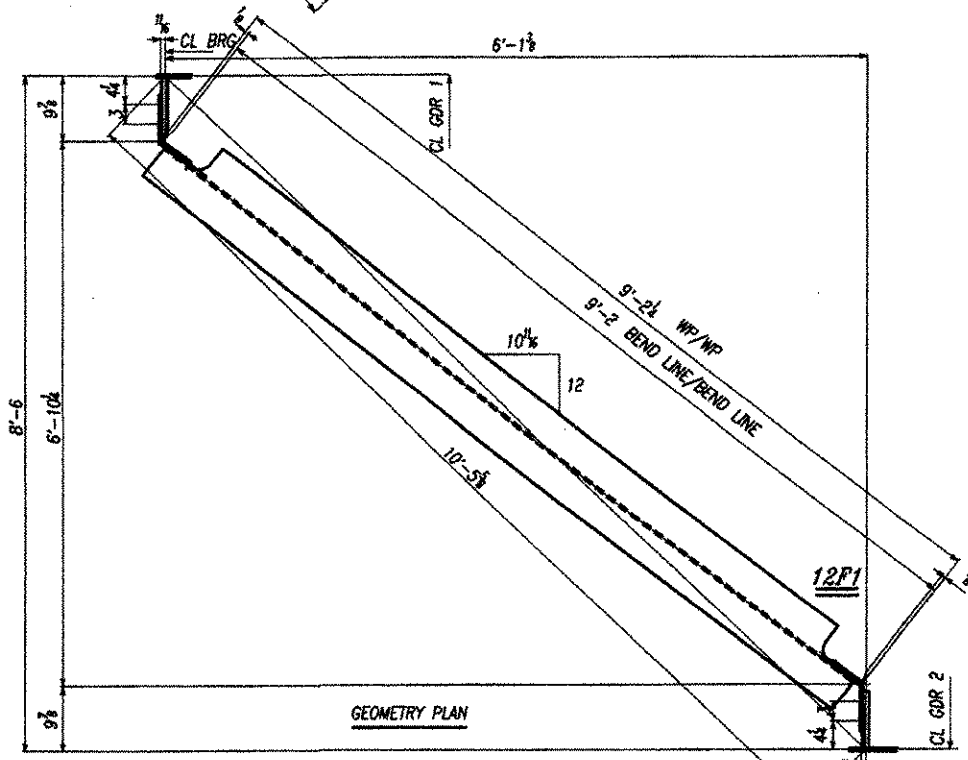
DATE: [Blank] BY: [Blank]



SHOP BILL

QUANTITY	UNIT	DESCRIPTION	LENGTH	WEIGHT	REMARKS	UNIT PRICE	TOTAL PRICE
1	TRUSS	CROSSFRAMES					
1	WT	12X48.5	8.115	A	V	ME70-SOM	27016.1
1	WT	6X12	8.115	C	V	ME70-SOM	17016.1
1	L	6X6	4.108	D	V	ME70-SOM	17016.1
1	L	6X6	4.108	E	V	ME70-SOM	17016.1
2	PL	3X12	1.6	G	V	BEND ME70-SOM	28016.1
2	PL	3X12	1.6	H	V	BEND ME70-SOM	28016.1

DOMESTIC MATERIAL



RECEIVED

OK'D BY: _____ OK'D BY: *[Signature]*

FEB 7 - 2007

RESUBMIT: _____ APPROVED: *[Signature]*

BY: *[Signature]* DATE: 2/22/07

STA 9+18.07 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (THRU 4)
 HUNTINGTON, VT

CHITTENDEN COUNTY

DESIGNED BY: *[Signature]* DATE: 1/24/07

CHECKED BY: *[Signature]* DATE: 1/24/07

PROJECT: BRIDGE # 42 EAST STREET THRU 4

CLASS: CLASS 3 LOCAL ROAD

LOCATION: HUNTINGTON, VT

OWNER: CHITTENDEN COUNTY

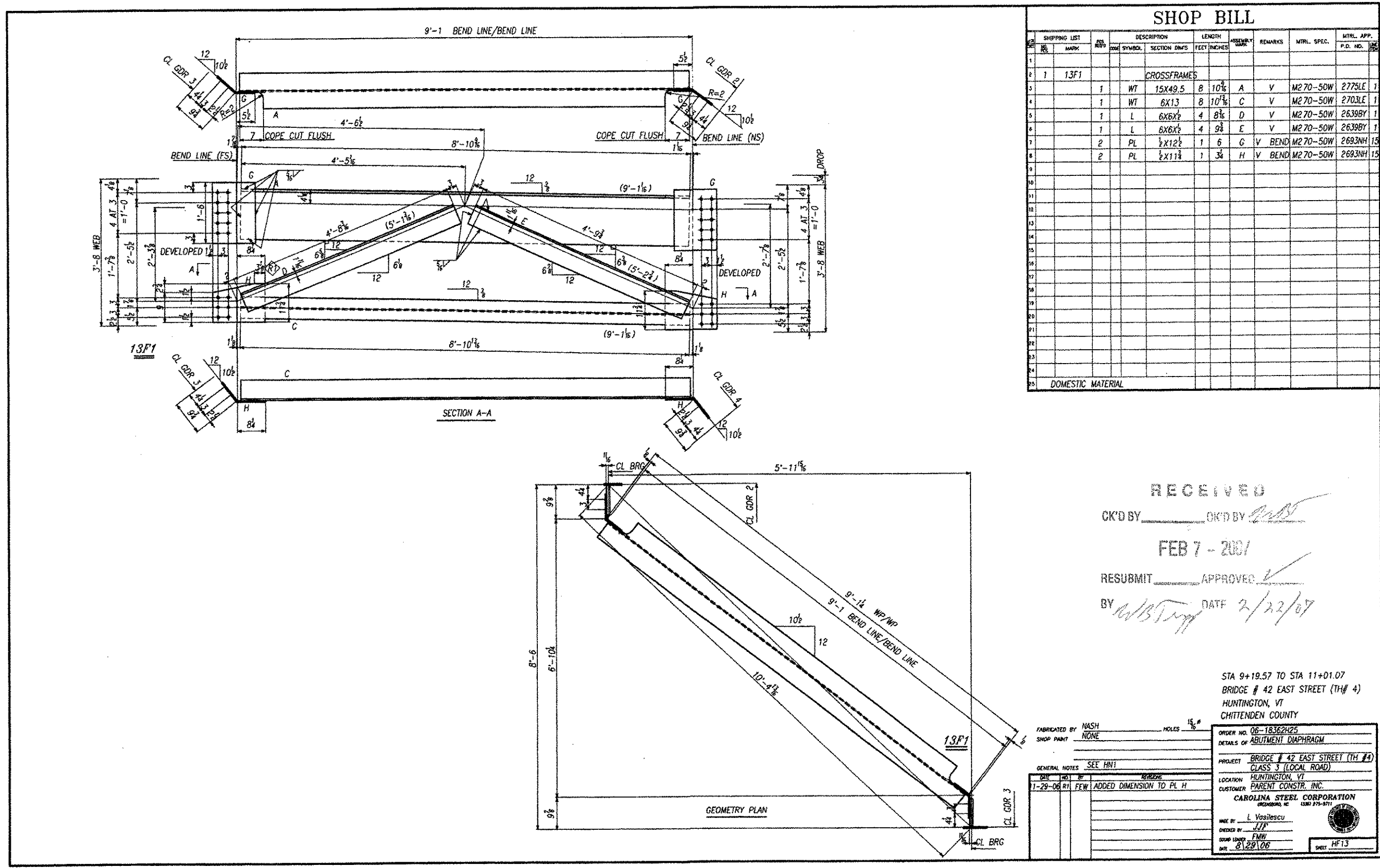
DESIGNER: PARSONS ENGINEERING

CANTONMENT: CANTONMENT CORPORATION

SCALE: 1" = 10'-0"

DATE: 1/24/07

BY: *[Signature]*



SHOP BILL

QTY	DESCRIPTION	UNIT	REMARKS	WH. SPEC.	UNIT PRICE	TOTAL AMT.
1	CROSSFRAME					
1	WT 12449.5	B 100	A V	ME 70-50M	27726.1	
1	WT 6813	B 100	C V	ME 70-50M	17308.1	
1	L 6885	4 80	D V	ME 70-50M	28387.1	
1	L 6885	4 80	E V	ME 70-50M	28387.1	
2	PL 12425	11 2	G V	ME 70-50M	28387.1	
2	PL 12425	11 2	H V	ME 70-50M	28387.1	

DOMESTIC MATERIAL

RECEIVED

CR'D BY _____ EN'D BY *CLB*

FEB 7 - 2007

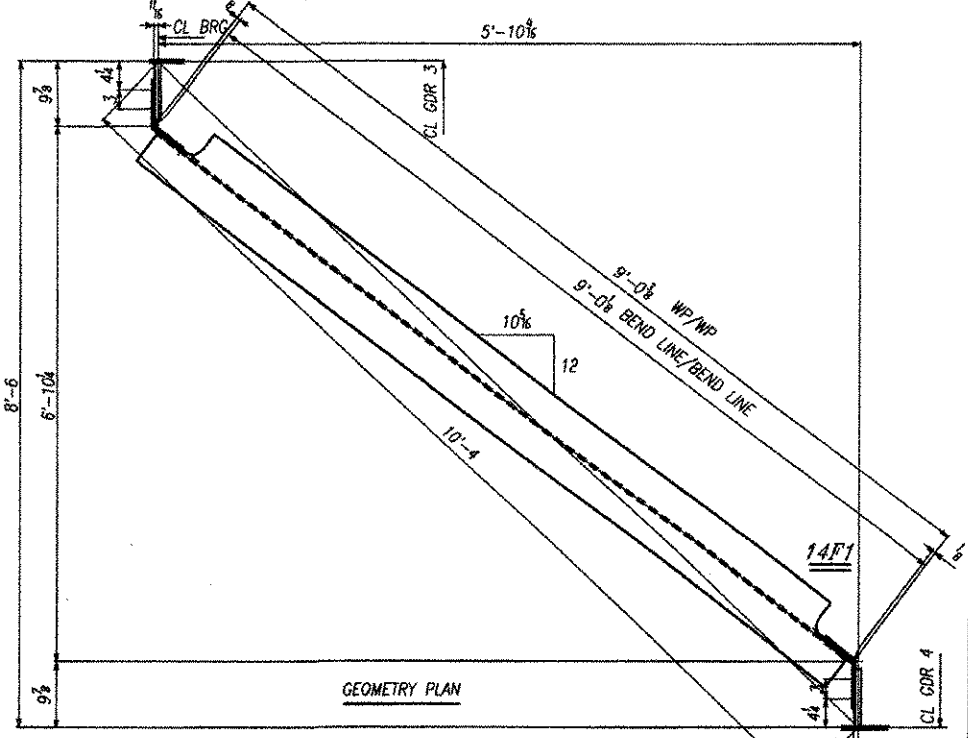
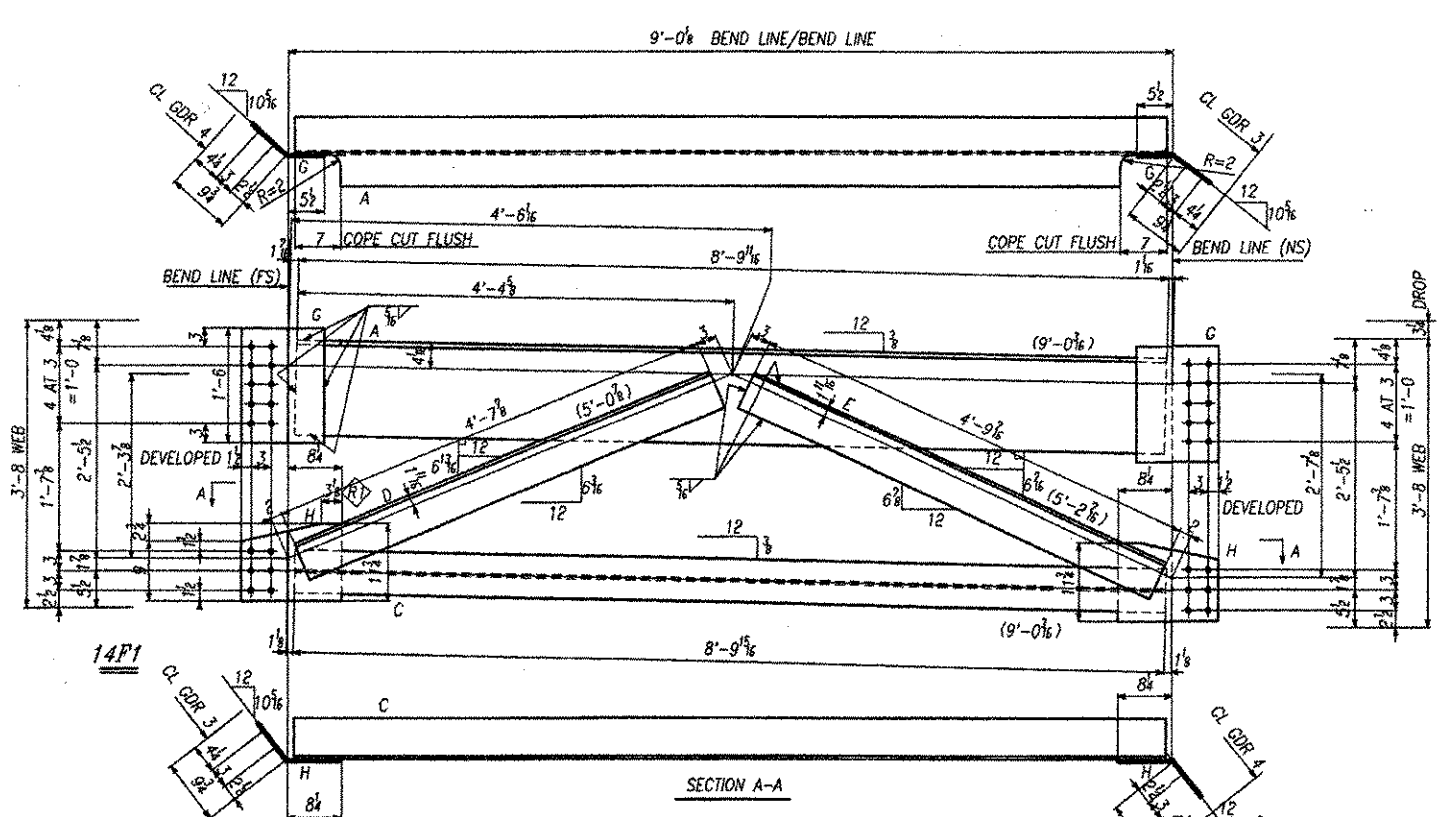
RESUBMIT _____ APPROVED *[Signature]*

BY *[Signature]* DATE 2/27/07

STA 9+10.57 TO STA 11+01.07
BRIDGE # 42 EAST STREET (TH# 4)
HUNTINGTON, VT
CHITTENDEN COUNTY

DESIGNED BY	DATE	CHECKED BY	DATE
APPROVED BY	DATE	SCALE	

PROJECT: BRIDGE # 42 EAST STREET (TH# 4)
 LOCATION: HUNTINGTON, VT
 DRAWING: BRIDGE ELEVATION, R.C.
 CAROLINA STEEL CORPORATION
 HUNTINGTON, VT
 DATE: 2/27/07
 DRAWN BY: [Signature]
 CHECKED BY: [Signature]



SHOP BILL

NO.	QTY	DESCRIPTION	UNIT	PRICE	TOTAL	REMARKS
1	1	CROSSBRACE	EA			
2	1	WT 15449.5	EA			
3	1	WT 8213	EA			
4	1	L 8887	EA			
5	1	L 6857	EA			
6	2	PL 3411	EA			
7	2	PL 3411	EA			

RECEIVED

CK'D BY _____ OK'D BY *WJ*

FEB 7 - 2007

RESUBMIT _____ APPROVED _____

BY *WJ* DATE *2/2/07*

STA 9+19.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (14' W)

CHITTENDEN COUNTY

DESIGNED BY: *SEE PLAN*

CHECKED BY: *SEE PLAN*

DATE: *2-2-07*

PROJECT: BRIDGE # 42 EAST STREET (14' W)

CONTRACT: *SEE PLAN*

CONTRACTOR: *SEE PLAN*

SCALE: *SEE PLAN*

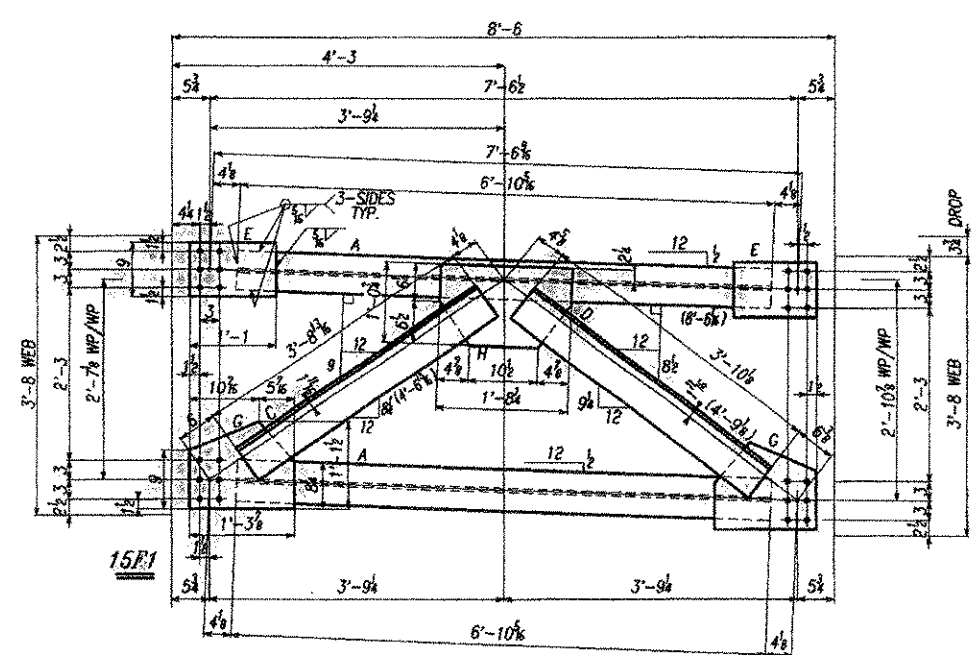
DATE: *2-2-07*

BY: *WJ*

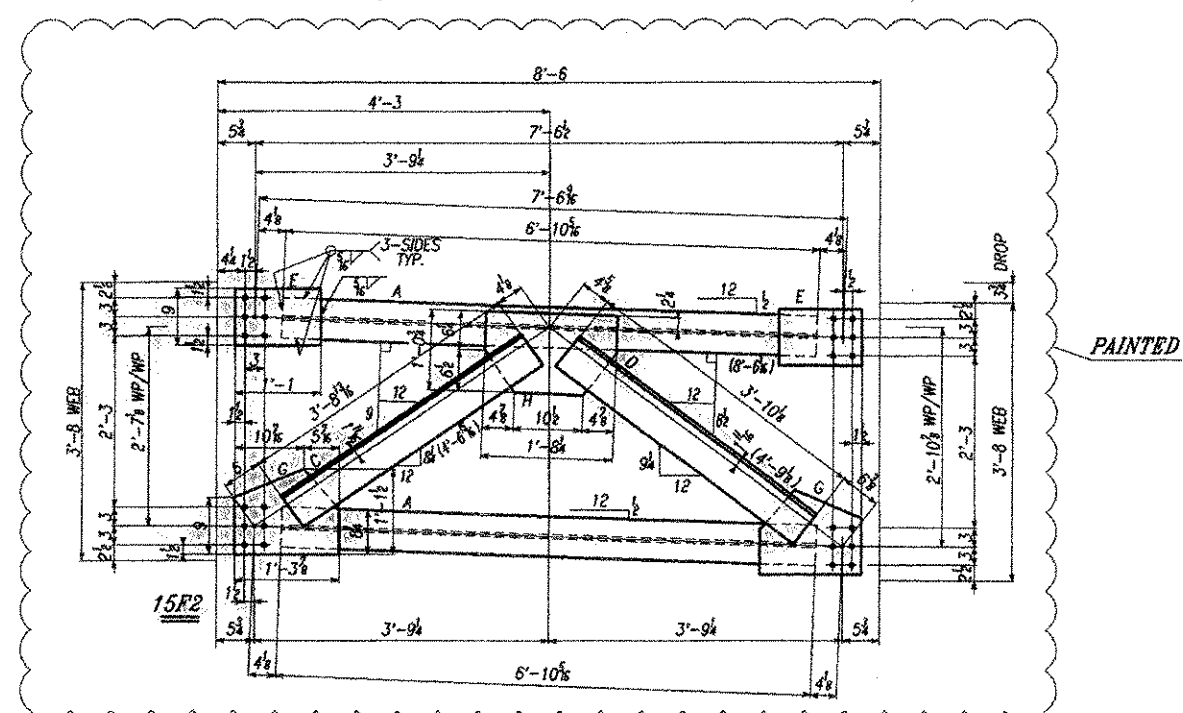
IN CHARGE: *WJ*

APP: *WJ*

DATE: *2-2-07*



SHOP BILL									
NO.	QTY.	DESCRIPTION	UNIT	REMARKS	DATE	BY	CHKD.	APPROV.	DATE
24	15F1	CROSSFRAMES							
48	WT	6x12	8	10 1/2	A	V	M270-50W		
24	L	6x8x5	3	10 1/2	C	V	M270-50W		
24	L	6x8x5	3	10 1/2	D	V	M270-50W		
48	PL	3/8	11	1	E	V	M270-50W		
48	PL	3/8	11	3/8	F	V	M270-50W		
24	PL	3x12x2	1	8 1/2	H	V	M270-50W		
4	15F2	CROSSFRAMES							
8	WT	6x12	8	10 1/2	A	V	M270-50W		
4	L	6x8x5	3	10 1/2	C	V	M270-50W		
4	L	6x8x5	3	10 1/2	D	V	M270-50W		
8	PL	3/8	11	1	E	V	M270-50W		
8	PL	3/8	11	3/8	F	V	M270-50W		
4	PL	3x12x2	1	8 1/2	H	V	M270-50W		



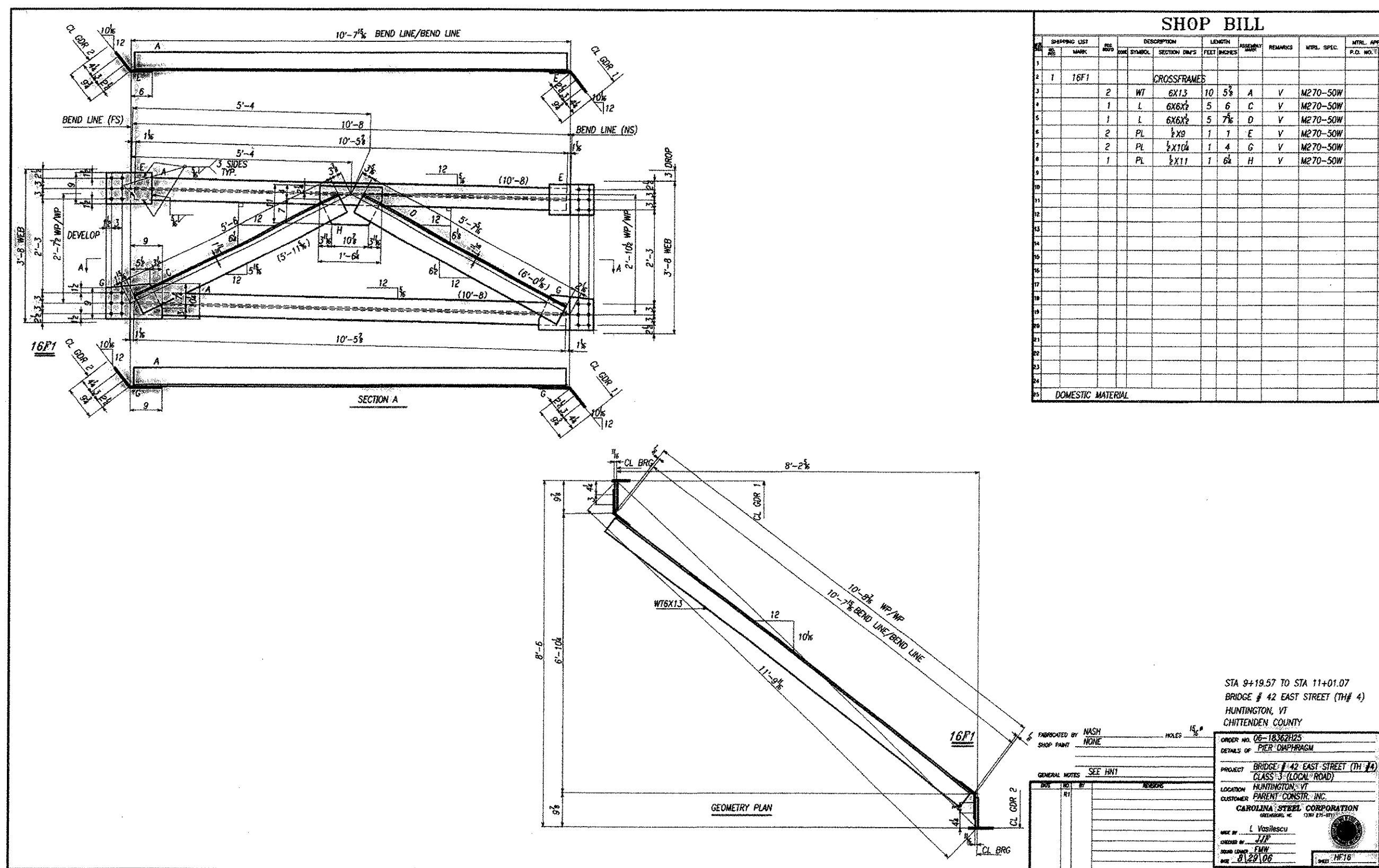
STA 8+19.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (NH 4)
 HUNTINGTON, VT
 CHITTENDEN COUNTY

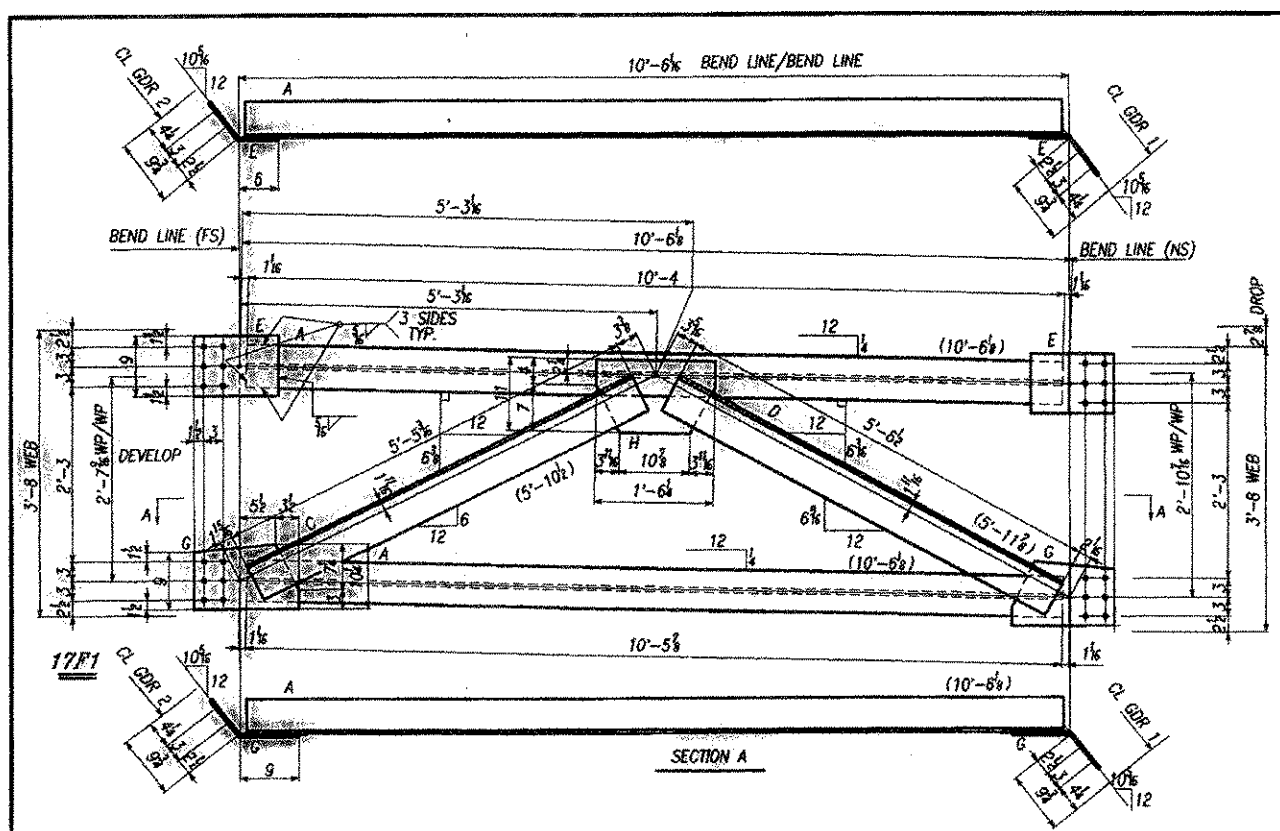
DESIGNED BY: [Signature]
 CHECKED BY: [Signature]
 DATE: [Date]

CONTRACT NO. [Number]
 SHEET NO. [Number]

APPROVED BY: [Signature]
 TITLE: [Title]

DATE: [Date]

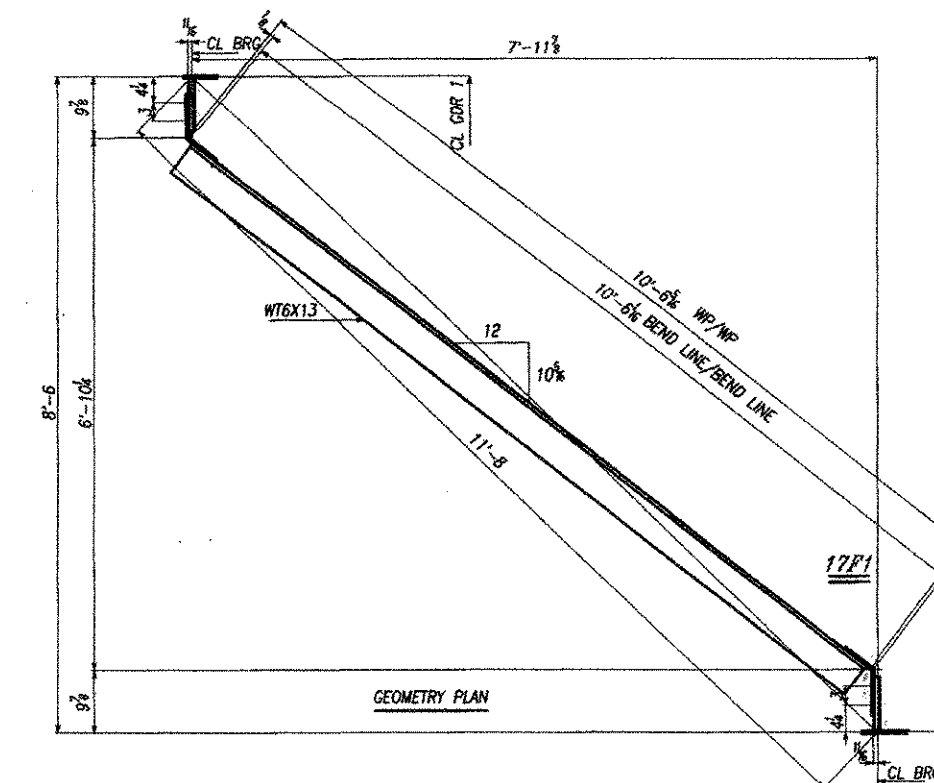




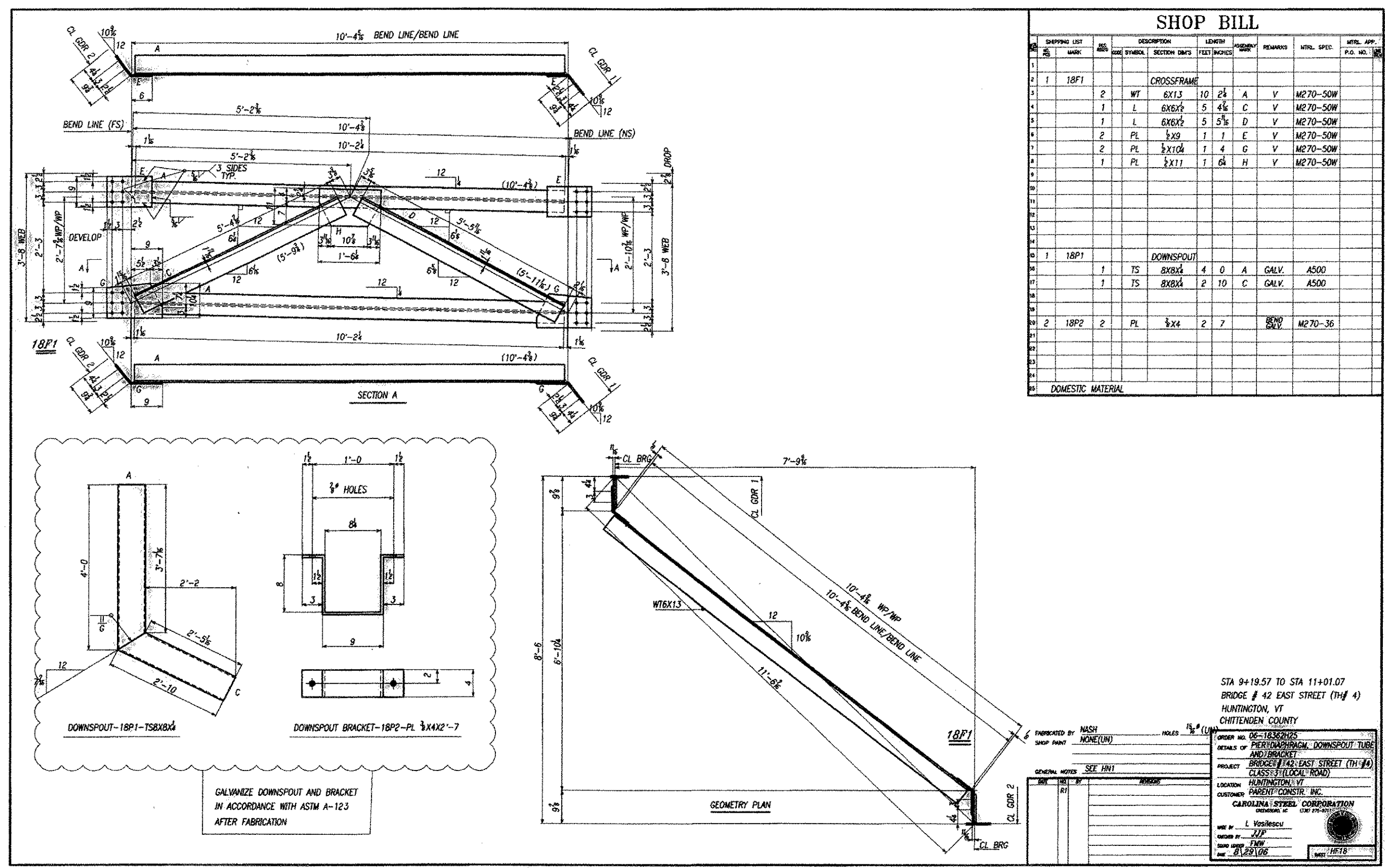
SHOP BILL

NO.	QTY	DESCRIPTION	UNIT	REMARKS	DATE
1	1	CROSSFRAME			
2	2	WT 6x12	10	A V	M270-50W
1	1	L 6x6	5	C V	M270-50W
1	1	L 6x6	5	D V	M270-50W
2	2	PL 3x3	1	E V	M270-50W
2	2	PL 3x3	1	F V	M270-50W
1	1	PL 3x3	1	G V	M270-50W

DOMESTIC MATERIAL



STA 9419.57 TO STA 11401.07
 BRIDGE # 42 EAST STREET (PM 4)
 HUNTINGTON, VT
 CHITTENDEN COUNTY
 DRAWN BY: [Signature]
 CHECKED BY: [Signature]
 PROJECT: BRIDGE # 42 EAST STREET (PM 4)
 CLASS: LOCAL ROAD
 CONTRACTOR: [Name]
 DATE: [Date]
 SCALE: [Scale]



SHOP BILL

ITEM NO.	QUANTITY	DESCRIPTION	UNIT	REMARKS	DATE	BY
1	1	TRUSS	EA			
2	2	WT 6013	120	24 A V	M270-50W	
3	1	L 6013	5	48 C V	M270-50W	
4	1	L 6013	5	54 D V	M270-50W	
5	2	PL 1/2"	1	1 C V	M270-50W	
6	2	PL 1/2"	1	4 C V	M270-50W	
7	1	PL 1/2"	1	4 H V	M270-50W	
8	1	TRUSS	EA			
9	1	TRUSS	EA			
10	1	TRUSS	EA			
11	1	TRUSS	EA			
12	1	TRUSS	EA			
13	1	TRUSS	EA			
14	1	TRUSS	EA			
15	1	TRUSS	EA			
16	1	TRUSS	EA			
17	1	TRUSS	EA			
18	1	TRUSS	EA			
19	1	TRUSS	EA			
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96	1	TRUSS	EA			
97	1	TRUSS	EA			
98	1	TRUSS	EA			
99	1	TRUSS	EA			
100	1	TRUSS	EA			

STA 94+19.57 TO STA 114+01.07
 BRIDGE # 42 EAST STREET (HJ 4)
 HARRINGTON, VT
 CHITTENDEN COUNTY

DESIGNED BY: [Signature]
 DRAWN BY: [Signature]
 CHECKED BY: [Signature]
 DATE: [Date]

APPROVED BY: [Signature]
 TITLE: [Title]

DRIVING LIST				
LINE NO.	QTY	DRY LENGTH	MEMBER SUPPORTED	CONNECTED TO
ABUTMENT DIMENSIONS (2)				
180	1/2	13	1/2 GUSSET PL	1/2 CONN PL
INTERMEDIATE (2)				
96	1/2	1	1/2 GUSSET PL	1/2 CONN PL-INTER
576	1/2	1	1/2 GUSSET PL	1/2 CONN PL-INTER
PIER DIMENSIONS (2)				
72	1/2	13	1/2 GUSSET PL	1/2 CONN PL
FIELD SPICES				
160	1/2	24	1/2 TOP FLG SPLICE PL	1 TOP FLG PL
224	1/2	44	1/2 BOT. FLG SPLICE PL	1/2 BOT. FLG PL
208	1/2	14	1/2 WEB SPLICE	1/2 WEB PL

ALL BOLTS TYPE 3 (UNLESS NOTED)
 * DENOTES LOCATIONS SHOWN ON NET OF TYPE 1 BOLTS
 TYPE 1 * SEE NET
 TYPE 1 AT PIERED CONN PLATES ON
 LOW SIDE OF CROSSFRAMES, (4 LOCATIONS) * SEE NET
 TYPE 3
 GALVANIZE DOWNSPOUT ANCHORS
 AND WASHERS IN ACCORDANCE WITH
 ASTM A513 (A513D ME30)

SHOP BILL									
LINE NO.	QTY	DESCRIPTION	UNIT	REMARKS	MTL SPEC.	WELL SPEC.			
1	200	18P13 200 F HSBT	1/2	0 23	TYPE 1	M164			
2	140	18P14 140 F HSBT	1/2	0 2	TYPE 1	M164			
3	300	18P15 300 F WASH	1/2		TYPE 1	F436			
4	500	18P17 500 F HSBT	1/2	0 2	TYPE 3	M164			
5	200	18P18 200 F HSBT	1/2	0 23	TYPE 3	M164			
6	200	18P19 200 F HSBT	1/2	0 23	TYPE 3	M164			
7	166	18P11 166 F HSBT	1/2	0 25	TYPE 3	M164			
8	1278	18P13 1278 F WASH	1/2		TYPE 3	F436			
NOTE TO BE AS-BUILT									
9	4	18P20 4 F BOLT	1/2	0 3	DOMESTIC	SEK			
10	4	18P21 4 F WASH	1/2		SEK	F436			
11	4	18P22 4 F NUT	1/2		SEK	A563			
DOMESTIC MATERIAL									

STA 9+19.57 TO STA 11+01.07
 BRIDGE # 42 EAST STREET (TH# 4)
 HUNTINGTON, VA
 CHESTERFIELD COUNTY

APPROVED BY: [Signature] DATE: [Date]
 CHECKED BY: [Signature] DATE: [Date]
 PROJECT: BRIDGE # 42 EAST STREET (TH# 4)
 CLASS: LOCAL ROAD
 CONTRACTOR: CAROLINA STEEL CORPORATION
 DRAWING NO: [Number]
 SHEET NO: [Number] OF [Total]
 DATE: [Date]



State of Vermont
PDD/Structures Design Section
National Life Building - Drawer 33
Montpelier, VT 05633-5001
www.aot.state.vt.us

Agency of Transportation

[phone] 802-828-2621
[fax] 802-828-3566
[td] 800-233-0191

DATE: January 8, 2007

Merrimack Sheet Metal
119 Hall Street
Concord, NH 03301

Project Name: **Huntington** Project #: **BRO 1445(29)**

Structure Identification: **East Street (TH #4) over Huntington River**

The following Expansion Joint details and Welding Procedures [Item # 516.10, Bridge Expansion Joint] for the above project (Vendor's Job # 0922) transmitted with Parent Construction Inc. letter dated 12/11/06 have been reviewed and are being returned herewith.

Sheets: Shop Drawings E1, E2, F1, F2, F3, and F4 and Welding Procedures DS-7, DS-13 and DS-16
are approved or approved "as noted".

Upon receipt of these "approved and "approved as noted" drawings, please make changes as appropriate and submit white prints for our use in the record plans for this project.

You must provide written notice to this office as to the date fabrication represented by these drawings will begin. That notice must be received at least seven days prior to that date, as per Specification 506.03. Any material fabricated prior to the notification date is subject to rejection without further cause.

Sincerely,

Warren Tripp, Project Manager

Attachments

cc: Resident Engineer Dale Norton, w/ prints
Shop Inspector Jeff Clark w/prints
Parent Construction, Inc. w/prints
Construction Division - letter only
Materials & Research Section (C&IA Unit) - letter only
Files (Structures & Central)



PROJECT NAME _____
 PREQUALIFIED JOINT WELDING PROCEDURE PROJECT NUMBER _____
 PROCEDURE SPECIFICATION

Material specification A36; A572; A588 SINGLE PASS ONLY
 Welding process FOAW
 Manual or machine SEMI-AUTOMATIC
 Position of welding FLAT
 Filler metal specification AWS 5.20
 Filler metal classification ALLOY RODS DUAL SHIELD IT 71 ULTRA
 Flux N/A
 Shielding gas CO₂ Flow rate .35 CFH
 Single or multiple pass MULTIPLE
 Single or multiple arc SINGLE ELECTRICAL STICK-OUT 3/8"-3/4"
 Welding current DC
 Polarity REVERSE
 Welding progression N/A
 Root treatment Back-Gouge root to sound metal before welding second side.
 Preheat and interpass temperature 50 to 3/4" INCL .70 to 1 1/4" INCL .150 to 2 1/4" INCL
 Postheat treatment NONE
 Supported by WPS 007 and 008

WELDING PROCEDURE

Pass no.	Electrode size	Welding current		Travel speed I.P.M.	Joint detail
		Amperes	Volts		
a11	.045	220-240	26-28	14-16	
<p>RECEIVED DEC 14 2006 APPROVED DATE 12-18-06</p>					

This procedure may vary due to fabrication sequence, fit-up, pass size, etc., within the limitation of variables given in Section 5.

Procedure no. DS-7 Contractor Merrimack Sheet Metal, Inc.
 Revision no. _____ Authorized by Scott Blanchette
 Form E-2 Date 1/26/01

PROJECT NAME _____
 QUALIFIED JOINT WELDING PROCEDURE PROJECT NUMBER _____
 PROCEDURE SPECIFICATION _____

Material specification A36: A572-65RR
 Welding process FCAW
 Manual or machine SEMI-AUTOMATIC
 Position of welding FLAT for groove welds Horizontal for fillet welds 1G: 2F
 Filler metal specification AWS 5.20
 Filler metal classification ALLOY RODS DUAL SHIELD TIG WELDS (E71T-1)
 Flux N/A Flow rate 35 CFH
 Shielding gas CO₂
 Single or multiple pass SINGLE AND MULTIPLE
 Single or multiple arc SINGLE ELECTRICAL STICK-ONHT 3/8"-3/8"
 Welding current DC
 Polarity REVERSE
 Welding progression N/A
 Root treatment NONE
 Preheat and interpass temperature 50 to 3/4" INCL. 70 to 1 1/2" INCL. 150 to 2 1/2" INCL.
 Postheat treatment NONE
 Supported by WPS 007 and 008

WELDING PROCEDURE

DEC 14 2006

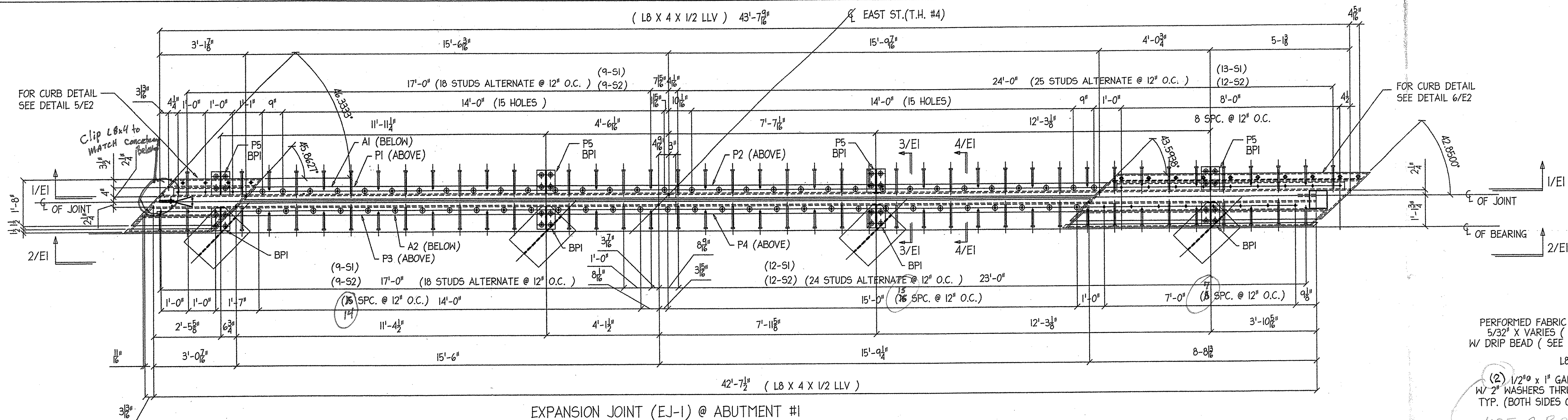
Pass no.	Electrode size	Welding current		Travel speed IPM	Weld size (S)	Joint detail
		Amperes	Volts			
GROOVE WELDS						
1	.045	220-240	26-28	16-18	3/16"	
1	.045	220-240	26-28	10-12	1/4"	
all	.045	220-240	26-28	14-16	5/16"	
FILLET WELDS						
FILLET WELDS SHALL EQUAL 1/4 OF t BUT NOT MORE THAN 3/8"						
SEE PROCEDURE DS-16 FOR PARAMETERS OF FILLET WELDS						

APPROVED BY: _____ DATE: 12-13-06

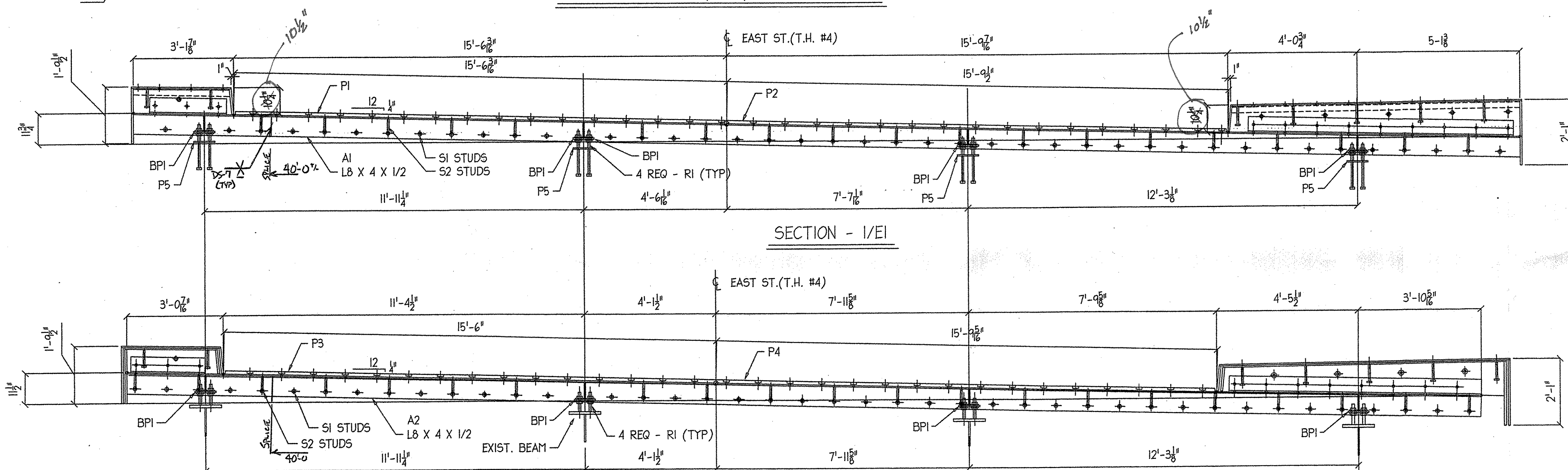
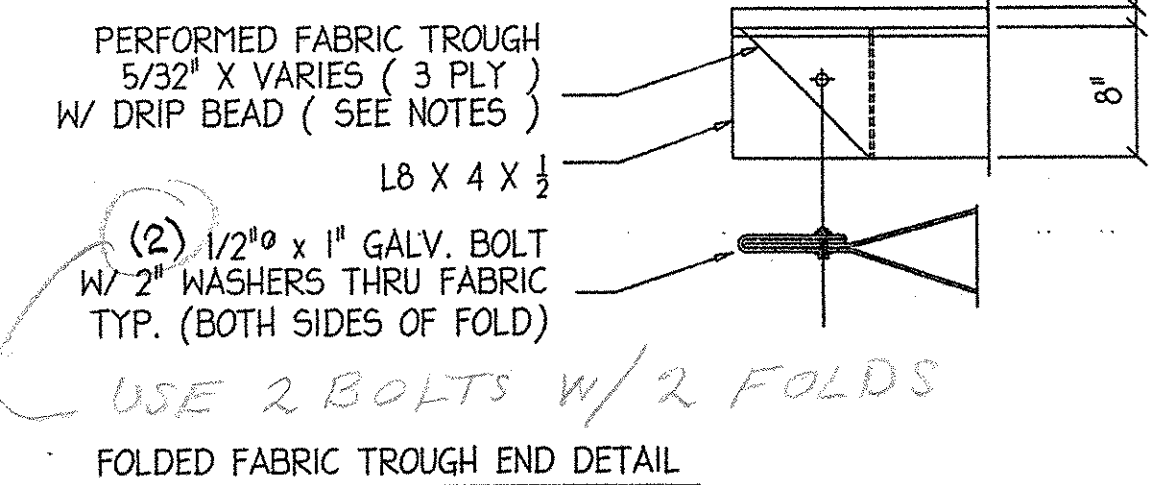
This procedure may vary due to fabrication sequence, fill-up, pass size, etc., within the limitation of variables given in Section 5.

Procedure no. DS-13 Contractor Merrimack Sheet Metal, Inc.
 Revision no. _____ Authorized by Sue Blawie
 Form E-2 Date 1/26/01

111 65

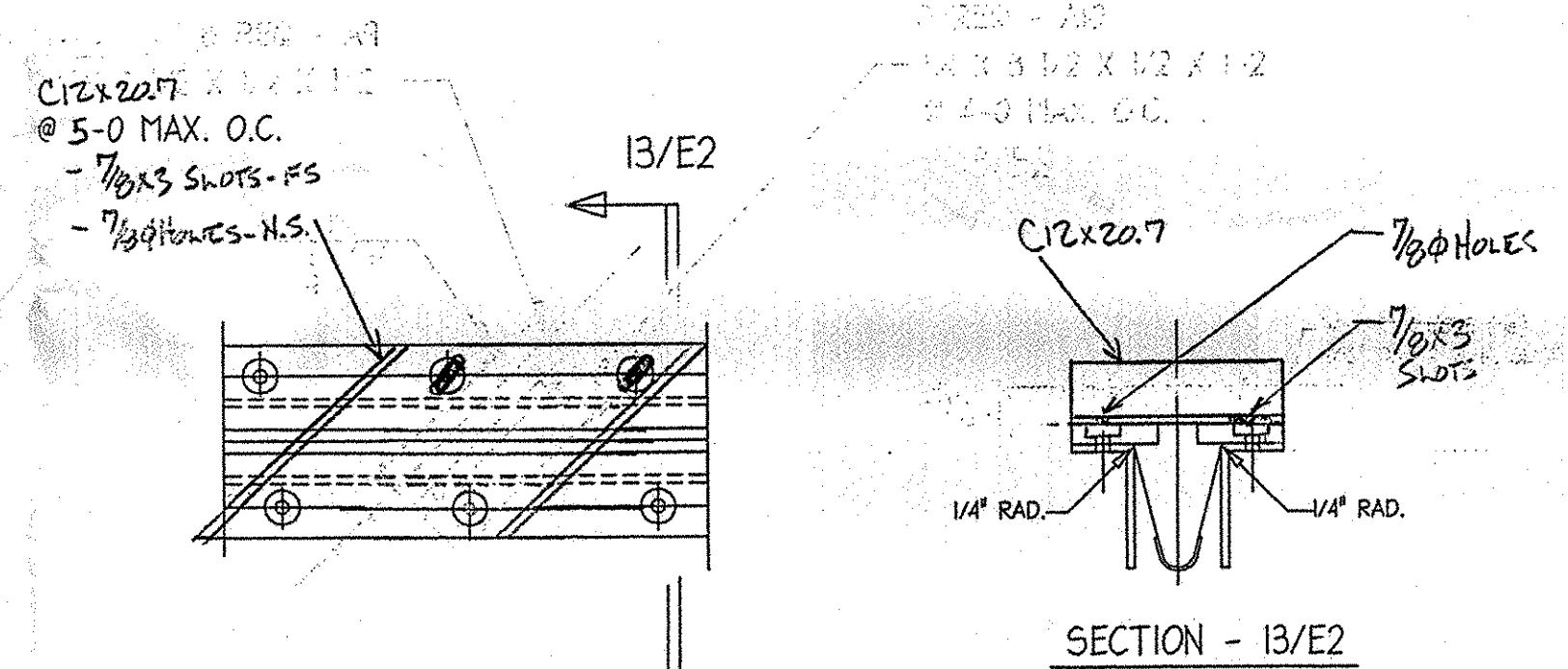


EXPANSION JOINT (EJ-1) @ ABUTMENT #1

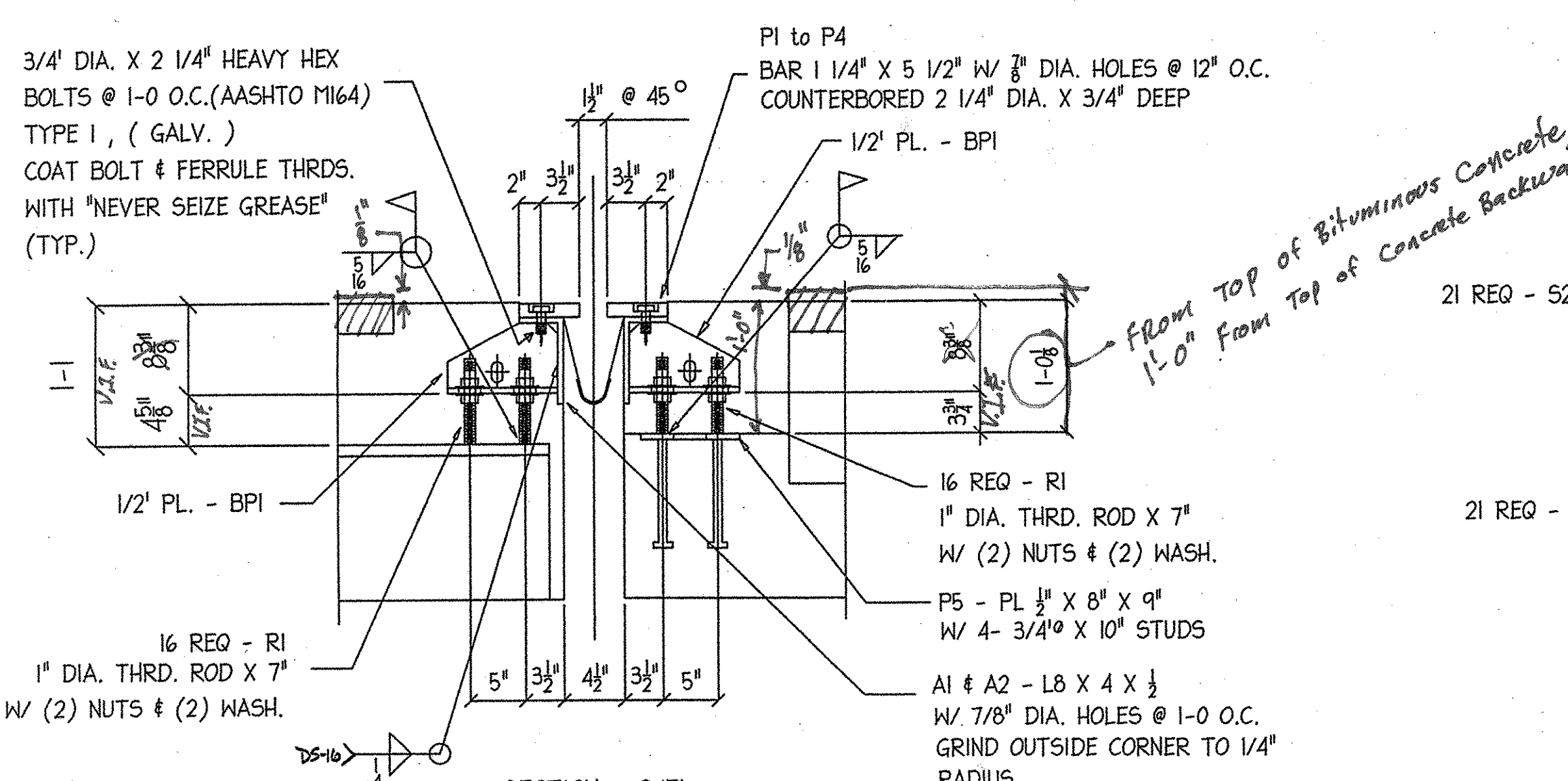


SECTION - 1/EI

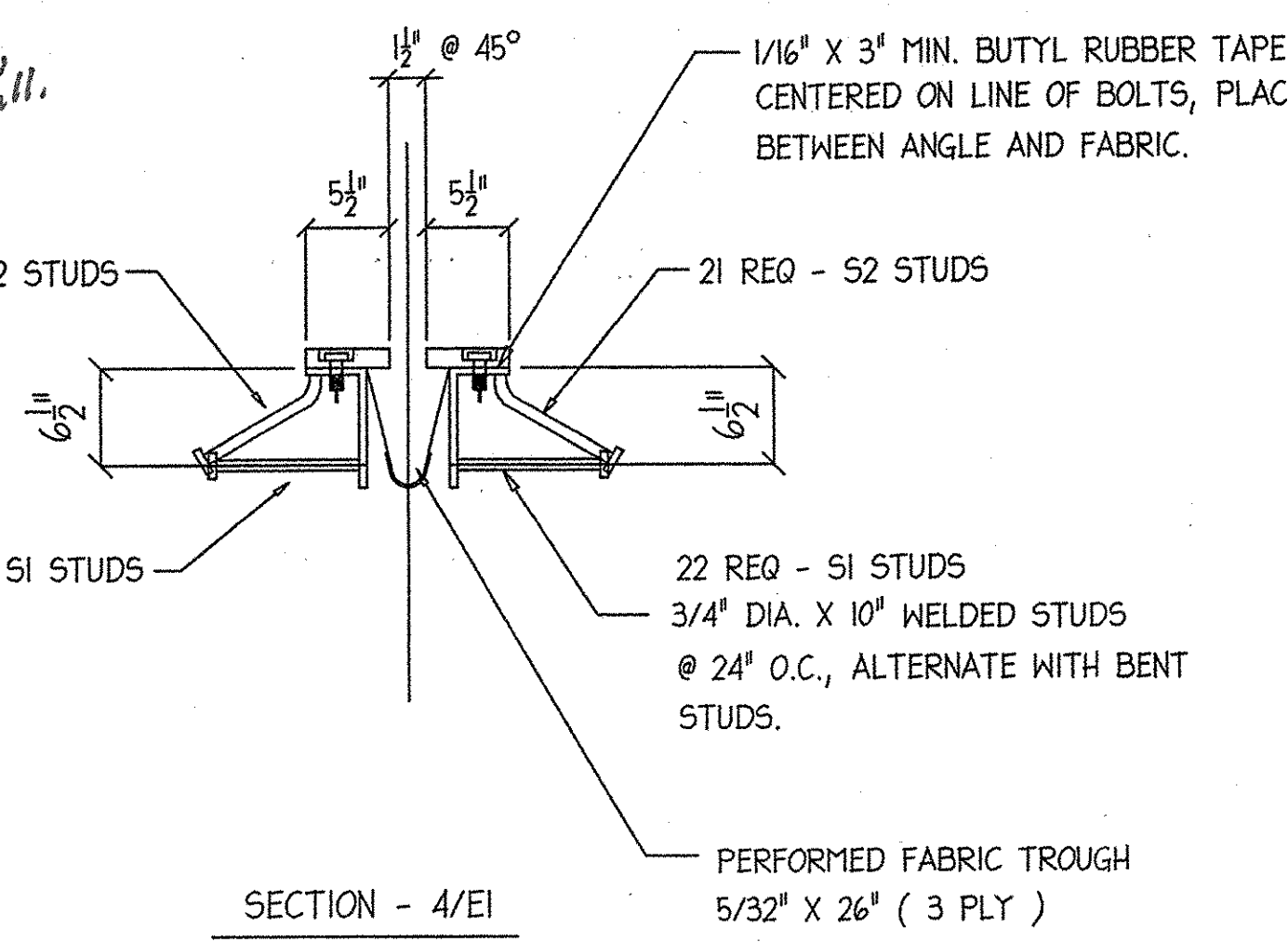
SECTION - 2/EI



PLAN - SHIPPING DEVICE



SECTION - 3/EI



SECTION - 4/EI

SHOP DRAWINGS ARE REVIEWED UNLESS NOTED OTHERWISE
 NO EXCEPTION TAKEN
 REVISION NOTED
 RESUBMISSION NOT REQUIRED
 REVISION NOTED
 RESUBMISSION REQUIRED
 REVISION NOTED
 RESUBMISSION REQUIRED
 DATE: January 2nd, 2007
 SIGNATURE: Scott Bruland
 REVIEW BY STARTED IS FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DESIGN. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONING, FABRICATION AND CONSTRUCTION METHODS. COORDINATION OF SUB-TENDERS, DETAIL DESIGN OF COMPONENTS, AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.

TEMP	"A" DIST.
0° F	2"
15° F	1 7/8"
30° F	1 11/16"
45° F	1 1/2"
60° F	1 5/16"
75° F	1 1/8"
90° F	1"
105° F	13/16"

"A" is the setting before dead loads are in place.

REV. NO.	DATE	DESCRIPTION
1	11-20-06	HOLES: 13/16" (U.N.O.)
2		PAINT:

STATE PROJECT # BRO 1445 (29)

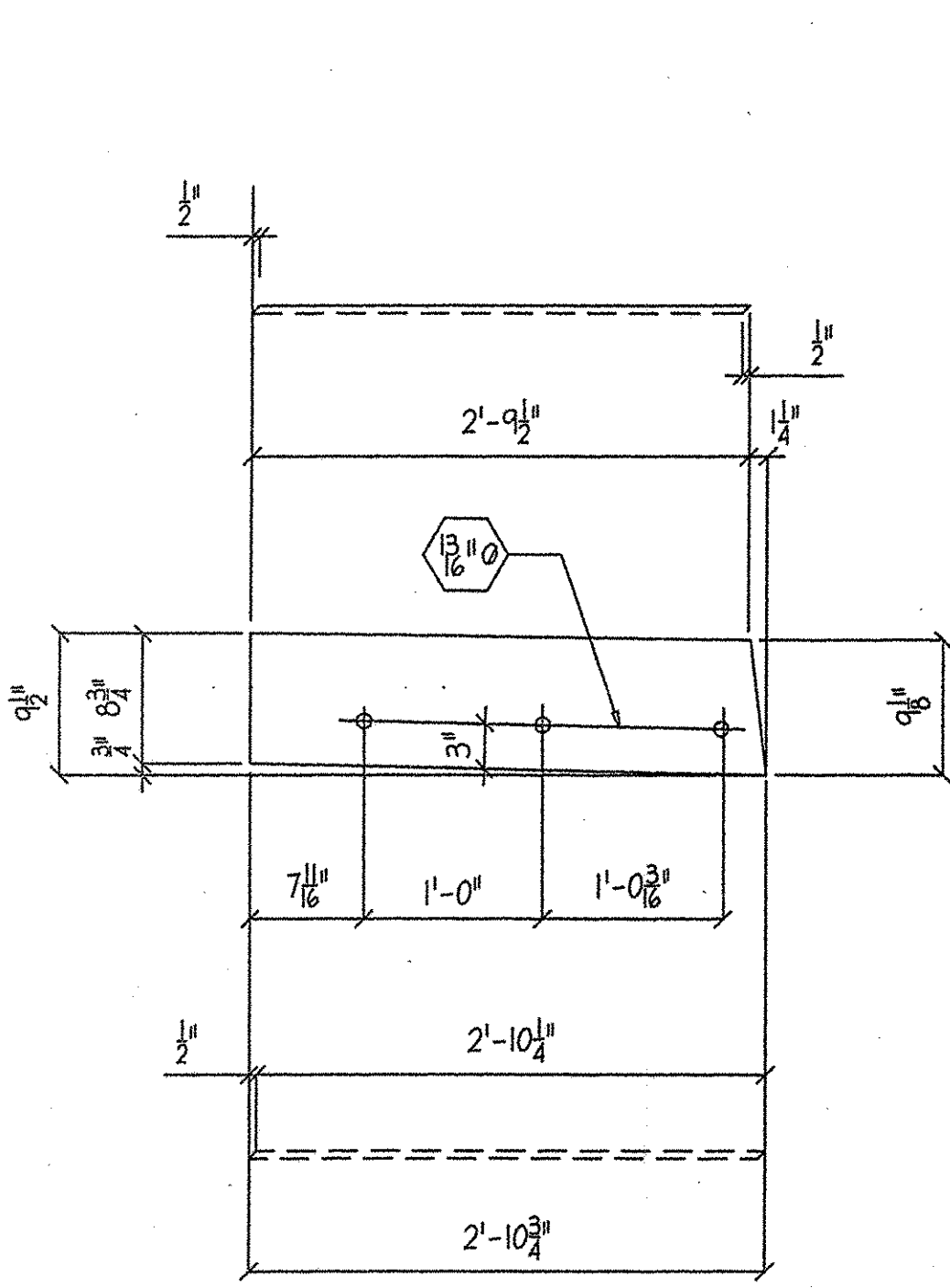
EXPANSION JOINT (EJ-1) @ ABUTMENT #1

EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

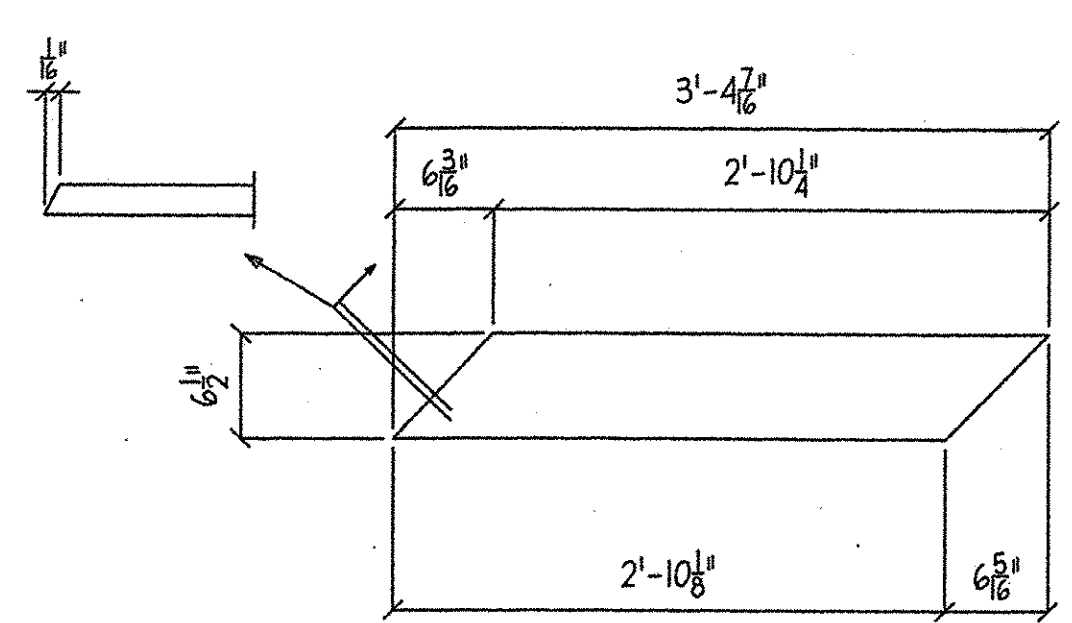
DATE SUB.	PROJECT	LOCATION	ARCHITECT
DATE APP.	ENGINEER	STATE OF VERMONT, AGENCY OF TRANSPORTATION	
DATE DIST.	CUSTOMER	PARENT CONSTRUCTION	

MERRIMACK MERRIMACK SHEET METAL
 119 HALL ST.
 CONCORD, N.H., 03301
 TEL. (603) 224-7766
 FAX. (603) 224-7925

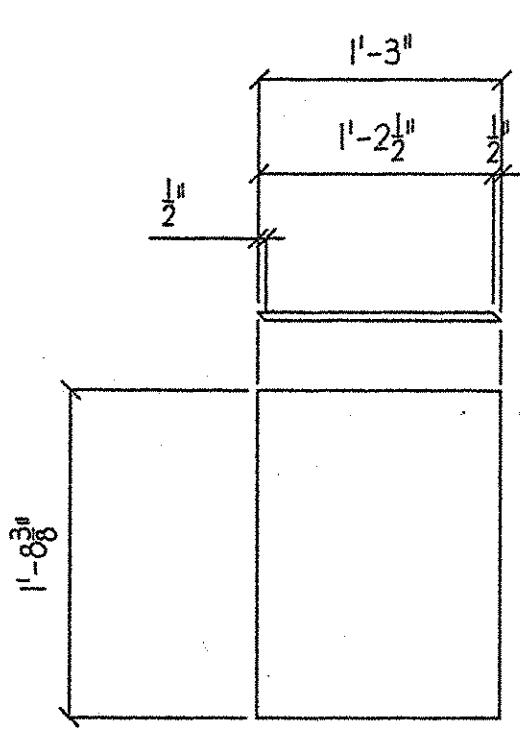
DR BY: KJM
 REV BY: JD
 JOB NO: 0922
 DWG NO: EI



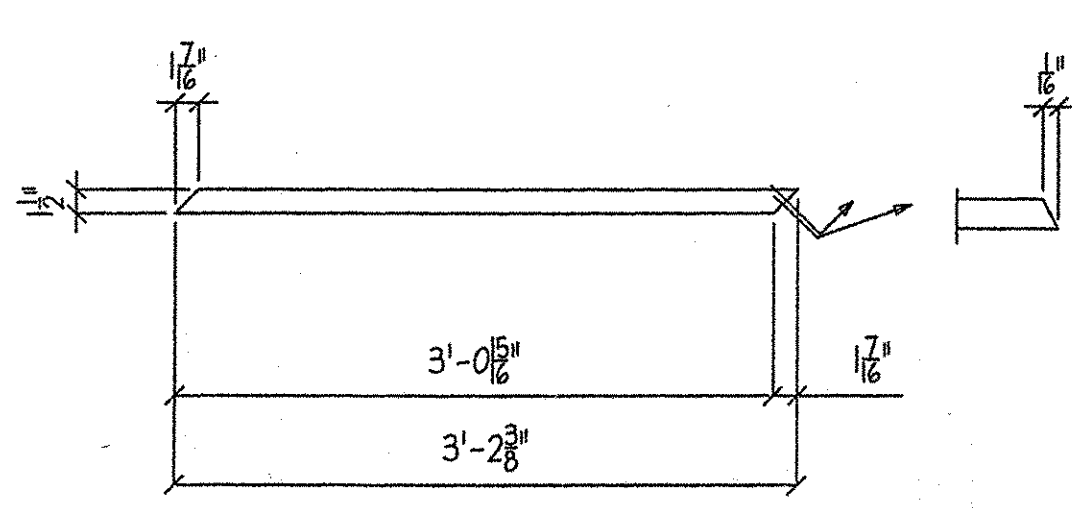
ONE REQ - PII



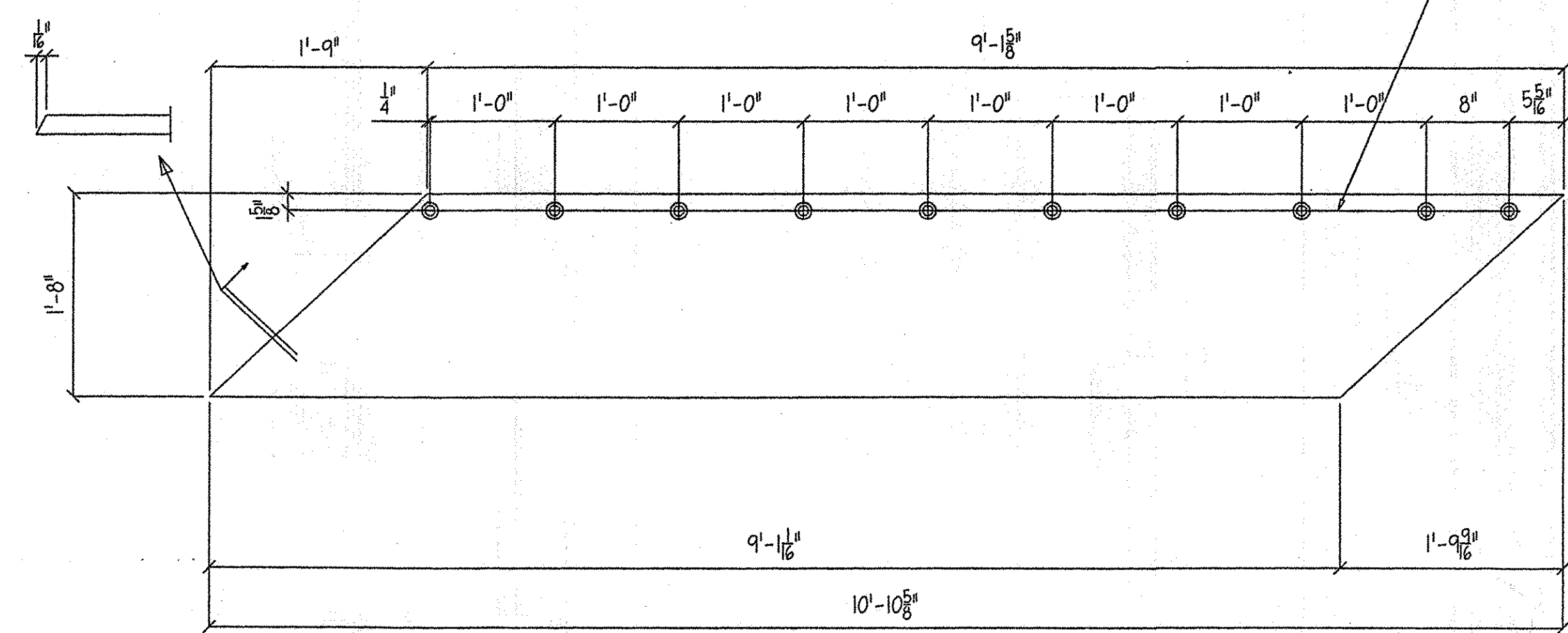
ONE REQ - PI2



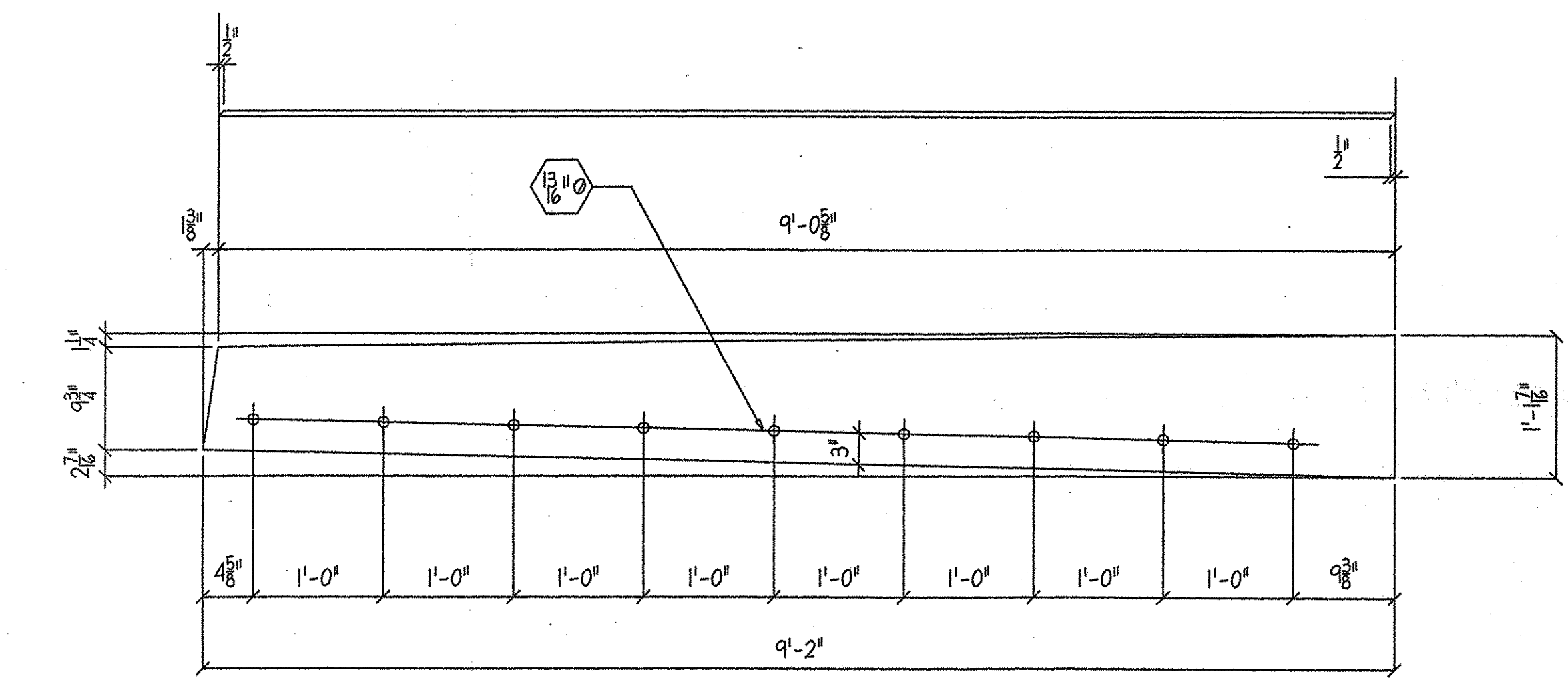
ONE REQ - PI3



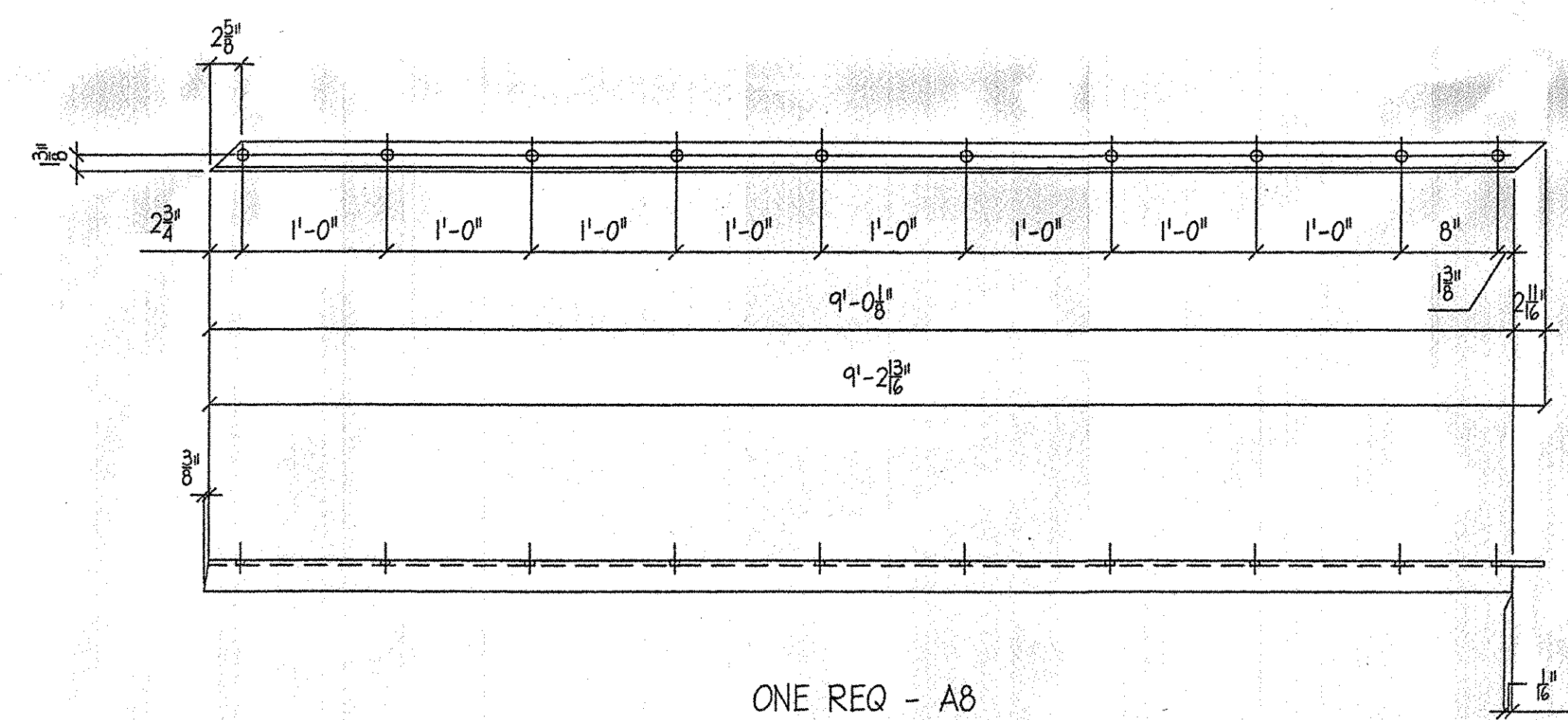
ONE REQ - PI4



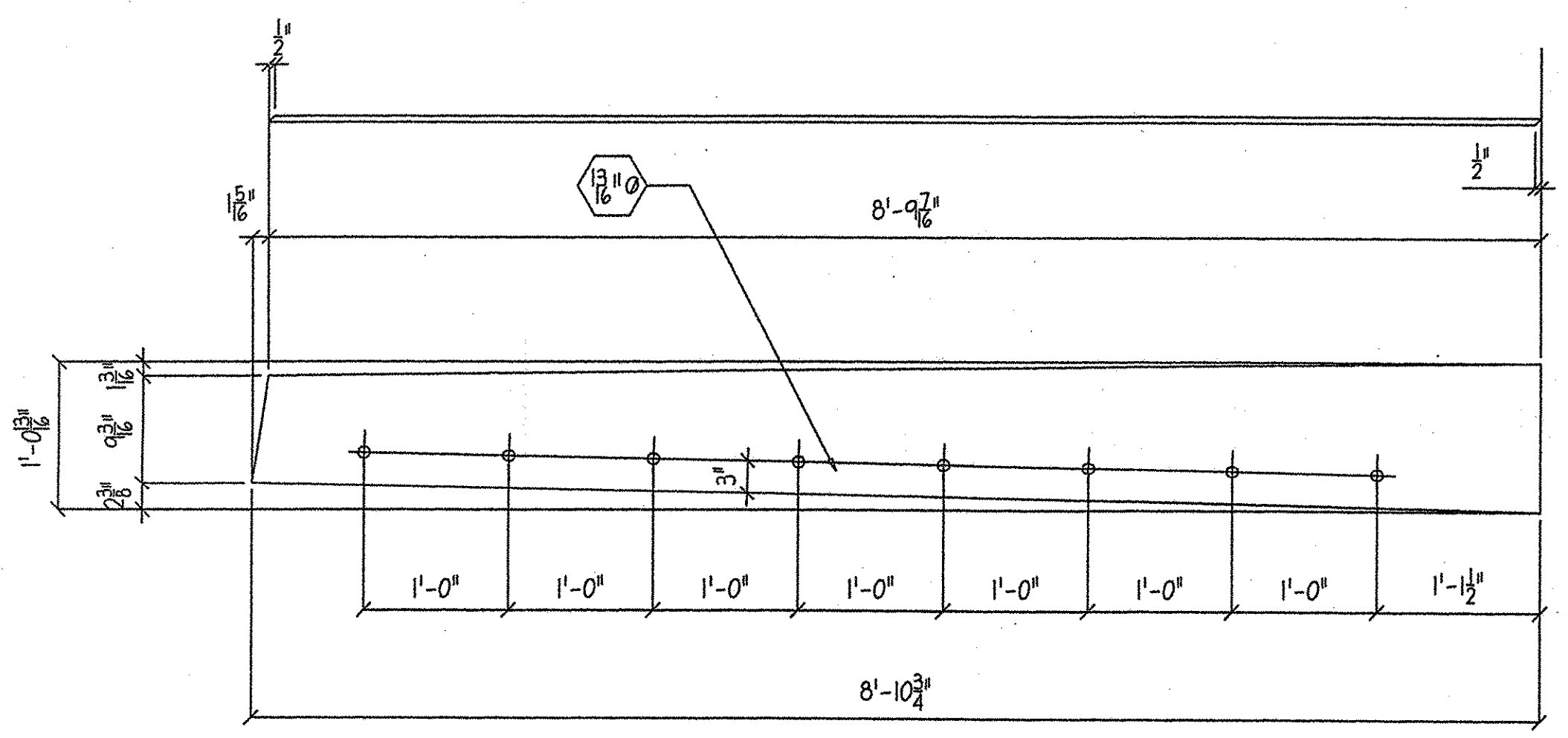
ONE REQ - PI7



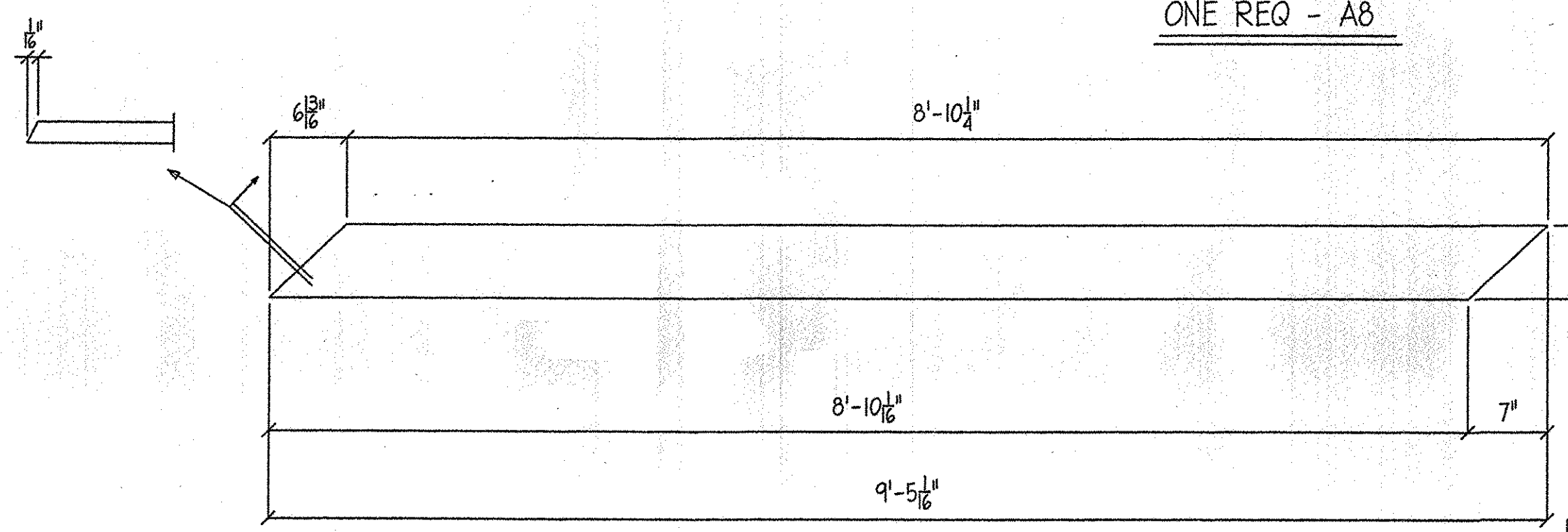
ONE REQ - PI5



ONE REQ - A8



ONE REQ - PI6



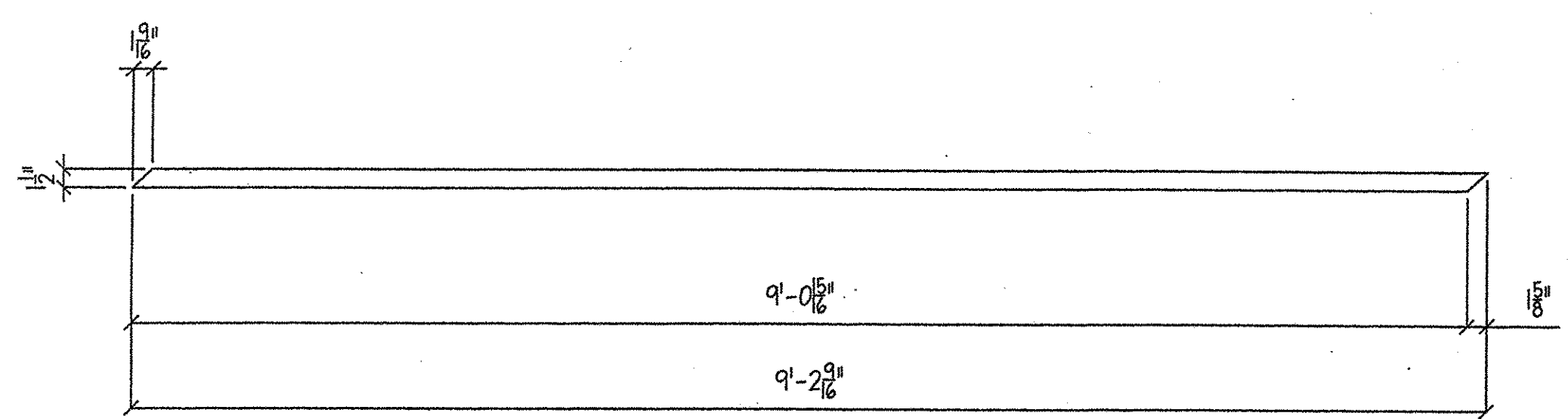
ONE REQ - PI8

BILL OF MATERIAL					
QTY	MARK	SHAPE	LENGTH		REMARKS
1	PII	PL 1/2" X 9 1/2"	2	10 3/4	GALV.
1	PI2	PL 1/2" X 6 1/2"	3	4 7/16	
1	PI3	PL 1/2" X 15"	1	8 3/8	
1	PI4	P 5/8" X 1 1/2"	3	2 3/8	
1	PI5	P 1/2" X 13 7/16"	9	2	
1	PI6	P 1/2" X 12 13/16"	8	10 3/4	
1	PI7	P 1/2" X 20"	10	10 5/8	
1	PI8	P 1/2" X 6 1/2"	9	5 1/16	
1	A8	L 2 1/2 X 2 1/ X 3/8	9	2 3/4	

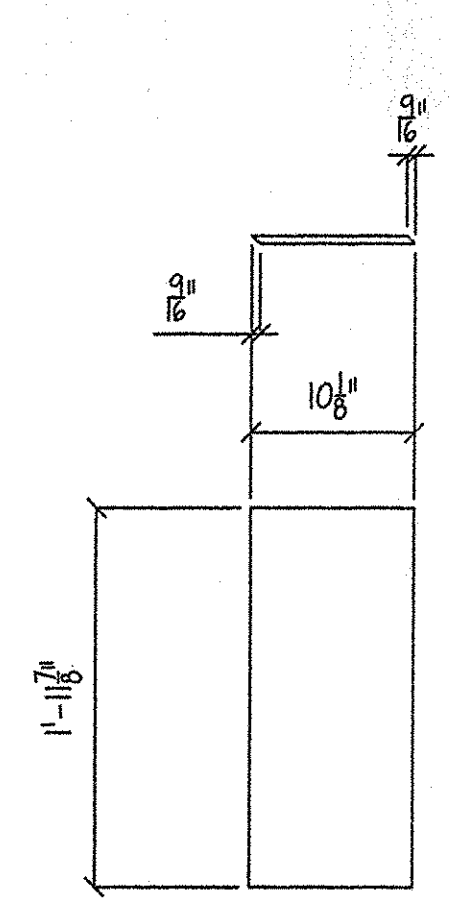
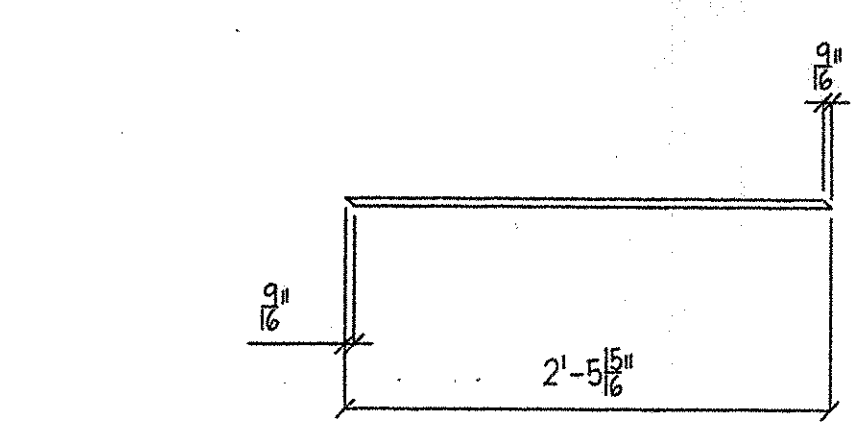
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 REVIEW BY STANTEC IS FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMANCE WITH DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS. COORDINATION OF SUB-TRENCH DETAIL DESIGN OF COMPONENTS AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.
 DATE: January 2nd, 2007
 SIGNATURE: *Scott Bullock*
 STATE: VERMONT

STATE PROJECT # BRO 1445 (29)
 EXPANSION JOINT (EJ-1) FAB DETAILS
 EAST STREET (T.H. #4) OVER HUNTINGTON RIVER
 HUNTINGTON VT.
 STATE OF VERMONT, AGENCY OF TRANSPORTATION
 MERRIMACK SHEET METAL
 119 HALL ST.
 CONCORD, N.H., 03301
 TEL. (603) 224-7766
 FAX. (603) 224-7925

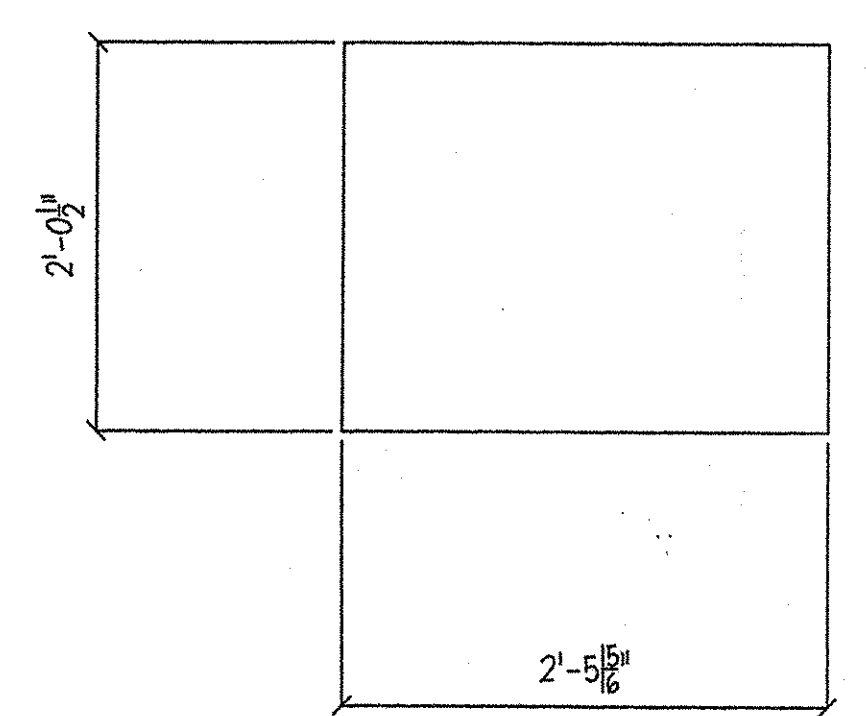
REV. NO.	DATE	DESCRIPTION
HOLES: 13/16" (U.N.O.)	DATE: 11-20-06	
PAINT:		



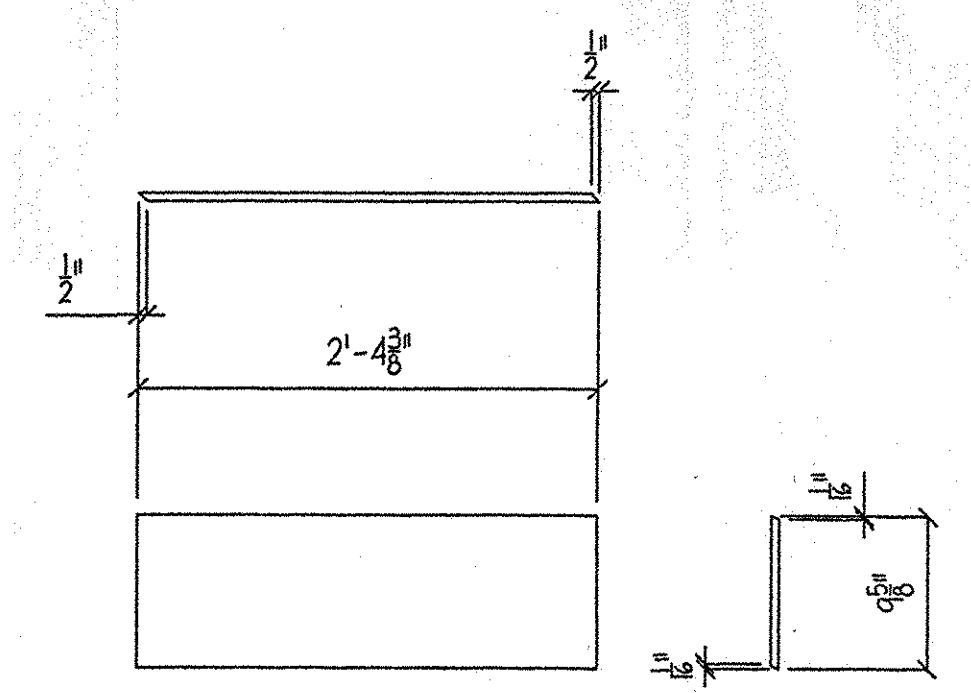
ONE REQ - P19



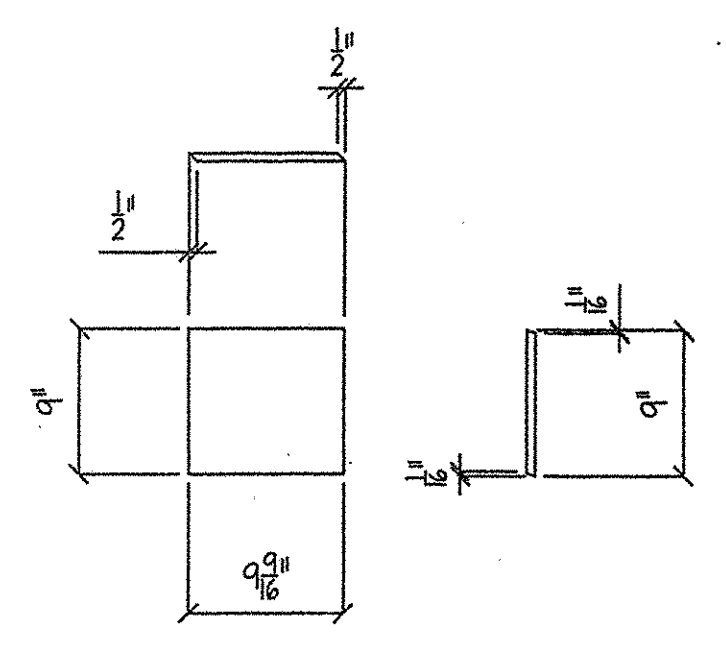
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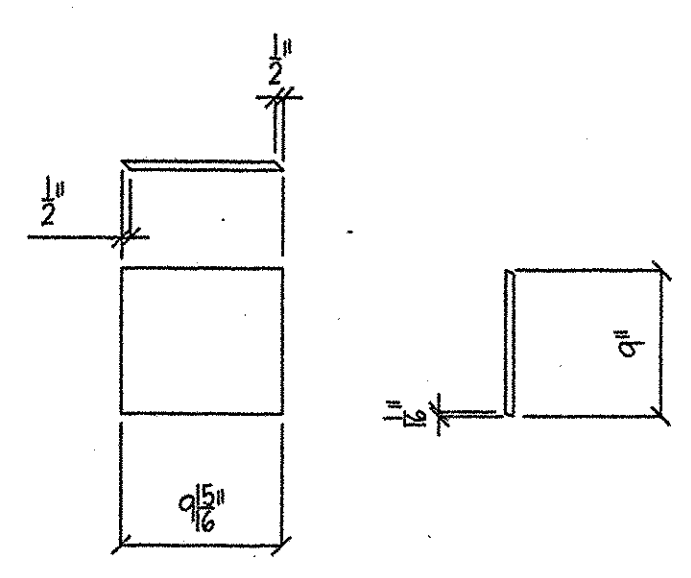
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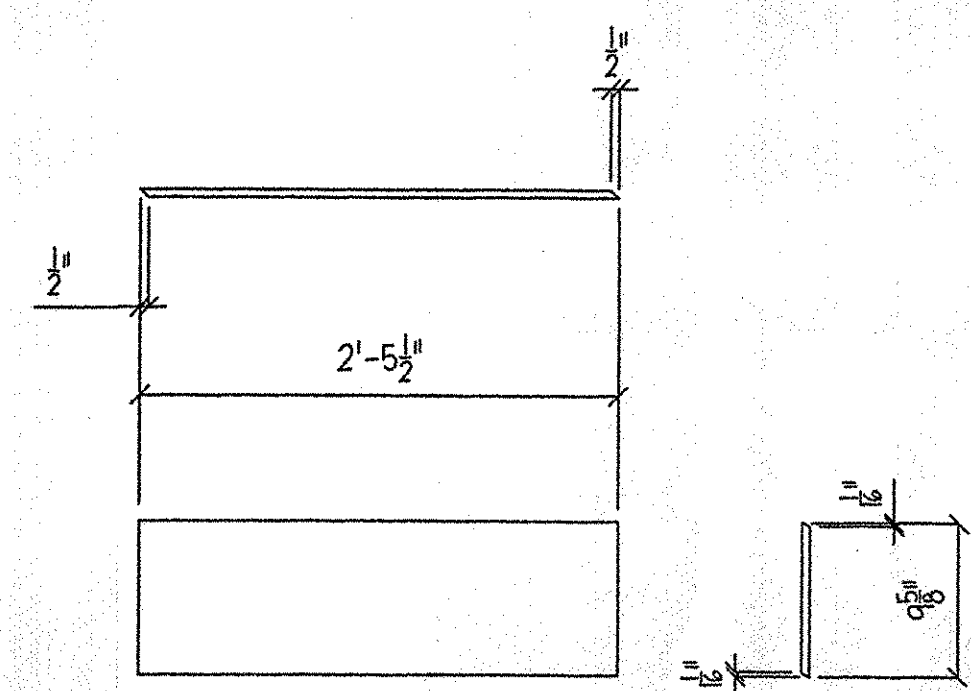
ONE REQ - P22



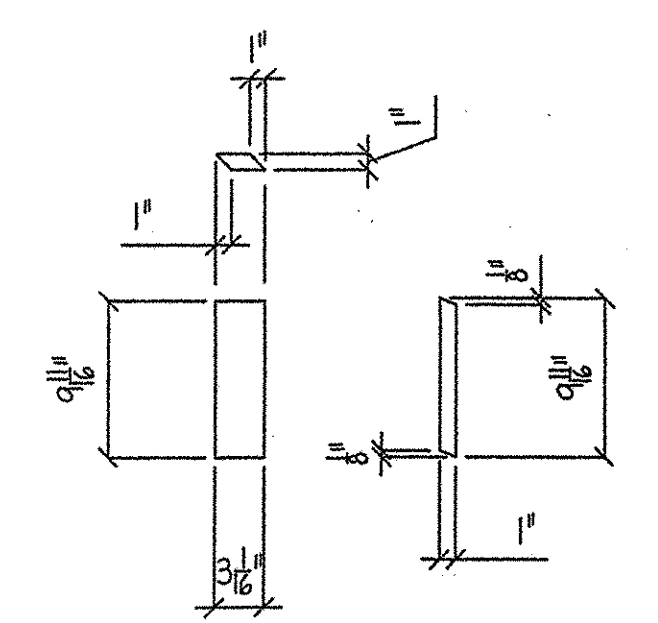
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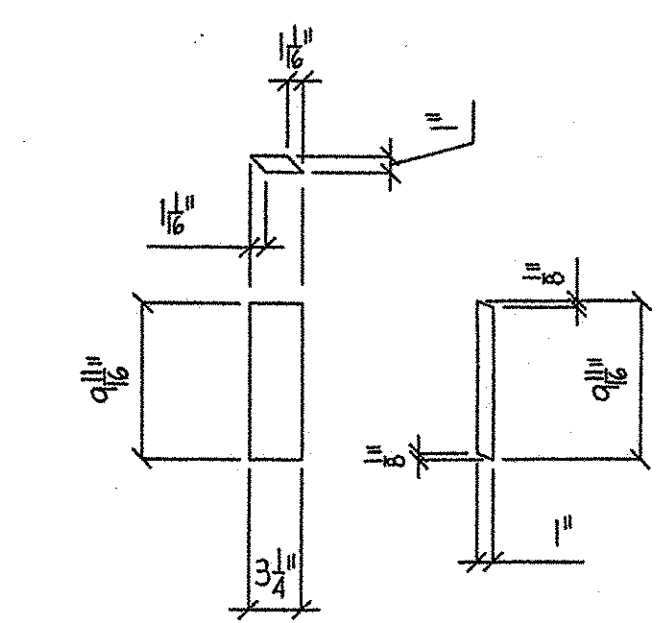
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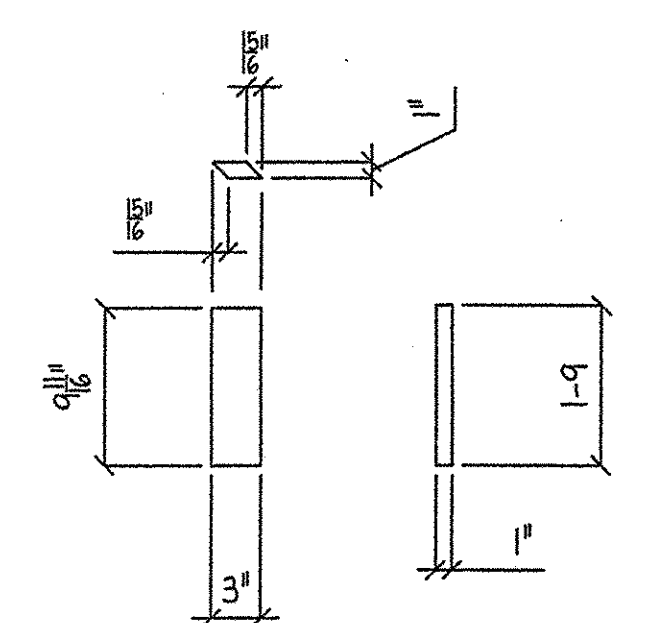
ONE REQ - P23



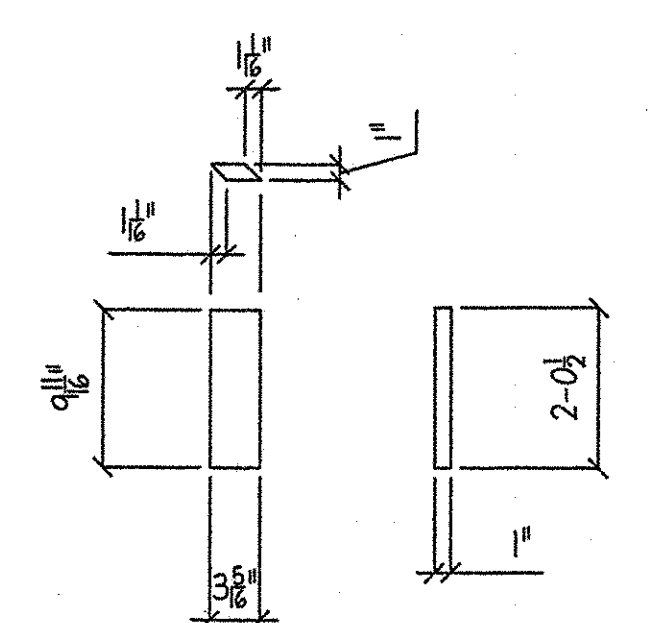
ONE REQ - P26



ONE REQ - P27

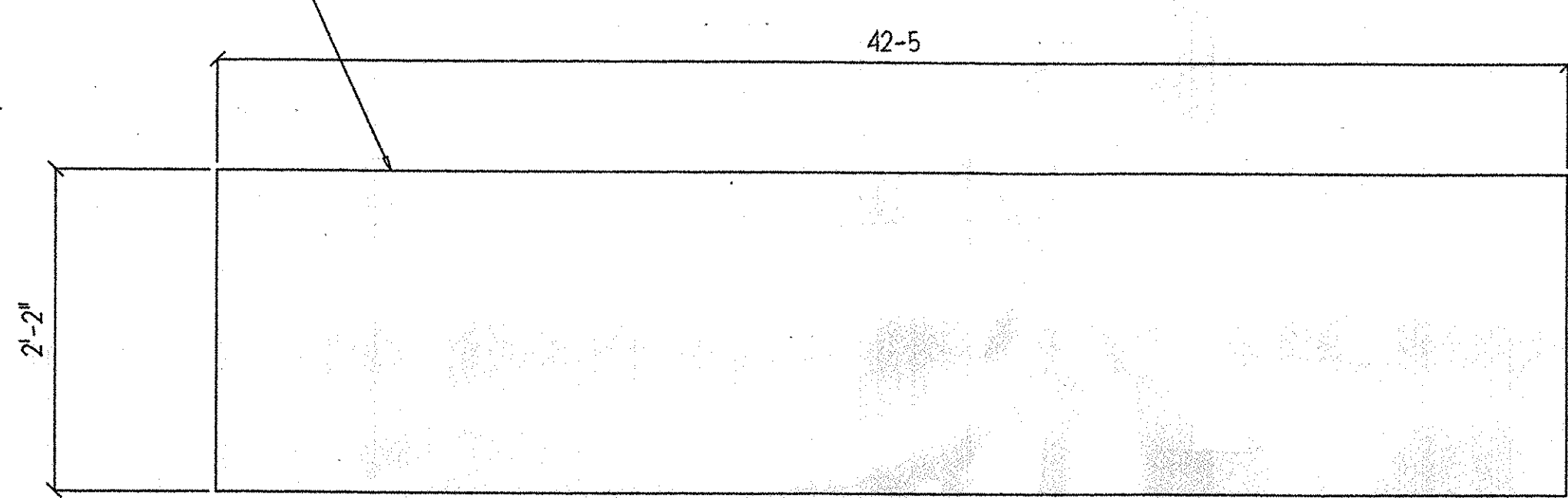


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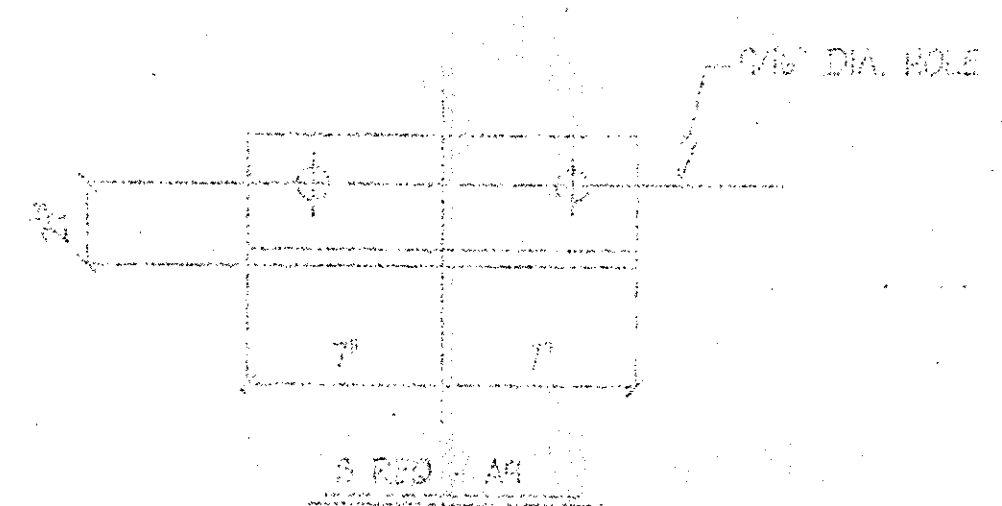
ONE REQ - P29

SEE FOLDED FABRIC TROUGH END DETAIL ON DWG. #E1

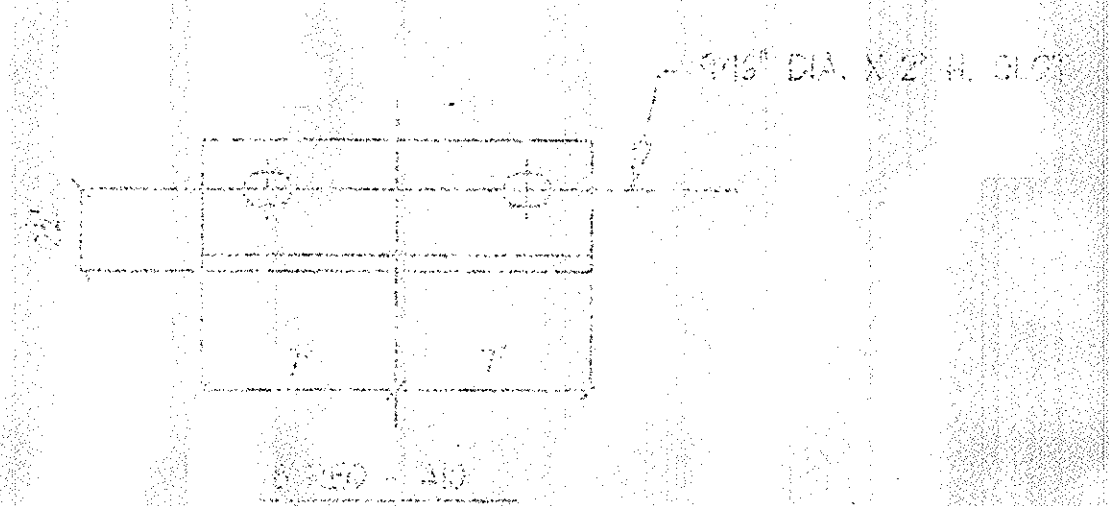


TROUGH LAYOUT

PERFORMED FABRIC TROUGH
5/32" X 26" (3 PLY)
W/ DRIP BEAD (SEE NOTES)



ONE REQ - A9



ONE REQ - A10

SHOP DRAWINGS ARE REVIEWED UNLESS NOTED OTHERWISE

NO EXCEPTION TAKEN

REVISE AS NOTED

RESUBMISSION NOT REQUIRED

REVISE AS NOTED

RESUBMISSION REQUIRED

REJECTED

DATE: January 2nd 2007

SIGNATURE: Scott Burckel

REVIEW BY STAFFED IS FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DESIGN. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS, COORDINATION OF SUB-TRACES, DETAIL DESIGN OF COMPONENTS AND BRACES OR OMISSIONS ON SHOP DRAWINGS.

BILL OF MATERIAL

QTY	MARK	SHAPE	LENGTH	REMARKS
1	P19	P 5/8" X 1 1/2"	9	2 9/16 GALV.
1	P20	P 1/2" X 24 1/2"	2	5 15/16
1	P21	P 1/2" X 10 1/8"	1	11 7/8
1	P22	P 1/2" X 9 5/8"	2	4 3/8
1	P23	P 1/2" X 9 5/8"	2	5 1/2
1	P24	P 1/2" X 9"	0	9 9/16
1	P25	P 1/2" X 9"	0	9 15/16
1	P26	P 1/2" X 3 1/16"	0	9 11/16
1	P27	P 1/2" X 3 1/4"	0	9 11/16
1	P28	P 1/2" X 3"	1	9
1	P29	P 1/2" X 3 5/16"	2	0 1/2

PERFORMED FABRIC TROUGH
5/32" X 26" (3 PLY)
W/ DRIP BEAD (SEE NOTES)

STATE PROJECT # BRO 1445 (29)

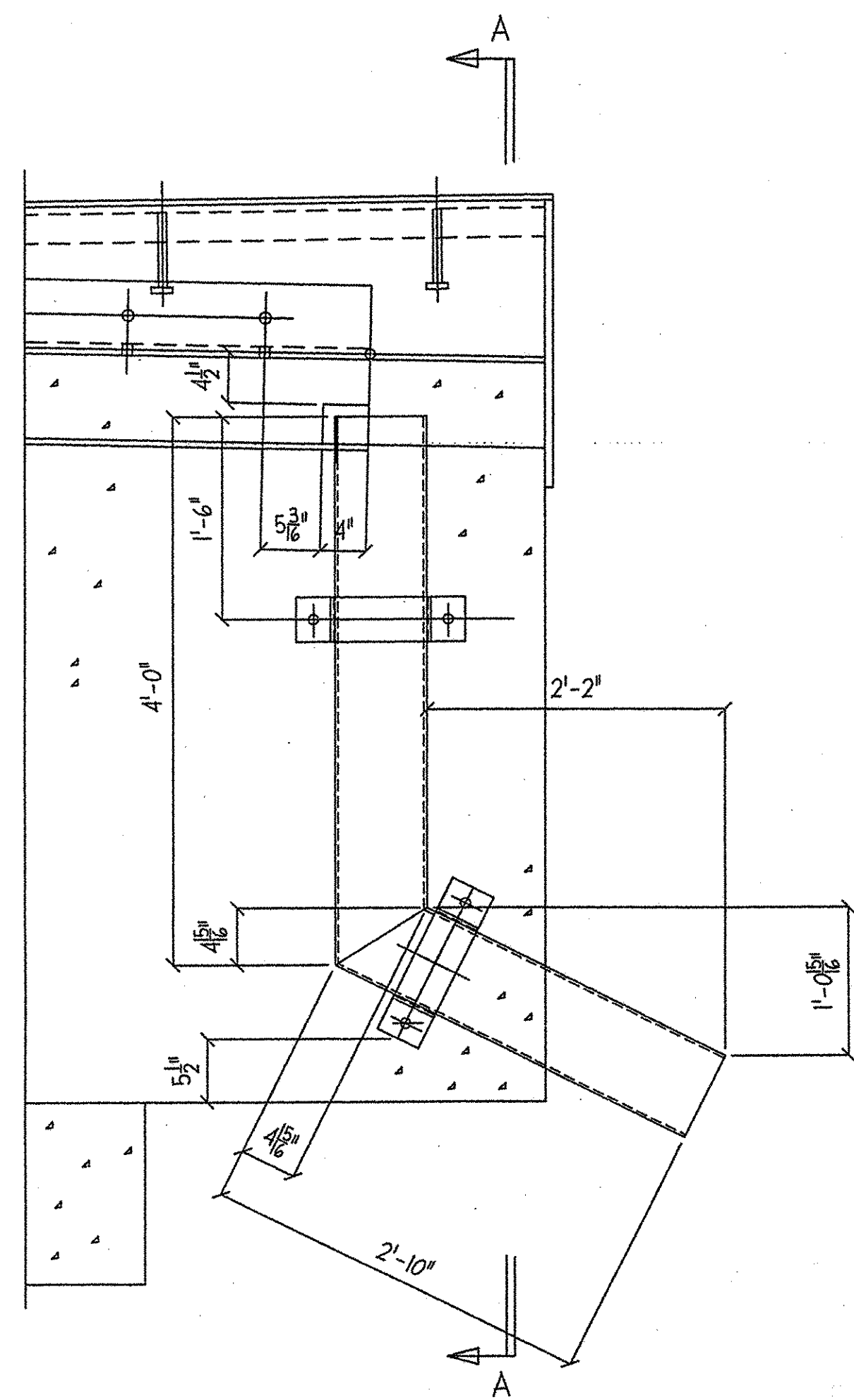
EXPANSION JOINT (EJ-1) FAB DETAILS

DATE SUB.	PROJECT	EAST STREET (T.H. #4) OVER HUNTINGTON RIVER
	LOCATION	HUNTINGTON VT.
	ARCHITECT	
DATE APP.	ENGINEER	STATE OF VERMONT, AGENCY OF TRANSPORTATION
DATE DIST.	CUSTOMER	PARENT CONSTRUCTION

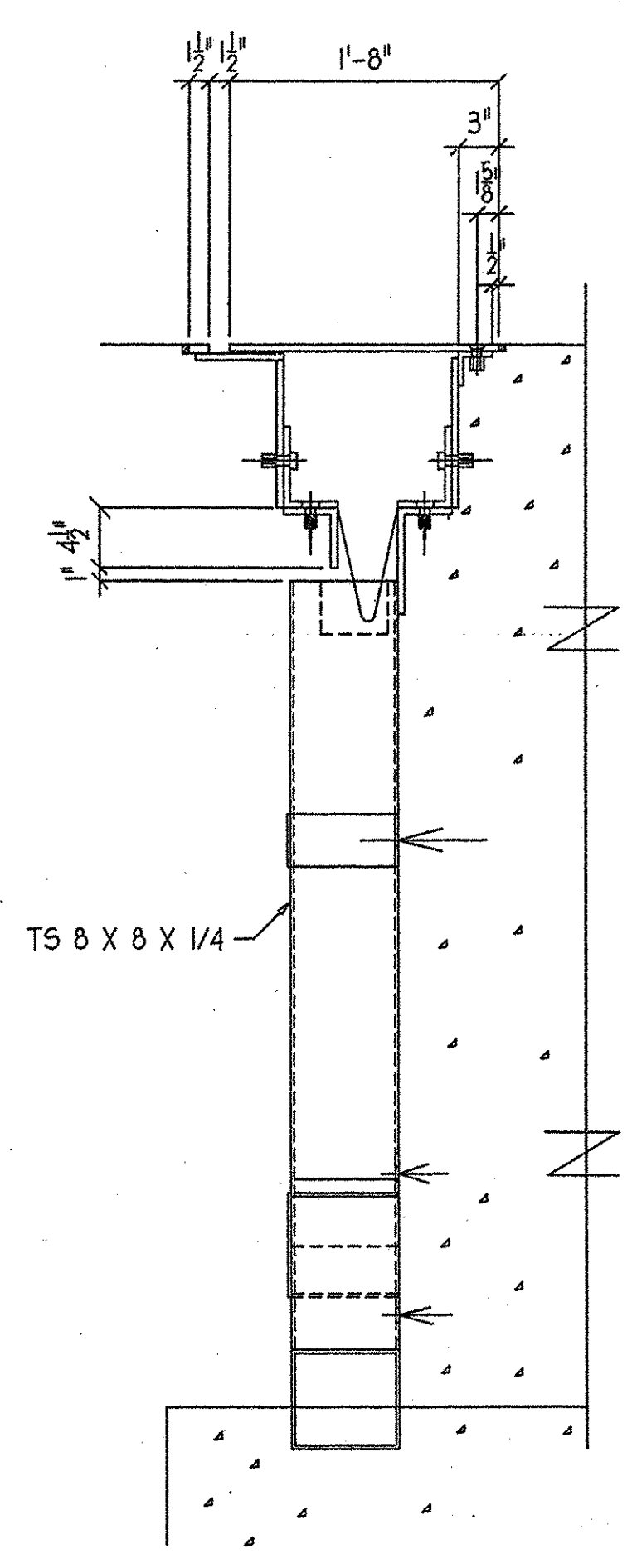
MERRIMACK SHEET METAL
119 HALL ST.
CONCORD, N.H., 03301
MISCELLANEOUS IRON STRUCTURAL STEEL FABRICATORS
TEL. (603) 224-7766
FAX. (603) 224-7925

DR BY	KJM
REV BY	JD
JOB NO.	0922
DWG NO.	F3

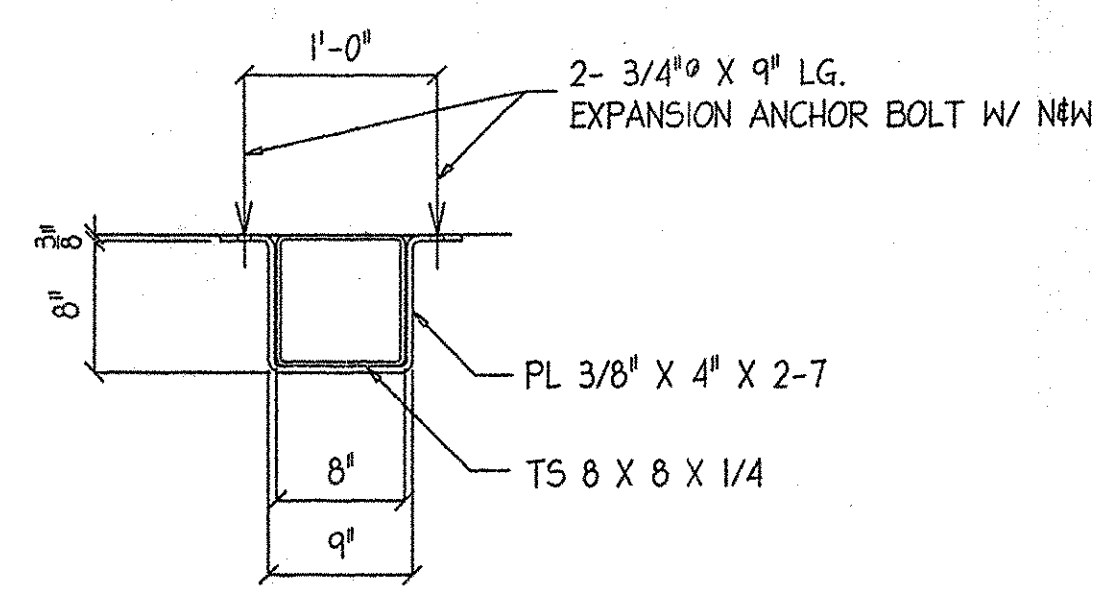
REV. NO.	DATE	DESCRIPTION
	13/16" (U.N.O.)	DATE: 11-20-06
PAINT:		



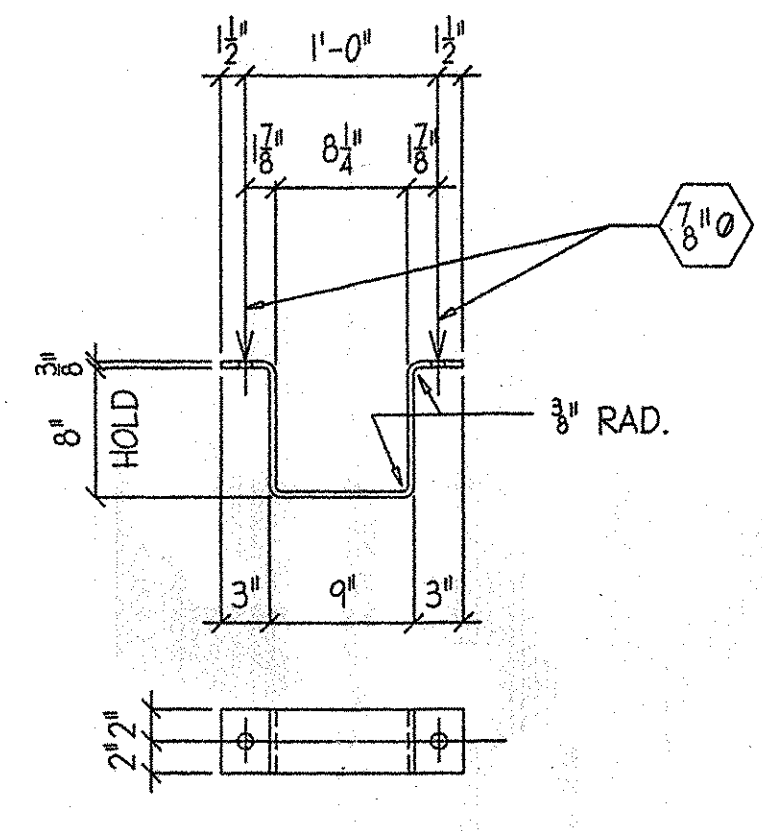
DOWNSPOUT FRONT ELEVATION



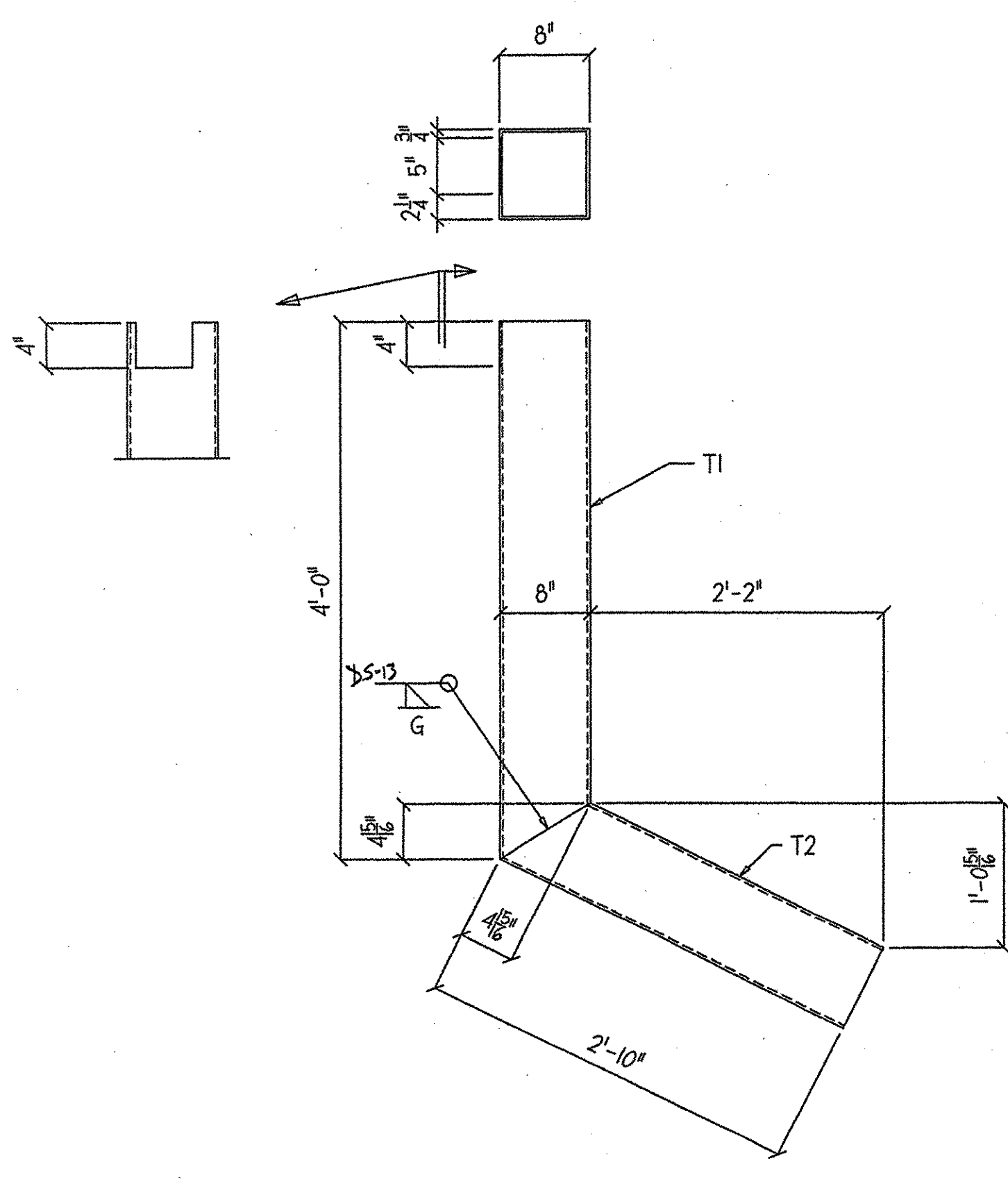
SECTION A-A



DOWNSPOUT BRACKET



2 REQ - DBI
BENT PL 3/8" X 4" X 2-6 1/2



ONE REQ - DSP-1

DOWNSPOUT NOTES

- 1.) HOLLOW STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A500.
- 2.) ALL PLATES, BARS, AND ANGLES SHALL CONFORM TO AASHTO M270, GR. 36.
- 3.) DOWNSPOUTS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A-123 AFTER FABRICATION.
- 4.) ALL BOLTS AND RELATED HARDWARE SHALL BE ASTM A307 AND SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A513 (AASHTO M232).

BILL OF MATERIAL

QTY	MARK	SHAPE	LENGTH	REMARKS
1	DSP-1	DOWNSPOUT		GALV.
1	T1	TS 8 X 8 X 1/4	4	0
1	T2	TS 8 X 8 X 1/4	2	10
2	DBI	DOWNSPOUT BRKT. BENT PL. 3/8" X 4"	2	6 1/4
FIELD BOLTS @ DOWNSPOUTS				
4		3/4" X 9" LG. EXPANSION ANCHOR BOLT W/ 2N#W		

SHOP DRAWINGS ARE REVIEWED UNLESS NOTED OTHERWISE

NO EXCEPTION TAKEN

REVISE AS NOTED

RESUBMISSION NOT REQUIRED

REVISE AS NOTED

RESUBMISSION REQUIRED

REJECTED

DATE: January 2nd, 2007

SIGNATURE: *Scott Burdick*

REVIEW BY STANTEC IS FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DESIGN. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS. COORDINATION OF SUB-TENDERS, DETAIL DESIGN OF COMPONENTS AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.

STATE PROJECT # BRO 1445 (29)

EXPANSION JOINT (EJ-1) DOWNSPOUT
EAST STREET (T.H. #4) OVER HUNTINGTON RIVER

DATE SUB.	PROJECT	LOCATION	HUNTINGTON VT.
DATE APP.	ENGINEER	ARCHITECT	STATE OF VERMONT, AGENCY OF TRANSPROTATION
DATE DIST.	CUSTOMER		PARENT CONSTRUCTION

REV. NO.	DATE	DESCRIPTION
HOLES:	13/16" (U.N.O.)	DATE: 11-20-06
PAINT:		

MERRIMACK MERRIMACK SHEET METAL
119 HALL ST.
CONCORD, N.H., 03301
TEL. (603) 224-7766
FAX. (603) 224-7925

DR BY	KJM
REV BY	
JOB NO.	0922
DWG NO.	F4



State of Vermont
PDD/Structures Design Section
National Life Building - Drawer 33
Montpelier, VT 05633-5001
www.aot.state.vt.us

Agency of Transportation

[phone] 802-828-2621
[fax] 802-828-3366
[td] 800-253-0191

DATE: March 29, 2007

Parent Construction, Inc.
P. O. Box 489
Hinesburg, VT 05461
Attention: Tim Parent

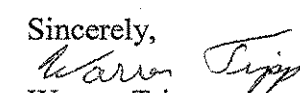
Dear Mr. Parent

Project Name: Huntington Project #: BRO 1445(29)
Structure Identification: East Street over Huntington River

The following Bridge and Approach Railing details [Item # 525.22, Bridge Railing - 3 Rail Aluminum, Item # 525.23, Bridge Railing - Aluminum/Pedestrian, and Item # 621.74, Aluminum Approach Railing] for the above project (Vendor's Job # BR 2898) along with the welding procedure transmitted with your letter dated 2/26/07 have been reviewed and two sets are being returned herewith. Please forward one set to your fabricator, including a copy of this letter for the fabricator's use.

Sheets 1 through 8 and the welding procedure # 44 are **approved or approved "as noted"**. Please note that upon receipt of these "as noted" or "approved" plans, the fabricator must make appropriate changes and submit white prints for our use in the record plans for this project.

The fabricator must provide notice to our fabrication inspector, Jeff Clark, as to the date fabrication represented by these drawings will begin. That notice must be received and acknowledged at least seven days prior to that date, as per Specification 506.03. Jeff may be contacted by phone at (802)828-0044 or email at jeff.clark@state.vt.us. Any material fabricated prior to the notification date is subject to rejection without further cause.

Sincerely,

Warren Tripp

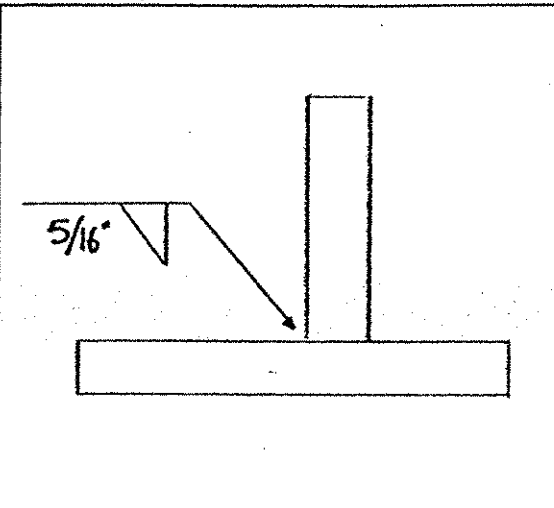
Attachments

cc: Auciello Iron Works
Resident Engineer w/prints Dale Norton
Shop Inspector w/prints Jeff Clark
Construction Division - letter only
Materials & Research Section (C&IA Unit) - letter only
Files (Structures & Central)

119 BR

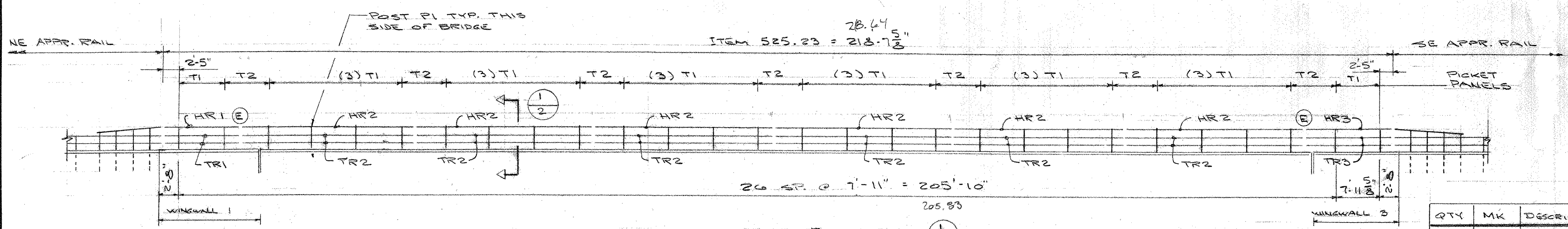


WELDING PROCEDURE SPECIFICATION (WPS) #44

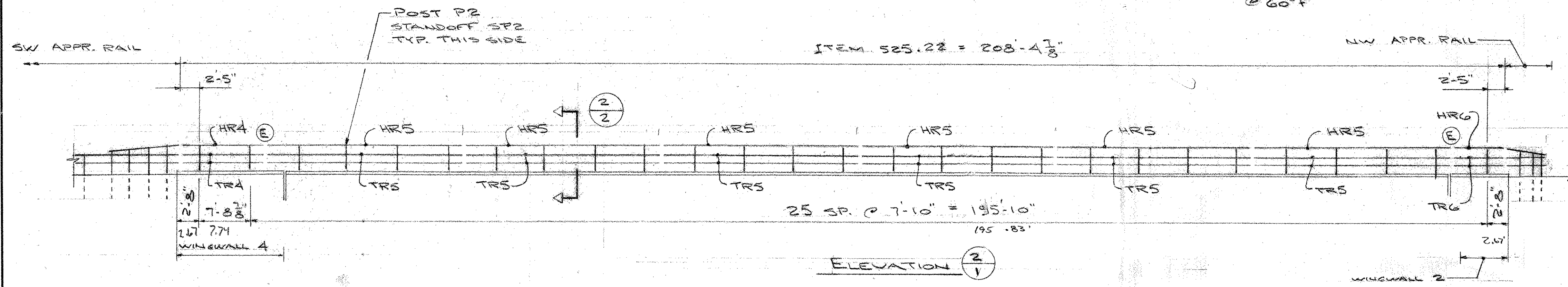
Welding Procedure Specification No. #44		Date 03.01.02	Approved Philippe
Revisions	Date	Approved	
Supporting PQR Numbers #15			
Joints Groove Design Sketch 		Filler Metal F-No. E22 AWS No. ER 5556 Size of electrode .045 ± 1/64 Type of electrode Aluminum Other -	
Backing Type _____ Permanent _____ Removed _____ Other _____		Shielding Gas Shielding gas(es) Helium - Argon Percent composition 75% - 25% Flow rate 55 CFH (+50% - 20%) Other -	
Base Metals Group No. M23 Thickness 3/4 to 3/4 Alloy and Temper 6061-T6		Position Position of groove Fillet - 2F Horizontal Welding progression Forehand Other -	
Preheat Preheat temperature 50°F Min Interpass temperature -			

Form E(a)

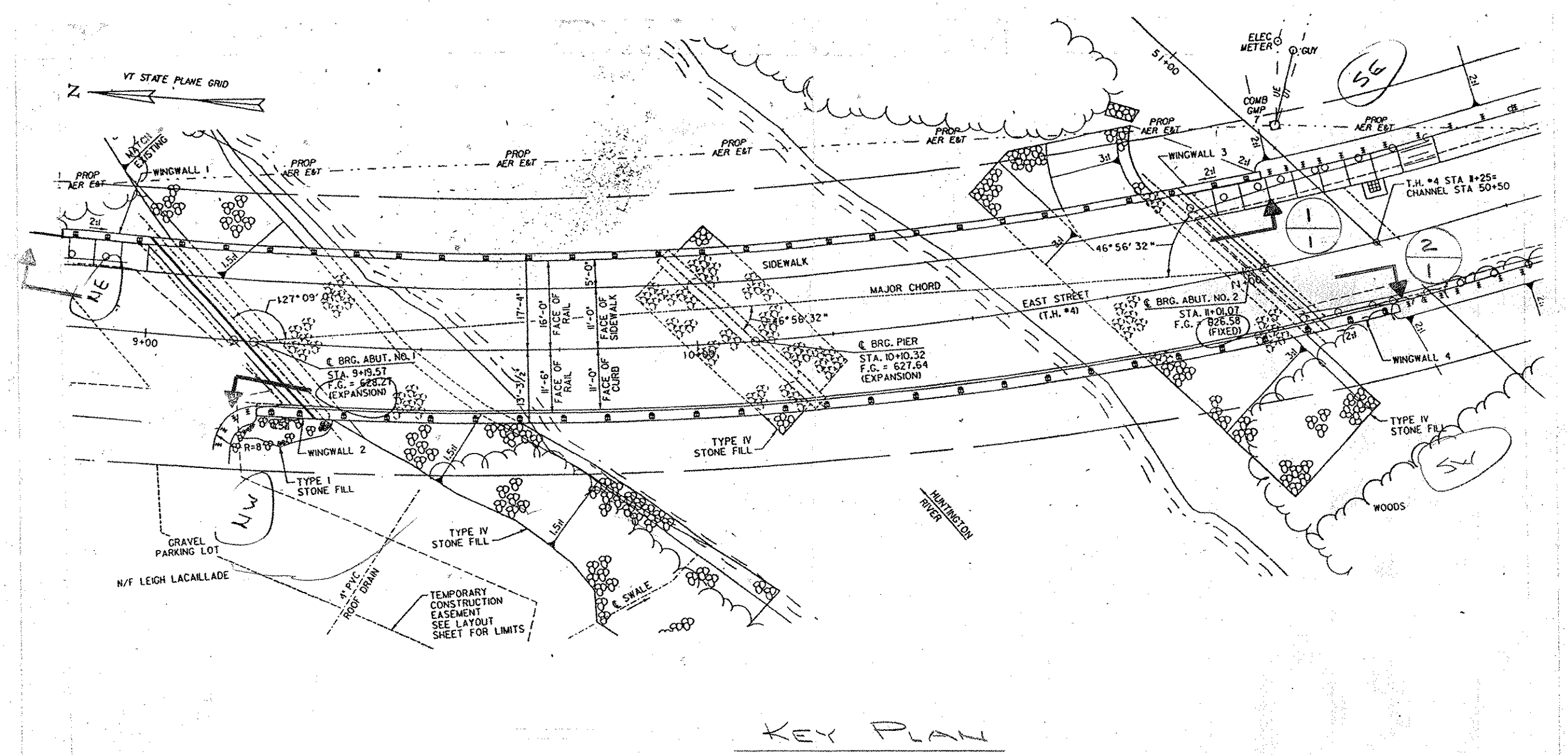
TRANS RECEIVED
 OK'D BY _____ OK'D BY JWC
 FEB 28 2007
 RESUBMIT APPROVED ✓ 1203R
 BY _____ DATE 3-22-07



ELEVATION 1



ELEVATION 2



KEY PLAN

QTY	MK	DESCRIPTION	HAANDRAIL
28	P1	RAIL POST	QTY MK LENGTH
27	P2	do	1 HR1 13-4 3/8
31	P8	APPROACH POST	6 HR2 31-7 1/2
1	P11		1 HR3 15-2 3/8
1	P12		1 HR4 13-2 5/8
1	P13		6 HR5 31-3 1/2
1	P14		1 HR6 7-2 1/2
1	P15		1 HR7 20-0
1	P16		3 HR8 11-9
1	AP1	APPROACH POST	
1	AP2		
1	AP3		
1	AP4		
1	TP1	STANDOFF	
1	TP2		
1	TP3		
1	TP4		
1	T31		
1	T32		
1	T33		
1	T34		
1	T35		
1	T36		
1	T37		
27	SP2		
31	T8	STANDOFF	
1	T11		
1	T12		
14	HS1	RAIL SPLICE	142 #4 CLAMP BAR
2	HS2		444 #1 CLAMP BAR
1	HS3		1020 1/2 x 3/4 S.S. BOLT
1	HS4		888 1/2 x 1 S.S. BOLT
1	HS4		640 1/2 x 1 1/2 S.S. BOLT
38	TS1		640 1/2 S.S. HEX NUT
4	TS2		2548 1/2 AL WASHER
2	TS3		64 1/2 S.S. WASHER
2	TS4		64 2 1/2 C BOLT
4	TS5		55 POST PAD
1	TS6		55 ANCHOR BOLT ASSEMBLY
1	TS7		18 1/4 x 3/4 ST STL SOCKET SET SCREW
1	TS7		10 SILVER COLOR DELIMINATOR
20	T1	PICKET PANEL	20 #3 x 3/4 ST STL SELF-TAPPING SCREW
7	T2	do	8 TRAFFIC RAIL CAP

REVIEW BY STANTEC IS FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DESIGN. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS, COORDINATION OF SUB TRADES, DETAIL DESIGN OF COMPONENTS AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.

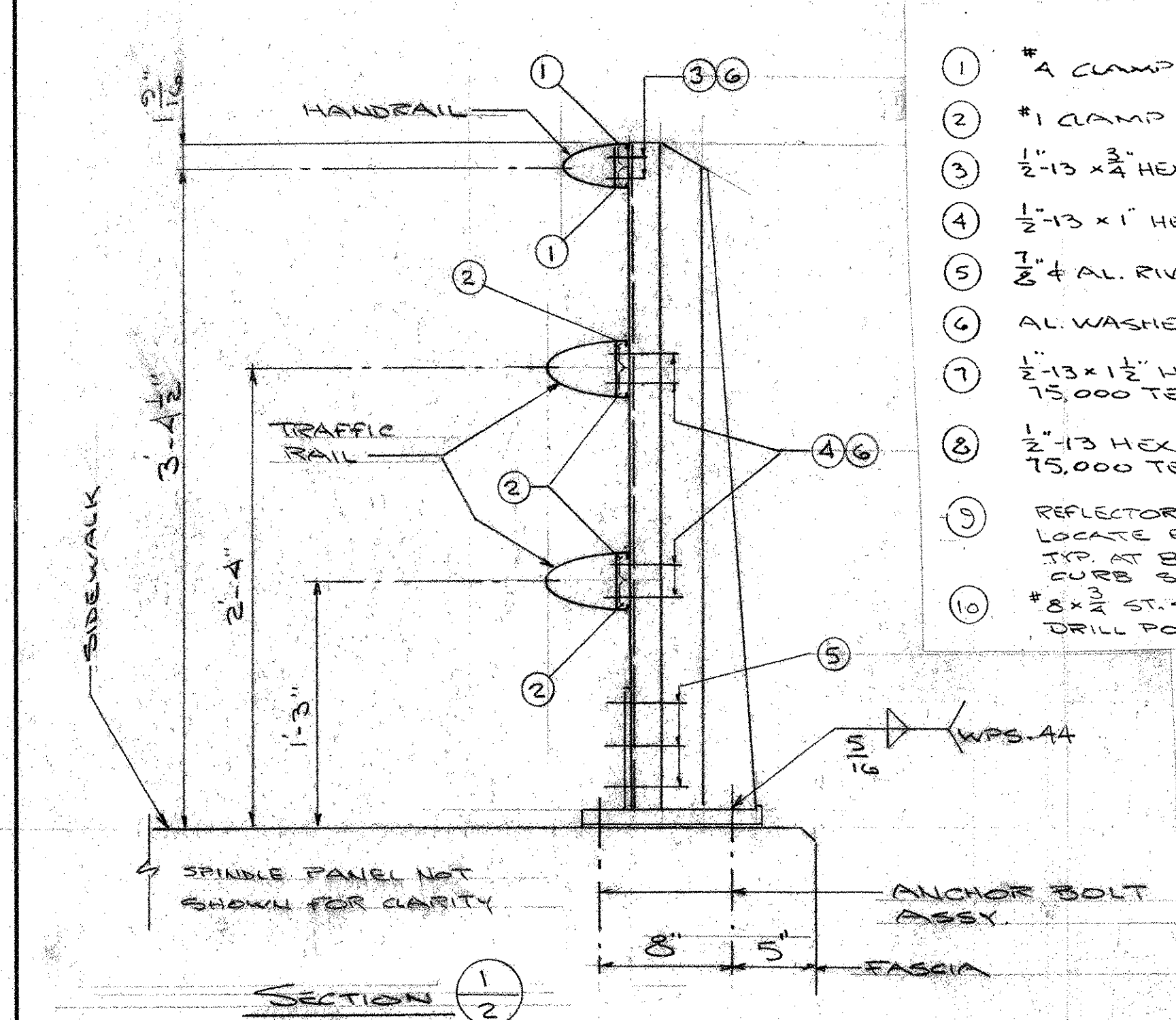
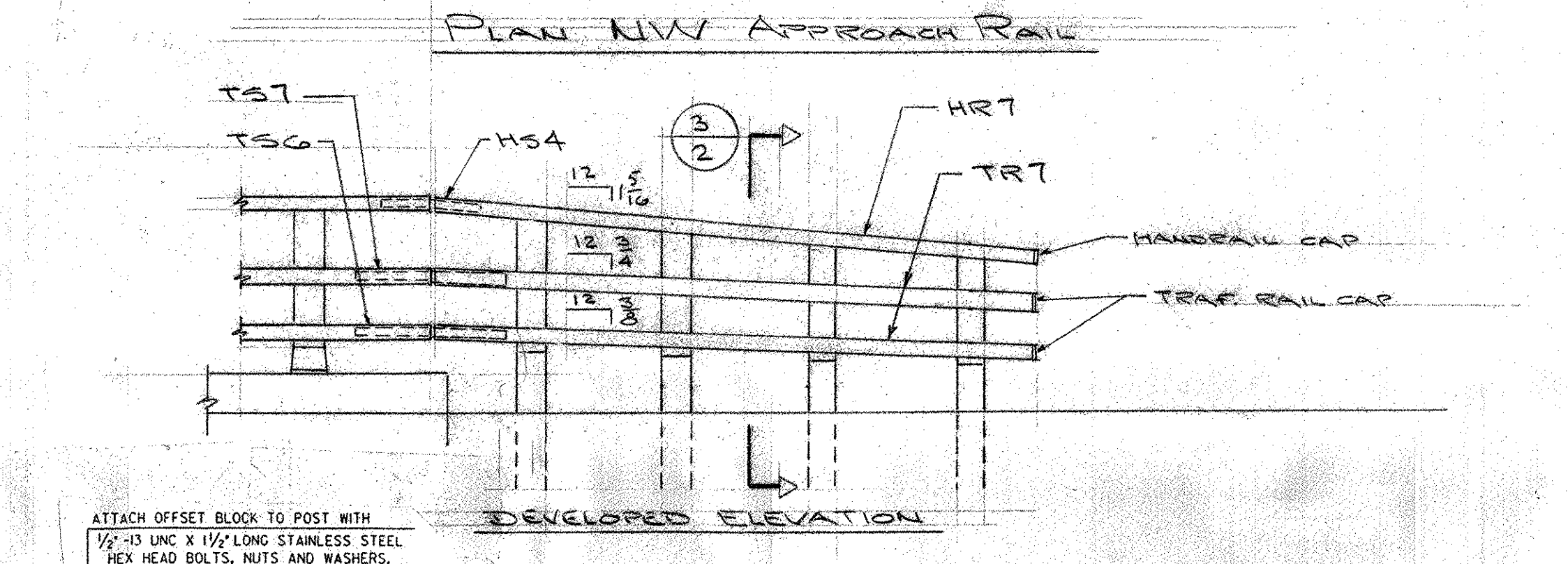
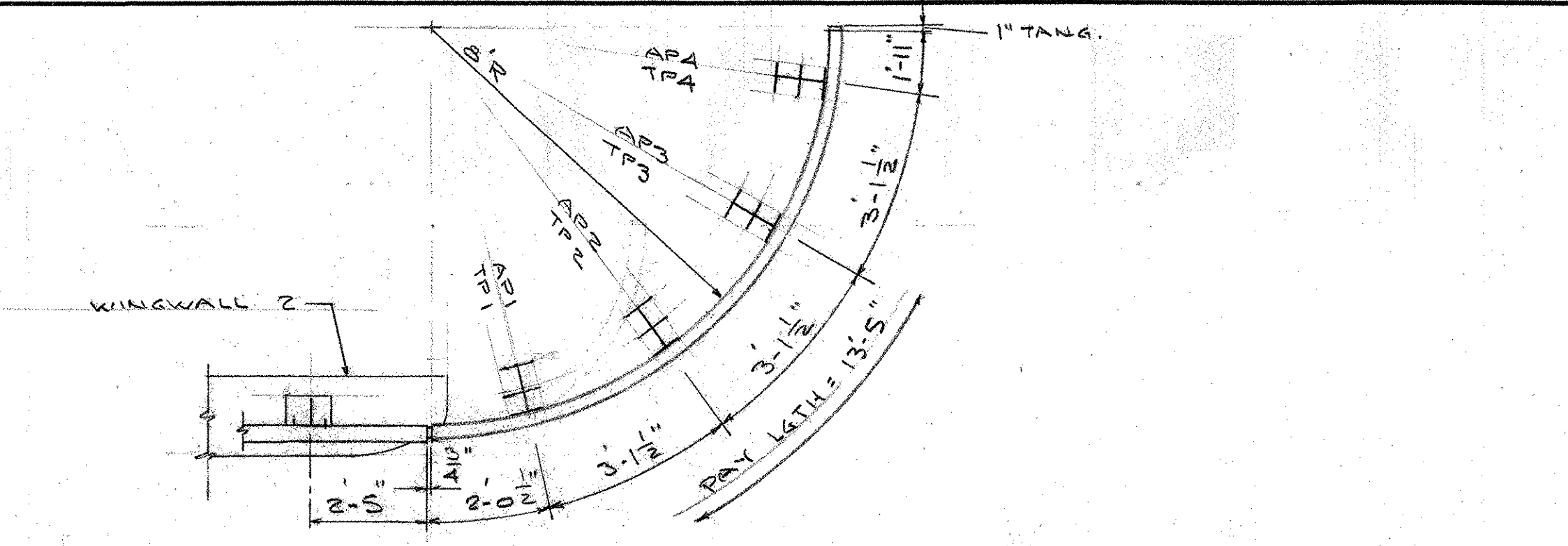
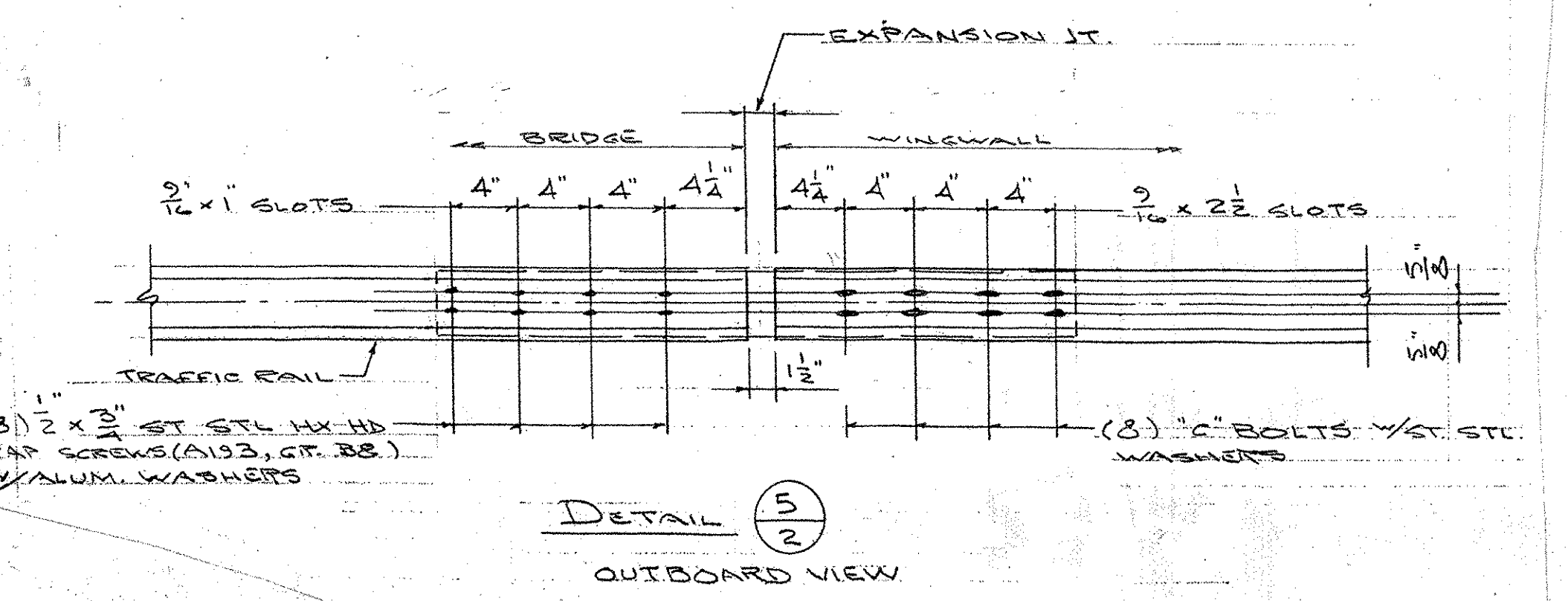
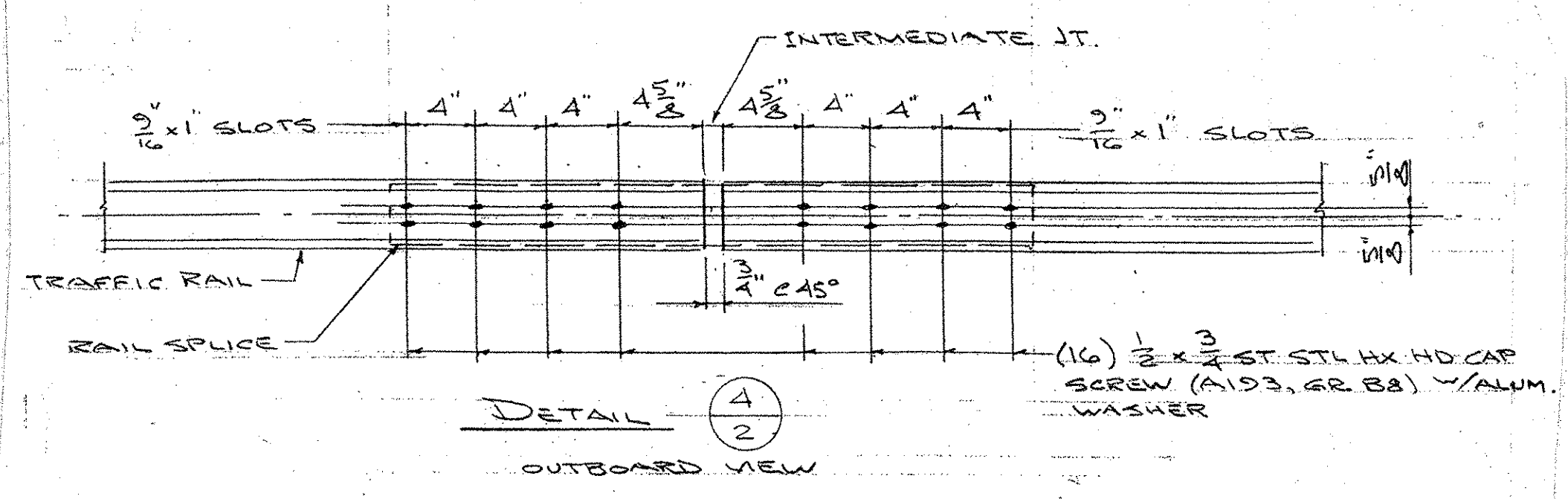
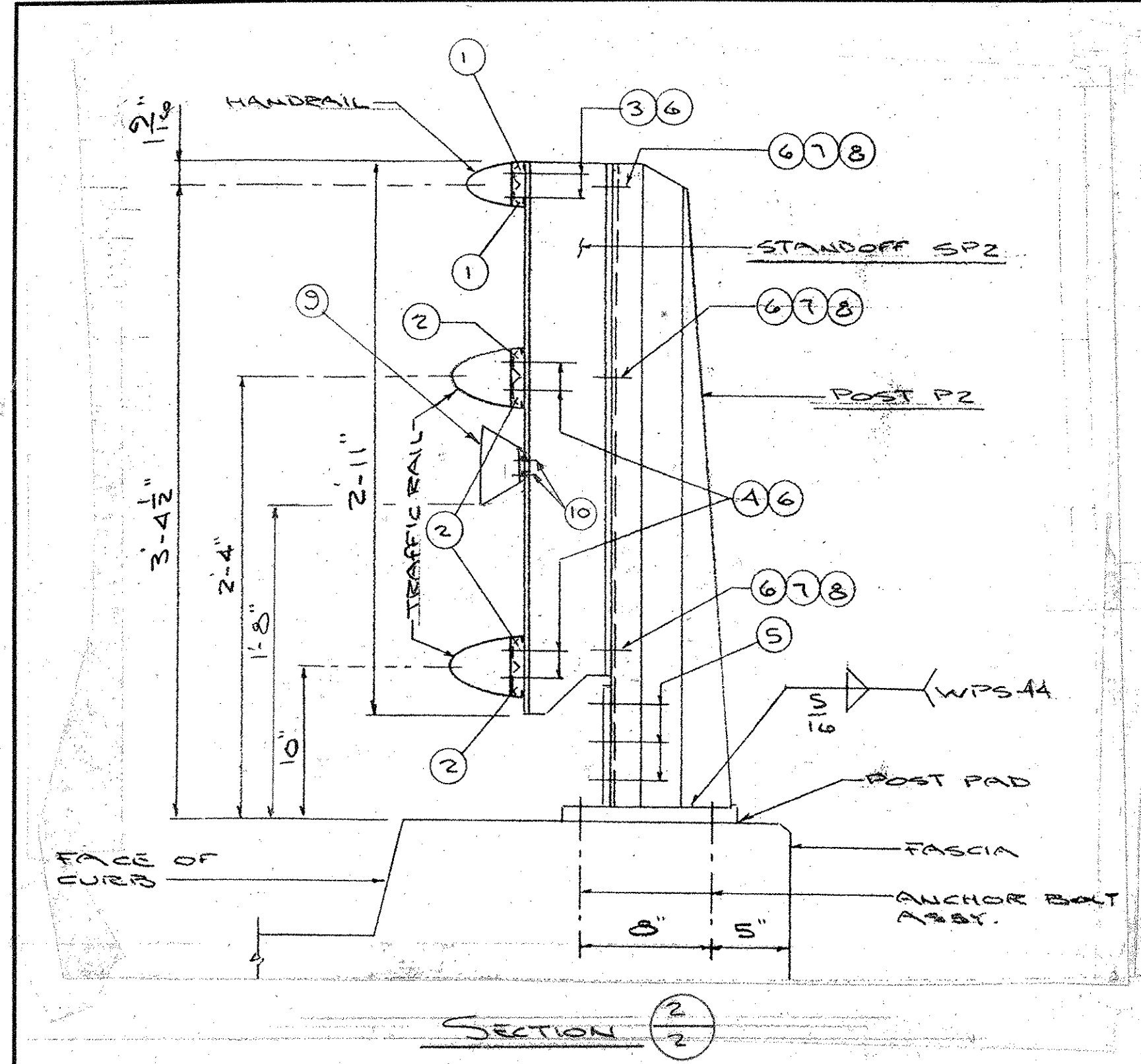
DATE: 3/26/07
SIGNATURE: *Michael Chiodi*

RECEIVED
FEB 28 2007
BY: *[Signature]*

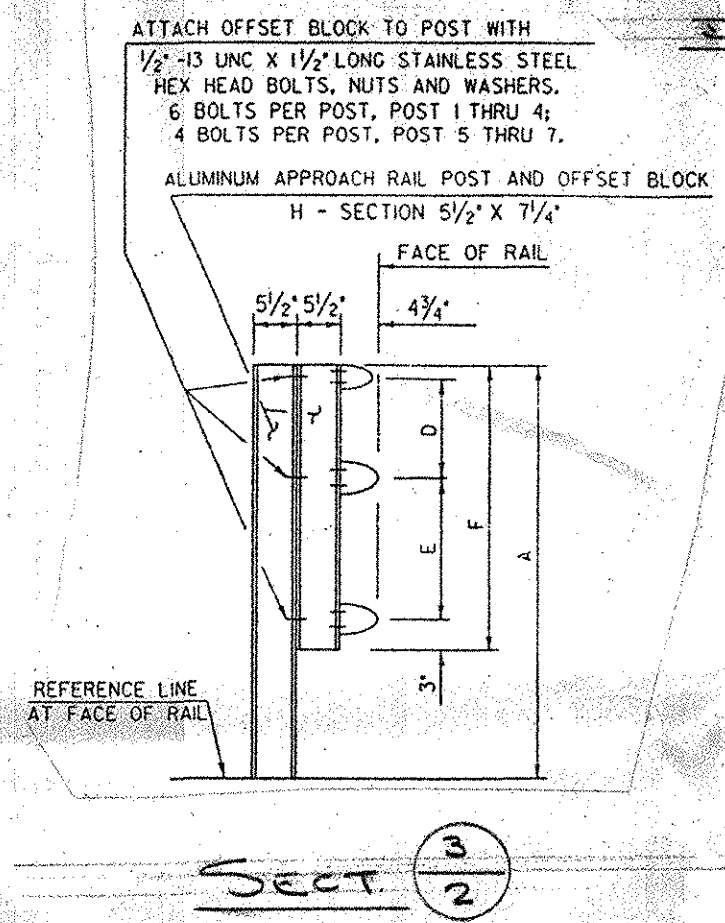
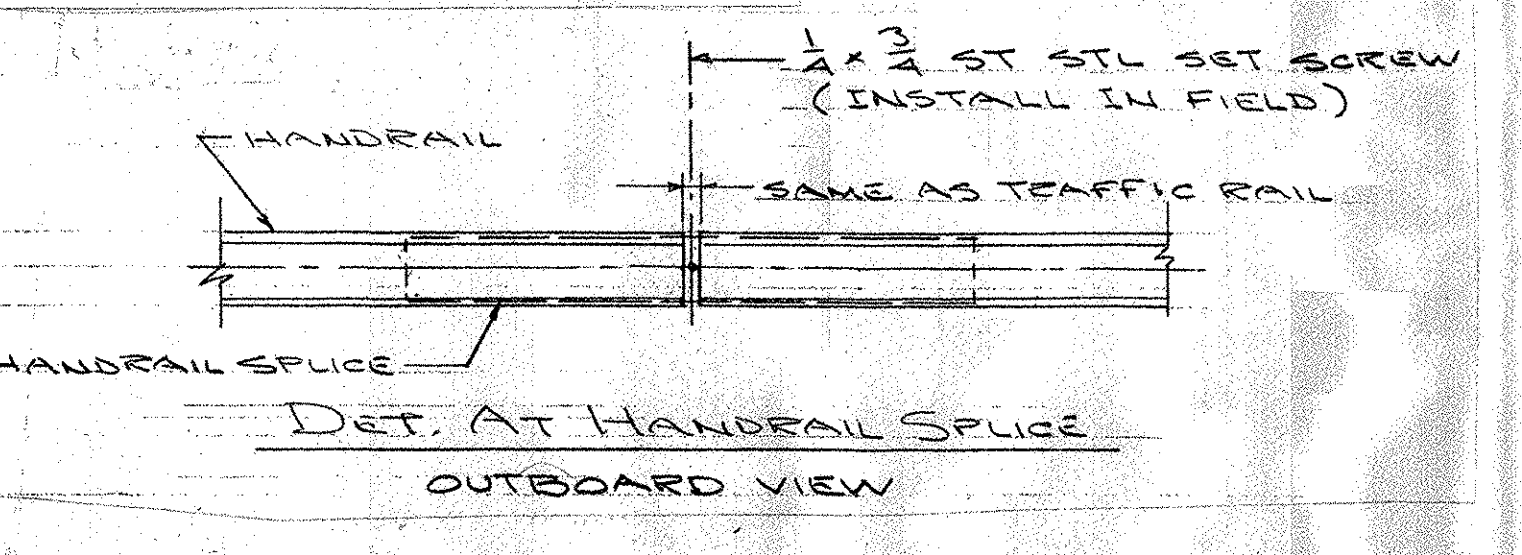
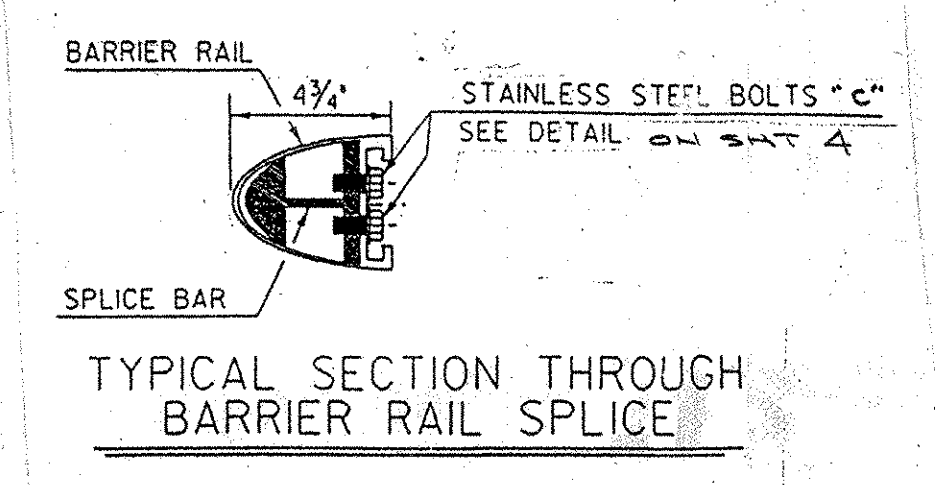
AUCIELLO IRON WORKS INC
560 MAIN ST. HUDSON, MA (978) 668-8382
VT AGENCY OF TRANSPORTATION
HUNTINGTON PROJ. BRO 1445 (29)
BRIDGE #42
EAST ST OVER HUNTINGTON RIVER

AL BRIDGE & APPROACH RAILING
SURFACE PREP: NONE FINISH: MILL FINISH
FOR: A.D. ROSSI CORP.
DR: VM 2-16-07 DWG NO.
CHK: E 2-18-07 BR: 2898
JOB NO. A870002-1001 SHEET 1 OF 3

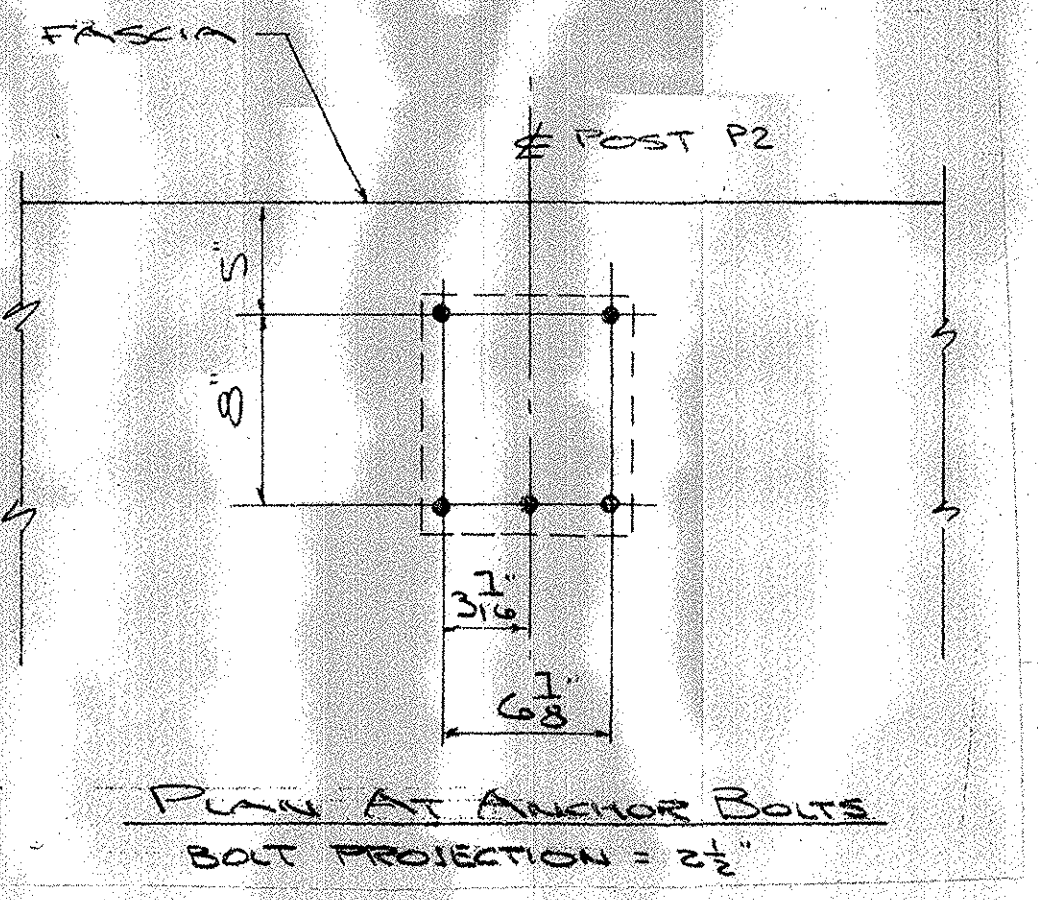
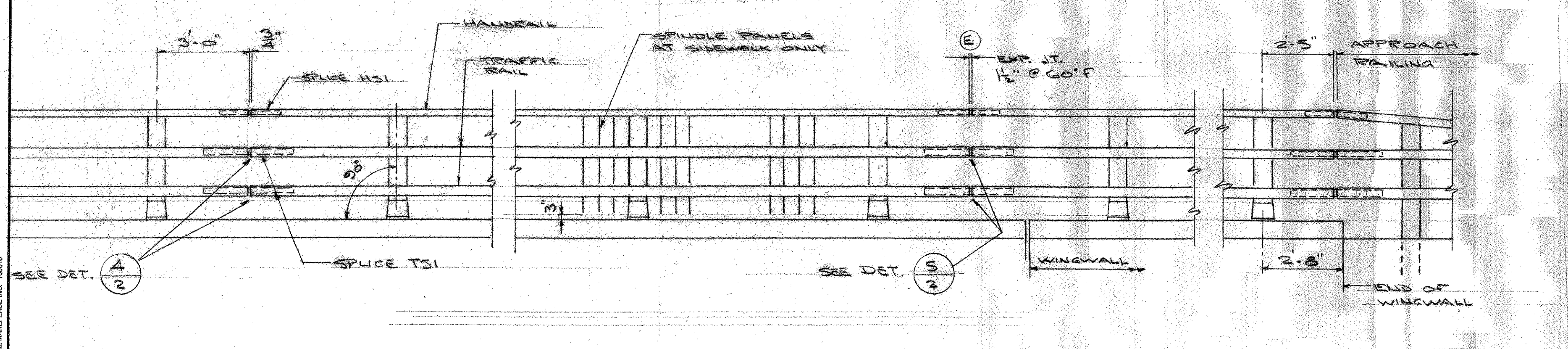
1	2-22-07	FOR APPROVAL	
ISSUE	DATE	DESCRIPTION	BY



- 1 #4 CLAMP BAR
- 2 #1 CLAMP BAR
- 3 1/2"-13 x 3/4" HEX HD CAP SCREW (A193, GR. B8)
- 4 1/2"-13 x 1" HEX HD CAP SCREW (A193, GR. B8)
- 5 1/2" #4 AL RIVET (6061-T6)
- 6 AL WASHER 1 1/2" OD x 1 1/2" ID x 3/32" THK.
- 7 1/2"-13 x 1 1/2" HEX HD CAP SCREW (304 ST. STL) 75,000 TENSILE
- 8 1/2"-13 HEX NUT (304 ST. STL) 75,000 TENSILE
- 9 REFLECTORIZED DELINEATOR (WHITE) PER SPECS. LOCATE EVERY 30' (OR CLOSEST POST). TOP AT BRIDGE RAIL & APPROACH RAIL AT CURB SIDE ONLY.
- 10 #8 x 3/4" ST. STL. SELF-TAPPING SCREW. DRILL POSTS IN FIELD.



POST NO.	RAIL HEIGHT DIMENSIONS			OFFSET BLOCK DIMENSIONS		
	A	B	C	D	E	F
1	4'-1 1/8"	3'-2 3/8"	1'-9"	11/16"	1'-5 1/8"	2'-9 1/8"
2	3'-9 3/8"	3'-0 3/8"	1'-8 3/4"	9/8"	1'-3 3/8"	2'-6 3/8"
3	3'-7 3/8"	2'-11 1/2"	1'-8 3/4"	8/8"	1'-2 3/4"	2'-3 1/4"
4	3'-4 1/8"	2'-10 1/8"	1'-9 3/8"	6/16"	1'-1 1/4"	2'-0 1/4"



REVIEW DRAWINGS ARE REVIEWED UNLESS NOTED OTHERWISE

NO EXCEPTION TAKEN	<input checked="" type="checkbox"/>
REVISE AS NOTED	<input checked="" type="checkbox"/>
RESUBMISSION NOT REQUIRED	<input checked="" type="checkbox"/>
REVISE AS NOTED	<input checked="" type="checkbox"/>
RESUBMISSION REQUIRED	<input checked="" type="checkbox"/>
REJECTED	<input type="checkbox"/>

DATE: 3/14/07

SIGNATURE: *Michael Chavitt*

REVIEWER'S STATEMENT: I AM THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS. COORDINATION OF SUB TRADES, DETAIL DESIGN OF COMPONENTS AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.

AUCIELLO IRON WORKS INC
560 MAIN ST. HUDSON, MA (978) 568-8382

VT AGENCY OF TRANSPORTATION
HUNTINGTON PROJ BRO 1445 (29)
BRIDGE # 42
EAST ST. OVER HUNTINGTON RIVER

AL. BRIDGE & APPROACH RAILING

SURFACE PREP: NONE	FINISH: MILL FINISH
FOR: A.D. ROSSI CORP.	
DRWM 2-16-07	DWG. NO. BR-2898
CHK'D 2-18-07	
JOB NO. AB10002-1001	SHEET 2 OF 8

RECEIVED

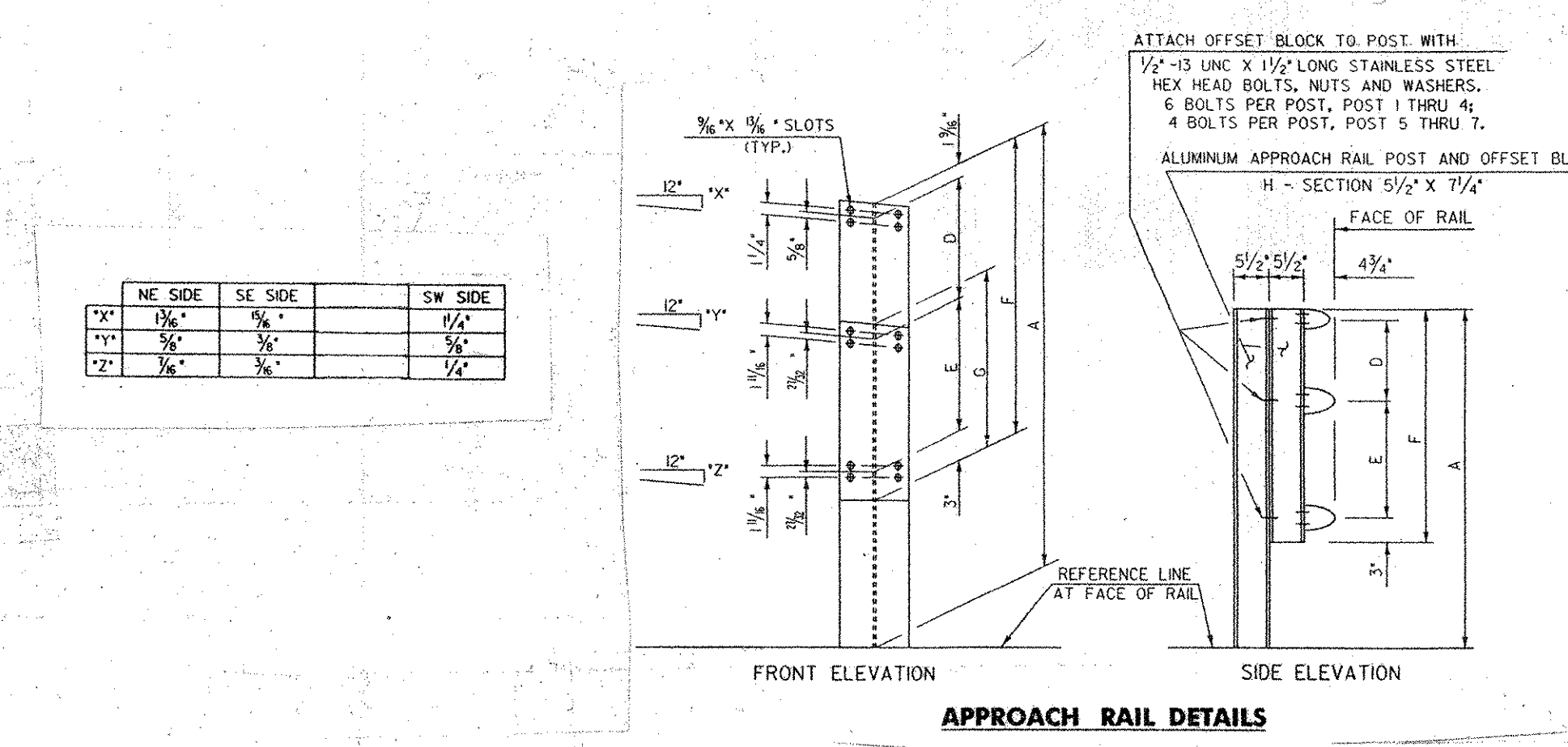
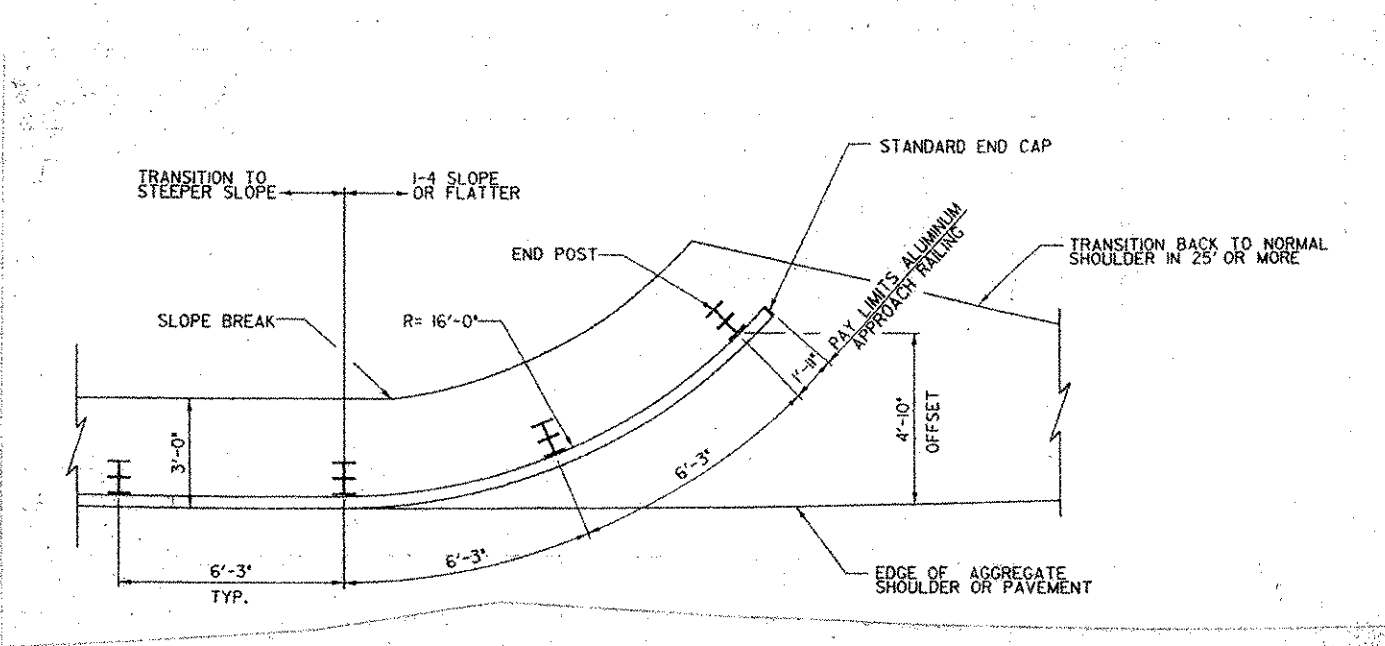
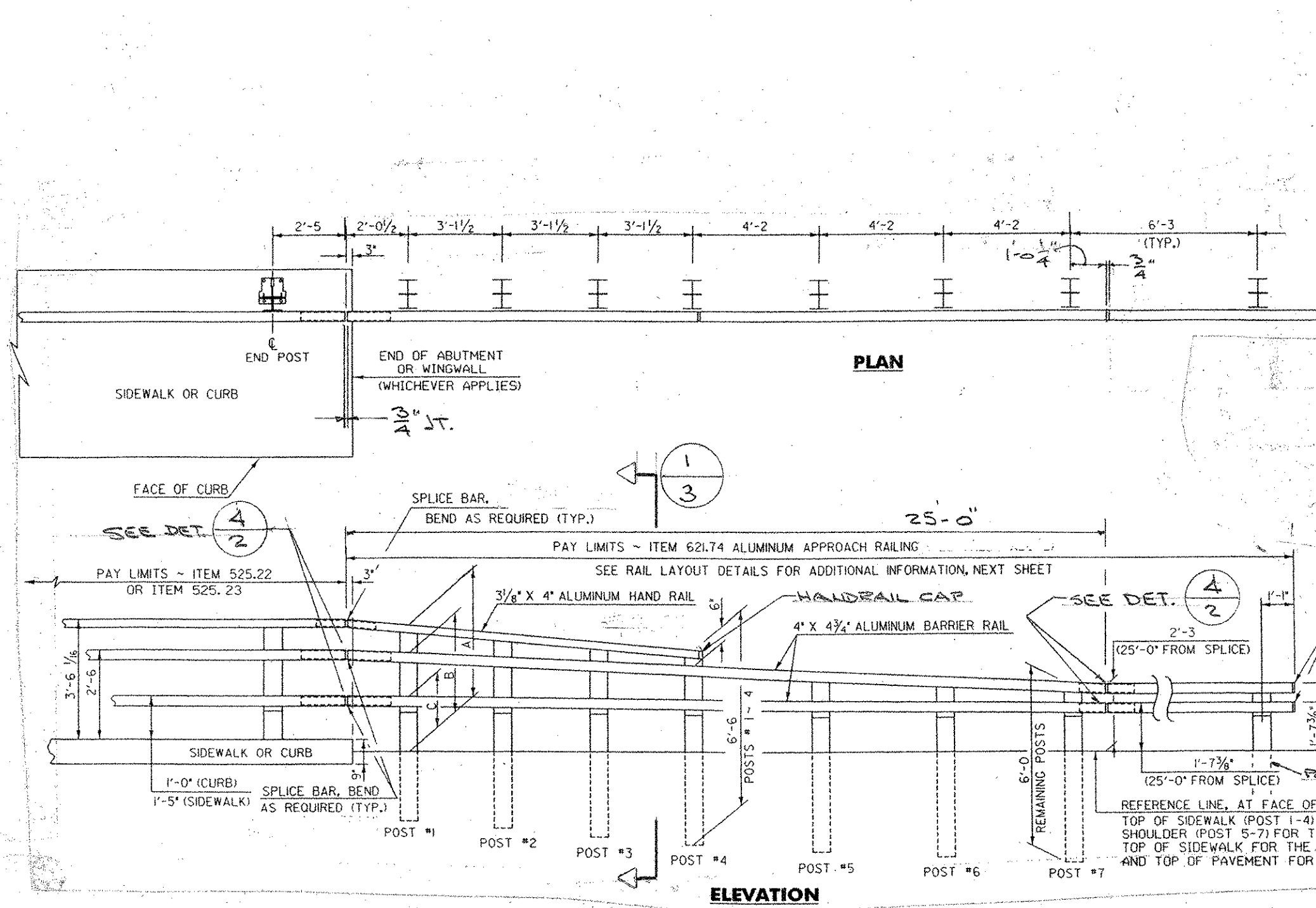
CHK'D BY: *[Signature]* DATE: *[Date]*

RESUBMITTED BY: *[Signature]* DATE: *[Date]*

APPROVED BY: *[Signature]* DATE: *[Date]*

1 2-22-07 FOR APPROVAL

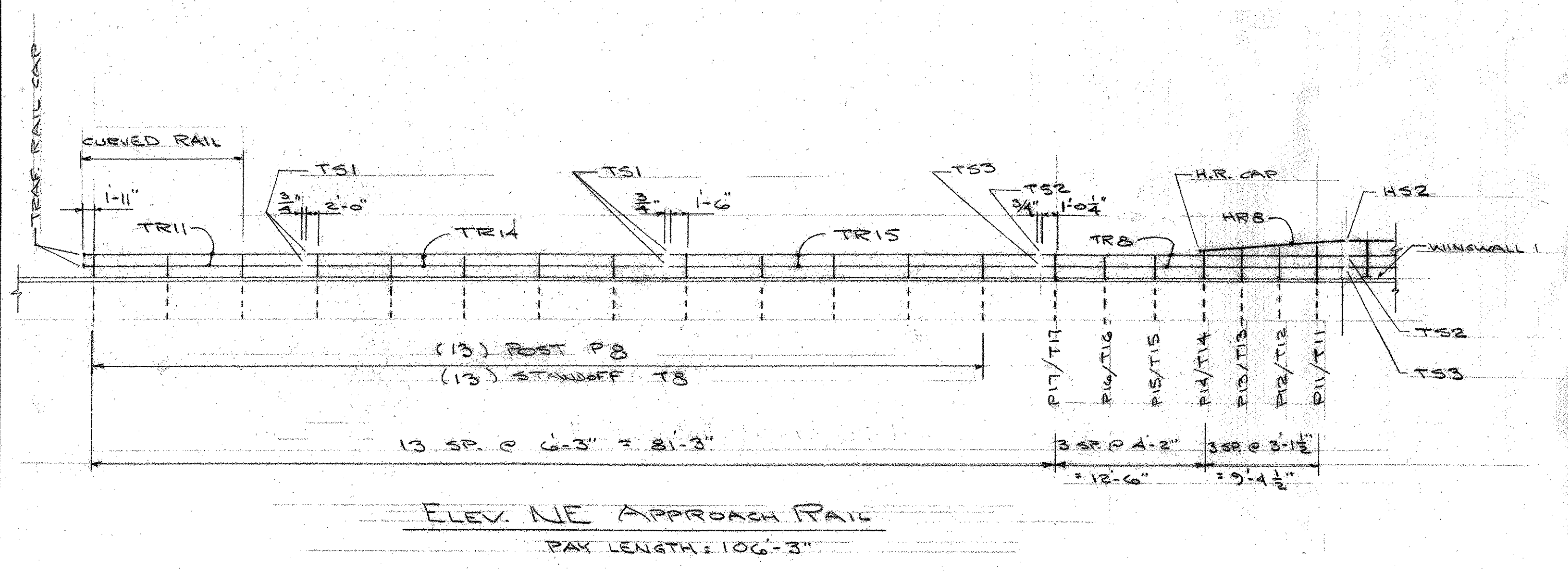
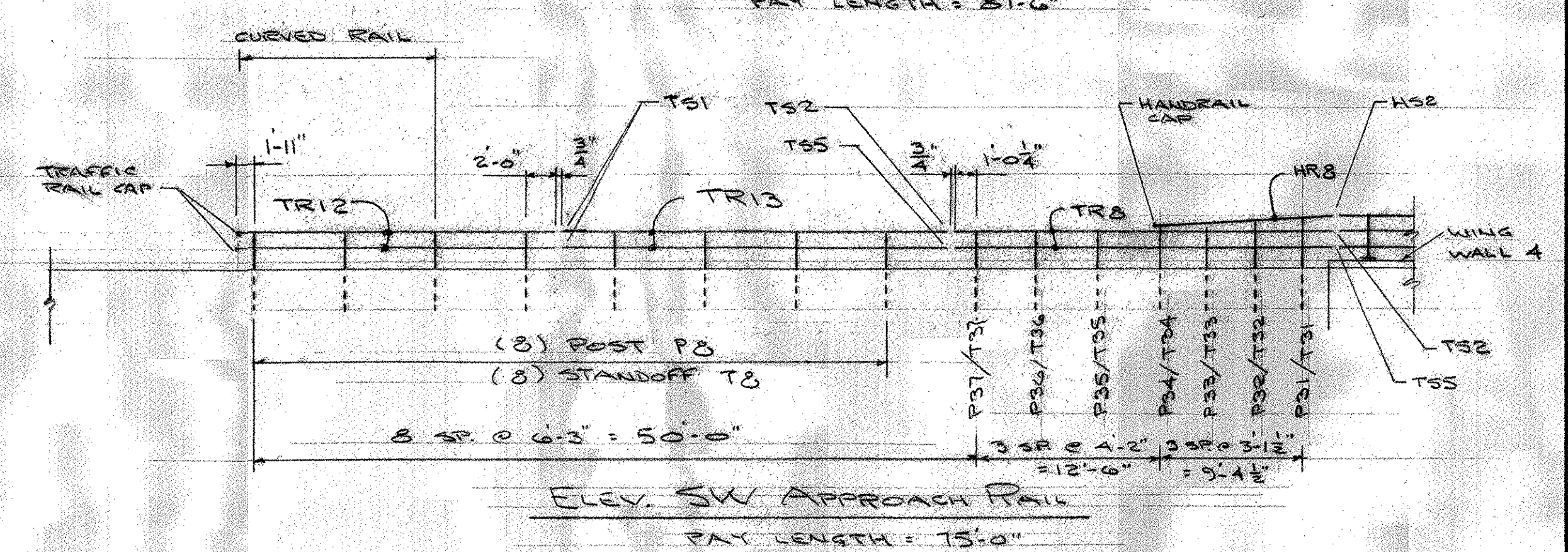
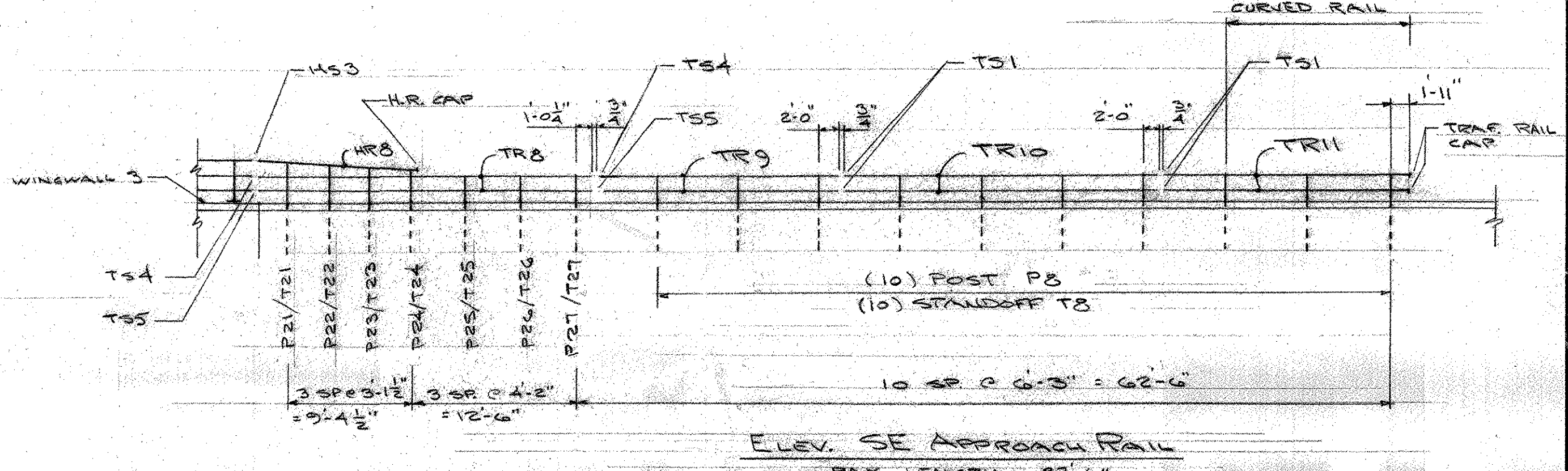
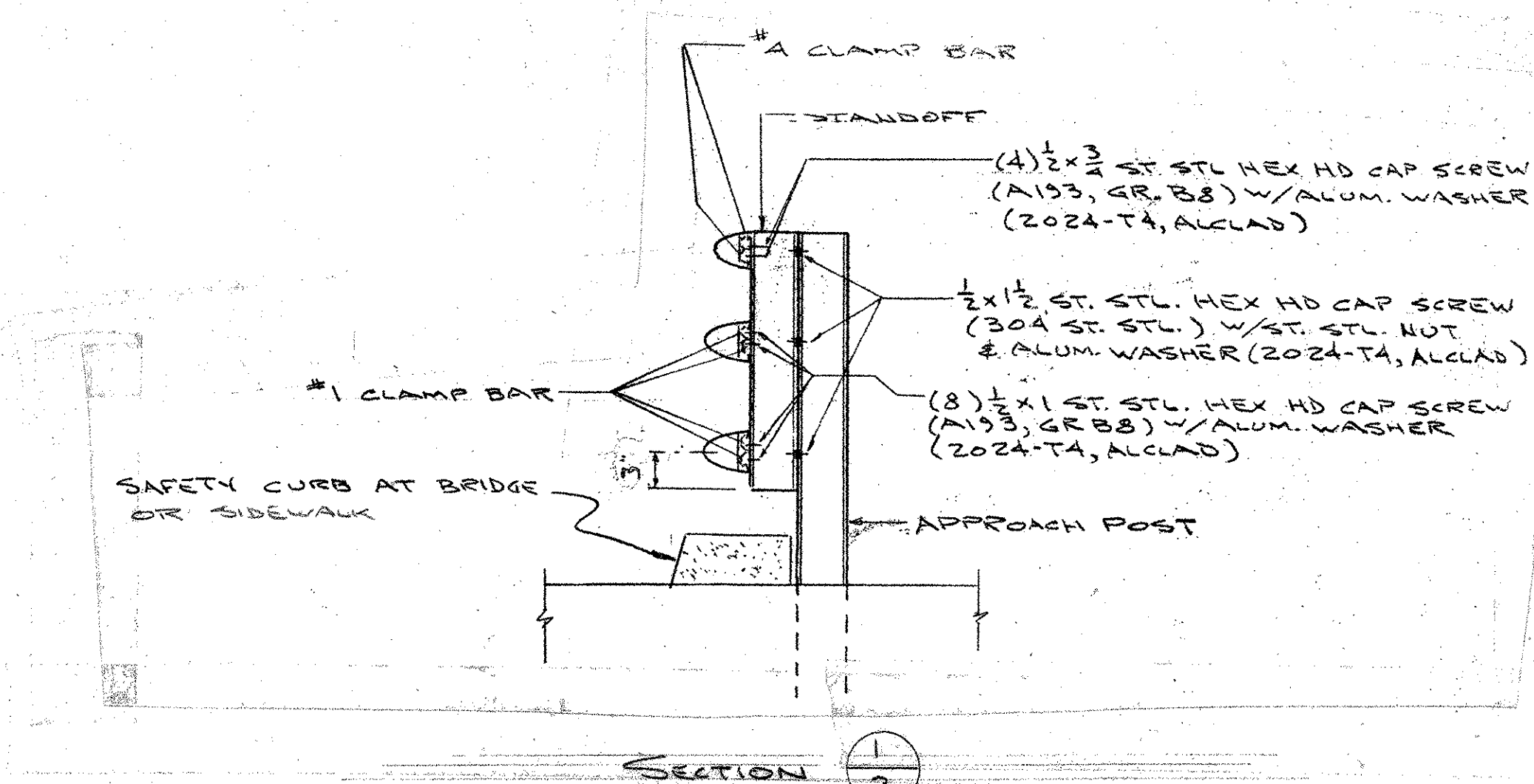
ISSUE	DATE	DESCRIPTION	BY
1	2-22-07	FOR APPROVAL	



NE ALUMINUM RAIL DIMENSIONS							SE ALUMINUM RAIL DIMENSIONS							SW ALUMINUM RAIL DIMENSIONS						
POST NO.	RAIL HEIGHT DIMENSIONS			OFFSET BLOCK DIMENSIONS			POST NO.	RAIL HEIGHT DIMENSIONS			OFFSET BLOCK DIMENSIONS			POST NO.	RAIL HEIGHT DIMENSIONS			OFFSET BLOCK DIMENSIONS		
A	B	C	D	E	F	G	A	B	C	D	E	F	G	A	B	C	D	E	F	G
1.1	3'-4"	2'-5 1/2"	1'-4 1/2"	8 1/2"	1'-0 1/2"	2'-4 1/2"	2.1	3'-4"	2'-5 1/2"	1'-4 1/2"	8 1/2"	1'-0 1/2"	2'-4 1/2"	3.1	4'-0 1/2"	3'-0 1/2"	1'-8 1/2"	8 1/2"	1'-5 1/2"	2'-9 1/2"
1.2	3'-0 1/2"	2'-3 1/2"	1'-3 1/2"	9 1/2"	1'-0 1/2"	2'-2 1/2"	2.2	3'-3 1/2"	2'-5 1/2"	1'-5 1/2"	9 1/2"	1'-0 1/2"	2'-2 1/2"	3.2	3'-5 1/2"	2'-10 1/2"	1'-7 1/2"	8 1/2"	1'-2 1/2"	2'-3 1/2"
1.3	2'-9 1/2"	2'-2 1/2"	1'-2 1/2"	8 1/2"	1'-8 1/2"	1'-8 1/2"	2.3	3'-0 1/2"	2'-5 1/2"	1'-6 1/2"	8 1/2"	1'-0 1/2"	1'-8 1/2"	3.3	3'-2 1/2"	2'-8 1/2"	1'-7 1/2"	6 1/2"	1'-4 1/2"	2'-0 1/2"
1.4	2'-9 1/2"	2'-3 1/2"	1'-4 1/2"	6 1/2"	1'-9 1/2"	1'-2 1/2"	2.4	2'-11 1/2"	2'-5 1/2"	1'-7 1/2"	6 1/2"	1'-0 1/2"	1'-9 1/2"	3.4	3'-2 1/2"	2'-8 1/2"	1'-7 1/2"	6 1/2"	1'-4 1/2"	2'-0 1/2"
1.5	2'-5 1/2"	2'-7 1/2"	1'-7 1/2"	9 1/2"	1'-2 1/2"	1'-2 1/2"	2.5	2'-5 1/2"	1'-7 1/2"	1'-7 1/2"	9 1/2"	1'-2 1/2"	1'-2 1/2"	3.5	2'-5 1/2"	1'-7 1/2"	1'-7 1/2"	9 1/2"	1'-2 1/2"	1'-4 1/2"
1.6	2'-4 1/2"	1'-7 1/2"	1'-7 1/2"	8 1/2"	1'-3 1/2"	1'-3 1/2"	2.6	2'-4 1/2"	1'-7 1/2"	1'-7 1/2"	8 1/2"	1'-3 1/2"	1'-3 1/2"	3.6	2'-5 1/2"	1'-7 1/2"	1'-7 1/2"	9 1/2"	1'-2 1/2"	1'-4 1/2"
1.7	2'-3 1/2"	1'-7 1/2"	1'-7 1/2"	7 1/2"	1'-0 1/2"	1'-0 1/2"	2.7	2'-3 1/2"	1'-7 1/2"	1'-7 1/2"	7 1/2"	1'-0 1/2"	1'-0 1/2"	3.7	2'-3 1/2"	1'-7 1/2"	1'-7 1/2"	8 1/2"	1'-0 1/2"	1'-4 1/2"

ALL REMAINING POSTS TO HAVE THE SAME DIMENSIONS AS POST NO. 7

TYP. DETAILS AT APPROACH RAILING
SE CORNER AS SHOWN
NE & SW CORNERS OPP. HAND



SHOP DRAWINGS ARE REVIEWED UNLESS NOTED OTHERWISE
NO EXCEPTION TAKEN
REVISE AS NOTED
RESUBMISSION NOT REQUIRED
REVISE AS NOTED
RESUBMISSION REQUIRED
REJECTED

DATE: 3/26/07
MICHAEL J. ROSSI
REVIEWED BY: MICHAEL J. ROSSI
PURPOSE OF ASBESTOS TESTING IS FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DESIGN. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS. COORDINATION OF SUB TRADE DETAIL DESIGN OF COMPONENTS AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.

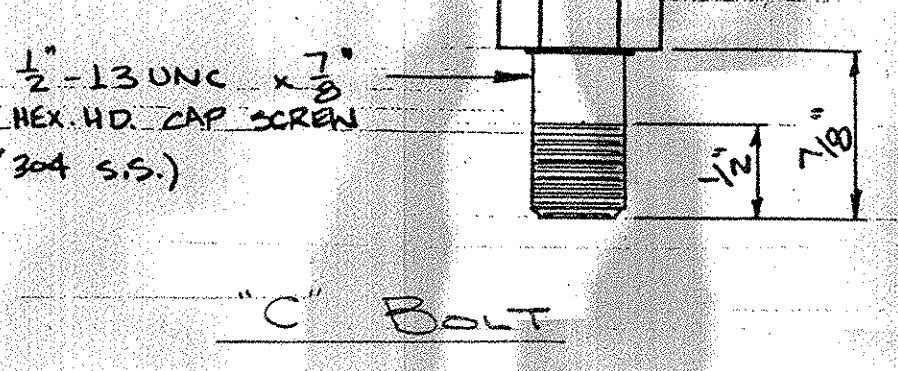
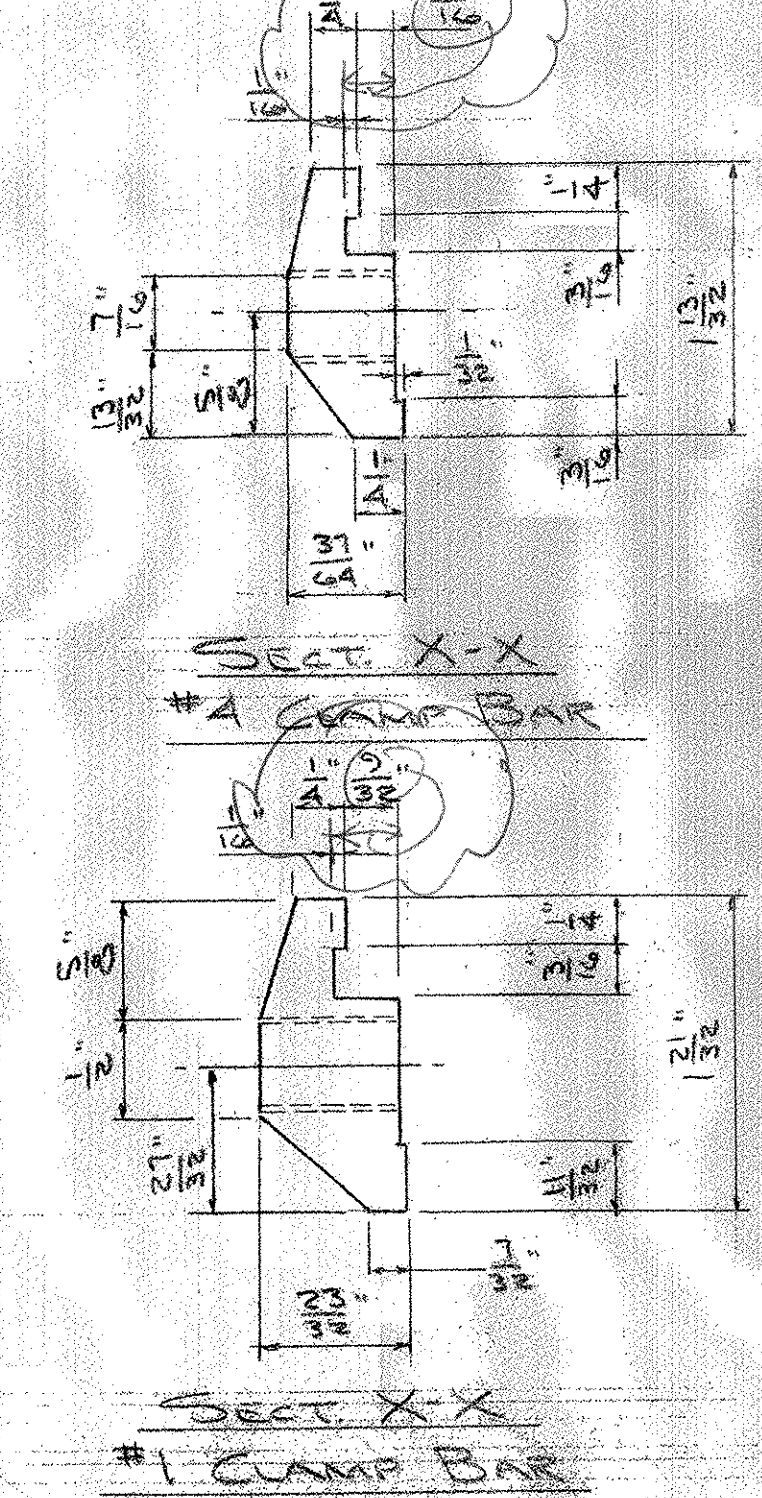
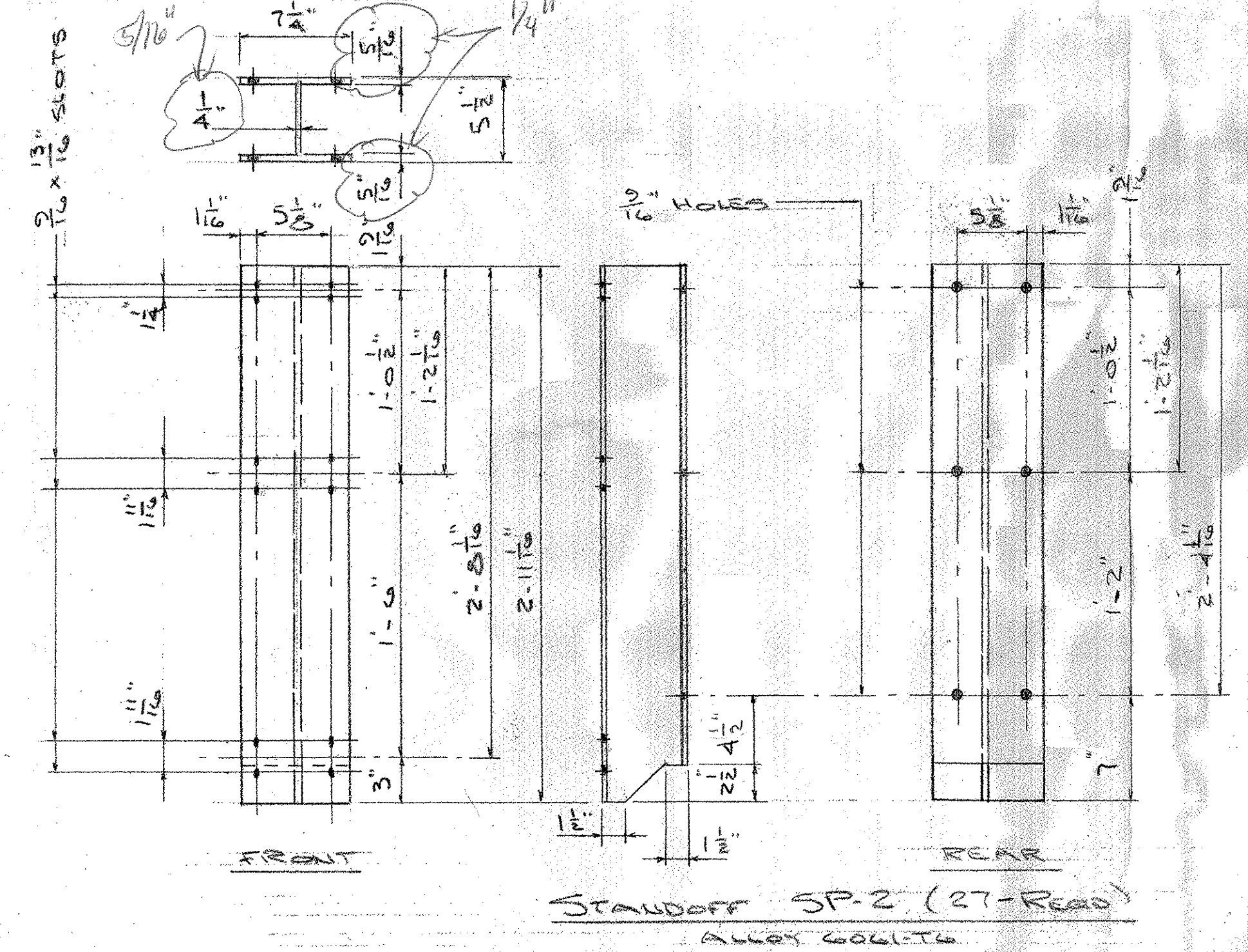
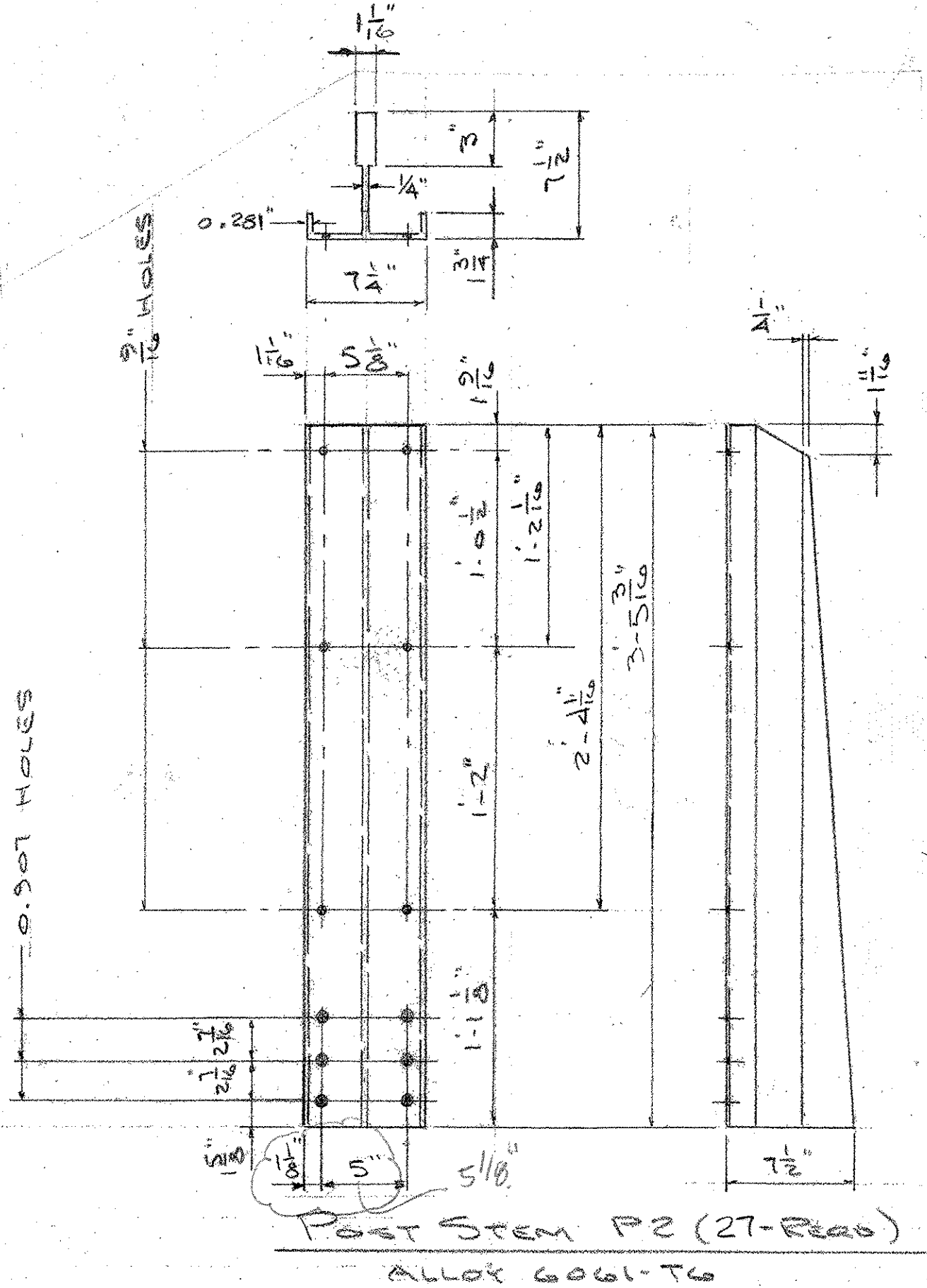
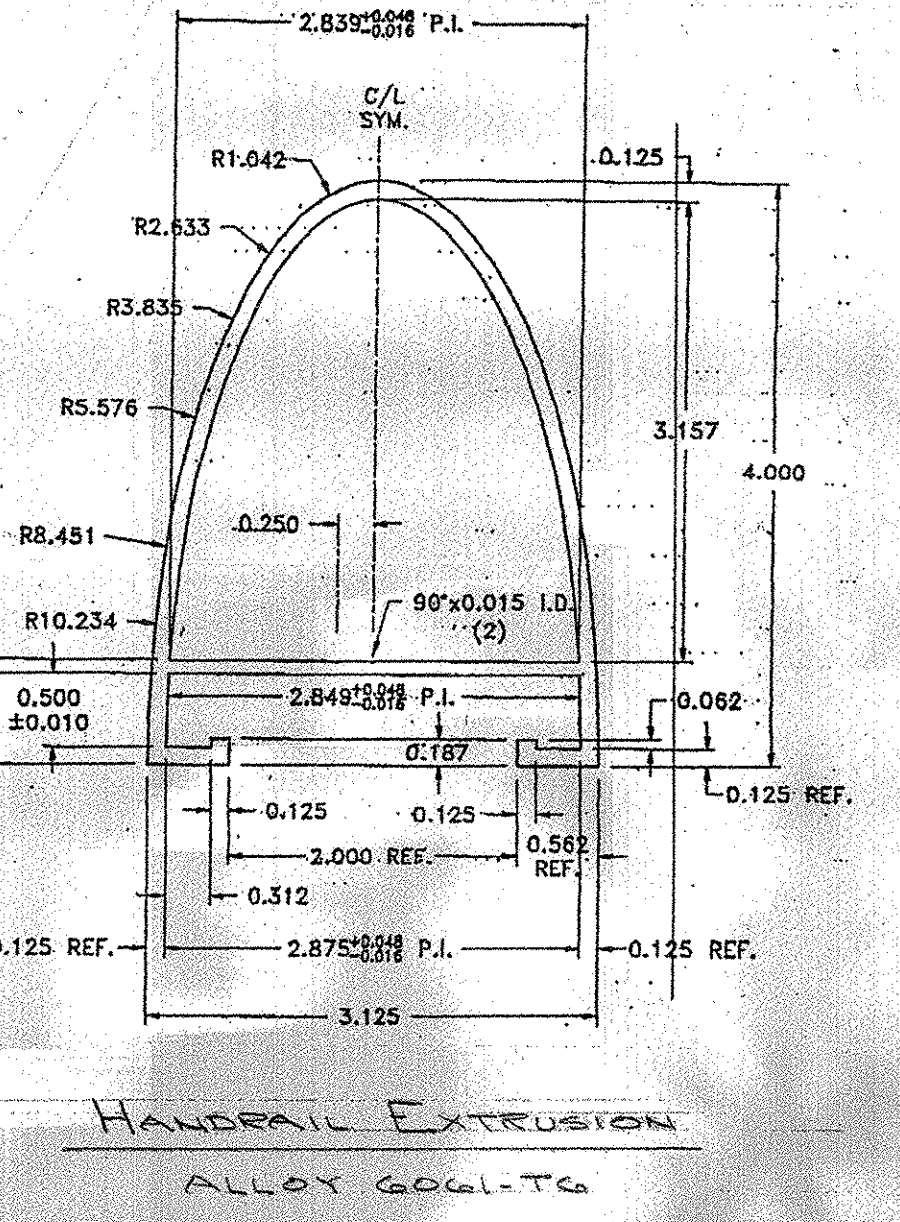
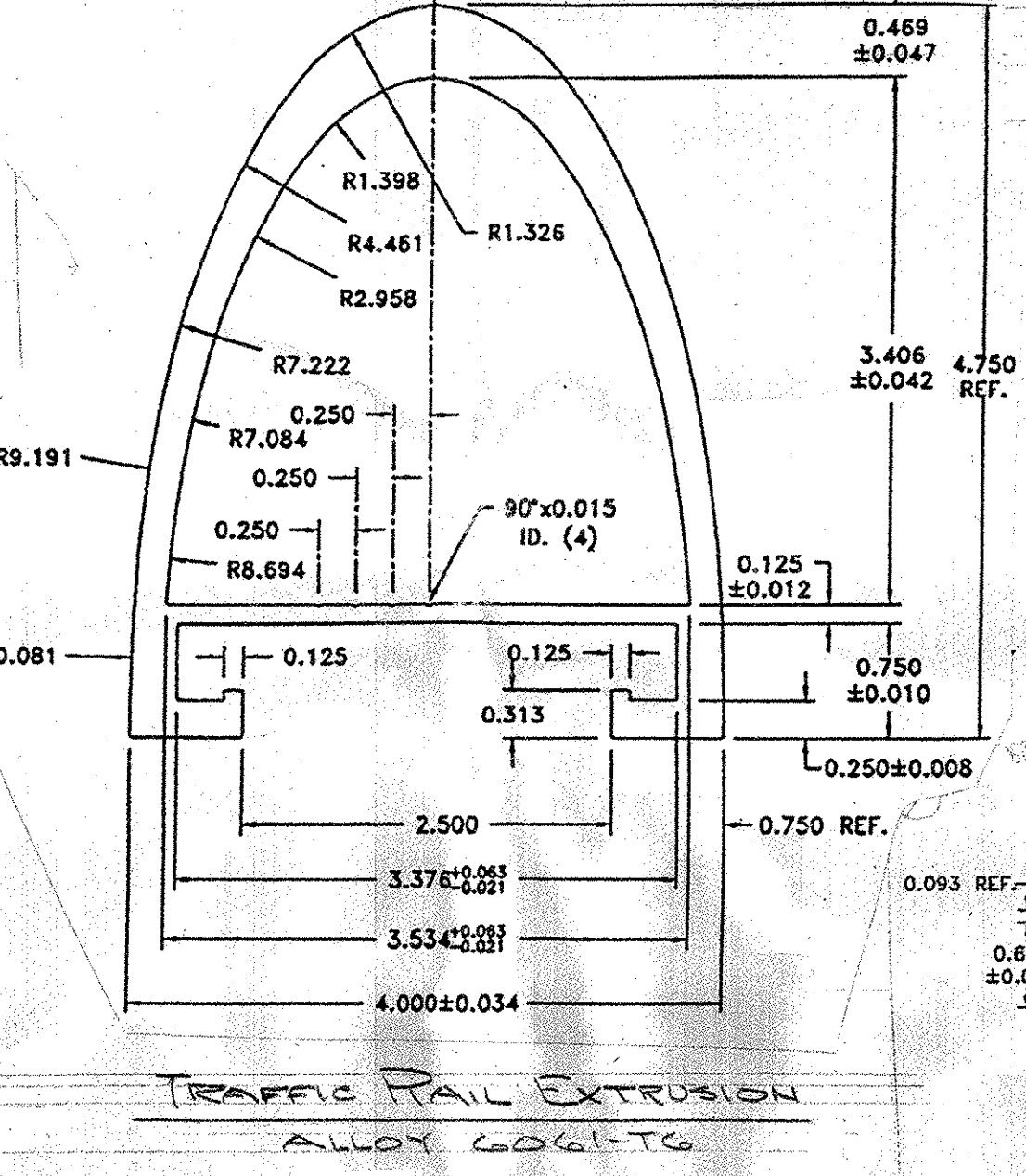
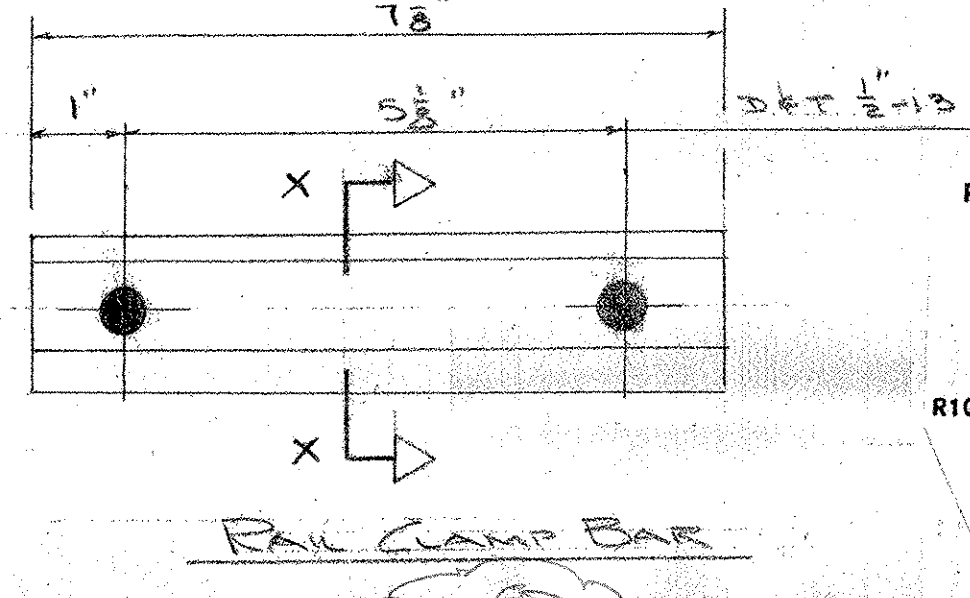
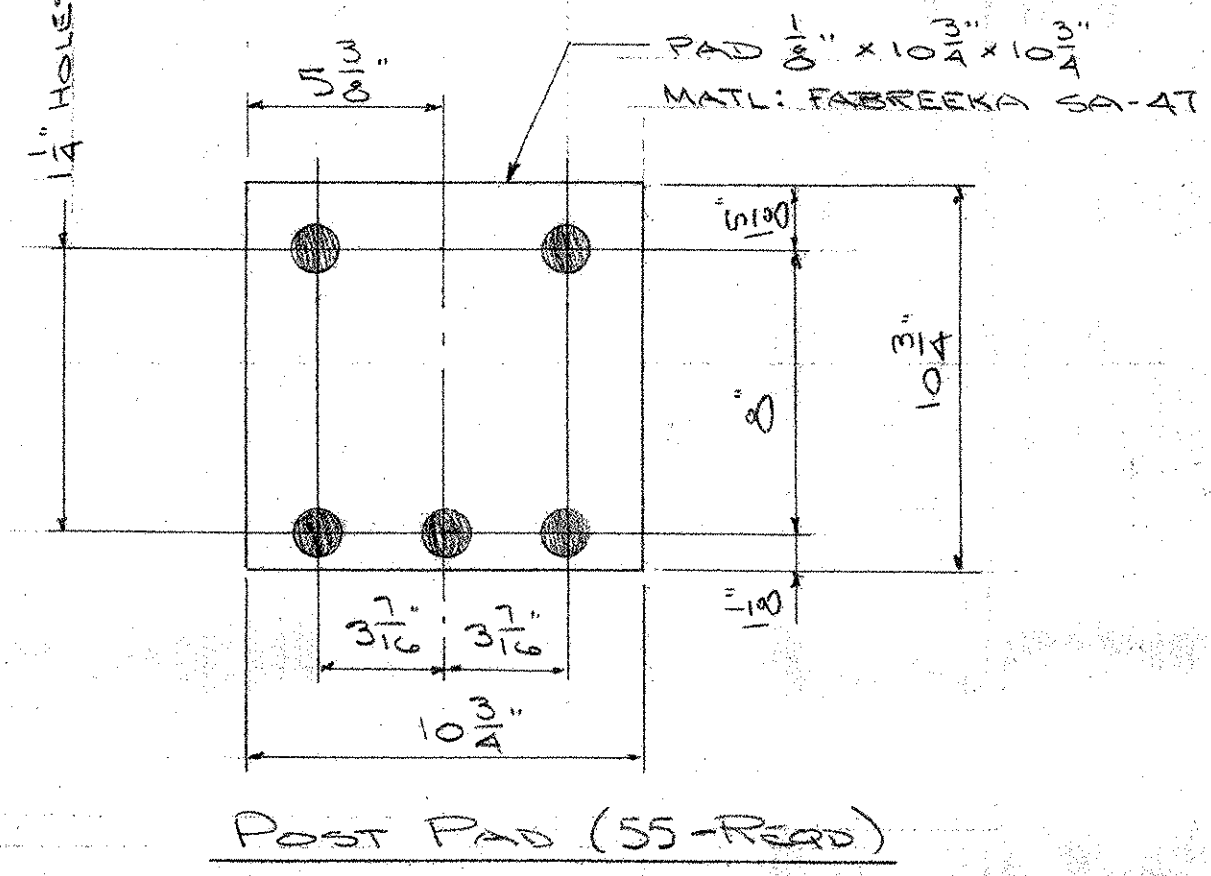
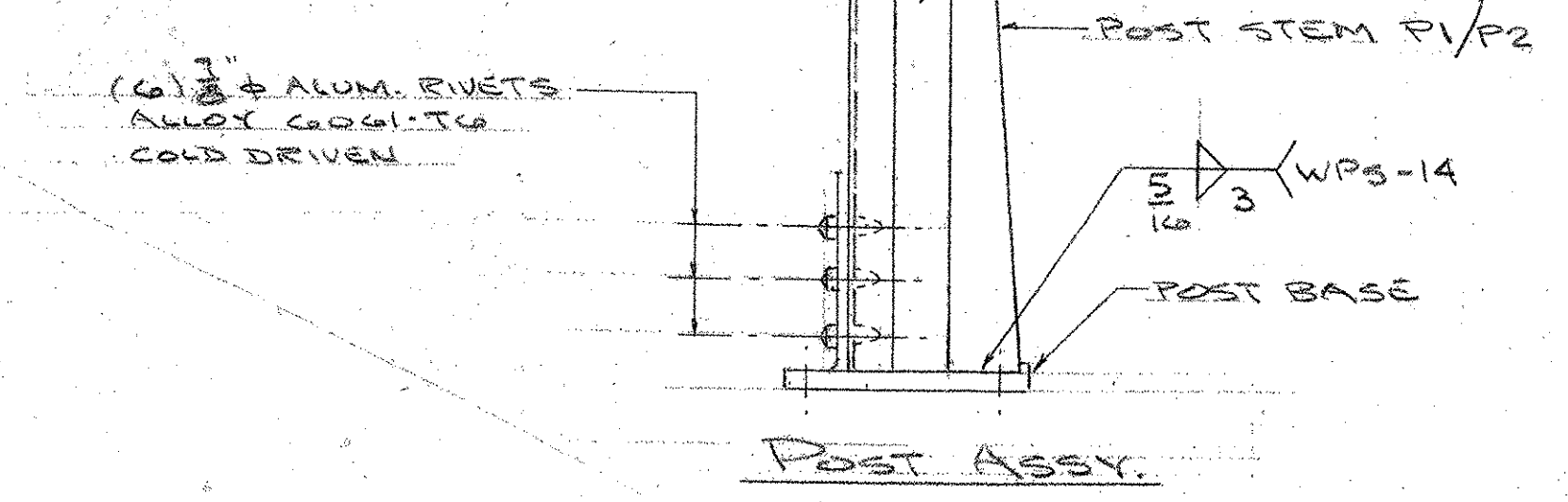
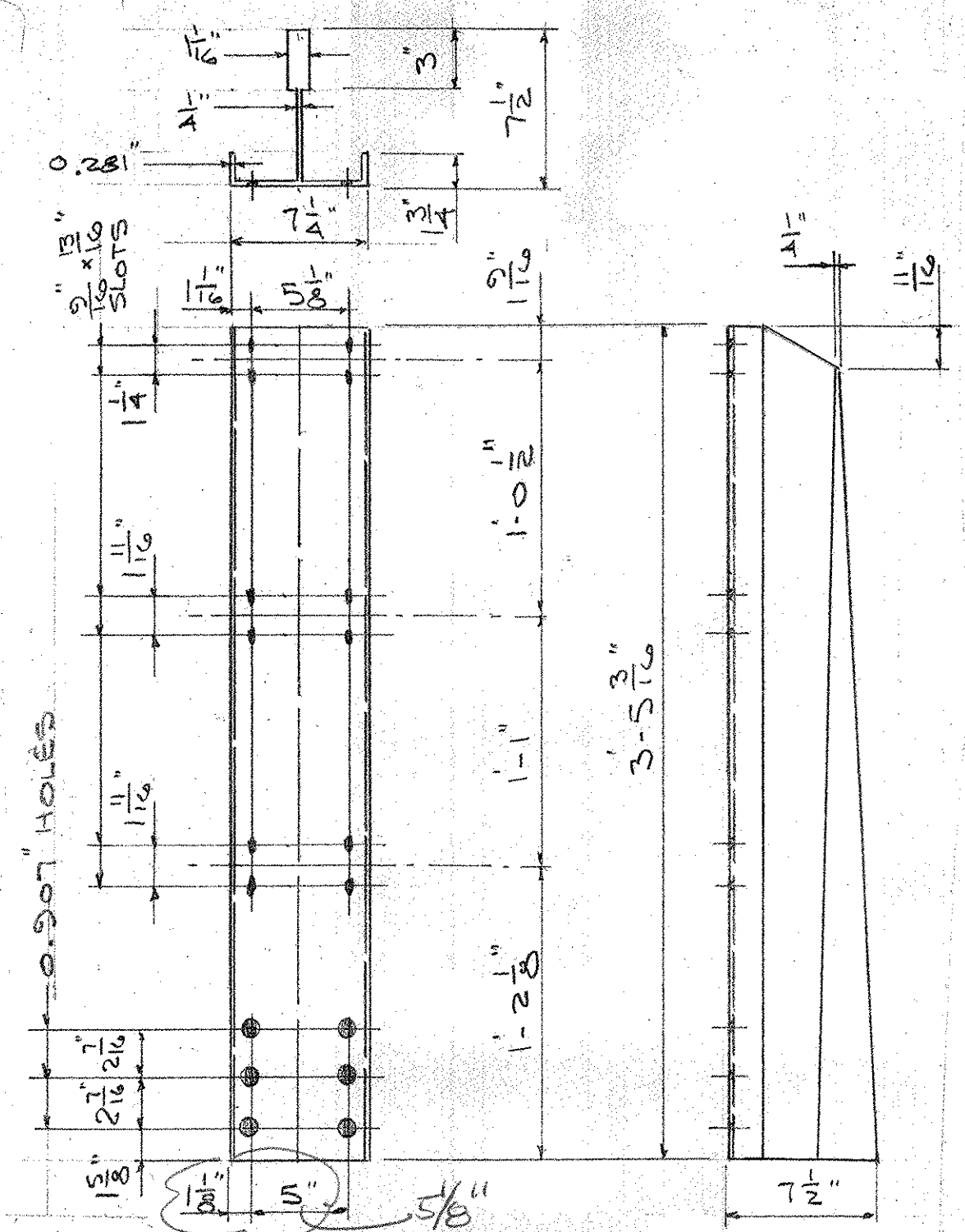
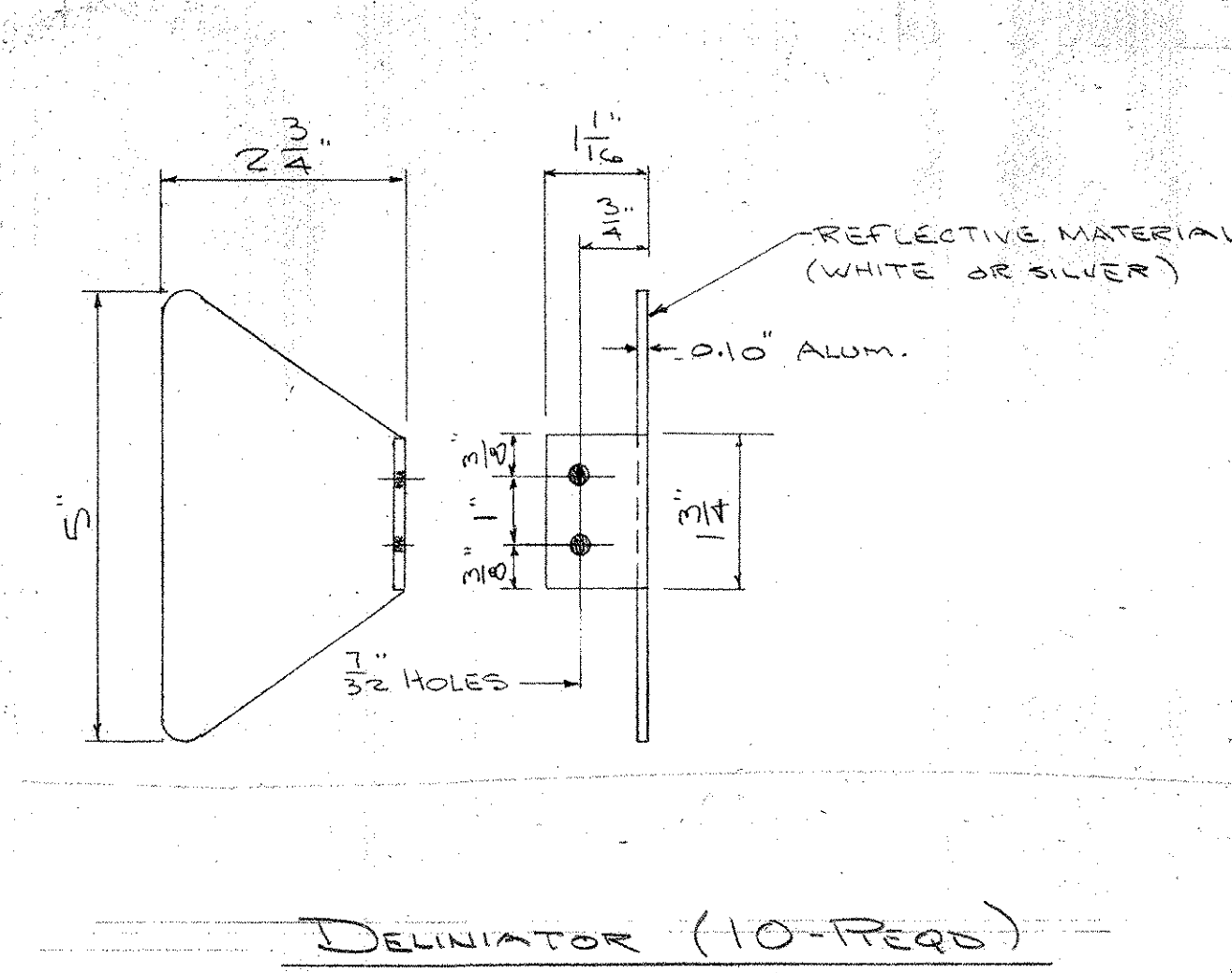
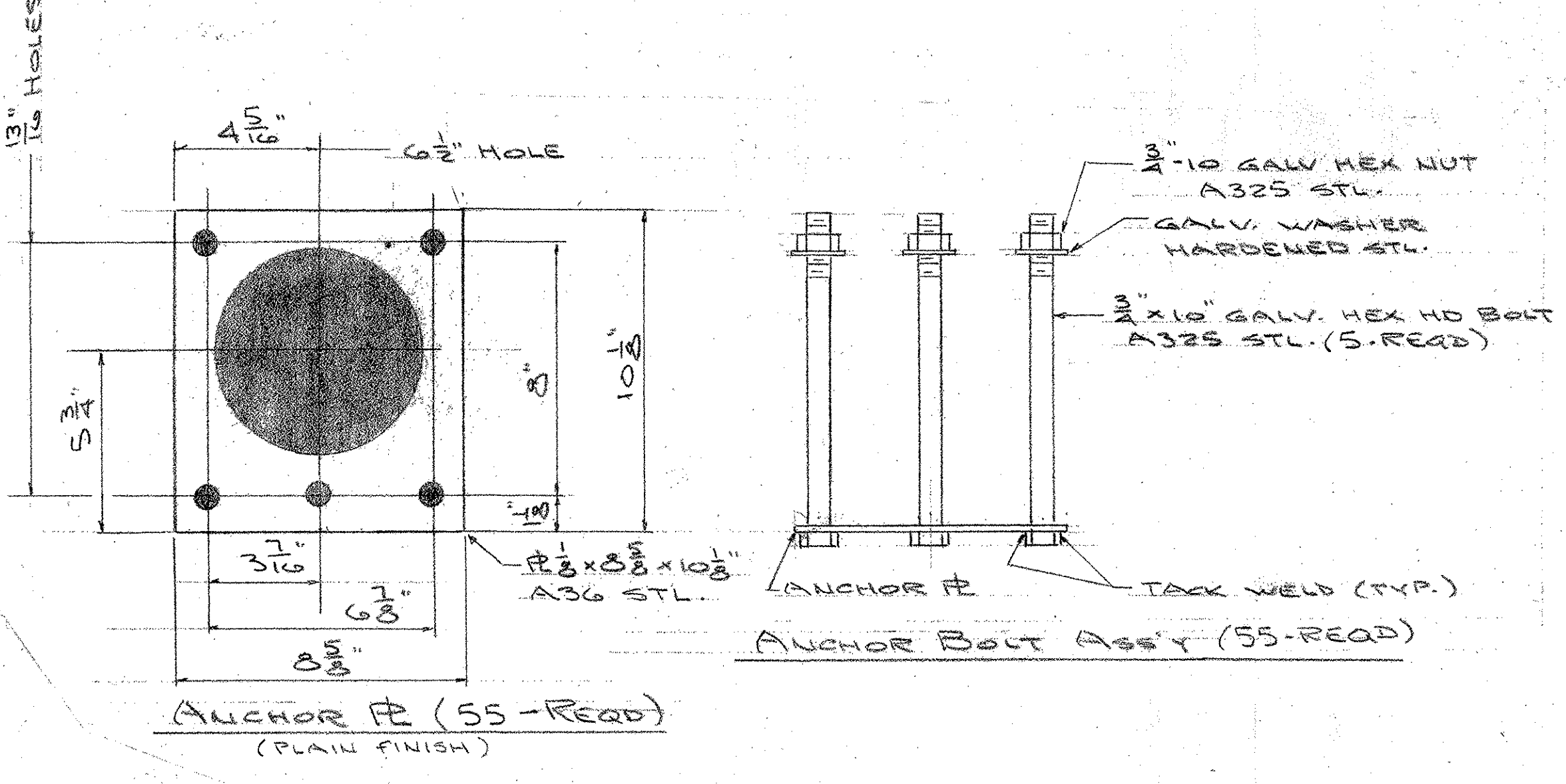
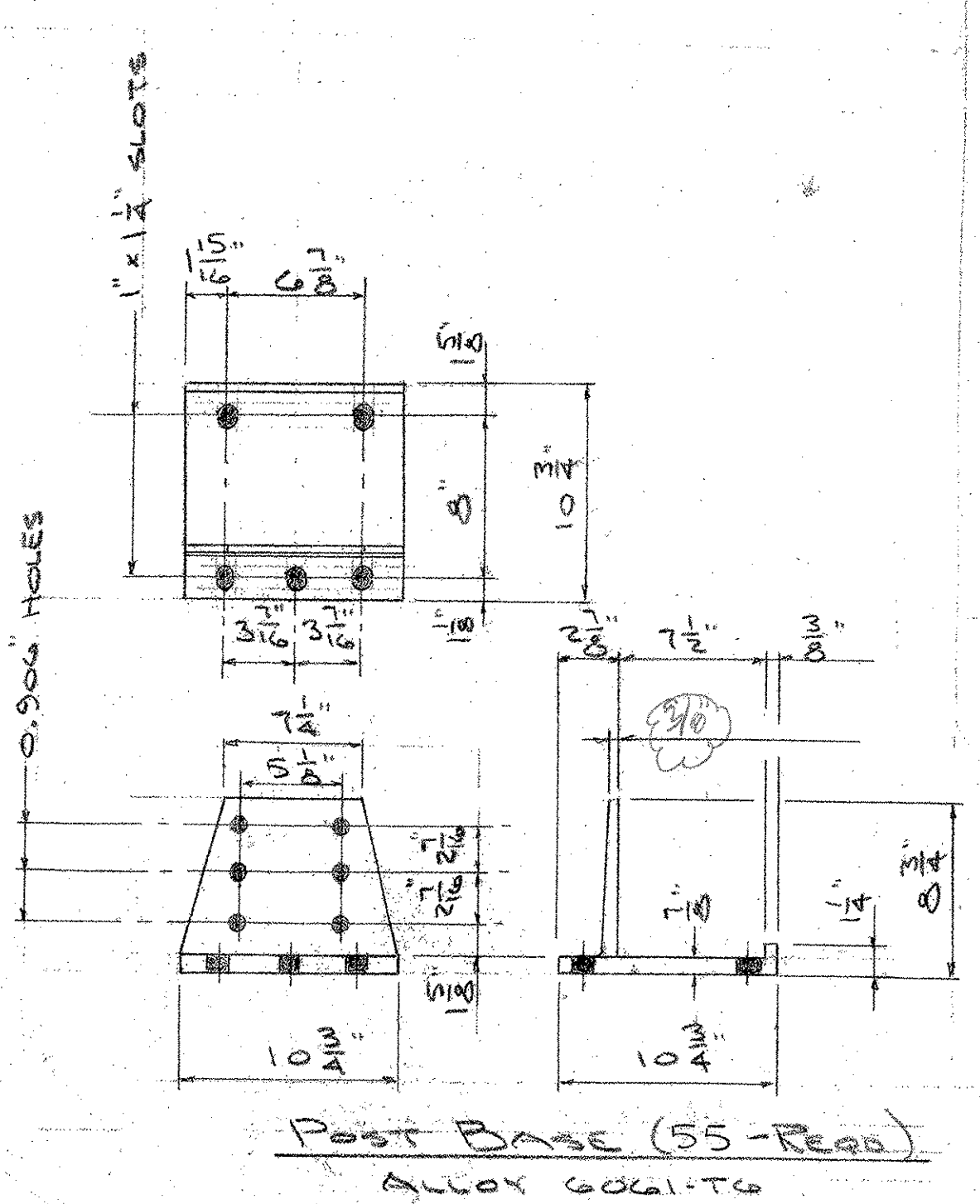
RECEIVED
CR'D BY: [Signature]
DATE: FEB 28 2007
RESUBMIT BY: [Signature]
APPROVED DATE: [Signature]

AUCIELLO IRON WORKS INC
560 MAIN ST. HUDSON, MA (978) 568-8382

VT. AGENCY OF TRANSPORTATION
HUNTINGTON PROJ. BRO 1445 (29)
BRIDGE # 42
EAST ST OVER HUNTINGTON RIVER

AL APPROACH RAILING

SURFACE PREP: NONE	FINISH: MILL FINISH
FOR: A.D. ROSSI CORP.	DR: [Signature] 2-16-07
CHK'D: [Signature] 2-18-07	DWG. NO. BR-2398
ISSUE DATE: 2-22-07	FOR APPROVAL
DESCRIPTION: APPROVAL	BY: [Signature]
JOB NO. AB70002-1001	SHEET 3 OF 3



REVISIONS:
NO EXCEPTION TAKEN
REVISIONS AS NOTED
RESUBMISSION NOT REQUIRED
REVISIONS AS NOTED
RESUBMISSION REQUIRED
REJECTED
DATE: 3/26/07
SIGNATURE: Michael Vant
REVIEWER'S STATEMENT: FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DESIGN, CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS, COORDINATION OF SUB-TRADES, DETAIL DESIGN OF COMPONENTS, AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.

RECEIVED	
OK'D BY	OK'D BY
FEB 28 2007	
RESUBMIT	APPROVED
BY	DATE
1	2/22/07 FOR APPROVAL
ISSUE	DATE DESCRIPTION

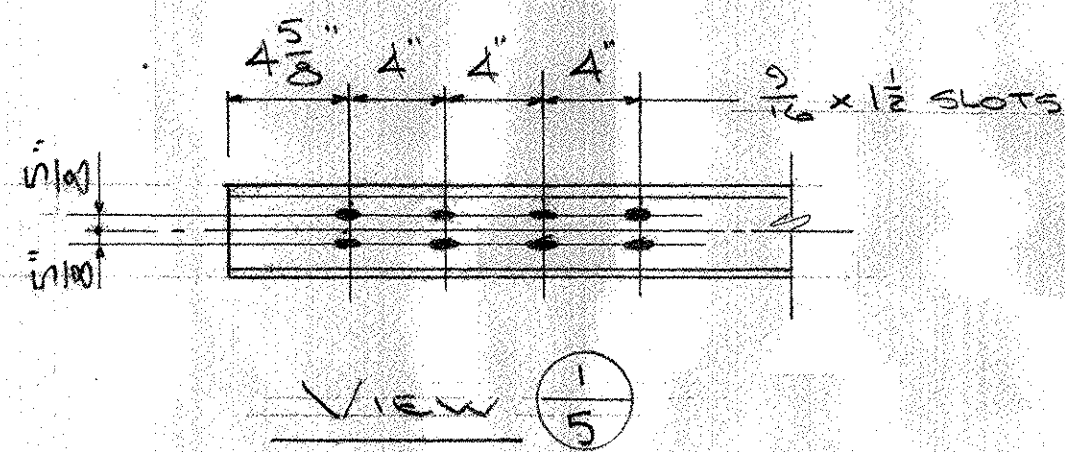
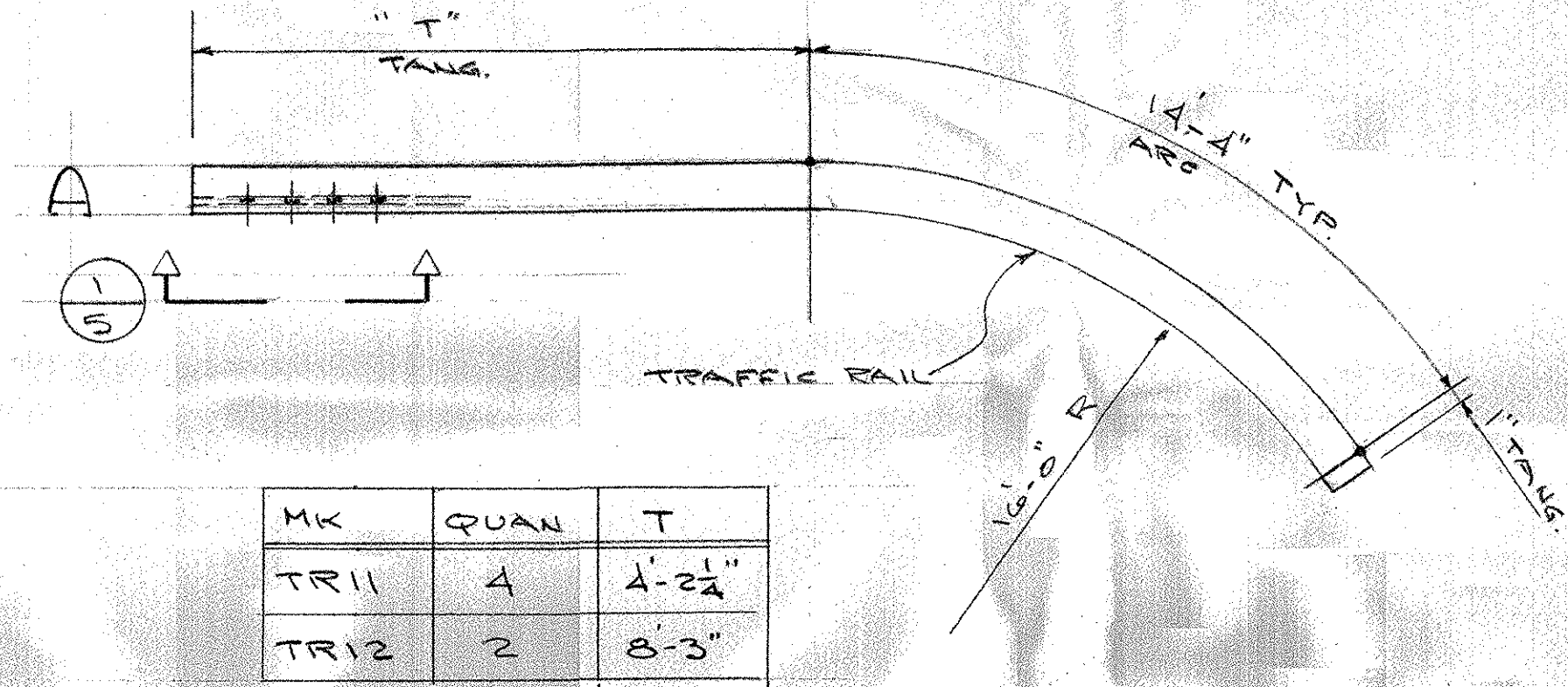
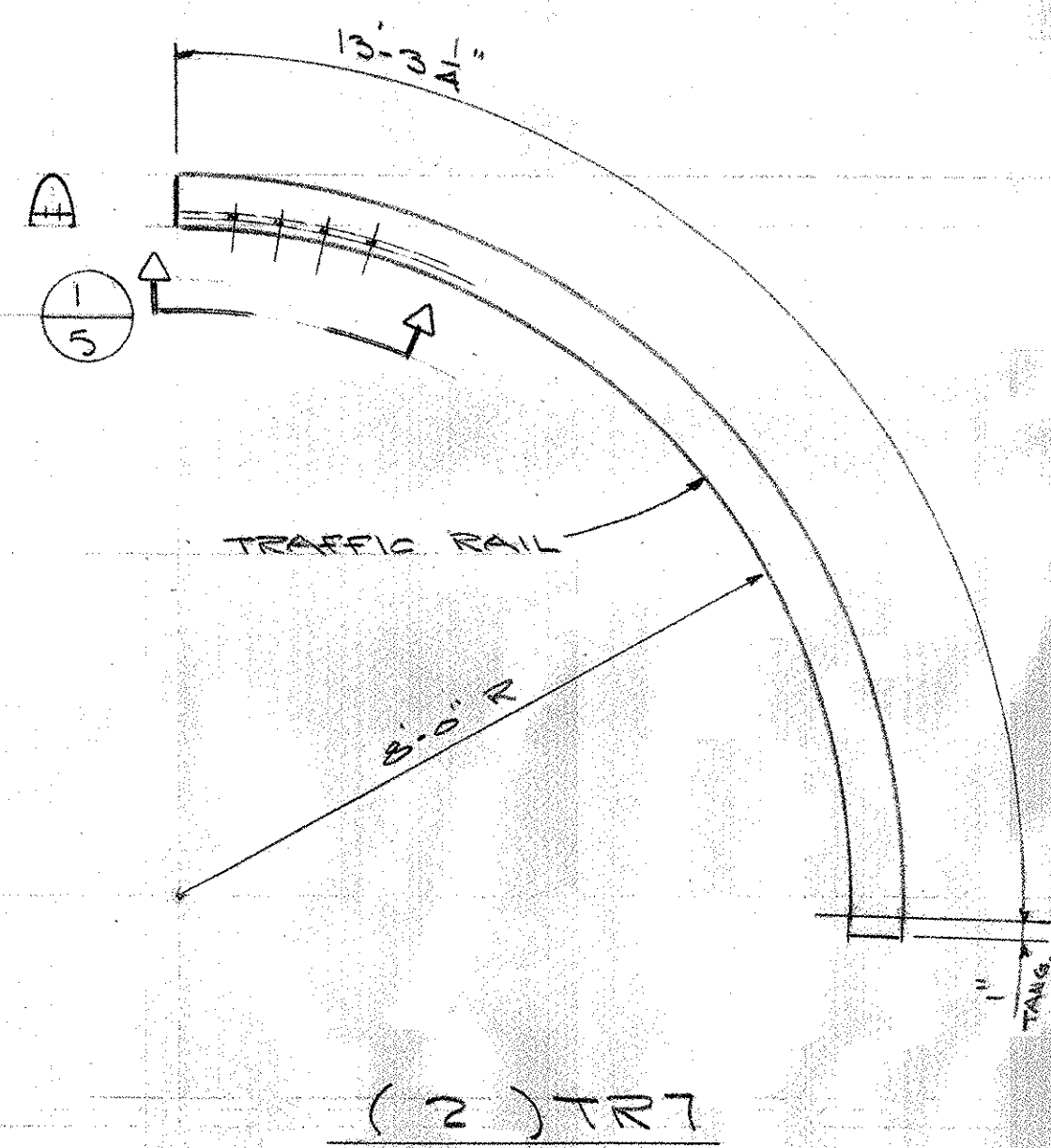
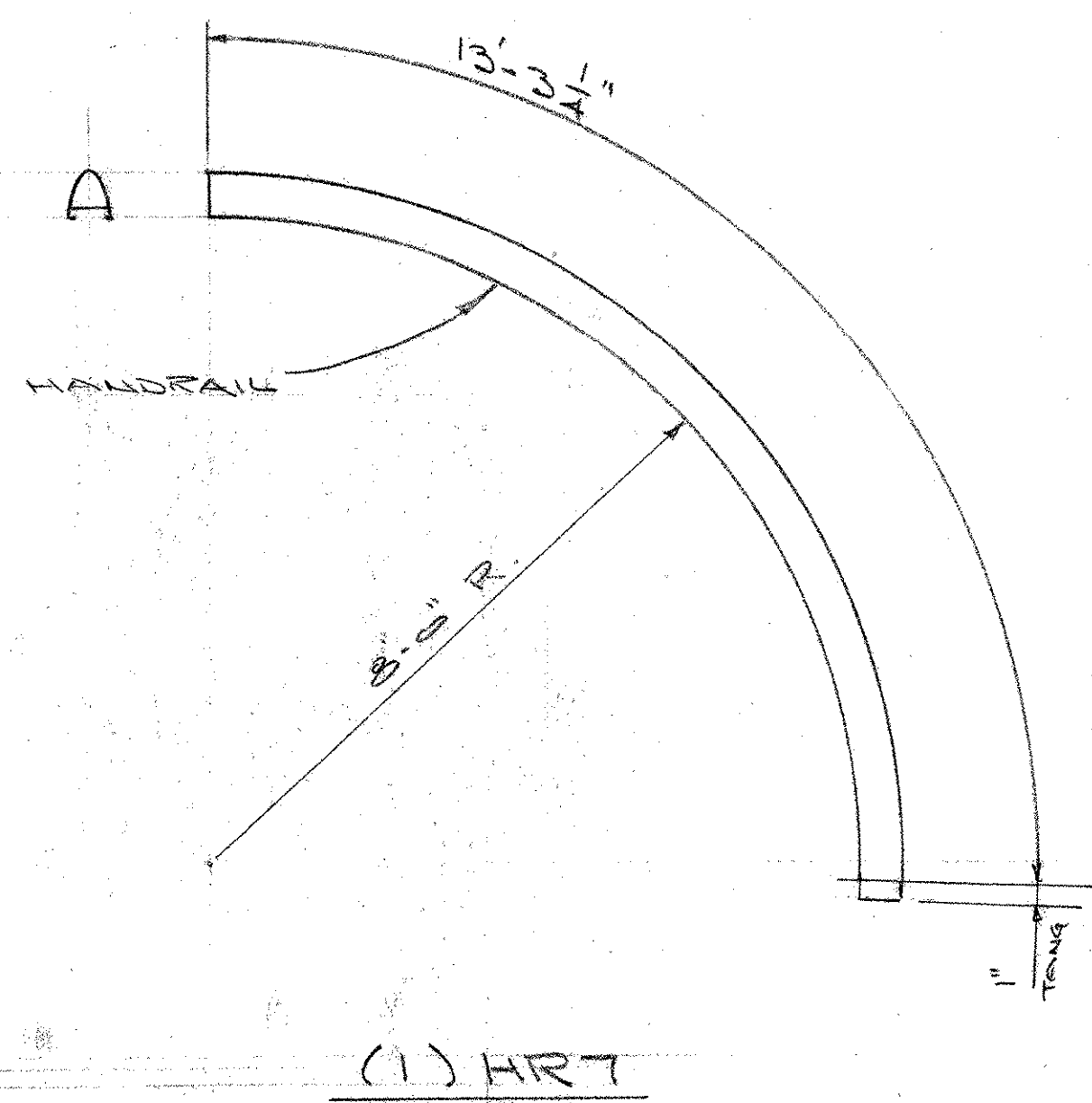
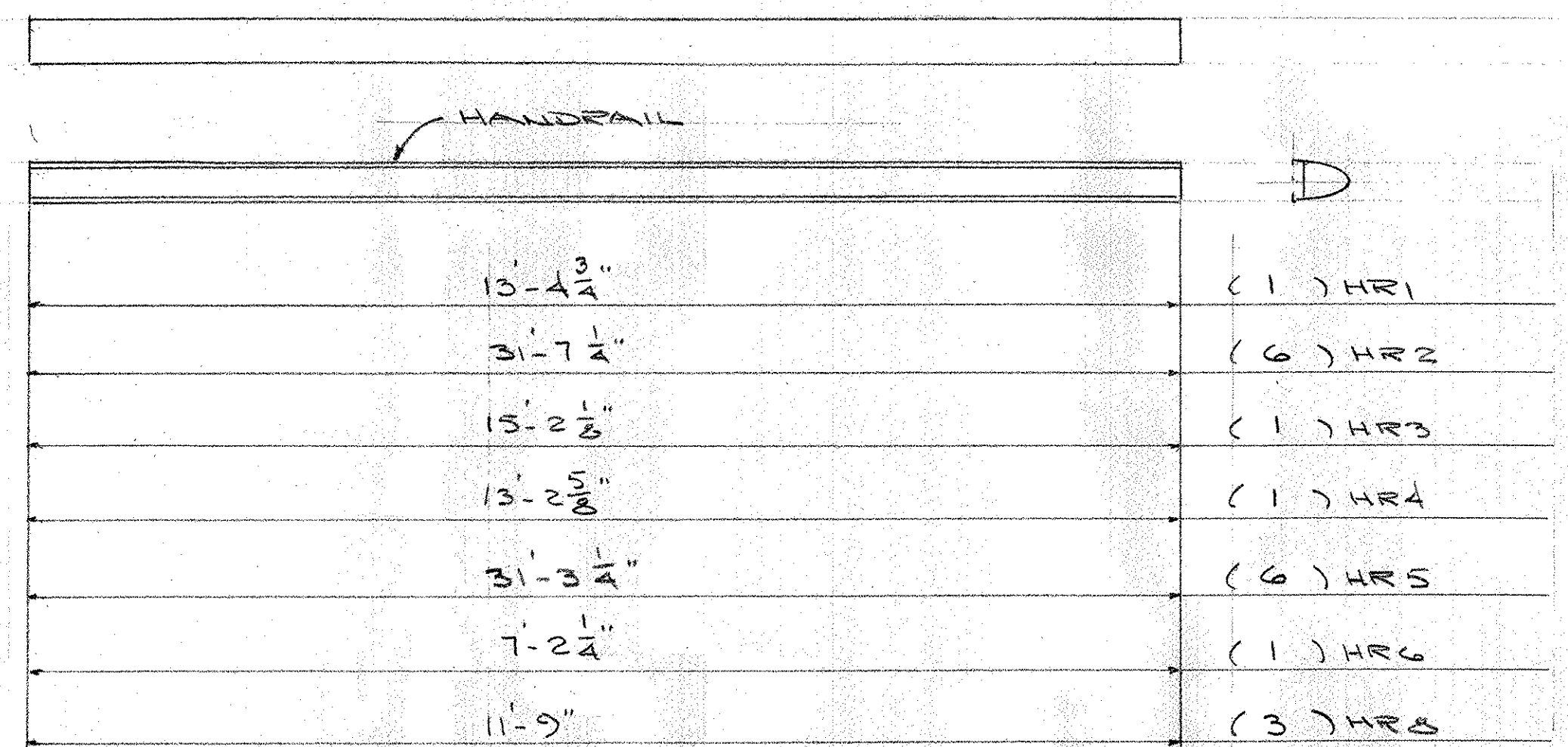
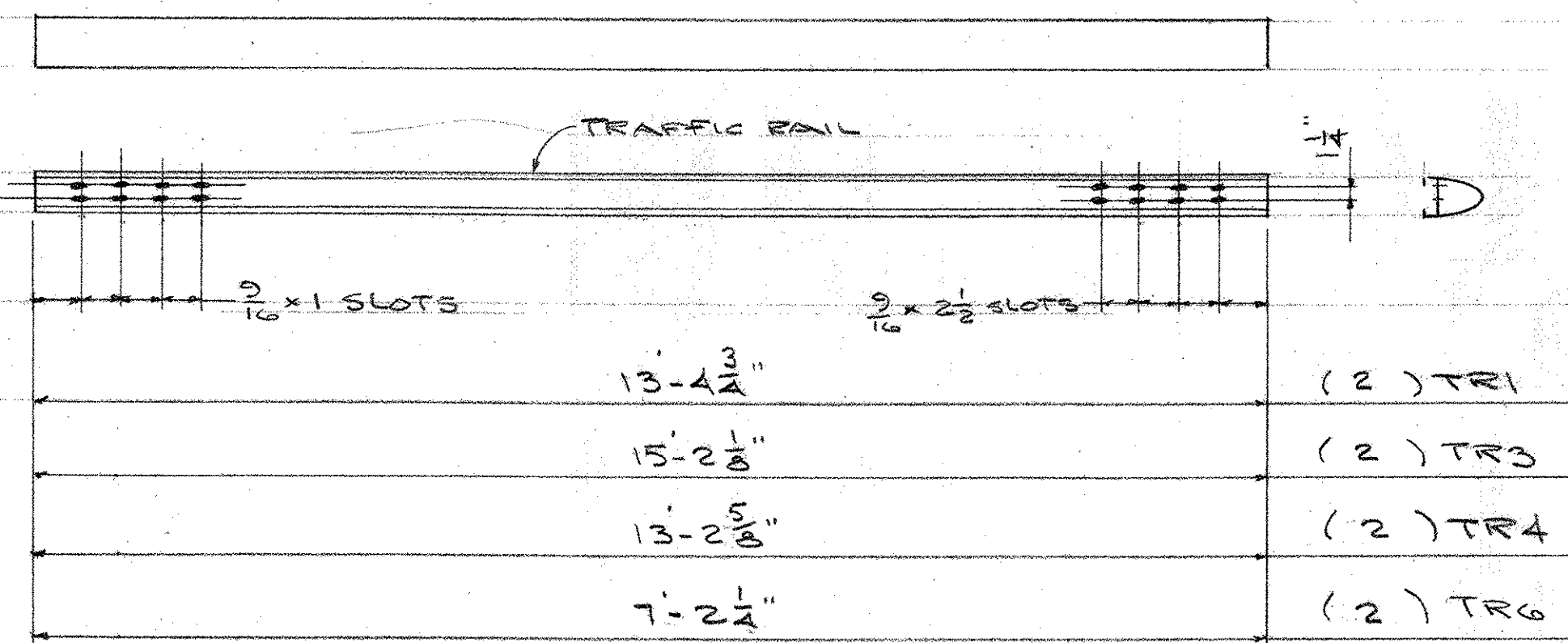
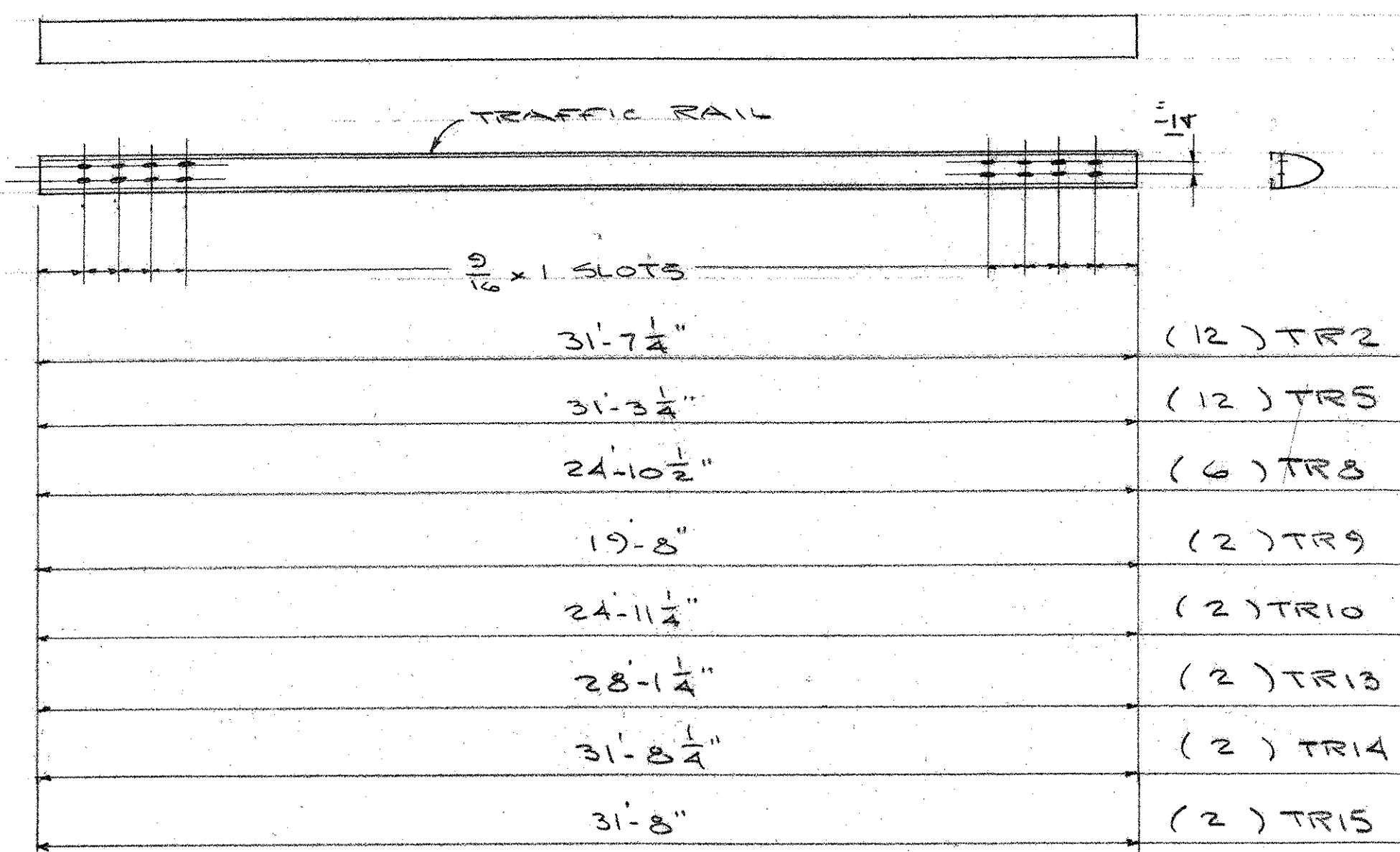
AUCIELLO IRON WORKS INC
560 MAIN ST. HUDSON, MA (978) 568-8382

VT AGENCY OF TRANSPORTATION
HUNTINGTON PROJ. BRO 1445 (29)
BRIDGE #42
EAST ST. OVER HUNTINGTON RIVER

AL BRIDGE # APPROACH RAILING

SURFACE PREP: None	FINISH: BLACK ANODIZE
--------------------	-----------------------

FOR A.D. ROSSI CORP.
DR: VM 02-12-07 DWG. NO.
CHK: B 02-13-07 BR-2898
JOB NO. AS10002-1001 SHEET 2 OF 3



IF ANY DIMENSIONS ARE REVISED, UNLESS NOTED OTHERWISE:

NO EXCEPTION TAKEN

REVISE AS NOTED

PERMISSION NOT REQUIRED

REVISE AS NOTED

PERMISSION REQUIRED

REFLECTED

DATE: 3/26/07

DESIGNED BY: Michael Chiodi

FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DESIGN, CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS, COORDINATION OF SUB-TRADER DETAIL DESIGN OF COMPONENTS AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.

RECEIVED

OK'D BY: [Signature]

DATE: FEB 28 2007

RESUBMIT BY: [Signature]

APPROVED DATE: [Signature]

FOR APPROVAL

ISSUE DATE DESCRIPTION BY

AUCIELLO IRON WORKS INC

590 MAIN ST. HUDSON, MA (978) 568-8382

VT. AGENCY OF TRANSPORTATION
HUNTINGTON PROJ. BRO (445 (29)
BRIDGE # 42

EAST ST. OVER HUNTINGTON RIVER

AL BRIDGE & APPROACH RAILING

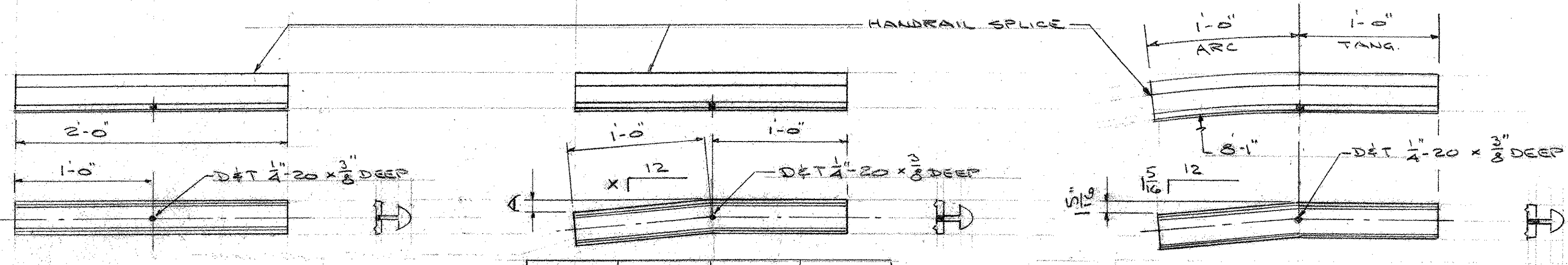
SURFACE PREP: NONE FINISH: MILL FINISH

FOR: A.D. ROSSI CORP.

DR: WM 2-16-07 DWG. NO. BR-2393

CHK: D 2-17-07

JOB NO. BR10002-1001 SHEET 5 OF 8

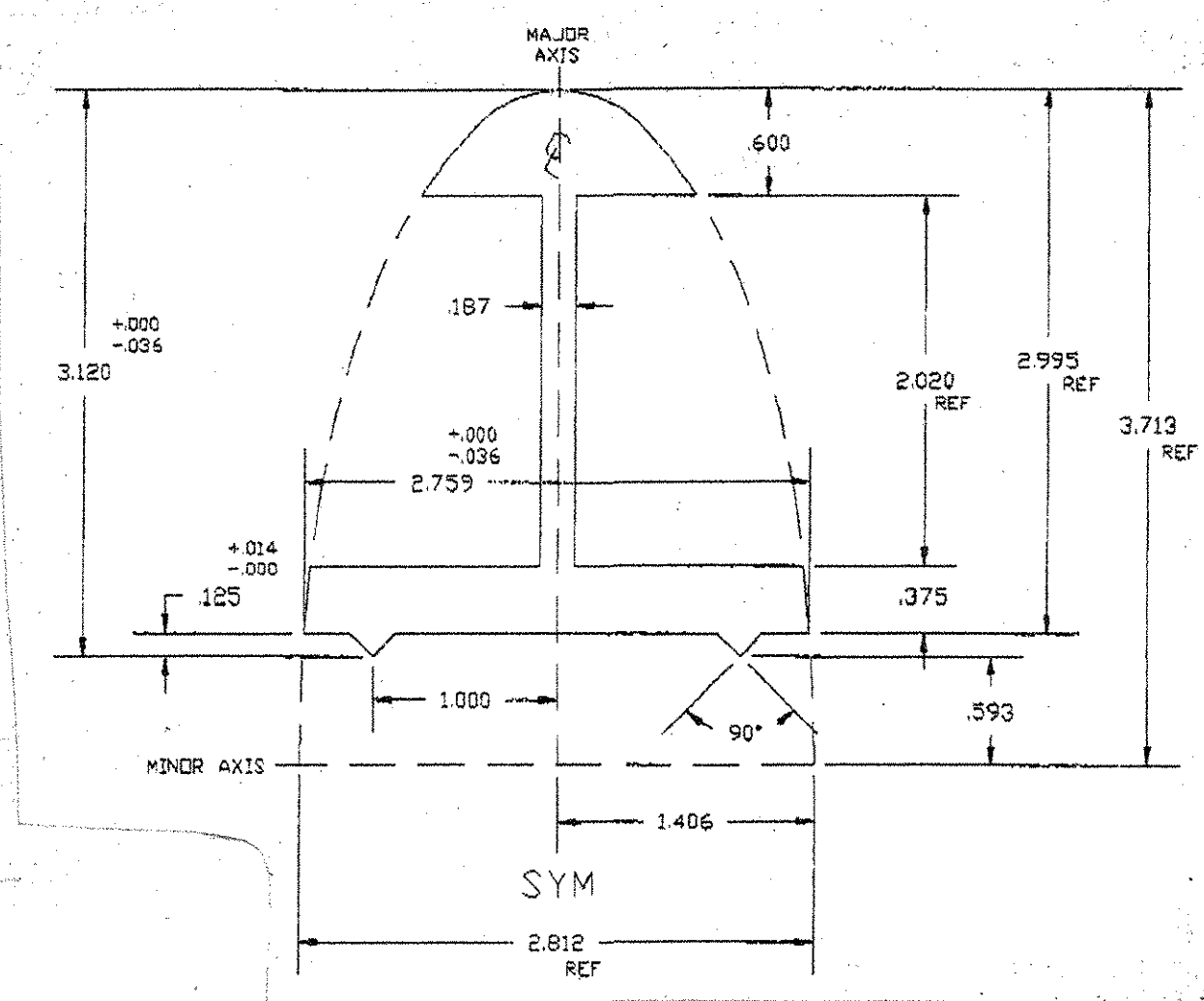


(14) HSI

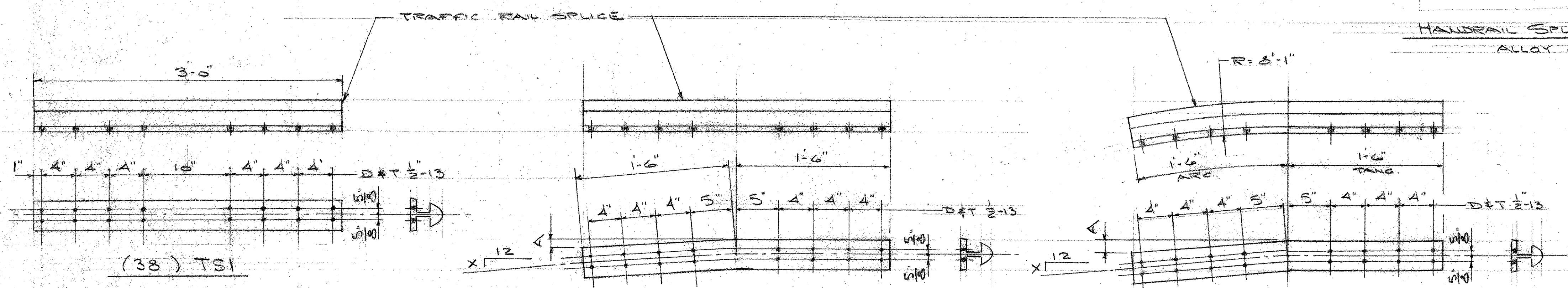
QUAN	MK	X	A
2	HS2	1 1/2	1 1/4
1	HS3	15/16	15/16

(1) HS4

SET SCREWS AT HANDRAIL SPLICES TO BE SHIPPED LOOSE



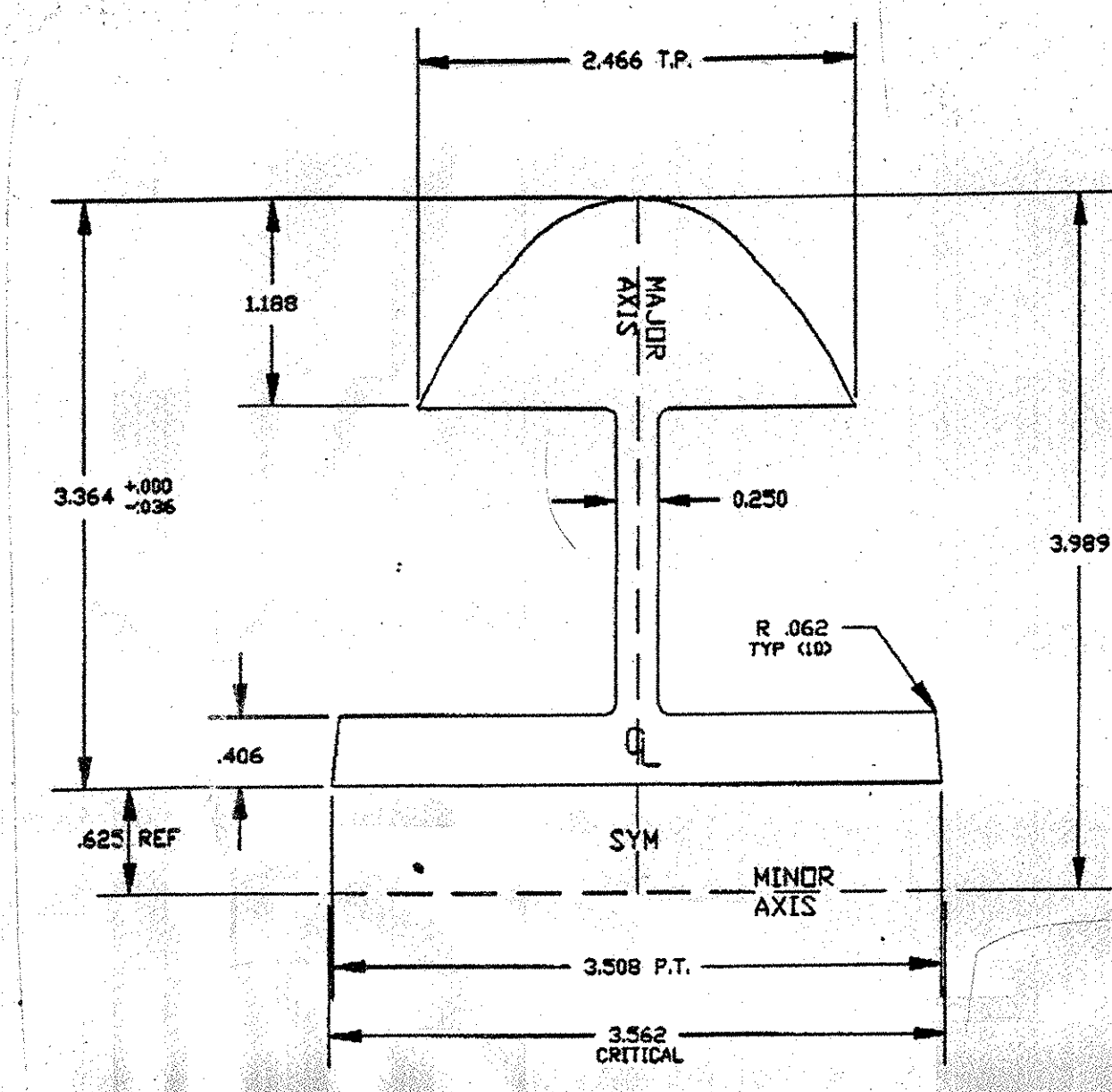
HANDRAIL SPLICE EXTRUSION
ALLOY 6061-T6



(38) TSI

QUAN	MK	X	A
TS2	4	1 1/8	1 1/8
TS3	2	1 1/16	1 1/16
TS4	2	1 1/16	1 1/16
TS5	4	1 1/16	1 1/16

QUAN	MK	X	A
1	TSG	1 1/8	1 1/8
1	TS7	1 1/8	1 1/8



TRAFFIC RAIL SPLICE EXTRUSION
ALLOY 6061-T6

REVISIONS ARE REVISED UNLESS NOTED OTHERWISE

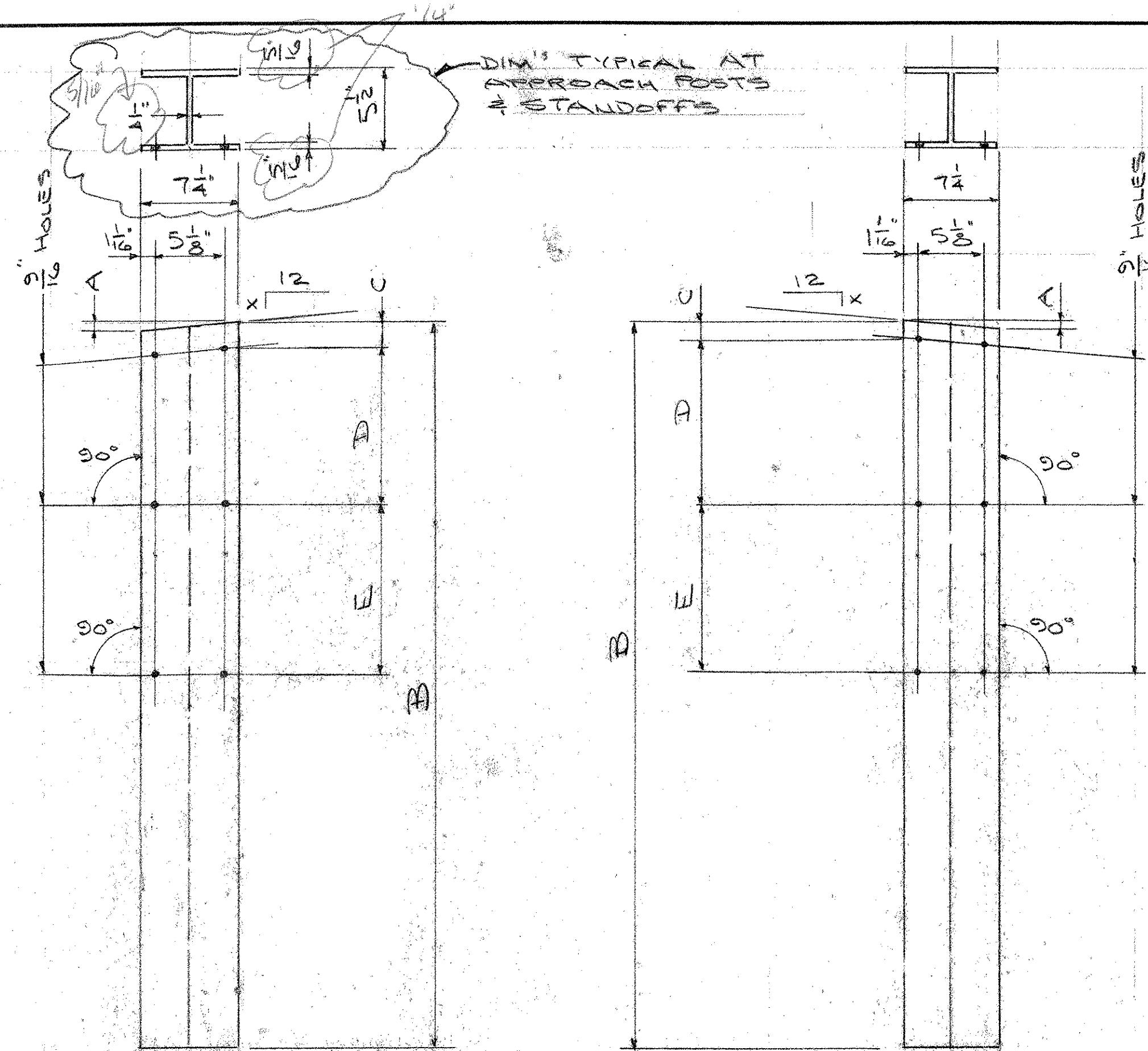
NO. DESCRIPTION TAKEN	DATE
REVISIONS NOTED	DATE
REVISIONS NOTED	DATE
REVISIONS NOTED	DATE
REVISIONS NOTED	DATE

DATE: 3/16/07
BY: Michael Smith

THIS DRAWING IS FOR THE SOLE PURPOSE OF ASSEMBLING GENERAL CONFORMITY WITH DESIGN. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS. COORDINATION OF SUBTRACTS, DETAIL DESIGN OF COMPONENTS AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.

RECEIVED	
CHK'D BY: _____	DATE: FEB 28 2007
RESUBMIT: _____	APPROVED: _____
BY: _____	DATE: _____
FOR APPROVAL	
ISSUE: 1	DATE: 2-28-07
DESCRIPTION: BRIDGE #42	BY: AB10002-1001

AUCIELLO IRON WORKS INC	
560 MAIN ST. HUDSON, MA (978) 568-8382	
VT. AGENCY OF TRANSPORTATION HUNTINGTON PROJ. BRO 1445 (29) BRIDGE #42	
EAST ST. OVER HUNTINGTON RIVER	
AL. BRIDGE # APPROACH RAILING	
SURFACE PREP: NONE	FINISH: MILL FINIS
FOR: A.D. ROSSI CORP.	
DR: WM 2-16-07	DWG. NO. BR-2898
CHK: B 2-17-07	JOB NO. AB10002-1001
BY: _____	SHEET 6 OF 8

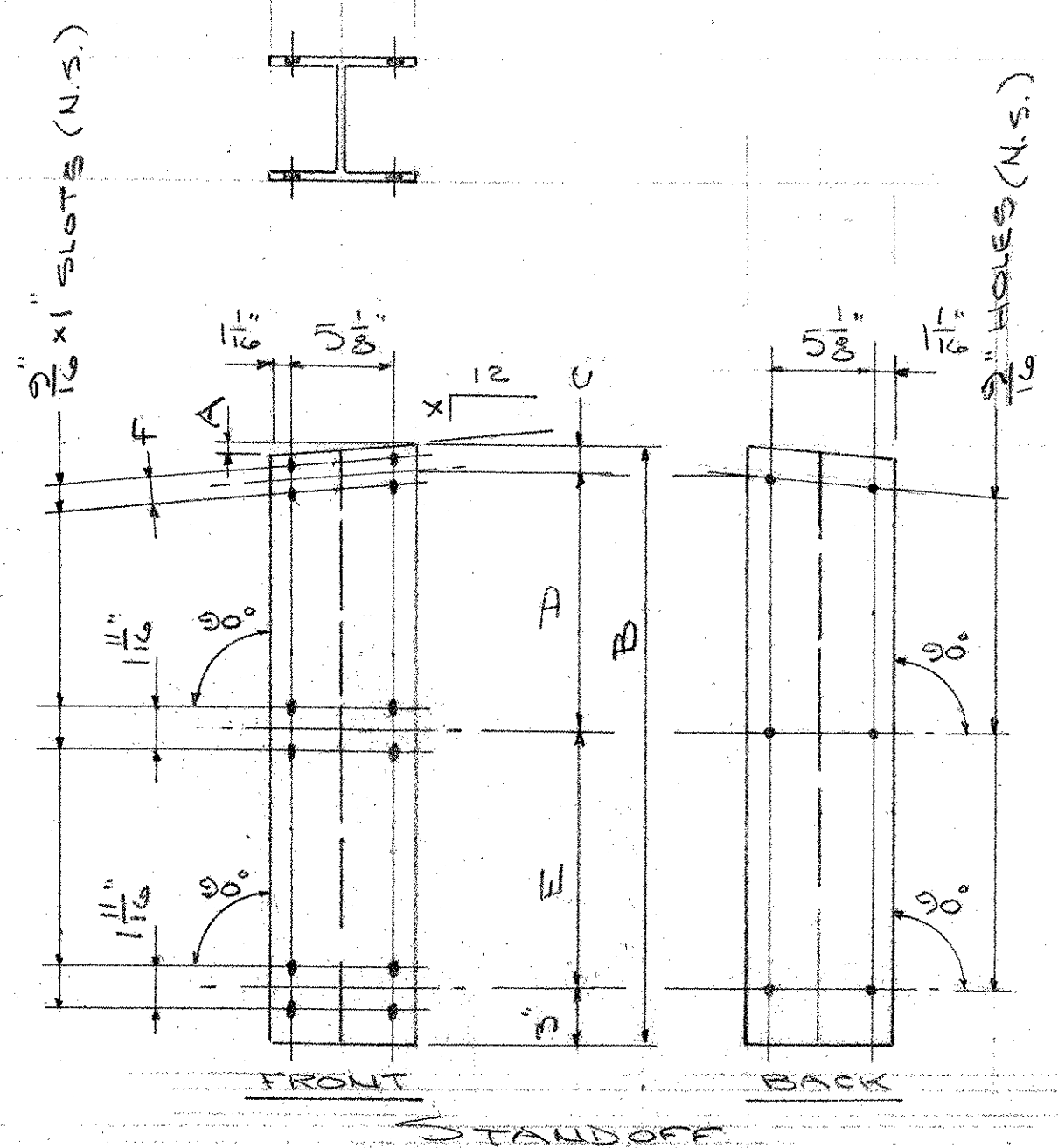


APPROACH POST

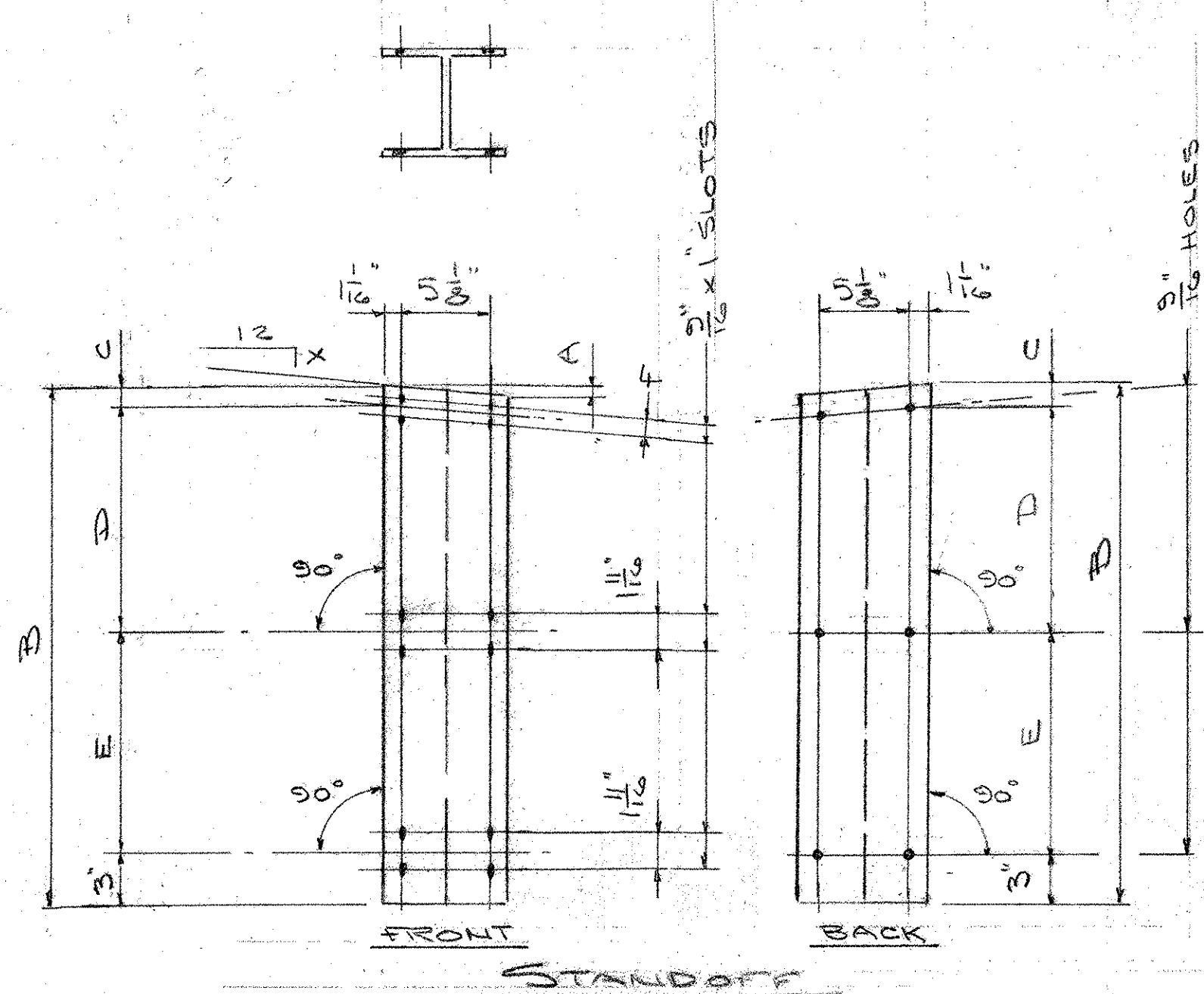
APPROACH POST

QTY	MK	X	A	B	C	D	E
1	P11	1 1/8"	1 1/8"	6'-6"	1 1/8"	11 7/16"	1'-0 9/16"
1	P12	1 1/8"	1 1/8"	6'-6"	1 1/8"	9 3/4"	11 7/16"
1	P13	1 1/8"	1 1/8"	6'-6"	1 1/8"	8 1/8"	11 7/16"
1	P14	1 1/8"	1 1/8"	6'-6"	1 1/8"	6 1/2"	10 9/16"
1	P15	1 1/8"	1 1/8"	6'-0"	2"	0	9 3/8"
1	P16	1 1/8"	1 1/8"	6'-0"	2"	0	8 1/2"
1	P17	1 1/8"	1 1/8"	6'-0"	2"	0	7 1/8"
1	P31	1 1/8"	1 1/8"	6'-6"	1 1/8"	11 7/16"	1'-5 3/8"
1	P32	1 1/8"	1 1/8"	6'-6"	1 1/8"	9 3/4"	1'-3 3/8"
1	P33	1 1/8"	1 1/8"	6'-6"	1 1/8"	8 1/8"	1'-2 9/16"
1	P34	1 1/8"	1 1/8"	6'-6"	1 1/8"	6 1/2"	1'-1 1/2"
1	P35	1 1/8"	1 1/8"	6'-0"	2"	0	11 1/2"
1	P36	1 1/8"	1 1/8"	6'-0"	2"	0	9 13/16"
1	P37	1 1/8"	1 1/8"	6'-0"	2"	0	8 1/2"
31	P8	0	0	6'-0"	2"	0	8"

QTY	MK	X	A	B	C	D	E
1	P21	1 5/8"	1 5/8"	6'-6"	1 5/8"	11 7/16"	1'-0 9/16"
1	P22	1 5/8"	1 5/8"	6'-6"	1 5/8"	9 3/4"	11 7/16"
1	P23	1 5/8"	1 5/8"	6'-6"	1 5/8"	8 1/8"	11 7/16"
1	P24	1 5/8"	1 5/8"	6'-6"	1 5/8"	6 1/2"	10 9/16"
1	P25	1 5/8"	1 5/8"	6'-0"	2"	0	9 3/8"
1	P26	1 5/8"	1 5/8"	6'-0"	2"	0	8 1/2"
1	AP1	1 5/8"	1 5/8"	6'-6"	1 5/8"	11 7/16"	1'-5 3/8"
1	AP2	1 5/8"	1 5/8"	6'-6"	1 5/8"	9 3/4"	1'-3 3/8"
1	AP3	1 5/8"	1 5/8"	6'-6"	1 5/8"	8 1/8"	1'-2 9/16"
1	AP4	1 5/8"	1 5/8"	6'-6"	1 5/8"	6 1/2"	1'-1 1/2"
1	P27	1 5/8"	1 5/8"	6'-0"	2"	0	7 1/8"



STANDOFF



STANDOFF

QTY	MK	X	A	B	C	D	E	F
1	T11	1 1/8"	1 1/8"	2'-4 1/8"	1 1/8"	11 7/16"	1'-0 9/16"	1'-1 1/2"
1	T12	1 1/8"	1 1/8"	2'-2 1/8"	1 1/8"	10 1/8"	11 7/16"	1'-1 1/2"
1	T13	1 1/8"	1 1/8"	2'-0 1/8"	1 1/8"	8 1/2"	11 7/16"	1'-1 1/2"
1	T14	1 1/8"	1 1/8"	1'-9 1/8"	1 1/8"	6 1/8"	10 9/16"	1'-1 1/2"
1	T15	1 1/8"	1 1/8"	1'-2 1/8"	2"	0	9 3/8"	1'-1 1/2"
1	T16	1 1/8"	1 1/8"	1'-1 1/8"	2"	0	8 1/2"	1'-1 1/2"
1	T17	1 1/8"	1 1/8"	1'-1 1/8"	2"	0	8 1/2"	1'-1 1/2"
1	T31	1 1/8"	1 1/8"	2'-9 1/8"	1 1/8"	11 7/16"	1'-5 3/8"	1'-1 1/2"
1	T32	1 1/8"	1 1/8"	2'-6 1/8"	1 1/8"	10 1/8"	1'-3 3/8"	1'-1 1/2"
1	T33	1 1/8"	1 1/8"	2'-3 1/8"	1 1/8"	8 1/2"	1'-2 9/16"	1'-1 1/2"
1	T34	1 1/8"	1 1/8"	2'-0 1/8"	1 1/8"	6 1/2"	1'-1 1/2"	1'-1 1/2"
1	T35	1 1/8"	1 1/8"	1'-1 1/8"	2"	0	11 1/2"	1'-1 1/2"
1	T36	1 1/8"	1 1/8"	1'-0 1/8"	2"	0	10"	1'-1 1/2"
1	T37	1 1/8"	1 1/8"	1'-1 1/8"	2"	0	8 1/2"	1'-1 1/2"
31	T8	0	0	1'-1 1/8"	2"	0	8"	1'-1 1/2"

QTY	MK	X	A	B	C	D	E	F
1	T21	1 5/8"	1 5/8"	2'-4 1/8"	1 5/8"	11 7/16"	1'-0 9/16"	1'-1 1/2"
1	T22	1 5/8"	1 5/8"	2'-2 1/8"	1 5/8"	10 1/8"	11 7/16"	1'-1 1/2"
1	T23	1 5/8"	1 5/8"	2'-0 1/8"	1 5/8"	8 1/2"	11 7/16"	1'-1 1/2"
1	T24	1 5/8"	1 5/8"	1'-9 1/8"	1 5/8"	6 1/8"	10 9/16"	1'-1 1/2"
1	T25	1 5/8"	1 5/8"	1'-2 1/8"	2"	0	9 3/8"	1'-1 1/2"
1	T26	1 5/8"	1 5/8"	1'-1 1/8"	2"	0	8 1/2"	1'-1 1/2"
1	T27	1 5/8"	1 5/8"	1'-1 1/8"	2"	0	8 1/2"	1'-1 1/2"
1	TP1	1 5/8"	1 5/8"	2'-9 1/8"	1 5/8"	11 7/16"	1'-5 3/8"	1'-1 1/2"
1	TP2	1 5/8"	1 5/8"	2'-6 1/8"	1 5/8"	10 1/8"	1'-3 3/8"	1'-1 1/2"
1	TP3	1 5/8"	1 5/8"	2'-3 1/8"	1 5/8"	8 1/2"	1'-2 9/16"	1'-1 1/2"
1	TP4	1 5/8"	1 5/8"	2'-0 1/8"	1 5/8"	6 1/2"	1'-1 1/2"	1'-1 1/2"

NO SHOP DRAWINGS ARE REVISED UNLESS NOTED OTHERWISE

NO EXCEPTION TAKEN

REVISE AS NOTED
RESUBMISSION NOT REQUIRED

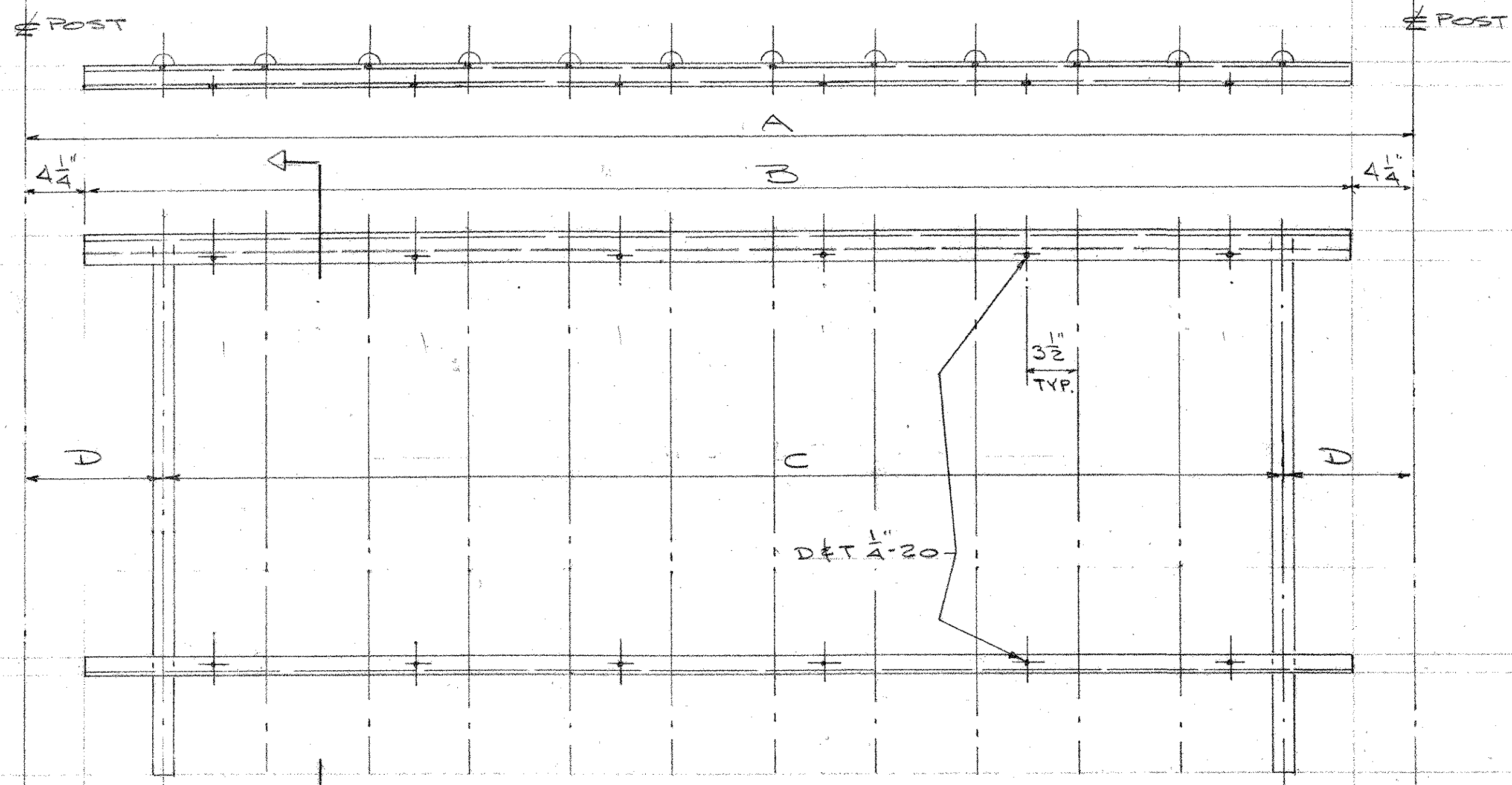
REVISE AS NOTED
RESUBMISSION REQUIRED

DATE: 3/26/07

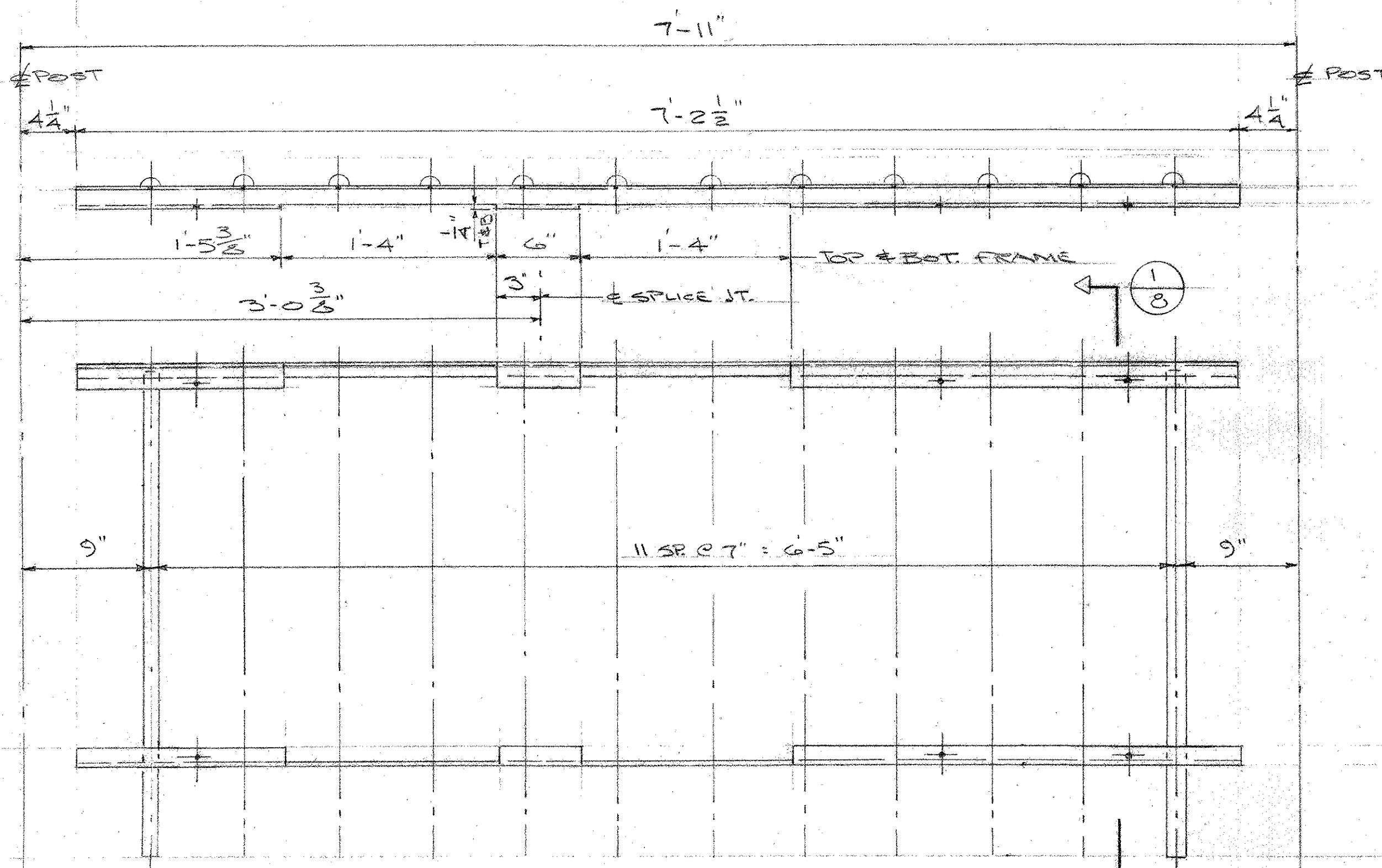
SIGNATURE: *M. Del Vecchio*

REVIEW BY STANTEC IS FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DESIGN. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS. COORDINATION OF SUB-TASKS, DETAIL DESIGN OF COMPONENTS AND ERRORS OR OMISSIONS ON SHOP DRAWINGS.

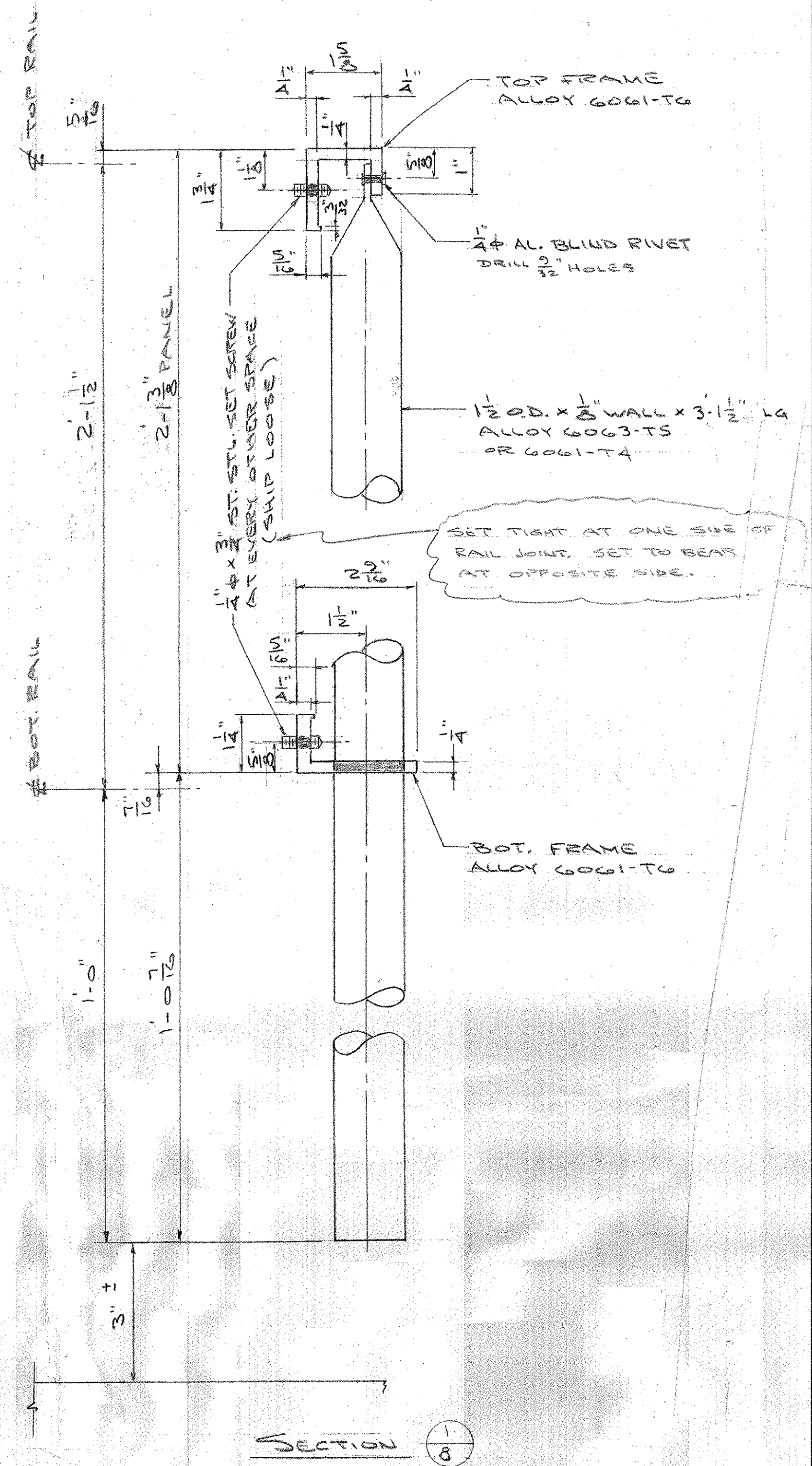
RECEIVED		AUCIELLO IRON WORKS INC 560 MAIN ST. HUDSON, MA (978) 568-8382	
CRD BY: _____	CHK BY: _____	VT. AGENCY OF TRANSPORTATION HUNTINGTON PROJ. BRD 1445 (23) BRIDGE # 42	
FEB 24 2007		EAST ST. OVER HUNTINGTON RIVER	
RESUBMIT BY: _____	APPROVED BY: _____	AL APPROACH RAILING	
DATE: _____	DATE: _____	SURFACE PREP: NONE FINISH: MILL FINISH	
FOR A.D. ROSSI CORP.		FOR A.D. ROSSI CORP.	
DR: WM 02-14-07		DWG. NO. BR-2398	
CHK: 3 02-17-07		JOB NO. 0310002-1001	
1	2-22-07	FOR APPROVAL	BY: _____
ISSUE	DATE	DESCRIPTION	BY



1/8	PANEL No.	QUAN	A	B	C	D
	T1	20	7'-11"	7'-2 1/2"	11 SP. @ 7" = 6'-5"	9"



PANEL T2 (7-REQD)



REVISIONS

NO EXCEPTION TAKEN	
REVISION AS NOTED	
RESUBMISSION NOT REQUIRED	
REVISION AS NOTED	
RESUBMISSION REQUIRED	
REJECTED	

DATE: 3/24/07

SIGNATURE: *Michael Chumt*

REVIEW BY STANTEC IS FOR THE SOLE PURPOSE OF ASCERTAINING GENERAL CONFORMITY WITH DESIGN. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, FABRICATION AND CONSTRUCTION METHODS, COORDINATION OF SUB-TRADES, DETAIL DESIGN OF COMPONENTS, AND ERRORS OR OMISSIONS ON EACH DRAWING.

RECEIVED

CHKD BY: *[Signature]* DATE: FEB 28 2007

RESUBMIT: *[Signature]* APPROVED: *[Signature]* DATE: *[Signature]*

1 2-22-07 FOR APPROVAL

ISSUE DATE DESCRIPTION BY

AUCIELLO IRON WORKS INC
560 MAIN ST. HUDSON, MA (978) 568-8382

VT AGENCY OF TRANSPORTATION
HUNTINGTON PROJ. BRO 1445 (29)
BRIDGE #42
EAST ST. OVER HUNTINGTON RIVER

ALUM. PICKET PANELS

SURFACE PREP: NONE	FINISH: MILL FINISH
FOR: A.D. ROSSI CORP.	DWG. NO. BR-2898
DR: WM 02-12-07	CHK'D: 02-17-07
JOB NO. AB10002-1001	SHEET 8 OF 8